



Auger Ref:



| Job Information | |
|-----------------|---------------|
| Client | Crawford & Co |
| Client ref | |
| Visit date | 05/07/2021 |
| Report date | 07/07/2021 |











lob Information

Overview

Brief

Auger were commissioned by Crawford & Co to undertake a site investigation and CCTV inspection of the underground drainage within the area of concern at the property.

Findings

Line 1 - From SVP1 downstream to MH1

There were no defects noted within the line which could be allowing an escape of water. The line was seen to be free flowing and serviceable. The SVP was found to extend into the manhole and then reached the channel within the manhole via an internal dropper.

Line 2 - From MH1 downstream

There were no defects noted within the line which could be allowing an escape of water. The line was seen to be free flowing and serviceable.

Line 3 - From MH1 upstream

There were no defects noted within the line which could be allowing an escape of water. The line was seen to be free flowing and serviceable.

Drain Survey

Line 4 - From SVP2 downstream to MH1

Our CCTV survey revealed joint displacements to the SVP rest bend and the pipework downstream from this point, this will be resulting in an escape of water. The SVP was found to extend below the ground to the level of the manhole before reaching the rest bend and then extending into the manhole.

RWP1

We were unable to survey this RWP as the above ground pipework was found to be of a small diameter in which we could not carry out a below ground break-in. We then scoped to attempt a below ground break-in on site, however, we discovered a gas main which runs in this location. We did however discover that the branch connection to this line on Line 2 contained no defects.

The above mentioned defects to the below ground drainage system have been caused by ground movement.

Recommendations

It is recommended that the following repairs are carried out to prevent an escape of water from the system:

Refer Back to Client

 $\begin{tabular}{ll} Line~2-Auger recommend to install~1.5m of 100mm flexi liner from MH1 upstream then install~a~100mm radial patch on the rest bend from MH1. \\ \end{tabular}$

Please Note: Deep MH entry will be required.

We will now refer the claim back to the client in order to progress.

Once repairs have been undertaken the customer should ensure the drainage system is periodically inspected in the future for any deterioration and kept free flowing / free of blockages. Any damage noted during future inspections should be repaired immediately in accordance with current Building Regulations.

With any repair process, complications and unforeseen circumstances can arise. These scenarios will be reported whilst on-site and could potentially cause an increase in repair costs and inconvenience.

Repair Caveats

If any of the above lining recommendations fail then excavation and replacement of the pipework would be required. This would severely increase the cost of repairs and would provide greater inconvenience to the residents.

Recommendations have been made to reline or patch reline sections of the drainage system at the property. This process combines a number of chemicals in a resin, which then harden in a fibreglass matting to create a new section of drain within the original. The reaction creates a strong smell which can linger for up to 72 hours once works are completed - this is not harmful. It is recommended that any areas where smells are experienced are kept well ventilated until the odour subsides.

Photographs

Trial Hole 1

Fig 1.1: Trial Hole 11 ocation



Fig 1.2: Trial Hole 1 Footing



Trial Hole 2

Fig 2.1: Trial Hole 2 Location



Fig 2.2: Trial Hole 2 Footing



CCTV Stills

Fig 3.1: Line 4, joint displacement



Fig 3.2: Line 4, joint displacement



Other Photos

Fig 4.1: Side of property







Fig 4.3: Side of property

Fig 4.4: Side of propert



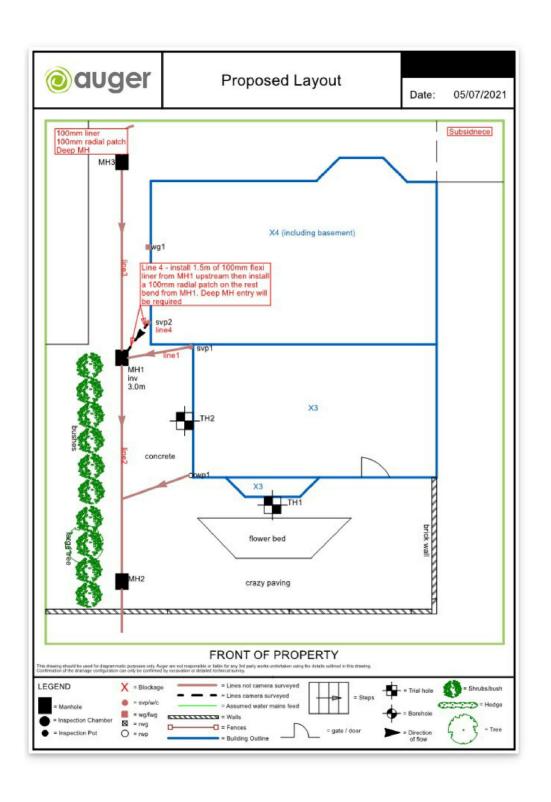


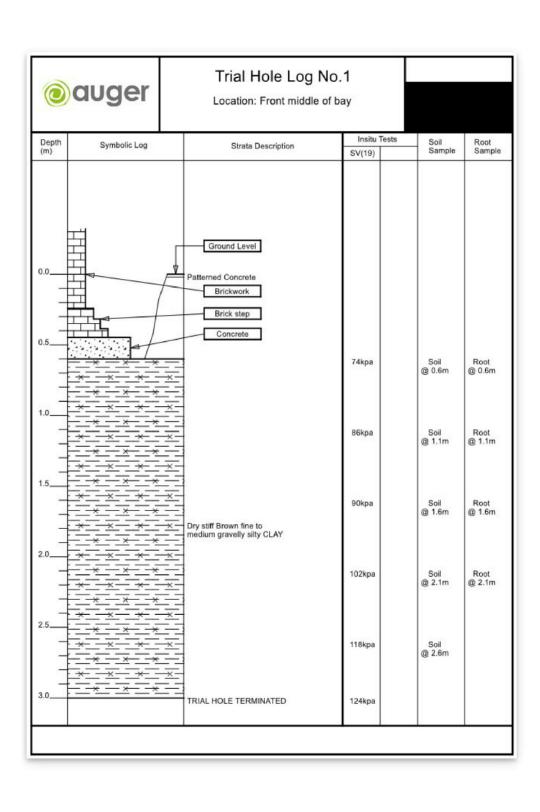
Fig 4.5: Soil samples collected

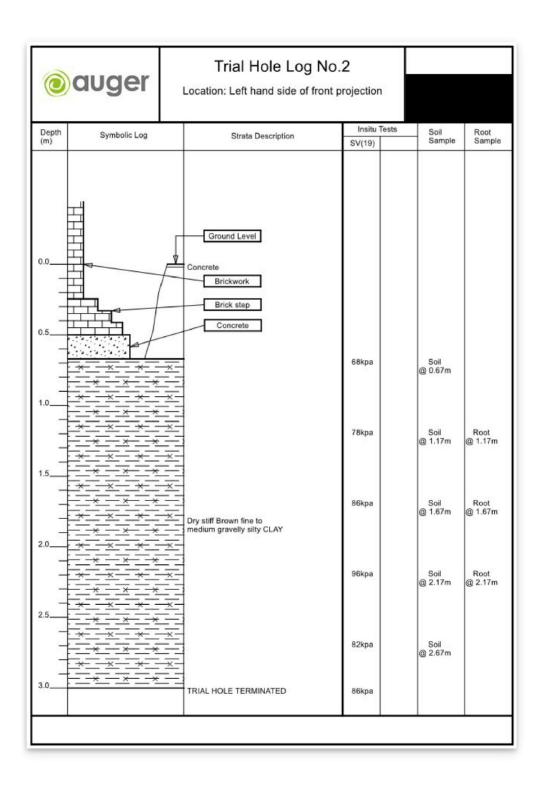
Fig 4.6: Root samples collected















16/07/2021



Dear Sirs

Root ID

The samples you sent in relation to the above on 05/07/2021 have been examined. Their structures were referable as follows:

| TH1, 0.60n | 1 | |
|------------|---|---|
| 3 no. | Examined root: TILIA (Lime). Less than 0.15mm in diameter. | Alive, recently*. |
| TH1, 1.10n | 1 | |
| 3 no. | Examined root: very THIN (under 0.1mm in diameter). We cannot rule out TILIA (Lime). | Dead* (note this 'dead' result can be unreliable with such thin samples). |
| 2 no. | Both pieces of BARK only - insufficient material for recognition. | |
| TH1, 1.60n | 1 | |
| 2 no. | Examined root: an unusual sample; also less than 0.1mm in diameter. NOT coniferous. Similar in some ways to the family MAGNOLIACEAE (Magnolia and Liriodendron (Tulip Tree)). Tentative. | Alive, recently*. |
| 2 no. | Both samples revealed too few cells for microscopic identification. | |
| TH1, 2.10n | 1 | |
| 1 no. | Examined root: again, tentatively like the family MAGNOLIACEAE (Magnolia and Liriodendron (Tulip Tree)). As above, under 0.1mm in diameter. | Dead* (as previously, this 'dead' result could be unreliable). |
| 3 no. | Unfortunately all with insufficient cells for identification. | |
| TH2, 1.17n | 1 | |
| 2 no. | Examined root: a SHRUB, similar in some ways to the family CAPRIFOLIACEAE (the most common members being Viburnum (Laurestinus and Guelder-rose), Weigela, Symphoricarpos (Snowberry), Lonicera (Honeysuckle)). | Alive, recently*. |
| 1 no. | A piece of BARK only, insufficient material for identification. | |
| | | |

/ continued overleaf

| TH2, 1.67r | ΓH2, 1.67m | | | | | | |
|------------|--|--|--|--|--|--|--|
| 1 no. | Examined root: most referable to TILIA (Lime). Not more than 0.15mm in diameter. | Dead* (as above, this 'dead' result could be an unreliable one). | | | | | |
| 3 no. | Unfortunately all with insufficient cells for identification. | | | | | | |
| TH2, 2.17r | ΓH2, 2.17m | | | | | | |
| 3 no. | Examined root: TILIA (Lime). Another THIN sample | Dead*. | | | | | |

Click here for more information: MAGNOLIACEAE TILIA

I trust this is of help. Please call us if you have any queries; our Invoice is enclosed.

Yours faithfully

Dr Ian B K Richardson

Based mainly on the lodine test for starch. Starch is present in some cells of a living woody root, but is more or less rapidly broken down by soil micro-organisms on death of the root, sometimes before decay is evident. This result need not reflect the state of the parent tree.

* * Try out our web site on www.botanical.net * *





Geotechnical Testing Analysis Report



Summary Of Claim Details

| Policy Holder | Unknown |
|--------------------|---------------|
| Risk Address | Unknown |
| SI Date | 05/07/2021 |
| Issue Date | 05/07/2021 |
| Report Date | 19/07/2021 |
| Auger Reference | |
| Insurance Company | Allianz |
| LA Claim Reference | |
| LA Co. Reference | Crawford & Co |

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced except in full, without the prior written approval of the laboratory.

| Checked | 19/07/2021 | Wayne Honey |
|----------|------------|-------------|
| Approved | 19/07/2021 | Paul Evans |

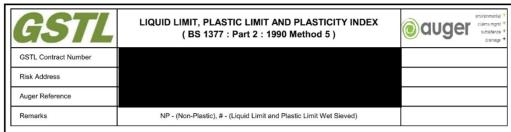


| GSTL | LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX (BS 1377 : Part 2 : 1990 Method 5) DESCRIPTIONS | auger | environmental claims mgmt subsidence dreinage |
|----------------------|--|-------|--|
| GSTL Contract Number | | | |
| Risk Address | | | |
| Auger Reference | | | |
| | | | |

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|------------|----------------|-----------|--|
| TH | Sample Type | Depth (m) | Sample Description |
| Trial Hole | | | |
| TH1 | D | 0.60 | Brown fine to medium gravelly silty CLAY |
| TH1 | D | 1.10 | Brown fine to medium gravelly silty CLAY |
| TH1 | D | 1.60 | Brown fine to medium gravelly silty CLAY |
| TH1 | D | 2.10 | Brown fine to medium gravelly silty CLAY |
| TH1 | D | 2.60 | Brown fine to medium gravelly silty CLAY |
| | | | |
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| | | | |
| TH2 | D | 0.67 | Brown fine to medium gravelly silty CLAY |
| TH2 | D | 1.17 | Brown fine to medium gravelly silty CLAY |
| TH2 | D | 1.67 | Brown fine to medium gravelly silty CLAY |
| TH2 | D | 2.17 | Brown fine to medium gravelly silty CLAY |
| TH2 | D | 2.67 | Brown fine to medium gravelly silty CLAY |
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| Test Operator | Checked | 19/07/2021 | Wayne Honey | |
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| Luke Williams | Approved | 19/07/2021 | Paul Evans | |





| TH Trial Hole | Sample Type | Depth (m) | Moisture Content % | Liquid Limit % | Plastic Limit % | Plasticity index % | Passing .425mm % | NHBC Chapter 4.2 | Remarks |
|------------------|----------------|-----------|-----------------------|----------------------|-----------------------|--|------------------------|------------------|--------------------|
| TH1 | D | 0.60 | 27 | 68 | 21 | 47 | 91 | HIGH VCP | CH High Plasticity |
| TH1 | D | 1.10 | 27 | | | | | | |
| TH1 | D | 1.60 | 26 | 57 | 19 | 38 | 90 | MEDIUM VCP | CH High Plasticity |
| TH1 | D | 2.10 | 30 | | | | | | |
| TH1 | D | 2.60 | 28 | 66 | 19 | 47 | 92 | HIGH VCP | CH High Plasticity |
| | | | | | | | | | |
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| TH2 | D | 0.67 | 30 | 51 | 22 | 29 | 93 | MEDIUM VCP | CH High Plasticity |
| TH2 | D | 1.17 | 27 | | | | | | |
| TH2 | D | 1.67 | 28 | 61 | 23 | 38 | 93 | MEDIUM VCP | CH High Plasticity |
| TH2 | D | 2.17 | 28 | | | | | | |
| TH2 | D | 2.67 | 19 | 61 | 24 | 37 | 93 | MEDIUM VCP | CH High Plasticity |
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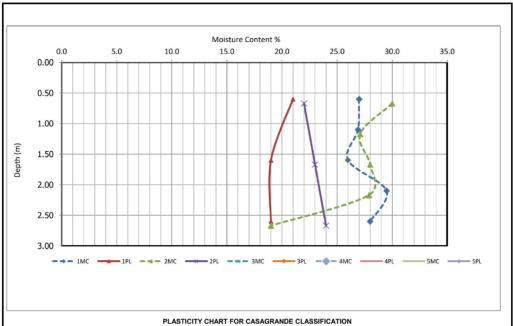
: Non Classified

Modified Plasticity Index (PI) <10 Modified PI = 10 to <20 Modified PI = 20 to <40 Low volume change potential (LOW VCP)
 Medium volume change potential (Med VCP)
 High volume change potential (HIGH VCP) Modified PI = 40 or greater

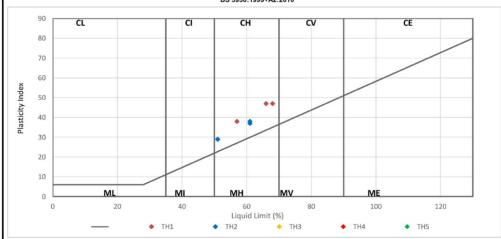
The Atterberg Limits May also be used to classify the volume change potential of fine soils using the National House building system, as given in the NHBC's Standards Chapter 4.2 (2003) "Building Near Trees"

| Test Operator | Checked | 19/07/2021 | Wayne Honey | |
|---------------|----------|------------|-------------|--|
| Luke Williams | Approved | 19/07/2021 | Paul Evans | |





BS 5930:1999+A2:2010



Modified Plasticity Index (PI) <10 Modified PI = 10 to <20

: Non Classified

Modified PI = 20 to <40 Modified PI = 40 or greater

: Low volume change potential (LOW VCP)
: Medium volume change potential (Med VCP)
: High volume change potential (HIGH VCP)

The Atterberg Limits May also be used to classify the volume change potential of fine soils using the National House building system, as given in the NHBC's Standards Chapter 4.2 (2003) "Building Near Trees"

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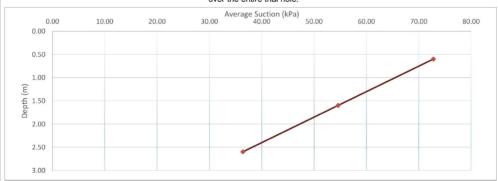
| GSTL | SUMMARY OF SOIL CLASSIFICATION TESTS, BRE Information Paper IP 4/93 February 1993 (CI/SfB p1), BRE Information Paper Digest 412 ci/SFb (A3s) February 1996 | envicormental. claims mgmt. 4 subsidence 4 chainage 4 |
|----------------------|--|--|
| GSTL Contract Number | | |
| Risk Address | | |
| Auger Reference | | |
| Remarks | D - Disturbed (Recompacted 2.5kg Rammer), U - Undisturbed Sample | |

| TH Trial Hole | Depth (m) | Filter Paper Location | Filter Paper | Sample Prep Method | Test Duration (Days) | Water Content (%) | Soil Suction Pk (kPa) | Average Soil Suction Pk (kPa) | Cumalative Heave Potential (mm) from bottom of the hole |
|------------------|--------------|--------------------------|-----------------|--------------------------|----------------------------|-------------------------|-----------------------------|----------------------------------|---|
| TH1 | | Тор | I | D | 5 | 54.1 | 56 | | |
| TH1 | 0.60 | Middle | П | D | 5 | 44.5 | 118 | 73 | 2 |
| TH1 | 1 | Bottom | III | D | 5 | 59.9 | 44 | | |
| TH1 | | | | | | | | | |
| TH1 | 1.10 | | | | | | | | |
| TH1 | 1 | | | | | | | | |
| TH1 | | Тор | I | D | 5 | 48.9 | 73 | | |
| TH1 | 1.60 | Middle | П | D | 5 | 62.9 | 39 | 55 | 0 |
| TH1 | 1 | Bottom | III | D | 5 | 55.7 | 53 | | |
| TH1 | | | | | | | | | |
| TH1 | 2.10 | | | | | | | | |
| TH1 | 1 | | | | | | | | |
| TH1 | | Тор | I | D | 5 | 87.4 | 17 | | |
| TH1 | 2.60 | Middle | П | D | 5 | 51.9 | 63 | 36 | 0 |
| TH1 | 1 | Bottom | III | D | 5 | 70.3 | 30 | | |
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Heave potential is calculated from the bottom of the hole and heaves above the bottom of the hole are reported as a cumalative value.

The values reported for heave above only apply to the strata the suction and plasticity have been performed on. The shallowest depth reported is assumed to be a strata thickness to GL and Heave is calculated based on that layer thickness, if the next sample is in 0.5m increments the heave is calculated based on the layer thickness of 0.5m and depths 1m from the sample above will include heave over 1m.

Consideration should be made for other stratas where values are not reported and when working out the heave potential over the entire trial hole.



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|---------------|----------|------------|-------------|
| Luke Williams | Approved | 19/07/2021 | Paul Evans |



| GSTL | SUMMARY OF SOIL CLASSIFICATION TESTS, BRE Information Paper IP 4/93 February 1993 (CI/SfB p1), BRE Information Paper Digest 412 ci/sFb (A3s) February 1996 | orwinomental claims more: substitutions of creatings of c |
|----------------------|--|--|
| GSTL Contract Number | | |
| Risk Address | | |
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| Remarks | D - Disturbed (Recompacted 2.5kg Rammer), U - Undisturbed Sample | |

| TH Trial Hole | Depth (m) | Filter Paper Location | Filter Paper | Sample Prep Method | Test Duration (Days) | Water Content (%) | Soil Suction Pk (kPa) | Average Soil Suction Pk (kPa) | Cumalative Heave Potential (mm) from bottom of the hole |
|------------------|--------------|--------------------------|-----------------|--------------------------|----------------------------|-------------------------|-----------------------------|----------------------------------|---|
| TH2 | 0.67 | Тор | I | D | 5 | 60.2 | 43 | | |
| TH2 | 0.67 | Middle | П | D | 5 | 57.2 | 49 | 47.4 | 0 |
| TH2 | 0.67 | Bottom | III | D | 5 | 56.9 | 50 | | |
| TH2 | 1.17 | | | | | | | | |
| TH2 | 1.17 | | | | | | | | |
| TH2 | 1.17 | | | | | | | | |
| TH2 | 1.67 | Тор | I | D | 5 | 64.2 | 37 | | |
| TH2 | 1.67 | Middle | П | D | 5 | 56.4 | 51 | 53.5 | 0 |
| TH2 | 1.67 | Bottom | III | D | 5 | 48.9 | 73 | | |
| TH2 | 2.17 | | | | | | | | |
| TH2 | 2.17 | | | | | | | | |
| TH2 | 2.17 | | | | | | | | |
| TH2 | 2.67 | Тор | I | D | 5 | 57.8 | 48 | | |
| TH2 | 2.67 | Middle | П | D | 5 | 67.0 | 33 | 41.3 | 0 |
| TH2 | 2.67 | Bottom | III | D | 5 | 60.5 | 43 | | |
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Heave potential is calculated from the bottom of the hole and heaves above the bottom of the hole are reported as a cumalative value.

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Consideration should be made for other stratas where values are not reported and when working out the heave potential over the entire trial hole.



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