PHILOSOPHY & DESIGN OVERVIEW



7 Hove Manor Parade, Hove Street, Hove, East Sussex, BN3 2DF T: 01273 358035: F: 01273 207451: W www.qedstructures.co.uk

Project:	205 Albany	Street							
Project No:	20-186	Date:	Mar 20	Engineer:	BS	Checked By:	SHL	Sheet No:	G3

Introduction

The proposed works at 205 Albany Street comprise of the extension of the existing lower ground floor and reducinf the existing lower ground floor level.

Ground Conditions

A soil investigation report carried out by ASL showed the ground build up to be approximately 2.0m overlaying London Clay. An allowable ground bearing pressure of 125kN/m2 has been recomended

Foundations

Foundations to be in the form of mass concrete underpinning of the existing walls at lower ground floor with reinforced concrete retaining walls underpinning the existing ground floor walls

Stability

N/A

Loadings All proposed loads are in line with Eurocodes.

Fire Protection

By others



205 Albany Street

2.0 PROJECT LOADINGS

Date:Mar 20Project Number:20-186Revision Number:BR1



Consulting Structural & Civil Engineers

								ve Street, Hove, East S 73 207451 : W www.qe	ussex, BN3
Project:	205 Alba	any Street							
Project No:	20-186	Date: N	/lar 20	Engineer:	BS	Checked:	SHL	Sheet No.	G4
Loadings									
lat Roof									
Р	ermanent	Loads							
	Flat Roof	f Finishes						0.45	
	Joists (20	00)						0.15	
	-	l (12mm)						0.09	
	insulatio							0.02	
	Plasterbo	oard (12.5mi	m)					0.09	
								0.80 k	N/m2
h	mposed Lo	bads							
	0.75 lmp	oosed Load						0.75 k	N/m2
Glazed Roof									
Р	ermanent	Loads							
	Glazing							0.50	
								0.50 k	N/m2
li	mposed Lo	oads Dosed Load						0.75 k	N/m2
	0.75 mp							0.75 K	N/1112
Ground floo	~								
	•								
Р	ermanent								
		(200mm)						5.00	
	Screed (7							1.80	
	Finishes	+ insulation						0.20	
Т	otal =						SA	Y 7.00 k	N/m2



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Project:	205 Alba	any Street							
Project No:	20-186	Date:	Mar 20	Engineer:	BS	Checked:	SHL	Sheet No.	G4
Ir	mposed Lo			•		•			_
	1.5 Impo	osed Load						1.50 k	«N/m2
Pitched Roo	of						Pitch =	28.00 °	
Р	ermanent	t Loads							
	Slate Til	es						0.30	
	Joists (1							0.08	
	Batterns	-						0.05	
	insulatio	on						0.02	
	Plasterb	oard (12.5	mm)					0.09	
						ON SLC	OPE = SAY	0.55	«N/m2
				0	N PLAN	= 0.55/COS	28 = SAY	0.70	«N/m2
Ir	mposed Lo	oads							
	0.6 Impo	osed Load						0.60	«N/m2
Timber floor									
Р	ermanent	t Loads							
	Services							0.05	
	Joists (2							0.15	
		oards/Ply						0.15	
	insulatio							0.02	
	Plasterb	oard (12.5	mm)					0.09	
							SAY	0.50	«N/m2
Ir	mposed Lo								
	1.5 Impo	osed + part	itions					2.00	«N/m2



Consulting Structural & Civil Engineers 7 Hove Manor Parade, Hove Street, Hove, East Sussex, BN3 2DF

						T: 01273 358	035: F: 0127	3 207451: W www.qedstructures.co
Project:	205 Alba	iny Street						
Project No:	20-186	Date:	Mar 20	Engineer:	BS	Checked:	SHL	Sheet No. G4
215 Solid wa	all	•				ŀ		
Р	ermanent	Loads						
	215mm	Brickworl	〈					4.30
	Plasterbo	oard (12.	5mm)					0.09
							SAY	′ 4.40 kN/m2
330 Solid wa	all							
Р	ermanent	Loads						
	330mm	brickworł	< C					6.60
	Plasterbo	oard (12.	5mm)					0.09
	Plasterbo	oard (12.	5mm)					0.09
							SAY	′ 6.80 kN/m2

100 Solid wall

	SAY	2.20 kN/m2
Plasterboard (12.5mm)		0.09
Plasterboard (12.5mm)		0.09
100mm brickwork		2.00
Permanent Loads		



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Project:	205 Albany Street											
Project No:	20-186	Date:	Mar 20	Engineer:	BS	Checked:	SHL	Sheet No.	G4			
Roof space		•					Pitch =	40.00 °				
P	ermanent	Loads										
	Services							0.05				
	Joists (10	00)					0.08					
	insulatio	n						0.02				
	Plasterb	oard (12.	5mm)					0.09				
							SAY	0.25 kM	\/m2			
li	mposed Lo	oads										
	0.6 Impc	sed Load						0.60 kM	N/m2			



205 Albany Street

3.0 MEMBER LOADS

Date:Aug 20Project Number:19-045Revision Number:BR1

CALCULATION SHEET 3.1 MEMBER LOADINGS

qed

QED STRUCTURES

Consulting Structural & Civil Engineers 7 Hove Manor Parade, Hove Street, Hove, East Sussex, BN3 2DF T: 01273 358035: F: 01273 207451: W www.qedstructures.co.uk

Project:	ct: 205 Albany Street								
Project No:	20-186	Date:	Mar 20	Engineer:	BS	Checked By:	SHL		
Ref.		1	Ca	lculations		ł			
RT1		UDL o	n top of wall	_					
Retaining wall at rear			Dead	Live	length				
wall at rear			(kN/m2)	(kN/m2)	/height	DL	LL		
	Ground floor		7.00	1.50	0.80	5.60	1.20		
	Glazed roof		0.50	0.75	2.00	1.00	1.50		
	215 Solid wall		4.40	0.00	3.00	13.20	0.00		
						19.80	2.70		



205 Albany Street

4.0 MEMBER DESIGN

Date:Mar 20Project Number:20-186Revision Number:BR1

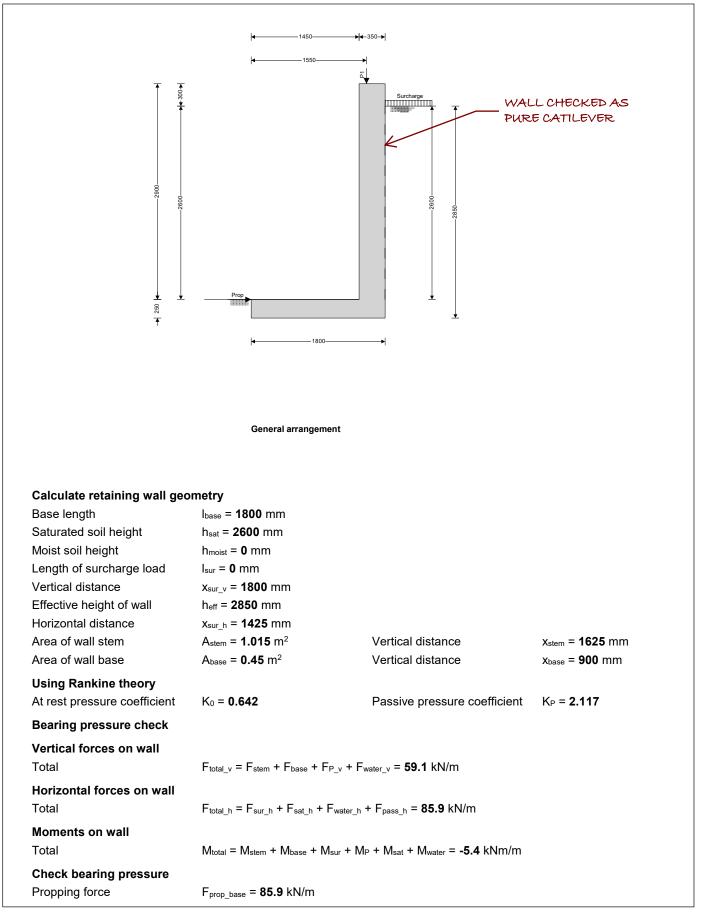
Tekla. Tedds	Project	205 Alba	any Street		Job no. 20-186	
	Calcs for	Retaining wa	all no top prop		Start page no./R	evision 1
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.11

Retaining wall details			
Stem type	Cantilever		
Stem height	h _{stem} = 2900 mm		
Stem thickness	t _{stem} = 350 mm		
Angle to rear face of stem	α = 90 deg		
Stem density	$\gamma_{\text{stem}} = 25 \text{ kN/m}^3$		
Toe length	I _{toe} = 1450 mm		
Base thickness	t _{base} = 250 mm		
Base density	γ _{base} = 25 kN/m ³		
Height of retained soil	h _{ret} = 2600 mm	Angle of soil surface	$\beta = 0 \operatorname{deg}$
Depth of cover	d _{cover} = 0 mm		
Height of water	h _{water} = 2600 mm		
Water density	γ _w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γmr = 21 kN/m ³		
Saturated density	γ _{sr} = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{r.k} = 21 deg	
Characteristic wall friction ang	le	δ _{r.k} = 10.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γь = 21 kN/m³		
Characteristic effective shear	resistance angle	φ' _{b.k} = 21 deg	
Characteristic wall friction and	le	δ _{b.k} = 10.5 deg	
Characteristic base friction an	gle	δ _{bb.k} = 14 deg	
Presumed bearing capacity	P _{bearing} = 125 kN/m ²		
Loading details			
Variable surcharge load	Surcharge _Q = 10 kN/m ²		
Vertical line load at 1550 mm	P _{G1} = 19.8 kN/m		
	P _{Q1} = 2.7 kN/m		

Tekla. Tedds	Project	205 Alba	any Street		Job no. 20-186	
	Calcs for	Retaining wa	all no top prop		Start page no./Re	evision 2
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



Tekla Tedds	Project	205 Alba	any Street		Job no. 20)-186
	Calcs for	Retaining w	all no top prop		Start page no./I	Revision 3
	Calcs by	Calcs date	Checked by	Checked date	Approved by	Approved date
	BS	12/03/2021	SHL	12/03/2021	SHL	12/03/2021
Bearing pressure at toe	$q_{toe} = 0 \text{ kN/m}^2$		Bearing press		q _{heel} = 0 kN/r	n ² side base area
Factor of safety	FoS _{bp} = 0					
	FAIL - M	laximum applie	d bearing pres	ssure exceeds a	llowable bea	ring pressure
				LL FAILS ON		
				ERTURNING. N DUGH TOE LEN		
			PRI	OVIDED. WALL ED TO BE PROP	WILL	
			HE		1027(1	

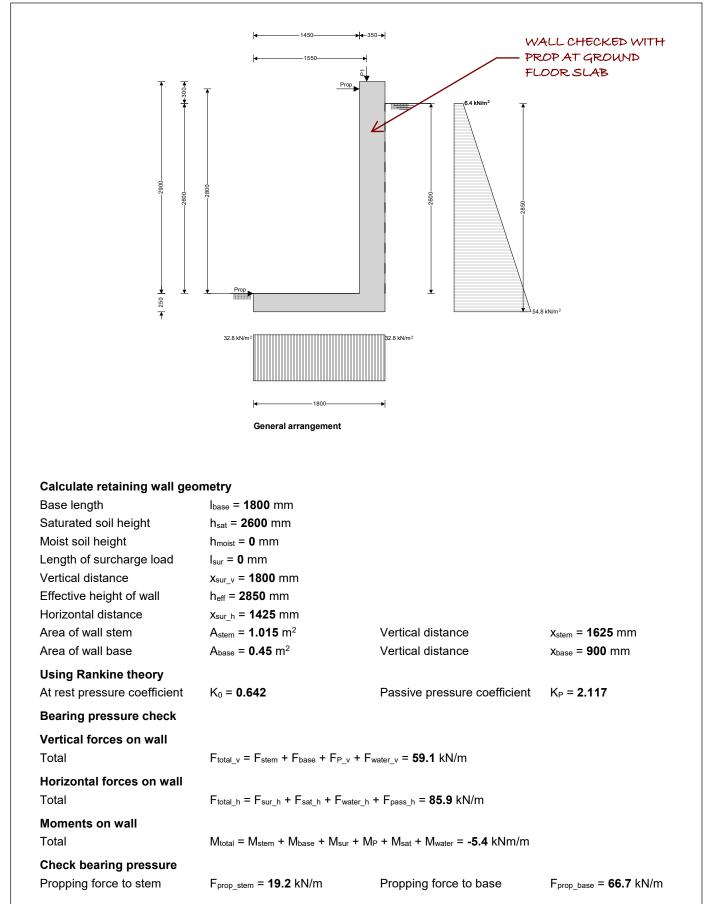
Tekla. Tedds	Project	205 Alba	iny Street		Job no. 20-186	
	Calcs for	Retaining wal	I with top prop		Start page no./Revision 1	
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

			Tedds calculation version 2.9.11
Retaining wall details			
Stem type	Propped cantilever		
Stem height	h _{stem} = 2900 mm		
Prop height	h _{prop} = 2800 mm		
Stem thickness	t _{stem} = 350 mm		
Angle to rear face of stem	α = 90 deg		
Stem density	γ _{stem} = 25 kN/m ³		
Toe length	I _{toe} = 1450 mm		
Base thickness	t _{base} = 250 mm		
Base density	γ _{base} = 25 kN/m ³		
Height of retained soil	h _{ret} = 2600 mm	Angle of soil surface	$\beta = 0 \deg$
Depth of cover	d _{cover} = 0 mm		
Height of water	h _{water} = 2600 mm		
Water density	γ _w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γ _{mr} = 21 kN/m ³		
Saturated density	γ _{sr} = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{r.k} = 21 deg	
Characteristic wall friction ang	le	δ _{r.k} = 10.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γ _b = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{b.k} = 21 deg	
Characteristic wall friction and	le	δ _{b.k} = 10.5 deg	
Characteristic base friction an	gle	δ _{bb.k} = 14 deg	
Presumed bearing capacity	P _{bearing} = 125 kN/m ²		
Loading details			
Variable aurobarga lood	Surphoreon = $10 \text{ kN}/\text{m}^2$		

Variable surcharge load	Surcharge _Q = 10 kN/m ²
Vertical line load at 1550 mm	P _{G1} = 19.8 kN/m
	P _{Q1} = 2.7 kN/m

Tekla. Tedds	Project	Project Jo 205 Albany Street					
	Calcs for	Calcs for Si Retaining wall with top prop				Start page no./Revision 2	
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021	



Tekla Tedds		205 Albany Street				-186
	Calcs for	Retaining wa	all with top prop		Start page no./Revision 3	
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/202
Bearing pressure at toe	q _{toe} = 32.8 kN/r	m²	Bearing press	sure at heel	q _{heel} = 32.8 kl	N/m ²
Factor of safety	FoS _{bp} = 3.805					
	PASS - A	llowable beariı	ng pressure ex	xceeds maximu	m applied bea	ring pressur
RETAINING WALL DESIGN						
	4 4 9 9 9 4 1					
In accordance with EN1992-	•	orating Corrige	naum dated J	anuary 2008 and	d the UK Natio	onal Annex
incorporating National Ame	nament No.1				Tedds calcula	tion version 2.9.
Concrete details - Table 3.1	- Strength and d	leformation ch	aractoristics f	or concrete		
Concrete strength class	C32/40					
Char.comp.cylinder strength	f _{ck} = 32 N/mm ²		Mean axial tensile strength		f _{ctm} = 3.0 N/mm ²	
Secant modulus of elasticity	E _{cm} = 33346 N/	mm ²	Maximum aggregate size		$h_{agg} = 20 \text{ mm}$	
Design comp.concrete strengt			$f_{cd} = 18.1 \text{ N/mm}^2$		Partial factor	γc = 1.50
						10 1100
Reinforcement details	f 500 bl/m	2		4: - :4 .		
Characteristic yield strength	f _{yk} = 500 N/mm		Modulus of elasticity		E _s = 200000 N/mm ²	
Design yield strength	f _{yd} = 435 N/mm	2	Partial factor		γs = 1.15	
Cover to reinforcement						
Front face of stem	c _{sf} = 35 mm		Rear face of s	stem	c _{sr} = 75 mm	
Top face of base	c _{bt} = 35 mm		Bottom face of	of base	c _{bb} = 75 mm	
•						
Loading details - Combination No	p.1 - kN/m ² Shea	ar force - Combination No.1 - kN/	m Bend	ding moment - Combination No.1 -	kNm/m	
Loading details - Combination N	o.1 - kN/m ² Shea	ar force - Combination No.1 - kN/	m Ben	ding moment - Combination No.1 -	kNm/m	
Loading details - Combination N	o.1 - kN/m ² Shea	r force - Combination No.1 - kN/	m Ben	ding moment - Combination No.1 -	kNm/m	
. Loading details - Combination N		ur force - Combination No.1 - kN/	8	ding moment - Combination No.1 -	KNm/m 0 0	
. Combination N	o.1 - kN/m ² Shea	n force - Combination No.1 - kN/	m Ben 0 21.9	ding moment - Combination No.1 -	kNm/m 0	
. Coading details - Combination N		rr force - Combination No.1 - kN/	8	ding moment - Combination No.1 -	KNm/m 0 0	

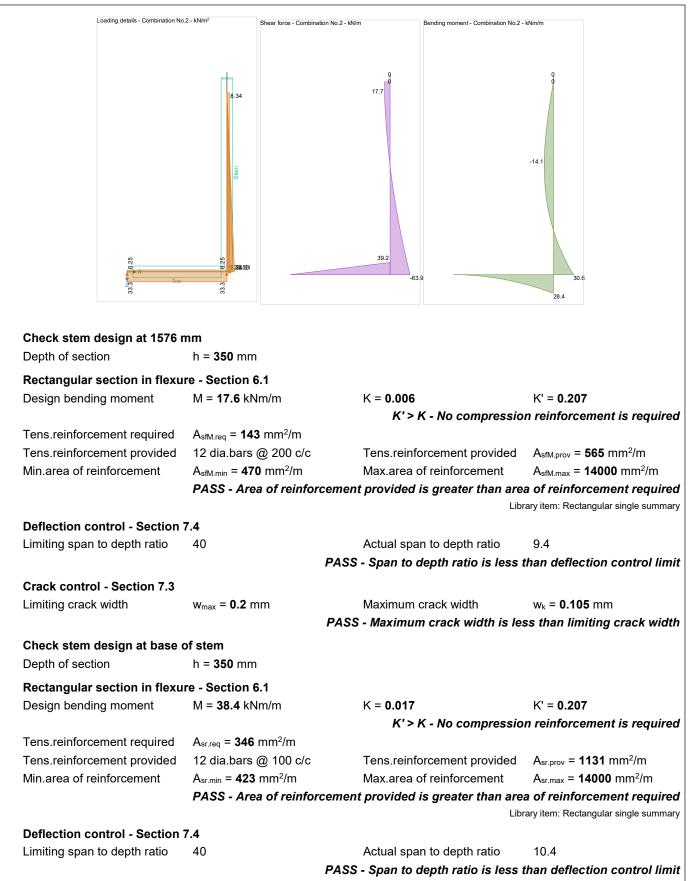
52.4

82243

44.57

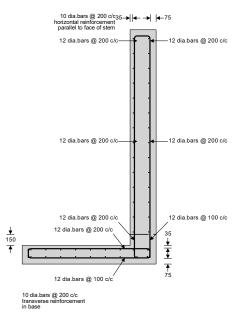
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Tekla. Tedds	Project	Project Jo 205 Albany Street				Job no. 20-186	
	Calcs for	Calcs for S Retaining wall with top prop				Start page no./Revision 4	
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021	



	Project	205 Alba	any Street		Job no. 20-	-186
	Calcs for	Retaining wa	II with top prop		Start page no./R	evision 5
		-				-
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved da 12/03/20
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm		Maximum crack	width	w _k = 0.117 mi	n
		PASS	- Maximum cra	ck width is les	s than limiting	g crack wie
Rectangular section in shea	r - Section 6.2					
Design shear force	V = 80.7 kN/m		Design shear re	sistance	V _{Rd.c} = 143 kN	l/m
		PAS	S - Design shea	ar resistance e	xceeds desig	n shear fo
Check stem design at prop						
Depth of section	h = 350 mm					
Rectangular section in shea	ur - Section 6.2					
Design shear force	V = 21.9 kN/m		Design shear re	sistance	V _{Rd.c} = 135.3	kN/m
Boolgh chodi loroo		PAS	SS - Design shea			
Horizontal reinforcement pa	rallal to face of a					
Min.area of reinforcement	$A_{sx.req} = 350 \text{ mm}$		Max.spacing of	reinforcement	s 400 i	mm
Trans.reinforcement provided	·		Trans.reinforcer			
Trans.reiniorcement provided	-		t provided is gre	-		
Check base design at toe						
Depth of section	h = 250 mm					
•						
Rectangular section in flexu			K = 0.042		K' = 0.207	
Design bending moment	M = 38 kNm/m			o compressior		nt is roqui
Tens.reinforcement required	A _{bb.req} = 544 mn	n²/m	K > K - K	o compression	i i ennoi cennei	n is requi
Tens.reinforcement provided	12 dia.bars @ 1		Tens.reinforcem	nent provided	Abb.prov = 1131	mm²/m
Min.area of reinforcement	A _{bb.min} = 266 mr		Max.area of reir	-	$A_{bb,max} = 1000$	
			t provided is gre		of reinforcen	nent requii
				Libra	ary item: Rectangul	ar single sumn
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm		Maximum crack	width	w _k = 0.181 mi	n
Limiting clack width			Movimum oro			
		PASS		ck width is les	s than limiting	g crack wie
Rectangular section in shea	r - Section 6.2	PASS		ck width is les	s than limiting	g crack wid
-	nr - Section 6.2 V = 52.4 kN/m	PASS	Design shear re		V _{Rd.c} = 112.6	
Rectangular section in shea				sistance	V _{Rd.c} = 112.6	kN/m
Rectangular section in shea	V = 52.4 kN/m	PAS	Design shear re	sistance	V _{Rd.c} = 112.6	kN/m
Rectangular section in shea Design shear force	V = 52.4 kN/m	PAS - Section 9.3	Design shear re	sistance ar resistance e	V _{Rd.c} = 112.6 exceeds desig	kN/m n shear fo i
Rectangular section in shea Design shear force Secondary transverse reinfo	V = 52.4 kN/m Difference in the base A _{bx.req} = 226 mm	PAS • - Section 9.3 n²/m	Design shear re S S - Design shea	esistance ar resistance e reinforcement	V _{Rd.c} = 112.6 exceeds design s _{bx_max} = 450 p	kN/m n shear foi mm

Tekla. Tedds	Project	Project Jo 205 Albany Street				Job no. 20-186	
	Calcs for	Calcs for Si Retaining wall with top prop			Start page no./Revision 6		
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021	



Reinforcement details

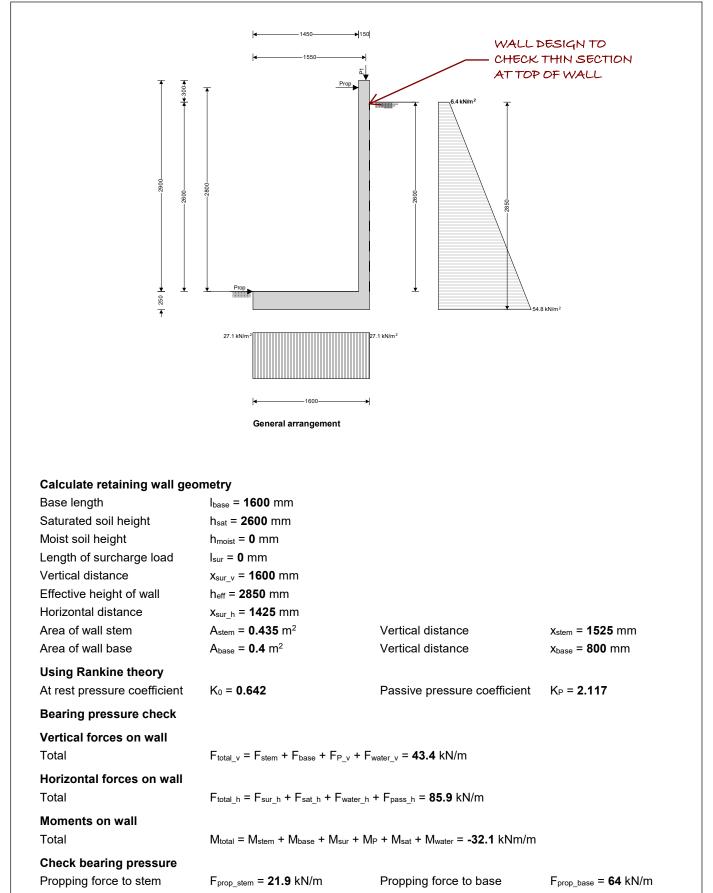
Tekla. Tedds	Project J 205 Albany Street					Job no. 20-186	
	Calcs for S Retaining wall - Top section check				Start page no./Revision 1		
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021	

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

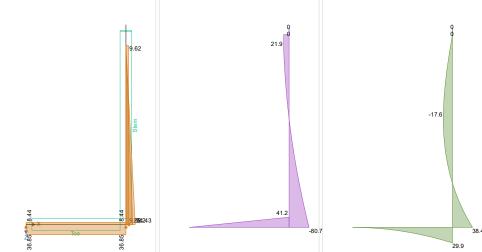
Tedds calcul	ation version	2.9.11
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Retaining wall details			
Stem type	Propped cantilever		
Stem height	h _{stem} = 2900 mm		
Prop height	h _{prop} = 2800 mm		
Stem thickness	t _{stem} = 150 mm		
Angle to rear face of stem	α = 90 deg		
Stem density	γ _{stem} = 25 kN/m ³		
Toe length	l _{toe} = 1450 mm		
Base thickness	t _{base} = 250 mm		
Base density	γ _{base} = 25 kN/m ³		
Height of retained soil	h _{ret} = 2600 mm	Angle of soil surface	$\beta = 0 \operatorname{deg}$
Depth of cover	d _{cover} = 0 mm		
Height of water	h _{water} = 2600 mm		
Water density	γ _w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γ _{mr} = 21 kN/m ³		
Saturated density	γ _{sr} = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{r.k} = 21 deg	
Characteristic wall friction ang	le	$\delta_{r.k}$ = 10.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γь = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{b.k} = 21 deg	
Characteristic wall friction ang	le	δ _{b.k} = 10.5 deg	
Characteristic base friction and	gle	δ _{bb.k} = 14 deg	
Presumed bearing capacity	P _{bearing} = 125 kN/m ²		
Loading details			
Variable surcharge load	Surcharge _Q = 10 kN/m ²		
Vertical line load at 1550 mm	P _{G1} = 19.8 kN/m		
	P _{Q1} = 2.7 kN/m		

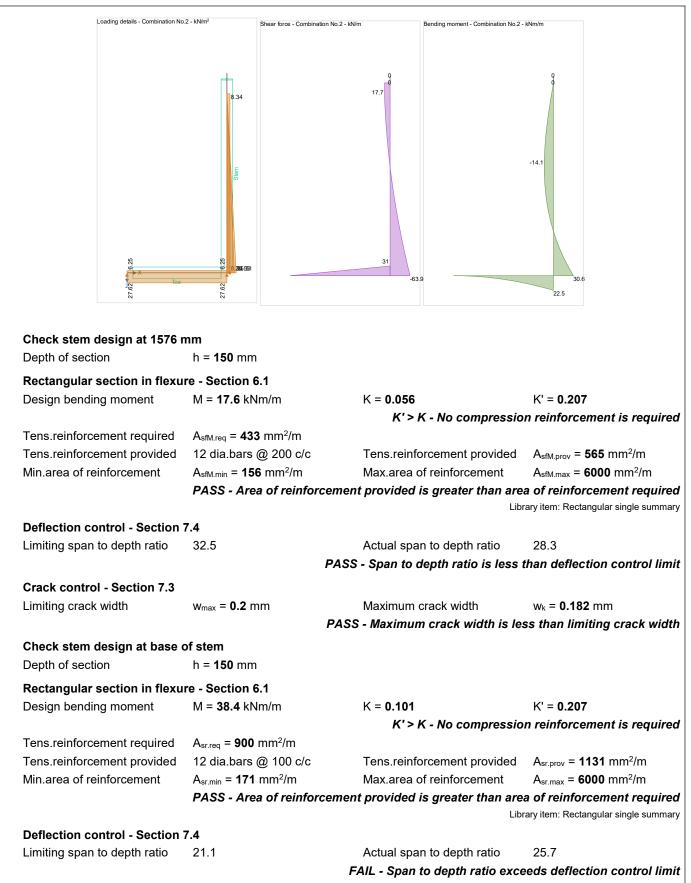
Tekla. Tedds	Project Jo 205 Albany Street					186	
	Calcs for	Calcs for Si Retaining wall - Top section check				Start page no./Revision 2	
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021	



Tekla. Tedd		205 Alb	Job no. 20-186			
	Calcs for	Retaining wall -	Top section ch	neck	Start page no./	Revision 3
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/202
Bearing pressure at toe	q _{toe} = 27.1 kN	I/m ²	Bearing press	sure at heel	q _{heel} = 27.1 k	:N/m ²
Factor of safety	FoS _{bp} = 4.61	1				
	PASS -	Allowable beari	ng pressure e	xceeds maximu	m applied bea	aring pressu
RETAINING WALL DESIGN	-					
In accordance with EN199		porating Corrige	endum dated J	January 2008 an	d the UK Nati	onal Annex
incorporating National Am	endment No.1				Toddo oplaul	ation version 2.9
					Teuus calcul	
Concrete details - Table 3.	-	deformation ch	aracteristics f	or concrete		
Concrete strength class	C32/40	-2	Maan avial tanaila atranath		$f = 2.0 \text{ M/mm}^2$	
Char.comp.cylinder strength			Mean axial tensile strength		$f_{ctm} = 3.0 \text{ N/mm}^2$	
Secant modulus of elasticity		N/mm²	Maximum aggregate size f _{cd} = 18.1 N/mm²		h _{agg} = 20 mm	
Design comp.concrete stren	gin		$I_{cd} = 18.1 \text{ N/fr}$	100-	Partial factor	γc = 1.5
Reinforcement details						
Characteristic yield strength			Modulus of elasticity		E _s = 200000 N/mm ²	
Design yield strength	f _{yd} = 435 N/m	m ²	Partial factor		γs = 1.15	
Cover to reinforcement						
Front face of stem	c _{sf} = 35 mm		Rear face of	stem	c _{sr} = 35 mm	
Top face of base	c _{bt} = 35 mm		Bottom face of	of base	c _{bb} = 75 mm	
Loading details - Combination	n No.1 - kN/m² s	Shear force - Combination No.1 - kN	/m Ben	ding moment - Combination No.1 -	kNm/m	
			<u>^</u>			
			21.9		0	
	9.62		21.5			
					-17.6	

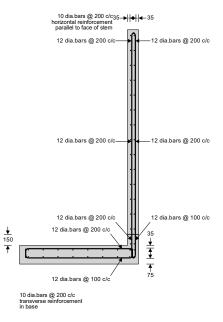


Tekla. Tedds	Project				Job no. 20-	186
	Calcs for S Retaining wall - Top section check				Start page no./Re	evision 4
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



Tekla Tedds	Project	205 Alba	any Street		Job no. 20-186		
	Calcs for	Retaining wall - ⁻	Start page no./F	Revision 5			
	Calcs by	Calcs date	Approved by	Approved date			
	BS	12/03/2021	Checked by SHL	Checked date 12/03/2021	SHL	12/03/202	
Crack control - Section 7.3							
Limiting crack width	w _{max} = 0.2 mm	PASS	Maximum crack - Maximum cra		w _k = 0.17 mm s than limitin		
Rectangular section in shear							
Design shear force	V = 80.7 kN/m	PAS	Design shear re S - Design shea		V _{Rd.c} = 84.1 k exceeds desig		
Check stem design at prop			, i i i i i i i i i i i i i i i i i i i		Ĭ		
Depth of section	h = 150 mm						
Rectangular section in shear							
Design shear force	V = 21.9 kN/m	DAS	Design shear re S - Design shea		$V_{Rd.c}$ = 66.7 k		
Horizontal reinforcement par	allal to face of s		-	ar resistance e	xceeus uesig	in Shear Toro	
Min.area of reinforcement	$A_{sx.req} = 283 \text{ mm}$		Max.spacing of	reinforcement	s _{sx max} = 400	mm	
Trans.reinforcement provided	10 dia.bars @ 2		Trans.reinforcer				
	PASS - Area of	reinforcement	provided is gre	eater than area	of reinforcer	ment require	
Check base design at toe							
Depth of section	h = 250 mm	λ					
Rectangular section in flexur					1/1 0 007		
Design bending moment	M = 29.9 kNm/n	1	K = 0.033 K' > K - N	o compressior	K' = 0.207 reinforceme	nt is require	
Tens.reinforcement required	A _{bb.req} = 428 mm	ı²/m		0 0011101 000101		in io require	
Tens.reinforcement provided	12 dia.bars @ 1	00 c/c	Tens.reinforcem	nent provided	Abb.prov = 113	1 mm²/m	
Min.area of reinforcement	A _{bb.min} = 266 mm		Max.area of reir		A _{bb.max} = 1000		
	PASS - Area of	reinforcement	provided is gre		ary item: Rectangul	-	
Crack control - Section 7.3						C C	
Limiting crack width	w _{max} = 0.2 mm		Maximum crack	width	w _k = 0.142 m	m	
		DVCC	- Maximum cra	ck width is les	s than limitin	a crack widt	
		FA33				J	
Rectangular section in shear		FASS				-	
-	- Section 6.2 V = 41.2 kN/m		Design shear re		V _{Rd.c} = 112.6	kN/m	
Rectangular section in shear Design shear force	V = 41.2 kN/m	PAS				kN/m	
Rectangular section in shear Design shear force Secondary transverse reinfor	V = 41.2 kN/m	PAS - Section 9.3	Design shear re S - Design shea	ar resistance e	xceeds desig	kN/m I n shear forc	
Rectangular section in shear Design shear force	V = 41.2 kN/m	<i>PAS</i> - Section 9.3 ^{2/} m	Design shear re	ar resistance e reinforcement	xceeds desig s _{bx_max} = 450	kN/m I n shear forc	

Tekla. Tedds	Project				Job no. 20-	186
	Calcs for S Retaining wall - Top section check				Start page no./Re	evision 6
	Calcs by BS	Calcs date 12/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



Reinforcement details

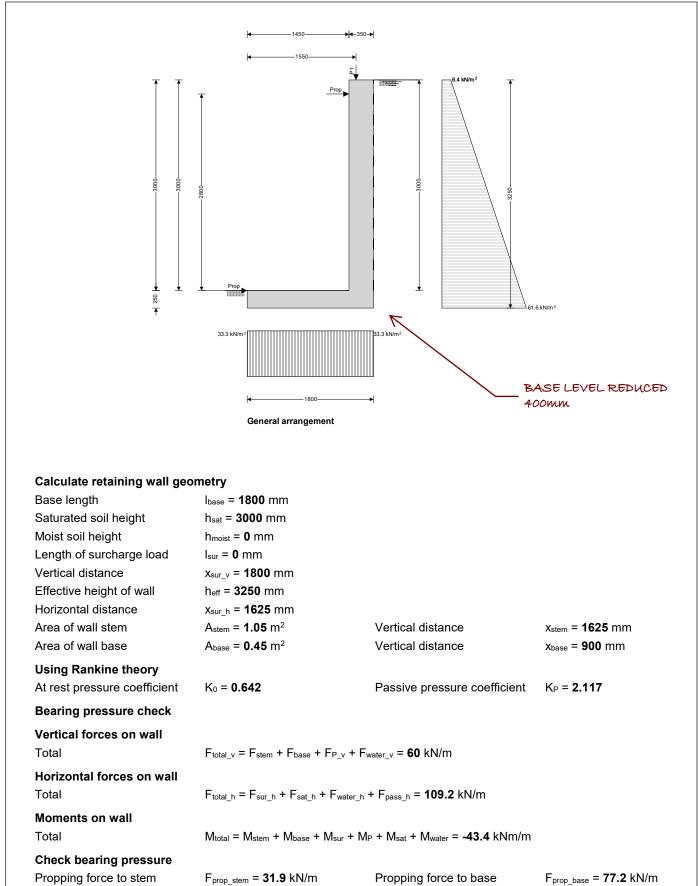
Tekla. Tedds	Project				Job no. 20-	-186
	Calcs for S Ret wall - reduced level check			Start page no./Revision 1		
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 29/03/2021	Approved by SHL	Approved date 29/03/2021

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.11

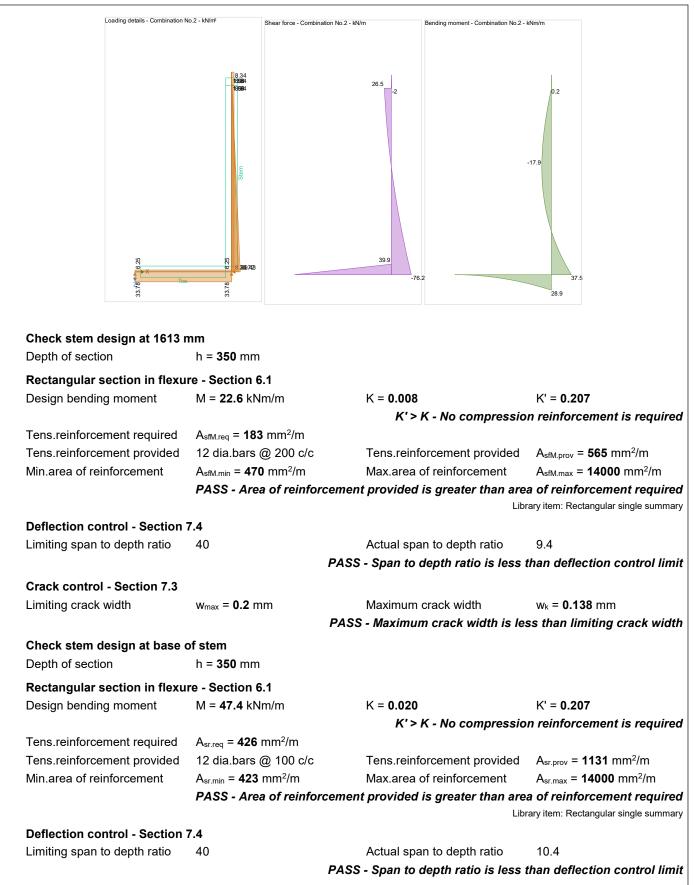
Retaining wall details			
Stem type	Propped cantilever		
Stem height	h _{stem} = 3000 mm		
Prop height	h _{prop} = 2800 mm		
Stem thickness	t _{stem} = 350 mm		
Angle to rear face of stem	α = 90 deg		
Stem density	γ _{stem} = 25 kN/m ³		
Toe length	I _{toe} = 1450 mm		
Base thickness	t _{base} = 250 mm		
Base density	γ _{base} = 25 kN/m ³		
Height of retained soil	h _{ret} = 3000 mm	Angle of soil surface	$\beta = 0 \deg$
Depth of cover	d _{cover} = 0 mm		
Height of water	h _{water} = 3000 mm		
Water density	γw = 9.8 kN/m³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γ _{mr} = 21 kN/m ³		
Saturated density	γ _{sr} = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{r.k} = 21 deg	
Characteristic wall friction ang	le	δ _{r.k} = 10.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γь = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{b.k} = 21 deg	
Characteristic wall friction ang	le	δ _{b.k} = 10.5 deg	
Characteristic base friction an	gle	δ _{bb.k} = 14 deg	
Presumed bearing capacity	P _{bearing} = 125 kN/m ²		
Loading details			
Variable surcharge load	Surcharge _Q = 10 kN/m ²		
Vertical line load at 1550 mm	P _{G1} = 19.8 kN/m		
	P _{Q1} = 2.7 kN/m		

Tekla. Tedds	Project				Job no. 20-186	
	Calcs for S Ret wall - reduced level check				Start page no./Re	evision 2
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 29/03/2021	Approved by SHL	Approved date 29/03/2021

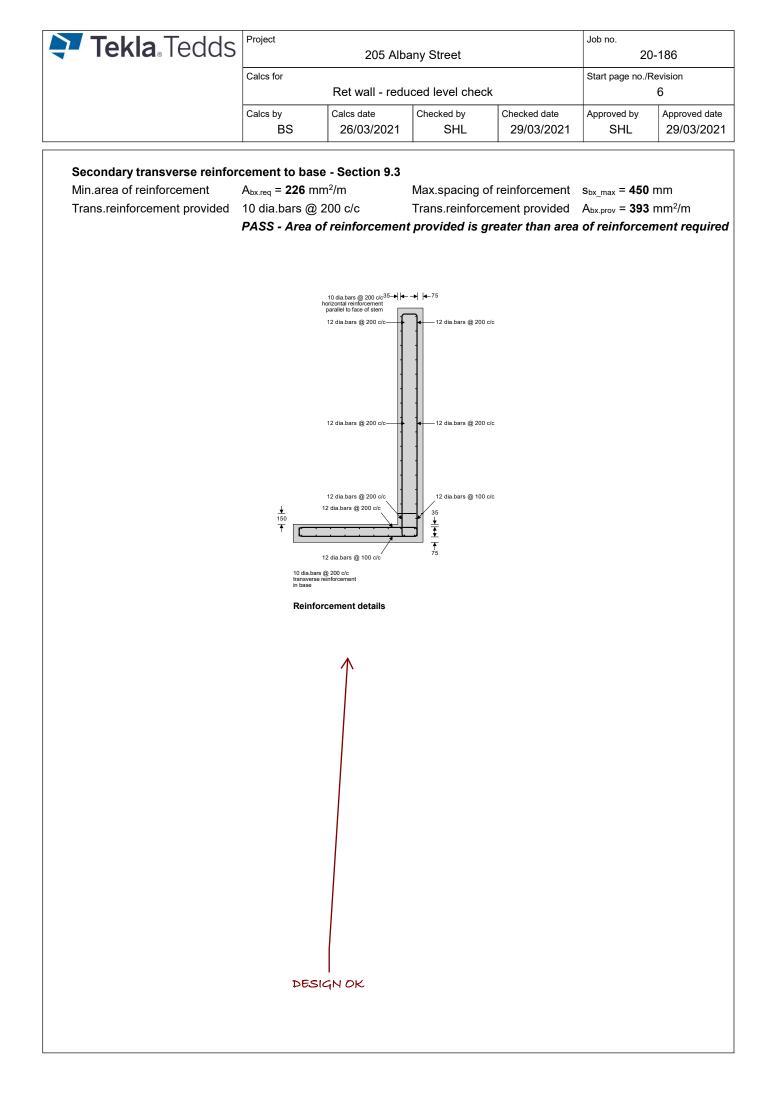


Tekla Tedds	Project	205 Alb	Job no. 20	-186			
	Calcs for	^{cs for} Ret wall - reduced level check			Start page no./Revision 3		
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 29/03/2021	Approved by SHL	Approved date 29/03/202	
Bearing pressure at toe	q _{toe} = 33.3 kN/r	m²	Bearing press	ure at heel	q _{heel} = 33.3 kl	N/m ²	
Factor of safety	FoS _{bp} = 3.75						
	PASS - A	llowable bearin	ng pressure ex	ceeds maximu	m applied bea	ring pressu	
RETAINING WALL DESIGN							
In accordance with EN1992-	-	orating Corrige	ndum dated Ja	anuary 2008 an	d the UK Natio	onal Annex	
incorporating National Amer	ndment No.1				To data a standa	tion version 2.9	
					ledds calcula	tion version 2.9	
Concrete details - Table 3.1	-	leformation cha	aracteristics fo	or concrete			
Concrete strength class	C32/40				(0.0 N/	0	
Char.comp.cylinder strength		f _{ck} = 32 N/mm ²		Mean axial tensile strength		f _{ctm} = 3.0 N/mm ²	
Secant modulus of elasticity	Ecm = 33346 N/	'mm²	Maximum aggregate size		h _{agg} = 20 mm		
Design comp.concrete strengt	h		f _{cd} = 18.1 N/m	m²	Partial factor	γc = 1.5 0	
Reinforcement details							
Characteristic yield strength	f _{yk} = 500 N/mm	2	Modulus of elasticity		E _s = 200000 N/mm ²		
Design yield strength	f _{yd} = 435 N/mm	2	Partial factor		γs = 1.15		
Cover to reinforcement							
Front face of stem	c _{sf} = 35 mm		Rear face of s	tem	c _{sr} = 75 mm		
Top face of base	c _{bt} = 35 mm		Bottom face o	fbase	c _{bb} = 75 mm		
Loading details - Combination N	lo.1 - kN/m² She	ar force - Combination No.1 - kN/	m Bendir	ng moment - Combination No.1 -	kNm/m		
	9.62 29662		33				
	29662		-2.4		0.2		

Tekla. Tedds	Project 205 Albany Street				Job no. 20-	-186
	Calcs for S Ret wall - reduced level check				Start page no./Revision 4	
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 29/03/2021	Approved by SHL	Approved date 29/03/2021



Tekla Tedds	Project	205 Alba	any Street		Job no.)-186
	Calcs for	Pot wall rodu	iced level check		Start page no./F	
		Calcs date	Checked by	Checked date	Approved by	5 Approved da
	Calcs by BS	26/03/2021	SHL	29/03/2021	SHL	29/03/20
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm		Maximum crac	k width	w _k = 0.147 m	m
		PASS	- Maximum cr	ack width is les	ss than limitin	g crack wid
Rectangular section in shear	- Section 6.2					
Design shear force	V = 96.7 kN/m		Design shear r	resistance	V _{Rd.c} = 143 k	N/m
		PAS	S - Design she	ear resistance e	exceeds desig	n shear foi
Check stem design at prop						
Depth of section	h = 350 mm					
Rectangular section in flexu	re - Section 6.1					
Design bending moment	M = 0.2 kNm/m		K = 0.000		K' = 0.207	
200.9.1.201.0.1.9	•••			No compressio		nt is reauir
Tens.reinforcement required	A _{sr1.req} = 2 mm ² /m	า		,		
Tens.reinforcement provided	12 dia.bars @ 20		Tens.reinforce	ment provided	A _{sr1.prov} = 565	mm²/m
Min.area of reinforcement	A _{sr1.min} = 423 mm	²/m	Max.area of re	inforcement	A _{sr1.max} = 140	00 mm²/m
	PASS - Area of	reinforcement	t provided is g	reater than area	of reinforcement requir	
				Libr	ary item: Rectangu	lar single sumn
Deflection control - Section 7	7.4					
Limiting span to depth ratio	16		Actual span to	depth ratio	0.7	
		PASS	- Span to dept	th ratio is less t	han deflectio	n control lii
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm		Maximum crac	k width	w _k = 0.001 m	m
		PASS	- Maximum cr	ack width is les	ss than limitin	g crack wie
Rectangular section in shear	r - Section 6.2					
Design shear force	V = 33 kN/m		Design shear r	resistance	V _{Rd.c} = 135.3	kN/m
		PAS	S - Design she	ear resistance e	exceeds desig	In shear foi
Horizontal reinforcement par	allel to face of st	em - Section 9	9.6			
Min.area of reinforcement	A _{sx.req} = 350 mm ²	/m	Max.spacing o	f reinforcement	s _{sx_max} = 400	mm
Trans.reinforcement provided	10 dia.bars @ 20)0 c/c	Trans.reinforce	ement provided	A _{sx.prov} = 393	mm²/m
	PASS - Area of I	reinforcement	t provided is g	reater than area	a of reinforce	ment requii
Check base design at toe						
Depth of section	h = 250 mm					
Rectangular section in flexu	re - Section 6.1					
Design bending moment	M = 38.7 kNm/m		K = 0.042		K' = 0.207	
			K' > K - I	No compressio	n reinforceme	nt is requir
Tens.reinforcement required	$A_{bb.req} = 554 \text{ mm}^2$	²/m				
Tens.reinforcement provided	12 dia.bars @ 10)0 c/c	Tens.reinforce	ment provided	Abb.prov = 113	1 mm²/m
Min.area of reinforcement	A _{bb.min} = 266 mm ²		Max.area of re		A _{bb.max} = 100	
	PASS - Area of I	reinforcement	t provided is g		a of reinforcei ary item: Rectangu	-
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm		Maximum crac	k width	w _k = 0.185 m	m
-		PASS		ack width is les		
Postangular costion in choose	- Section 6.2					
Rectallular section in shear						
Rectangular section in shear Design shear force	V = 53.3 kN/m		Design shear r	resistance	V _{Rd.c} = 112.6	kN/m



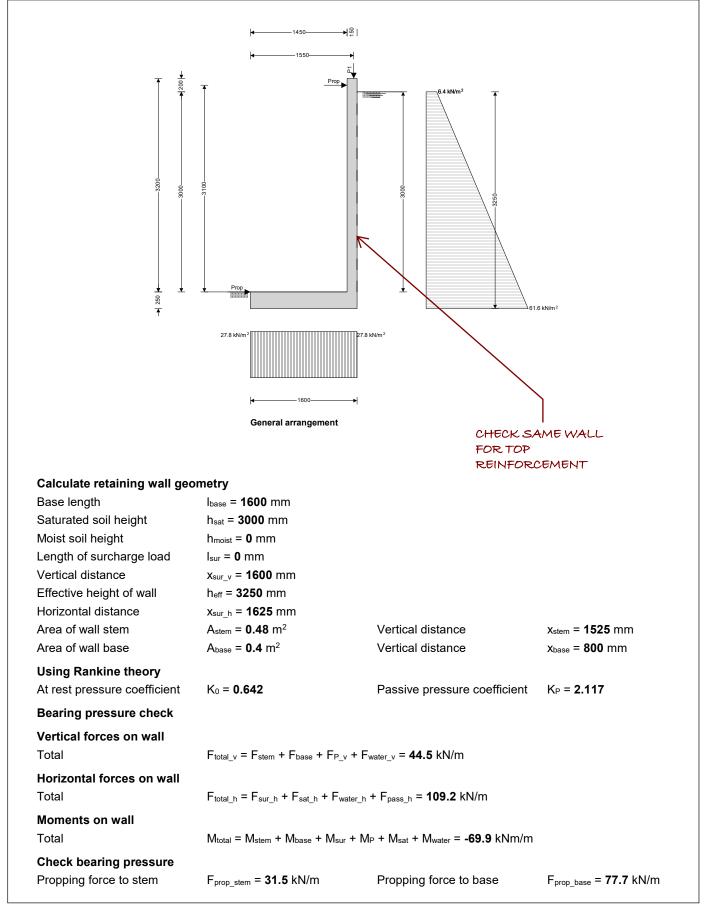
Tekla. Tedds	Project 205 Albany Street				Job no. 20	-186
	Calcs for S Top section check (reduced level)				Start page no./Revision 1	
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.11

Retaining wall details			
Stem type	Propped cantilever		
Stem height	h _{stem} = 3200 mm		
Prop height	h _{prop} = 3100 mm		
Stem thickness	t _{stem} = 150 mm		
Angle to rear face of stem	α = 90 deg		
Stem density	γ _{stem} = 25 kN/m ³		
Toe length	I _{toe} = 1450 mm		
Base thickness	t _{base} = 250 mm		
Base density	γ _{base} = 25 kN/m ³		
Height of retained soil	h _{ret} = 3000 mm	Angle of soil surface	$\beta = 0 \operatorname{deg}$
Depth of cover	d _{cover} = 0 mm		
Height of water	h _{water} = 3000 mm		
Water density	γ w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γ _{mr} = 21 kN/m ³		
Saturated density	γ _{sr} = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{r.k} = 21 deg	
Characteristic wall friction ang	le	$\delta_{r.k}$ = 10.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γь = 21 kN/m ³		
Characteristic effective shear	resistance angle	φ' _{b.k} = 21 deg	
Characteristic wall friction ang	le	δ _{b.k} = 10.5 deg	
Characteristic base friction an	gle	δ _{bb.k} = 14 deg	
Presumed bearing capacity	P _{bearing} = 125 kN/m ²		
Loading details			
Variable surcharge load	Surcharge _Q = 10 kN/m ²		
Vertical line load at 1550 mm	P _{G1} = 19.8 kN/m		
	P _{Q1} = 2.7 kN/m		

Tekla. Tedds	Project				Job no. 20-186	
	Calcs for Top section check (reduced level)			Start page no./R	evision 2	
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



Tekla. Tedds	Project	205 Alba	Job no. 20-186			
	Calcs for	op section che	ck (reduced lev	/el)	Start page no./R	Revision 3
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021
Bearing pressure at toe	q _{toe} = 27.8 kN/m ²	2	Bearing press	ure at heel	q _{heel} = 27.8 kl	N/m²
Factor of safety	•	FoS _{bp} = 4.494 PASS - Allowable bearing pressu		ceeds maximur	m applied bea	rina pressure
			5 /			
RETAINING WALL DESIGN In accordance with EN1992-	1-1·2004 incorpor	ating Corrige	ndum dated J	anuary 2008 and	d the LIK Natio	nal Annex
incorporating National Amer	=	ating ourriger		2000 and		
					Tedds calcula	tion version 2.9.11
Concrete details - Table 3.1 ·		formation cha	racteristics fo	or concrete		
Concrete strength class	C32/40	formation cha			6 0 0 0 0	2
Concrete strength class Char.comp.cylinder strength	C32/40 f _{ck} = 32 N/mm ²		Mean axial ter	nsile strength	f _{ctm} = 3.0 N/m	
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m		Mean axial ter Maximum agg	nsile strength regate size	h _{agg} = 20 mm	
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity Design comp.concrete strengt	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m		Mean axial ter	nsile strength regate size	oun	
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity Design comp.concrete strengt Reinforcement details	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m h		Mean axial ter Maximum agg f _{cd} = 18.1 N/m	nsile strength regate size m²	h _{agg} = 20 mm	γc = 1.50
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity Design comp.concrete strengt	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m		Mean axial ter Maximum agg	nsile strength regate size m²	h _{agg} = 20 mm Partial factor	γc = 1.50
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity Design comp.concrete strengt Reinforcement details Characteristic yield strength	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m h f _{yk} = 500 N/mm ²		Mean axial ter Maximum agg f _{cd} = 18.1 N/m Modulus of ela	nsile strength regate size m²	$h_{agg} = 20 \text{ mm}$ Partial factor E _s = 200000	γc = 1.50
Concrete strength class Char.comp.cylinder strength Secant modulus of elasticity Design comp.concrete strengt Reinforcement details Characteristic yield strength Design yield strength	C32/40 f _{ck} = 32 N/mm ² E _{cm} = 33346 N/m h f _{yk} = 500 N/mm ²		Mean axial ter Maximum agg f _{cd} = 18.1 N/m Modulus of ela	nsile strength regate size m ² asticity	$h_{agg} = 20 \text{ mm}$ Partial factor E _s = 200000	γc = 1.50

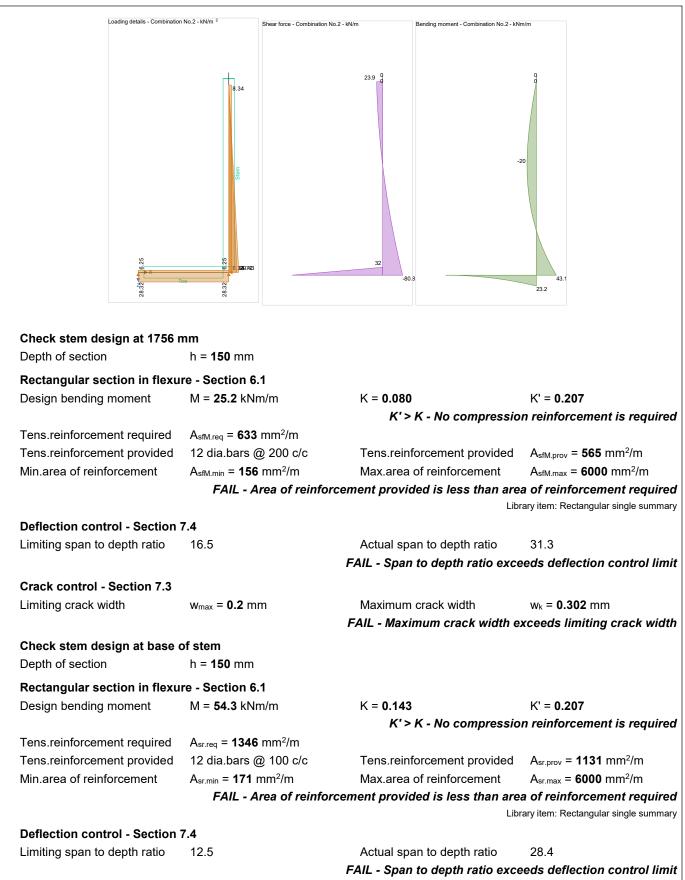
42.6

37.8

102.3

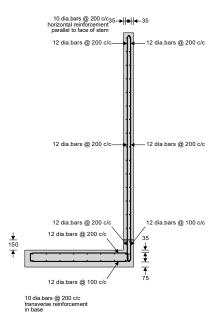
30.9

Tekla. Tedds	Project				Job no. 20-186	
	Calcs for S Top section check (reduced level)			Start page no./Revision 4		
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



Tekla. Tedds	Project	205 Alba	any Street		Job no.)-186
	Calcs for	Fop section che	ck (reduced leve	1)	Start page no./F	Revision 5
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved 12/03/2
Crack control - Section 7.3	Ч	I	1	1	1	
Limiting crack width	w _{max} = 0.2 mm	F	Maximum crack AIL - Maximum		w _k = 0.277 m cceeds limitin	
Rectangular section in shear						
Design shear force	V = 102.3 kN/m		Design shear re Design shear re		V _{Rd.c} = 84.1 k ss than desig	
Check stem design at prop						
Depth of section	h = 150 mm					
Rectangular section in shear Design shear force	- Section 6.2 V = 29.7 kN/m		Design shear re	sistance	V _{Rd.c} = 66.7 k	(N/m
		PAS	S - Design shea			
Horizontal reinforcement par	allel to face of s	tem - Section S	9.6			
Min.area of reinforcement	A _{sx.req} = 283 mm		Max.spacing of			
Trans.reinforcement provided	10 dia.bars @ 2 PASS - Area of		Trans.reinforce	-		
Check base design at toe		N	,			
Depth of section	h = 250 mm		ONLY THIS	CHECK		
Rectangular section in flexur	e - Section 6.1		REQUIRED			
Design bending moment	M = 30.9 kNm/n	n	K = 0.034 K' > K - N	o compressioi	K' = 0.207 n reinforceme	ent is requ
Tens.reinforcement required	A _{bb.req} = 442 mm					_
Tens.reinforcement provided Min.area of reinforcement	12 dia.bars @ 1 A _{bb.min} = 266 mn		Tens.reinforcen Max.area of reir	-	$A_{bb.prov} = 113$ $A_{bb.max} = 100$	
	PASS - Area of			eater than area		ment requ
Crack control - Section 7.3						
Limiting crack width	w _{max} = 0.2 mm	PASS	Maximum crack - Maximum cra		w _k = 0.147 m s <i>than limitin</i>	
Rectangular section in shear						
Design shear force	V = 42.6 kN/m	PAS	Design shear re S - Design shea		V _{Rd.c} = 112.6 exceeds desig	
Secondary transverse reinfor						
Min.area of reinforcement	A _{bx.req} = 226 mm		Max.spacing of		_	
Trans.reinforcement provided	10 dia.bars @ 2 PASS - Area of		Trans.reinforce	•	-	

Tekla. Tedds	Project				Job no. 20-186	
	Calcs for S Top section check (reduced level)			Start page no./Re	evision 6	
	Calcs by BS	Calcs date 26/03/2021	Checked by SHL	Checked date 12/03/2021	Approved by SHL	Approved date 12/03/2021



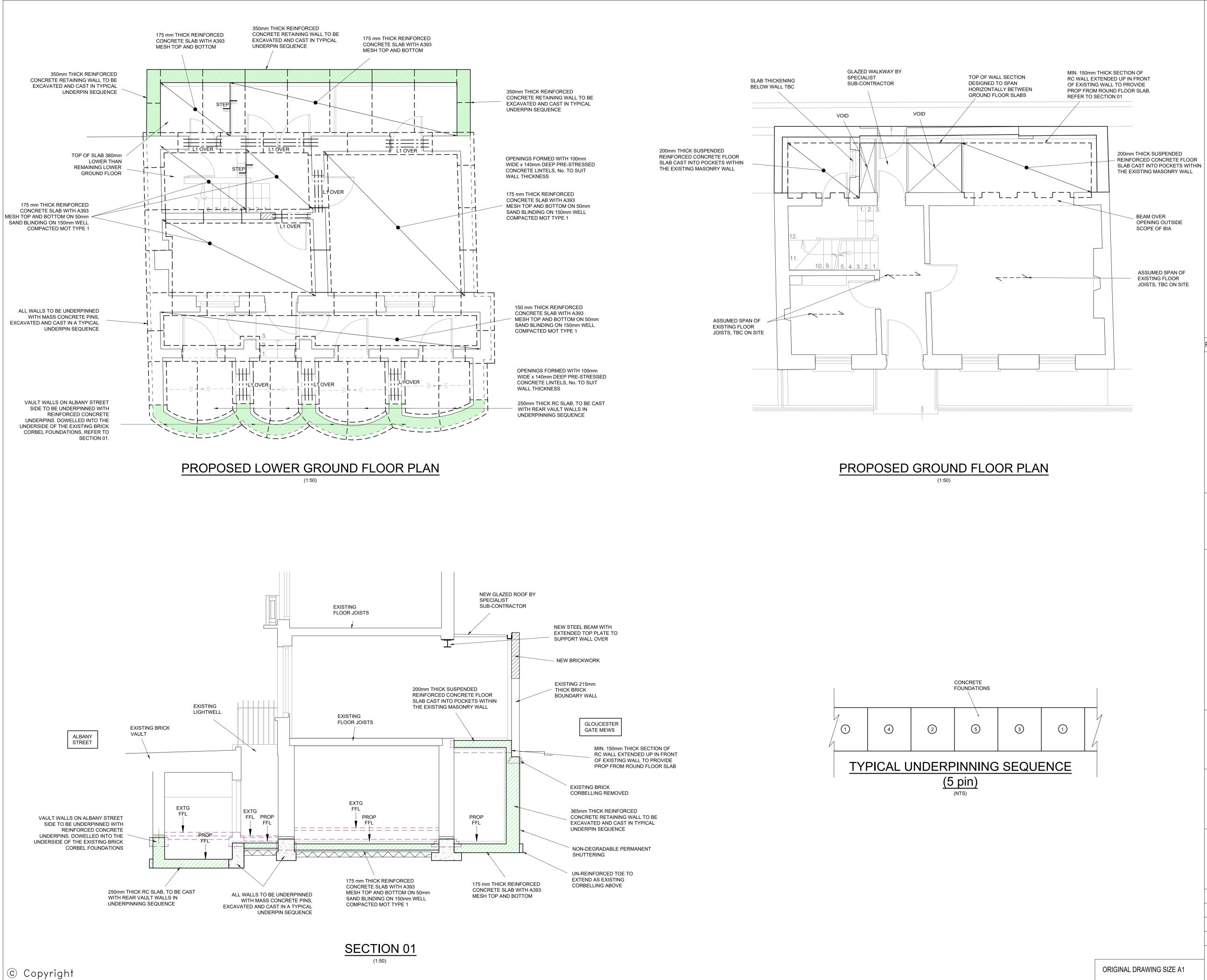
Reinforcement details



205 Albany Street

5.0 QED DRAWINGS

Date:Mar 20Project Number:20-186Revision Number:BR1



NOTES:-

- 1. THIS DRAWING IS NOT TO BE SCALED, WORK FROM GIVEN DIMENSIONS ONLY.
- 2. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, SERVICE ENGINEERS, SPECIALISTS, QED STRUCTURES DRAWINGS, SPECIFICATIONS AND BILL OF QUANTITIES.
- 3. ANY DISCREPANCIES OR VARIATIONS ARE TO BE REPORTED IMMEDIATELY TO QED STRUCTURES FOR CLARIFICATION BEFORE THE AFFECTED WORK COMMENCES.
- 4. ALL DIMENSIONS ARE TO BE CHECKED ON SITE, ANY DISCREPANCIES TO BE REPORTED TO QED STRUCTURES IMMEDIATELY BEFORE THE AFFECTED WORK COMMENCES.
- 5. ALL PROPRIETARY PRODUCTS ARE TO BE USED IN ACCORDANCE WITH THE MANUFACTURERS DETAILS AND SPECIFICATIONS.
- 6. THE CONTRACTOR IS TO ENSURE THAT TEMPORARY LOADS PLACED ONTO THE NEW STRUCTURES SHALL BE LESS THAN THE LOADS FOR WHICH IT HAS BEEN DESIGNED.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY WORKS AND FOR THE STABILITY OF THE WORKS IN PROGRESS.
- 8. FOR THE FULL COMPREHENSIVE GENERAL NOTES SEE GENERAL NOTES DRAWING 100.

SCAN QR CODE FOR ALL THE LATEST DRAWINGS ON THE PROJECT

Rev	Description	Date	Ву	Checked
P1	FOR BIA ONLY	12.03.21	BS	SHL
P2	FLOOR LEVELS REVISED	29.03.21	BS	SHL
l r				

FOR BASEMENT IMPACT ASSESSMENT ONLY



CONSULTING STRUCTURAL & CIVIL ENGINEERS

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Title

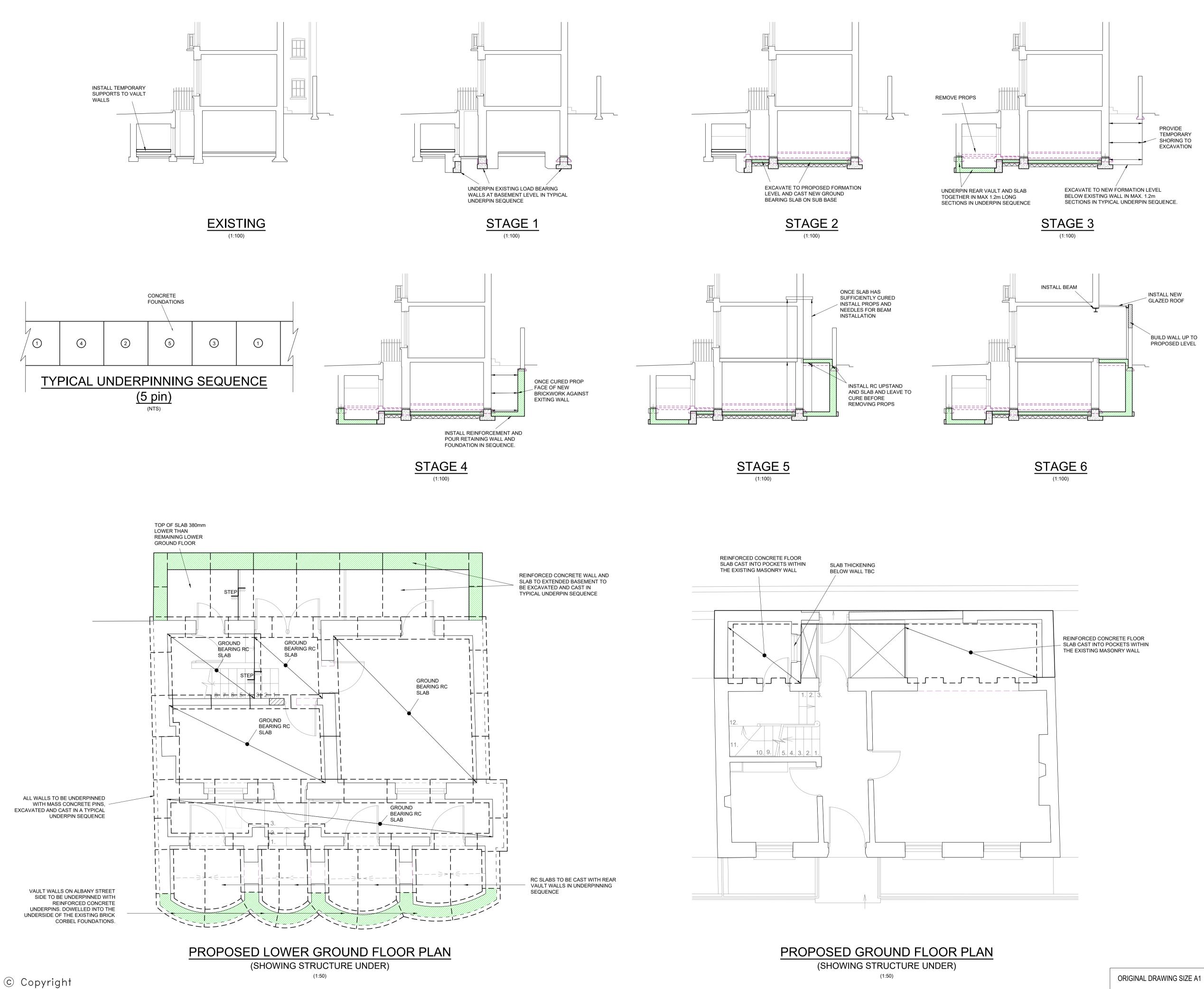
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AFRIJANUS PROPERTY **UK LIMITED**

Project 205 ALBANY STREET LONDON

GENERAL ARRANGEMENT **PLANS & SECTIONS**

Date MAR 21	Drawn BS		Checked	SHL	
Distribution Office LONDON					
Scale AS INDICATE	ED	Engineer	BS		
Approved SHL		Status	FOR BIA		
Project Number Dr		rawing Number		Revision	
20-306	BIA		1	P2	



NOTES:-

- 1. THIS DRAWING IS NOT TO BE SCALED, WORK FROM GIVEN DIMENSIONS ONLY.
- 2. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, SERVICE ENGINEERS, SPECIALISTS, QED STRUCTURES DRAWINGS, SPECIFICATIONS AND BILL OF QUANTITIES.
- 3. ANY DISCREPANCIES OR VARIATIONS ARE TO BE REPORTED IMMEDIATELY TO QED STRUCTURES FOR CLARIFICATION BEFORE THE AFFECTED WORK COMMENCES.
- 4. ALL DIMENSIONS ARE TO BE CHECKED ON SITE, ANY DISCREPANCIES TO BE REPORTED TO QED STRUCTURES IMMEDIATELY BEFORE THE AFFECTED WORK COMMENCES.
- 5. ALL PROPRIETARY PRODUCTS ARE TO BE USED IN ACCORDANCE WITH THE MANUFACTURERS DETAILS AND SPECIFICATIONS.
- 6. THE CONTRACTOR IS TO ENSURE THAT TEMPORARY LOADS PLACED ONTO THE NEW STRUCTURES SHALL BE LESS THAN THE LOADS FOR WHICH IT HAS BEEN DESIGNED.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY WORKS AND FOR THE STABILITY OF THE WORKS IN PROGRESS.
- 8. FOR THE FULL COMPREHENSIVE GENERAL NOTES SEE GENERAL NOTES DRAWING 100.

SCAN QR CODE FOR ALL THE LATEST DRAWINGS ON THE PROJECT

Rev	Description	Date	Ву	Checked
P1 P2	FOR BIA ONLY FLOOR LEVELS REVISED	12.03.21 29.03.21	BS BS	SHL SHL

FOR BASEMENT IMPACT ASSESSMENT ONLY



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RADLETT

Title

CONSTRUCTION METHOD STATEMENT

Date MAR 21	Drawn BS		Checked	SHL	
Distribution Office LONDON					
Scale AS INDICATE	ED	Engineer	BS		
Approved SHL		Status	FOR BIA		
Project Number	Project Number D		ber	Revision	
20-306	20-306		BIA 02 P2		