Job Number: 210221

Basement Impact Assessment

For proposed alterations at:

40 Leighton Road Kentish Town Camden NW5 2QE

For:

Matthew Jones & Louise Willcocks

Report by	Sandy Caulee
Report Reviewed by	Geoff Watson
Structural Design Reviewed by	Chris Tomlin
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Rev	Date	Author	Checker	Comment
-	13.04.2021	SC	GW	First Issue
1	17.11.2021	SC	GW	Audit Amendments
2	8.12.21	SC	CT	Soil parameter
3	8.2.22	ct	ct	Walls top propped





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1. Non-Technical Summary

1.1. Existing Property, Site & Neighbouring Sites

The property comprises a mid-terrace building which is two storeys high above street level. There is an existing Lower Ground Floor. This has restricted headroom and leads directly to the rear garden, which is higher than the internal Lower Ground Floor but slightly lower than street level at the front. The neighbouring properties are of a similar age and construction and already have full height Lower Ground Floors.

1.2. Proposed Development

The proposed works involve deepening and extending the existing Lower Ground Floor (also referred to as the Basement, in this report). This will include the creation of a light-well at the front of the property.



Figure 1: Aerial view with approx. site area indicated

1.3. Land Stability, Hydrogeology and Surface Water

Summaries of the impacts relating to Land Stability, Hydrogeology and Surface Water are contained within the BIA by Ground and Project Consultants Ltd.



2. Report Authors and Qualifications

2.1 Land Stability / Slope Stability

Croft has appointed the following suitably qualified professional to assess the impacts related to Land Stability:

Mr Jon Smithson of Ground and Project Consultants Ltd

This assessment has been reviewed by:

Phil Henry MEng CEng MICE Croft Structural Engineers

2.2 Hydrogeology and Groundwater Flooding

Croft has appointed the following suitably qualified professional to assess the impacts related to Hydrogeology and Groundwater Flooding:

Mr Jon Smithson of Ground and Project Consultants Ltd

2.3 Hydrology, Surface Water Flooding and Sewer Flooding

The following individuals have reviewed the impacts related to Surface Water and Flooding:

Phil Henry MEng CEng MICE Croft Structural Engineers

Chris Tomlin MEng CEng MIStructE Croft Structural Engineers

2.4 Sources of Information

The following baseline data have been referenced to complete the BIA in relation to the proposed development:

• Site walkover survey (08/03/2021);



- LB Camden, Strategic Flood Risk Assessment (produced by URS, 2014);
- LB Camden, Floods in Camden, Report of the Floods Scrutiny Panel (2013);
- LB Camden, Planning Guidance (CPG) Basements (January 2021);
- LB Camden, Camden Geological, Hydrogeological and Hydrological Study Guidance for

Subterranean Development (produced by Arup, 2010);

- LB Camden, Local Plan Policy A5 Basements (2017);
- LB Camden's Audit Process Terms of Reference;

Other sources of data are referred to within the relevant sections of this report.







3. Introduction

3.1. Site & location

The property is situated within Kentish Town, as shown below. .



Figure 2: Plan view of site (approx. area outlined in red) and the surrounding properties

Currently there is and external patio level with the Lower Ground Floor with stepped access to the raised rear garden.

Further information on the site and the surrounding area is contained in Section 4.

3.2. Proposed works

The proposed works involve deepening and extending the existing Lower Ground Floor *. This will include the creation of a light-well at the front of the property. Drawings illustrating the extent of the proposals are appended. Proficiency Design & Build has produced drawings showing the proposed architectural layouts, which are available separately.

* Throughout this report, the Lower Ground Floor may also be referred to as the Basement.



4. Desk Study & Walk over Survey

4.1. General Desk Study

4.1.1. Site History

The property is believed to have existed for over 150 years. Since the original construction, the property has been extended to the rear. This is understood to have been done towards the end of the 20th century.

4.1.2. Listed Buildings

The existing building is not listed. Data from Historic England shows that there are no listed buildings immediately adjacent to the site.



Figure 3 Extract from Historic England (listed buildings shown by blue triangle)

The site is in a conservation area.

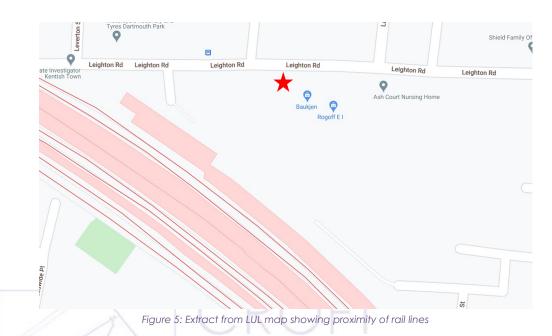


Figure 4 Extract from Camden Council's website (orange area indicates the coverage of a conservation area)



4.1.3. London Under Ground and Network Rail Infrastructure

The site is more than 80m away from the nearest national rail line and the nearest subterranean train line. These are unlikely to be affected by the new basement.



4.1.4. Highways

The site is within 5m of the public highway and the front lightwell is within 5m of the pavement.

4.1.5. UK Power Network

There are no significant items of electrical infrastructure (such as pylons, substations or tunnels) in the immediate vicinity.

4.1.6. Utility Search

A utility search has been completed and is appended. After the planning application is concluded, the design team should carry out an additional search to confirm that they have the most up to date information on buried services.

4.2. Walk Over Survey

A structural engineer from Croft Structural Engineers visited the site on 8th March 2021.

4.2.1. Site and Existing Property

The property is constructed from traditional building materials (brickwork and timber). The construction appears to differ slightly in elevation from the neighbouring properties suggesting that these were built at different times. The walls of the main building are shared with the properties either side and the front façade adjoins the neighbouring buildings forming a terrace property.





Figure 6 - 40 Leighton Road

The property sits on a gentle slope. Further descriptions of the topography are contained within the BIA report by Ground and Project Consultants Ltd. . The properties at either side of 40 Leighton Road already have basements constructed which will provide favourable conditions when designing the basement for 40 Leighton Road.

The external area in front of the property is covered with paving stones.

4.2.2. Proximity of Trees

The back garden of 40 Leighton Road has a few trees present, with the closest to the back wall of the building being a Japanese Laurel tree within 6m of the existing building. Other tress, located at a distance of 10m away or more from the back wall include a common persimmon, a river birch, arrow bamboo and butternut trees.

Guidance contained in BS 5837: 2012 Trees in relation to design, demolition and construction – Recommendations may need to be incorporated into the final construction method statement.

4.2.3. Adjacent Properties

4.2.3.1. 42 Leighton Road – Property to Left

42 Leighton Road is of a similar age and construction to the subject property. It was noted that during the Engineer's site visit, the property has a full height Lower Ground Floor. .





Figure 7 Neighbouring property

4.2.3.2. 38 Leighton Road – Property to Right

38 Leighton Road is of a similar age and construction to the subject property. This property has a full height Lower Ground Floor which is understood to have been constructed within the last two years. This is known to extend from below the middle of the main building into the rear garden.



Figure 8 Neighbouring property



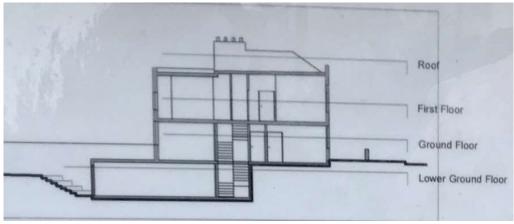


Figure 9 Cross section showing extent of basement at No. 38

5. Construction Methodology and Engineer Statements

5.1. Outline Geotechnical Design Parameters

From the Geological report and soil investigation reasonably conservative geotechnical parameters typical for London Clay should be applied to the design.

5.2. Hydro Static Pressure

Initial readings did not record ground water.

The permanent design should account for water at higher levels to allow for accidental hydrostatic loading (burst mains etc).

5.2.1. Intended Use & Loadings

	UDL kN/m ²	Concentrated Load kN
Domestic Single Dwellings	1.5	2.0

Below ground level, the reinforced concrete retaining walls are designed to carry the lateral loading applied from above.

The lateral earth pressure exerts a horizontal force on the retaining walls. The retaining walls will be checked for resistance to the overturning force this produces.

Lateral forces will be applied from:

- Soil loads
- Hydrostatic pressures
- Surcharge loading from behind the walls.

These produce retaining wall thrust. This will be restrained by the opposing retaining wall.



5.2.1.1. Surcharge Loading

The following will be applied as surcharge loads to the front/ front lightwell retaining walls:

- 10kN/m² if within 45° of road
- 100kN point loads if under road or within 1.5m
- 5kN/m² if within 45° of Pavement

Other areas:

- Garden Surcharge 2.5kN/m² + 1 m of soil (if present above basement ceiling) 20kN/m²
- Surcharge for adjacent property 1.5kN/m² + 4kN/m² for concrete ground bearing slab

5.3. Permanent Design Proposals

Reinforced concrete cantilevered retaining walls will form the new foundation of the property.

Appendix A shows the calculations of one of the most heavily loaded retaining wall. The most critical parameters have been used for this. The design of the retaining walls was calculated using software by TEDDS. The software is specifically designed for retaining walls and ensures that the construction is kept to a limit to prevent damage to the adjacent property.

The overall stability of the walls is designed using $K_a \& K_p$ values, while the design of the wall structure uses K_0 values. This approach minimises the level of movement from the concrete affecting the adjacent properties.

The design also considers floatation as a risk. The design should account for the weight of the building and the uplift forces from the water. The weight of the building is greater than the uplift, resulting in a stable structure.

5.3.1. Temporary works

Walls are designed to be structural stable with top and bottom propping. Temporary propping details will be required to be provided by the contractor and must be completed by a suitability qualified professional.

To demonstrate the feasibility of the works, a proposed basement construction sequence is appended.

5.4. Ground Movement Assessment

A Ground Movement Assessment has been produced by Ground and Project Consultants.



5.5. Control of Construction Works

5.5.1. Control of Construction Works

A construction sequence is proposed and appended. The method has been formulated with Croft's experience of over 500 basements. The procedures described in this statement will mitigate the impacts that the construction of the basement will have on nearby properties.

To reduce the risk to the development:

- Employ a reputable firm that has extensive knowledge of basement works.
- Employ suitably qualified consultants Croft Structural Engineers has completed over 500 basements in the last five years.
- Provide method statements for the contractors to follow
- Investigate the ground this has now been done.
- Record and monitor the properties close by. This is completed by a condition survey under the Party Wall Act, before and after the works are completed.

With the measures listed above, the maximum level of cracking anticipated is 'Hairline' cracking. This can be repaired with normal decorative works. Under the Party Wall Act, minor damage, although unwanted, can be tolerated it is permitted to occur to a neighbouring property as long as repairs are suitability undertaken to rectify this. To mitigate this risk, the Party Wall Act is to be followed and a Party Wall Surveyor will be appointed.

5.5.2. Noise and Nuisance Control

The contractor is to follow the good working practices and guidance laid down in the 'Considerate Constructors Scheme'.

The hours of working will be limited to those allowed; 8am to 5pm Monday to Friday and Saturday Morning 8am to 1pm.

None of the practices cause undue noise that one would typically expect from a construction site (a conveyor belt typically runs at around 70dB).

The site will be hoarded with 8' site hoarding to prevent access.

The hours of working will further be defined within the Party Wall Act.

The site is to be hoarded to minimise the level of direct noise from the site.



Working in the basement generally requires hand tools to be used. The level of noise generally will be no greater than that of digging of soil. The noise is reduced and muffled by the works being undertaken underground. The level of noise from basement construction works is lower than typical ground level construction due to this.

5.5.3. Construction Management Plan

For the Construction Phase Management Plane it may be beneficial to compile a CMP (Construction Management Plan). A suitably qualified person, typically the contractor, would provide the CMP. The items that should be considered are:

- Delivery routes and times
- Expected working hours
- Times when local roads may become bust: school times, other construction sites.
- Volume of muck away, how this is managed and when.
- Required plant
- Noise dust and Vibration
- Waste Management

This is outside the brief of the Basement Impact Assessment and is not covered within Croft's brief.

5.5.4. Monitoring

In order to safeguard the existing structures during underpinning and new basement construction, movement monitoring using total stations or similar is to be undertaken.

Before the works begin, a detailed monitoring report is required to confirm the implementation of the monitoring. The items that this should cover are:

- Risk Assessment to determine level of monitoring
- Scope of Works
- Applicable standards
- Frequency of Monitoring
- Specification for Instrumentation
- Monitoring of Existing cracks
- Monitoring of movement
- Reporting

Croft would recommend that the monitoring frequency should follow:

Pre-construction: Monitored once.

During construction: Monitored after every pin is cast for first 4 no. pins to gauge effect of underpinning. If all is well, monitor after every other pin.

Post construction works: Monitored once.



Trigger values and contingency actions are noted in the table below.

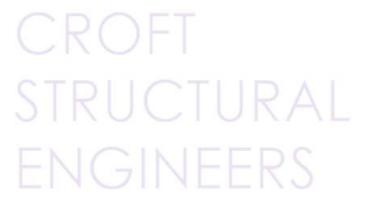
MOVEMENT		CATEGORY	ACTION
Vertical	Horizontal		
Lower limit, 0mm.	Lower limit, 0mm.	Green	No action required
Upper limit to match predicted ground movement from GMA.	Upper limit to match predicted ground movement from GMA.		
Lower limit to match predicted ground movement from GMA. Upper limit = lower limit from GMA +3mm	Lower limit to match predicted ground movement from GMA. Upper limit = lower limit from GMA +3mm	AMBER	Detailed review of Monitoring: Check studs are OK and have not moved. Ensure site staff have not moved studs. If studs have moved reposition. Relevel to ensure results are correct and tolerance is not a concern. Inform Party Wall surveyors of amber readings. Double the monitoring for 2 further readings. If stable revert back. Carry out a local structural review and inspection. Preparation for the implementation of remedial measures should be required. Double number of lateral props
Lower limit = predicted ground movement from GMA + 3mm Upper limit = lower limit from GMA + 5mm	Lower limit = predicted ground movement from GMA + 3mm Upper limit = lower limit from GMA + 5mm	RED	Implement remedial measures review method of working and ground conditions Implement structural support as required; Cease works with the exception of necessary works for the safety and stability of the structure and personnel; Review monitoring data and implement revised method of works



6. Basement Impact Assessment

Impacts on Land Stability, Hydro-geology and Surface Water have been assessed in detail by Ground and Project Consultants Ltd. This contains proposals relating to the design and construction of the basement, which have been accounted for within Croft's proposals in Section 5.







Appendix A: Structural Calculations

Building Regulations will be required after planning. As part of the building control pack full calculations must be undertaken and provided at detailed design stage once planning permission is granted. The calculations must be completed to a recognised Standard (BS or Euro Codes). The calculations must take into account the findings of this report and the recommendations of the auditors.

The design must resist:

- Vertical loads from the proposed works and adjacent properties
- Lateral loads from wind, soil water and adjacent properties
- Loadings in the temporary condition
- All other applied loads on the building
- Uplift forces from hydrostatic effects and soil heave

The final proposed scheme must:

- Provide stability in the temporary condition to all forces
- Provide stability to all forces in the permanent condition

As part of the planning Croft structural engineers has considered some of the pertinent parts of the basement structure to ensure that it can be constructed. The following calculations are not a full set of calculations for the final design which must be provided for building regulations.

Project

40 Leighton Road, NW5 2QE

Basement - Retaining wall design

Section Nos /Page

No. /Revision

Structure



Job No. 210221

RETAINING WALL (BELOW PARTY WALL)

Wall with the most lateral pressures will be a full height retaining wall to the rear of the property, along the boundary with 42 Leighton Road.

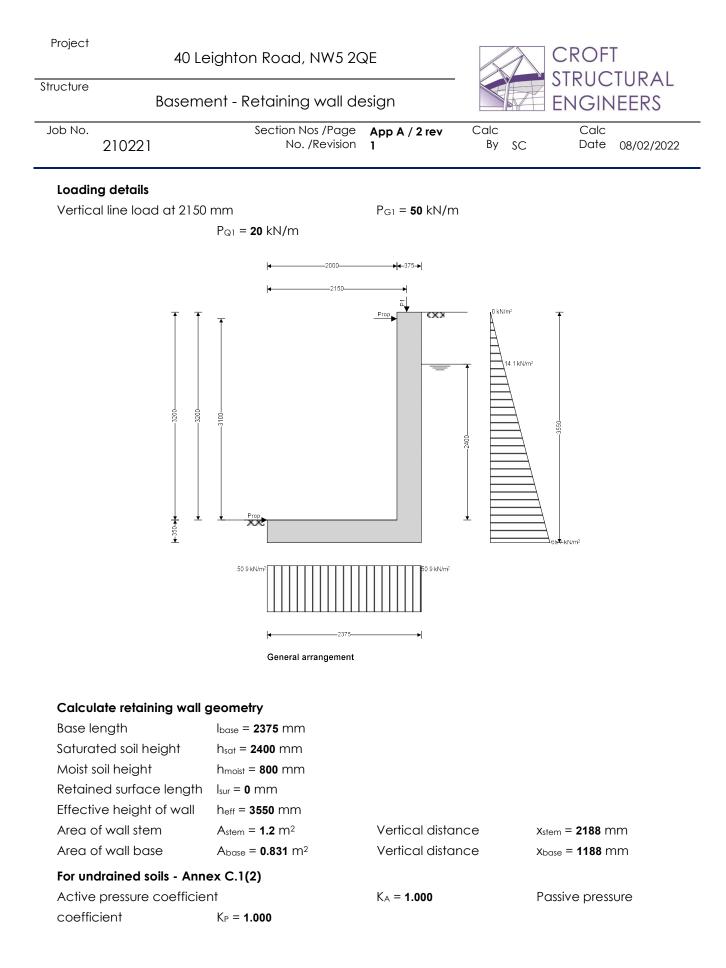
1

RETAINING WALL ANALYSIS

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.14

Retaining wall details			
Stem type	Propped cantilever		
Stem height	h _{stem} = 3200 mm		
Prop height	h _{prop} = 3100 mm		
Stem thickness	t _{stem} = 375 mm		
Angle to rear face of ste	m	α = 90 deg	
Stem density	$\gamma_{stem} = 25 \text{ kN/m}^3$		
Toe length	I _{toe} = 2000 mm		
Base thickness	t _{base} = 350 mm		
Base density	$\gamma_{\text{base}} = 25 \text{ kN/m}^3$		
Height of retained soil	h _{ret} = 3200 mm	Angle of soil surface	$\beta = 0 \deg$
Depth of cover	$d_{cover} = 0 mm$		
Height of water	h _{water} = 2400 mm		
Water density	γ _w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Stiff clay		
Moist density	γ _{mr} = 18 kN/m ³		
Saturated density	γ _{sr} = 19 kN/m ³		
Characteristic effective s	shear resistance angle	$\phi'_{r,k} = 22 \text{ deg}$	
Characteristic wall frictio	n angle	$\delta_{r,k}$ = 12.5 deg	
Base soil properties			
Soil type	Stiff clay		
Soil density	γ _b = 19 kN/m ³		
Characteristic effective s	shear resistance angle	φ' _{b.k} = 22 deg	
Characteristic wall frictio	n angle	$\delta_{b,k}$ = 12.5 deg	
Characteristic base friction	on angle	$\delta_{bb,k}$ = 16.7 deg	
Presumed bearing capa	city	$P_{bearing} = 100 \text{ kN/m}^2$	
	•		



Project

Structure

40 Leighton Road, NW5 2QE



Basement - Retaining wall design

ob No. 210221	Section Nos /Page No. /Revision	App A / 3 rev 1	Calc By SC	Calc Date	08/02/202
Bearing pressure check					
Vertical forces on wall					
Total	$F_{total_v} = F_{stem} + F_{base} + F_{P_v}$	+ F _{water_v} = 120.8	kN/m		
Horizontal forces on wal	1				
Total	F _{total_h} = F _{sat_h} + F _{water_h} + F	_{moist_h} + F _{pass_h} = 1	114.2 kN/m		
Moments on wall					
Total	$M_{total} = M_{stem} + M_{base} + M_{f}$	P + Msat + Mwater	+ M _{moist} = 10)5.6 kNm/m	
Check bearing pressure	à				
Propping force to stem	F _{prop_stem} = 11 kN/m	Propping forc	e to base	F _{prop_base} = 10)3.2 kN/m
Bearing pressure at toe	$q_{toe} = 50.9 \text{ kN/m}^2$	Bearing press			
Factor of safety	FoS _{bp} = 1.966			·	
PASS - AI	lowable bearing pressure	e exceeds ma	ximum ap	plied bearing	g pressure
		e exceeds ma	ximum ap	plied bearing	g pressure
PASS - AI		e exceeds ma	ximum ap _l	plied bearing	g pressure
RETAINING WALL DESIGN				-	
<u>RETAINING WALL DESIGN</u> In accordance with EN1	<u>1</u>	Corrigendum d		-	
<u>RETAINING WALL DESIGN</u> In accordance with EN1 National Annex incorpo	<u>N</u> 992-1-1:2004 incorporating arating National Amendmen	Corrigendum d nt No.1	lated Januc	ary 2008 and th Tedds calculatio	he UK
<u>RETAINING WALL DESIGN</u> In accordance with EN1 National Annex incorpo Concrete details - Table	N 992-1-1:2004 incorporating prating National Amendmen 2 3.1 - Strength and deformed	Corrigendum d nt No.1	lated Januc	ary 2008 and th Tedds calculatio	he UK
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RETAINING WALL DESIGN In accordance with EN1 National Annex incorpo Concrete details - Table Concrete strength class Char.comp.cylinder stre	N 992-1-1:2004 incorporating prating National Amendmen e 3.1 - Strength and deformed C32/40 ength	Corrigendum d nt No.1	lated Januc	ary 2008 and th Tedds calculatio	ne UK n version 2.9.1
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RETAINING WALL DESIGN In accordance with EN1 National Annex incorpo Concrete details - Table Concrete strength class Char.comp.cylinder strength Secant modulus of elast size Design comp.concrete Reinforcement details	992-1-1:2004 incorporating prating National Amendment 3.1 - Strength and deformed C32/40 ength form = 3.0 N/mm ² ticity hagg = 20 mm strength ngth	Corrigendum d at No.1 ation characteris f _{ck} = 32 N/mm E _{cm} = 33346 N/ f _{cd} = 18.1 N/m	ated Januc stics for cor 2 (mm ² m ²	Tedds calculation Tedds calculation Tedds calculation Mean axial f Maximum ag Partial facto	he UK In version 2.9.1 tensile ggregate Ir $\gamma_{\rm C}$ = 1.50
RETAINING WALL DESIGN In accordance with EN1 National Annex incorpo Concrete details - Table Concrete strength class Char.comp.cylinder stre strength Secant modulus of elast size Design comp.concrete Reinforcement details Characteristic yield stree	992-1-1:2004 incorporating prating National Amendment a 3.1 - Strength and deformed C32/40 ength $f_{ctm} = 3.0 \text{ N/mm}^2$ ticity $h_{agg} = 20 \text{ mm}$ strength $f_{s} = 200000 \text{ N/mm}^2$	Corrigendum d ht No.1 htion characteris f _{ck} = 32 N/mm E _{cm} = 33346 N/ f _{cd} = 18.1 N/m f _{yk} = 500 N/mn	ated Januc stics for cor 2 (mm ² m ²	Tedds calculation Tedds calculation Tedds calculation Mean axial the Maximum as Partial facto Modulus of e	he UK In version 2.9.1 tensile ggregate Ir $\gamma_{\rm C}$ = 1.50
RETAINING WALL DESIGN In accordance with EN1 National Annex incorpo Concrete details - Table Concrete strength class Char.comp.cylinder strength Secant modulus of elast size Design comp.concrete Reinforcement details Characteristic yield strength	992-1-1:2004 incorporating prating National Amendment a 3.1 - Strength and deformed C32/40 ength $f_{ctm} = 3.0 \text{ N/mm}^2$ ticity $h_{agg} = 20 \text{ mm}$ strength $f_{s} = 200000 \text{ N/mm}^2$	Corrigendum d ht No.1 htion characteris f _{ck} = 32 N/mm E _{cm} = 33346 N/ f _{cd} = 18.1 N/m f _{yk} = 500 N/mn	lated Janua stics for cor 2 'mm ² m ²	Tedds calculation Tedds calculation Tedds calculation Mean axial the Maximum as Partial facto Modulus of e	he UK In version 2.9.1 tensile ggregate Ir $\gamma_{\rm C}$ = 1.50



40 Leighton Road, NW5 2QE



-23.4

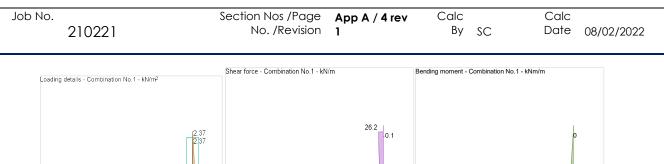
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Structure



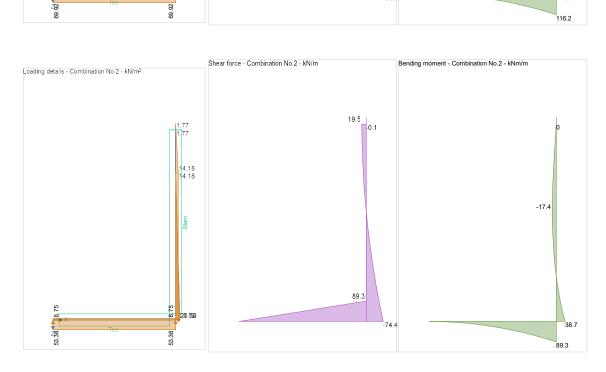
18.98 18.98

60. F 263 9365



116.2

مم د



Check stem design at 1547 mm

Depth of section h = 375 mm

Rectangular section in flexure - Section 6.1

Design bending moment M = 23.4 kNm/m

K = **0.007**

K' = **0.207**

K' > K - No compression reinforcement is required

A_{sfM.req} = **176** mm²/m

Tens.reinforcement required

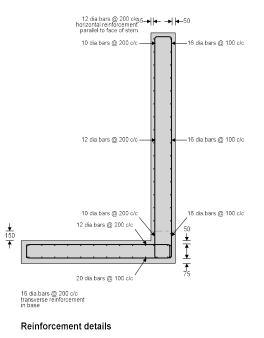
	40 L	eighton Road, NW5 20	QE		CROF	
ructure	Baserr	nent - Retaining wall de	esign		ENGIN	
lob No. 21	10221	Section Nos /Page No. /Revision	App A / 5 rev 1	Calc By SC	Calc Date	08/02/2022
Tens.reinfo	orcement prov	rided	12 dia.bars @ 2	200 c/c	Tens.reinforce	ement
provided		$A_{sfM.prov} = 565 \text{ mm}^2/\text{m}$				
Min.area mm²/m	of reinforceme	entA _{sfM.min} = 506 mm²/m	Max.area of re	einforcemer	nt A _{sfM.ma}	_x = 15000
Р	ASS - Area o	f reinforcement provided	is greater than	area of re	inforcement	required
				Library i	item: Rectangular si	ngle summary
Deflectior	n control - Sec	lion 7.4				
Limiting sp	oan to depth r	atio	40		Actual span	to depth
ratio		9.6				
		PASS - Span t	o depth ratio is	less than a	leflection co	ntrol limit
Crack co	ntrol - Section	7.3				
Limiting c	rack width	w _{max} = 0.3 mm	Maximum crac	ck width	w _k = 0.156 mr	n
PAS	S - Maximum	crack width is less than	limiting crack w	vidthCheck	stem design	at base of
			5		U	stem
Depth of	section	h = 375 mm				
Rectanau	lar section in f	exure - Section 6.1				
-		nt M = 51.9 kNm/m	K = 0.016		K' = 0.207	
0	C		> K - No compre	ession rein	forcement is	required
			A _{sr.req} = 396 mm			
Tens.reinfo	orcement reau		-			
	orcement requ orcement prov	vided	16 dia.bars @ 1	100 c/c	Tens.reinforc	ement
Tens.reinfo	orcement requ orcement prov		16 dia.bars @ 1	100 c/c	Tens.reinforce	ement
Tens.reinfo provided Min.area	orcement prov	vided A _{sr.prov} = 2011 mm²/m entA _{sr.min} = 498 mm²/m	16 dia.bars @ 1 Max.area of re			ement = 15000
Tens.reinfo provided Min.area mm²/m	orcement prov	A _{sr.prov} = 2011 mm ² /m	Max.area of re	einforcemer area of re	nt A _{sr.max}	= 15000 required
Tens.reinfo provided Min.area mm²/m P	orcement prov of reinforceme PASS - Area of	A _{sr.prov} = 2011 mm²/m entA _{sr.min} = 498 mm²/m f reinforcement provided	Max.area of re	einforcemer area of re	nt A _{sr.max}	= 15000 required
Tens.reinfo provided Min.area mm ² /m P Deflection	orcement prov of reinforceme PASS - Area of n control - Sec	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provided	Max.area of re I is greater than	einforcemer area of re	nt A _{sr.max} • inforcement item: Rectangular si	= 15000 required ngle summary
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp	orcement prov of reinforceme PASS - Area of	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provideo tion 7.4 atio	Max.area of re	einforcemer area of re	nt A _{sr.max}	= 15000 required ngle summary
Tens.reinfo provided Min.area mm ² /m P Deflection	orcement prov of reinforceme PASS - Area of n control - Sec	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provided tion 7.4 atio 9.8	Max.area of re I is greater than 40	einforcemer area of re Library i	nt A _{sr.max} F inforcement item: Rectangular si Actual span	= 15000 required ngle summary to depth
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio	orcement prov of reinforceme PASS - Area of n control - Sec pan to depth r	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provided tion 7.4 atio 9.8 PASS - Span te	Max.area of re I is greater than	einforcemer area of re Library i	nt A _{sr.max} F inforcement item: Rectangular si Actual span	= 15000 required ngle summary to depth
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio	orcement prov of reinforceme PASS - Area of n control - Sector pan to depth re ntrol - Section	A _{sr.prov} = 2011 mm²/m entA _{sr.min} = 498 mm²/m f reinforcement provided tion 7.4 atio 9.8 PASS - Span to 7.3	Max.area of re I is greater than 40 o depth ratio is	einforcemer area of re Library i	nt A _{sr.max} tinforcement item: Rectangular si Actual span deflection co	= 15000 required ngle summary to depth ntrol limit
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio	orcement prov of reinforceme PASS - Area of n control - Sec ban to depth re ntrol - Section rack width	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provided tion 7.4 atio 9.8 PASS - Span for 7.3 w _{max} = 0.3 mm	Max.area of re I is greater than 40 o depth ratio is Maximum crac	einforcemer area of re ^{Library i} less than a	nt A _{sr.max} Sinforcement item: Rectangular si Actual span deflection co w _k = 0.061 mr	= 15000 required ingle summary to depth ntrol limit
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio	orcement prov of reinforceme PASS - Area of n control - Sec ban to depth re ntrol - Section rack width	A _{sr.prov} = 2011 mm²/m entA _{sr.min} = 498 mm²/m f reinforcement provided tion 7.4 atio 9.8 PASS - Span to 7.3	Max.area of re I is greater than 40 o depth ratio is Maximum crac	einforcemer area of re ^{Library i} less than a	nt A _{sr.max} Sinforcement item: Rectangular si Actual span deflection co w _k = 0.061 mr ngular section	= 15000 required ngle summary to depth ntrol limit n in shear -
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio Crack con Limiting cu PAS	orcement prov of reinforceme PASS - Area of n control - Sector ban to depth re ntrol - Section rack width S - Maximum	A _{sr.prov} = 2011 mm ² /m entA _{sr.min} = 498 mm ² /m f reinforcement provided tion 7.4 atio 9.8 PASS - Span for 7.3 w _{max} = 0.3 mm crack width is less than	Max.area of re I is greater than 40 o depth ratio is Maximum crac Iimiting crack w	einforcemer area of re Library i less than a ck width vidthRectar	Int $A_{sr.max}$ Sinforcement item: Rectangular si Actual span Solution $w_k = 0.061 mr$ ingular section S	= 15000 required ingle summary to depth ntrol limit n in shear - ection 6.2
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio	orcement prov of reinforceme PASS - Area of n control - Sector ban to depth re ntrol - Section rack width S - Maximum	$A_{sr,prov} = 2011 \text{ mm}^2/\text{m}$ ent $A_{sr,min} = 498 \text{ mm}^2/\text{m}$ f reinforcement provided tion 7.4 atio 9.8 PASS - Span for 7.3 $w_{max} = 0.3 \text{ mm}$ crack width is less than V = 99.9 kN/m	Max.area of re I is greater than 40 o depth ratio is Maximum crac	einforcemen area of re Library i less than a ck width vidthRectar esistance	nt A _{sr.max} Sinforcement item: Rectangular si Actual span deflection co W _k = 0.061 mr ngular section S V _{Rd.c} = 186.2 k	= 15000 required ngle summary to depth ntrol limit n in shear - ection 6.2
Tens.reinfo provided Min.area mm²/m P Deflection Limiting sp ratio Crack con Limiting cu PAS. Design sho	orcement prov of reinforceme PASS - Area of n control - Sector ban to depth re ntrol - Section rack width S - Maximum	$A_{sr,prov} = 2011 \text{ mm}^2/\text{m}$ entA _{sr,min} = 498 mm ² /m f reinforcement provided tion 7.4 atio 9.8 PASS - Span for 7.3 $w_{max} = 0.3 \text{ mm}$ crack width is less than V = 99.9 kN/m PASS - Desig	Max.area of re 1 is greater than 40 o depth ratio is Maximum crac limiting crack w Design shear re	einforcemen area of re Library i less than a ck width vidthRectar esistance	nt A _{sr.max} Sinforcement item: Rectangular si Actual span deflection co W _k = 0.061 mr ngular section S V _{Rd.c} = 186.2 k	= 15000 required ngle summary to depth ntrol limit n in shear - ection 6.2

App A . 5

210221 Retaining wall

Project	40 L	eighton Road, NW5 20	ŞΕ		CROFT
ructure	Basem	nent - Retaining wall de	esign		STRUCTURAL ENGINEERS
lob No.	210221	Section Nos /Page No. /Revision	App A / 6 rev 1	Calc By SC	Calc Date 08/02/202
Rectar	ngular section in s	hear - Section 6.2			
Design	shear force	V = 26.2 kN/m	Design shear re	esistance	V _{Rd.c} = 186.2 kN/m
		PASS - Desig	ın shear resistan	ice excee	ds design shear force
Horizor	ntal reinforcemen	t parallel to face of stem - :	Section 9.6		
Min.are	ea of reinforceme	entA _{sx.req} = 503 mm²/m	Max.spacing c	of reinforcer	nent s _{sx_max} = 400 mm
Trans.re	einforcement pro	vided	12 dia.bars @ 2	200 c/c	Trans.reinforcement
provide	ed	A _{sx.prov} = 565 mm ² /m			
	PASS - Area of	f reinforcement provided	l is greater than	area of re	inforcement required
Check	base design at to	De			
	of section	h = 350 mm			
		exure - Section 6.1			
	-	nt M = 116.2 kNm/m	K = 0.052		K' = 0.207
200.9.1				ession rein	forcement is require
Tens.re	inforcement requ		A _{bb.req} = 1062 m		
	inforcement prov		20 dia.bars @ 1		Tens.reinforcement
provide		Abb.prov = 3142 mm ² /m		·	
	ea of reinforceme	entA _{bb.min} = 417 mm²/m	Max.area of re	einforcemer	14000 Abb.max = 14000
	PASS - Area of	f reinforcement provided	l is greater than	area of re	inforcement required
			-	Library i	tem: Rectangular single summa
Crack	control - Section	7.3			
Limiting	g crack width	w _{max} = 0.3 mm	Maximum crac	ck width	w _k = 0.118 mm
P	ASS - Maximum	crack width is less than	limiting crack w	/idthRectar	gular section in shear
					Section 6.
Design	shear force	V = 116.2 kN/m	Design shear re	esistance	V _{Rd.c} = 199.7 kN/m
		PASS - Desig	ın shear resistan	ice excee	ds design shear force
Secon	dary transverse re	inforcement to base - Sect	tion 9.3		
		entA _{bx.req} = 628 mm²/m	Max.spacing c	of reinforcer	nent s _{bx_max} = 450 mm
	einforcement pro	vided	16 dia.bars @ 2	200 c/c	Trans.reinforcement
Trans.re		naca			

Project	40	Leighton Road, NW5 2G					
Structure	Baser	ment - Retaining wall de		ENGINEERS			
Job No.	210221	Section Nos /Page No. /Revision	App A / 7 rev 1	Calc By SC	Calc Date 08/02/2022		





Appendix B: Construction programme

The Contractor is responsible for the final construction programme

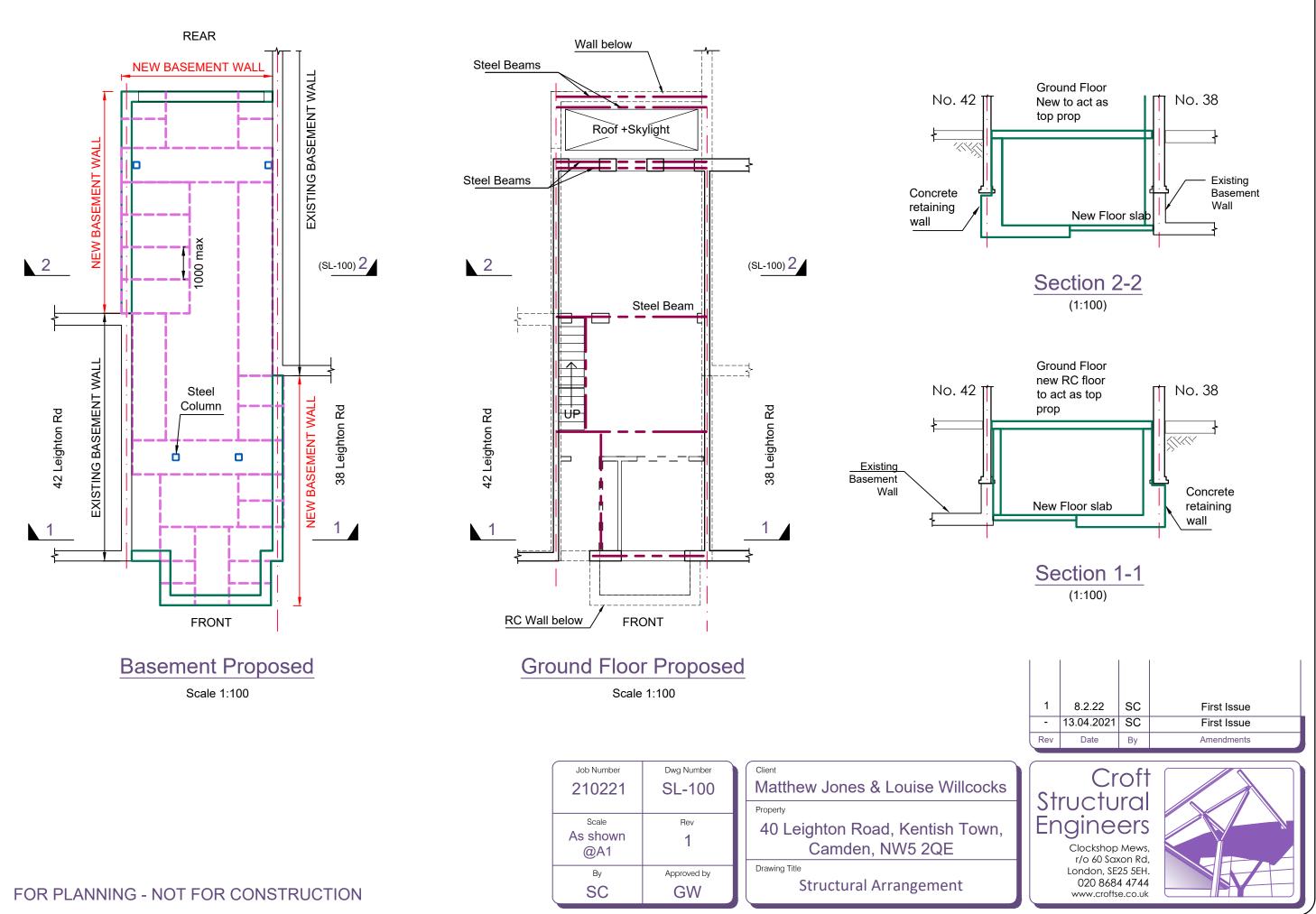
Outline cor	nstru	ctic	n P	rogr	am											
(For planning p	urpos	es or	nly)													
	Months															
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Planning																
approval																
Detailed																
Design																
Tender																
Party Walls																
Monitoring of	1			1	1		6									
Adjacent	1				1											
structures	1															
Enabling works	/			Č	N T		11	1.5					- 'A			
Basement						K										
Construction			- 25	~	2.1	1.5	10	1	-							
Superstructure		754	7	-	_	1.00	_			-						
alterations	100															
P	11					N		1				. 1 \	0			

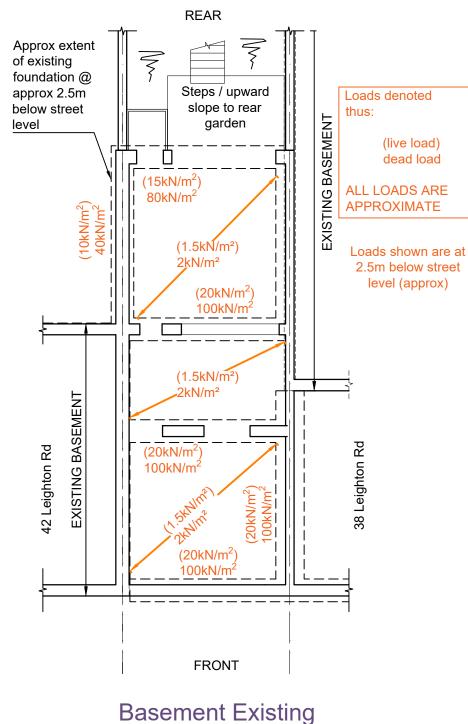


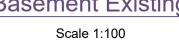
Appendix C : Structural Drawings

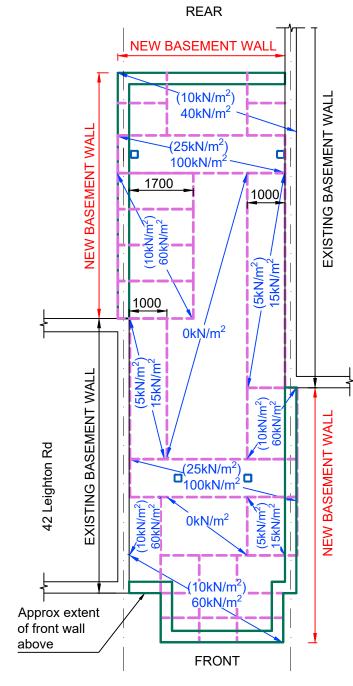


CROFT STRUCTURAL ENGINEERS









Basement Proposed

Scale 1:100

	Job Number 210221	Dwg Number SL-101	Client Matthew Jones & Louise Willcocks Property
	_{Scale} As shown @A1	Rev –	40 Leighton Road, Kentish Town, Camden, NW5 2QE
Issued for INFORMATION ONLY	By SC	Approved by	Drawing Title Foundation Loading

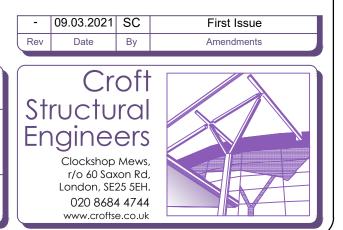
Loads denoted thus:

(live load) dead load

ALL LOADS ARE APPROXIMATE

Loads shown are at 3.5m below street level (approx)







Appendix D : Temporary Works Sequence



CROFT STRUCTURAL ENGINEERS

PHASE 1

- 1.1. Remove ground floor structure and excavate to level of front wall foundation
- 1.2. Excavate and form front light-well
 - 1.2.1. De-water ground if required
 - 1.2.2. Needle and prop front wall, constructing temporary mass concrete pads where necessary.
 - 1.2.3. Prop initial excavations; install lintels with props where soil is loose
 - 1.2.4. Place reinforcement and cast retaining wall
- 1.3. Excavate progressively towards the rear, between the existing adjacent basements
- 1.4. Excavate and cast underpins below party wall with No 42
 - 1.4.1. Outline of pin segments are shown in drawing SL-100; underpin sequence TBA at detailed design stage
 - 1.2.5. Excavation below existing wall to be carried out in segments not exceeding 1m
 - 1.2.1. Prop pits as excavation progresses
 - 1.2.2. When excavating below rear wall, install vertical props to brickwork above

PHASE 2

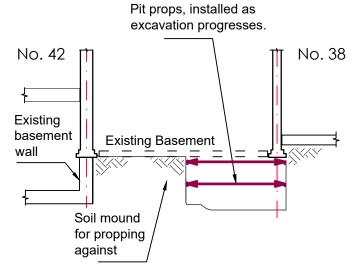
- 2.1. Continue with excavation and casting of pins in a hit and miss sequence
 - 2.1.1. When reaching formation level, excavate further for mass concrete thrust block
 - 2.1.2. Install sacrificial raking prop from wall from thrust block
 - 2.1.3. Place reinforcement and cast retaining wall
- 2.2. Continue for remaining underpins below party wall
 - 2.2.1. Do not commence excavation for pin until at least 48 hours after dry-packing for adjacent pin is complete
- 2.3. Needle and prop rear wall
- 2.4. Excavate and cast pins for rear light-well, installing props excavations progress

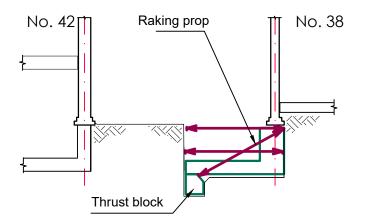
PHASE 3

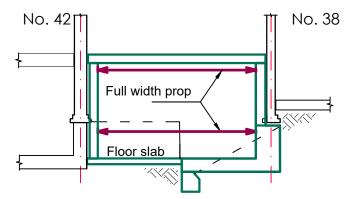
- 3.1. Excavate remaining soil mass below building
 - 3.1.1. Initial horizontal props may be removed as excavation progresses
- 3.1.2. Install full width cross prop
- 3.2. Install below slab drainage
- 3.3. Cast concrete floor slab.
- 3.4. After basement slab has gained sufficient strength, remove raking props or trim if cast as sacrificial. Full width horizontal props may also be removed after Basement structure is complete
- 3.5. Proceed with construction of internal walls, columns and Ground floor support

PHASE 4, 5 & 6

Repeat Phases 1, 2 and 3 for Phases 4 , 5 and 6 respectively for excavations in rear garden



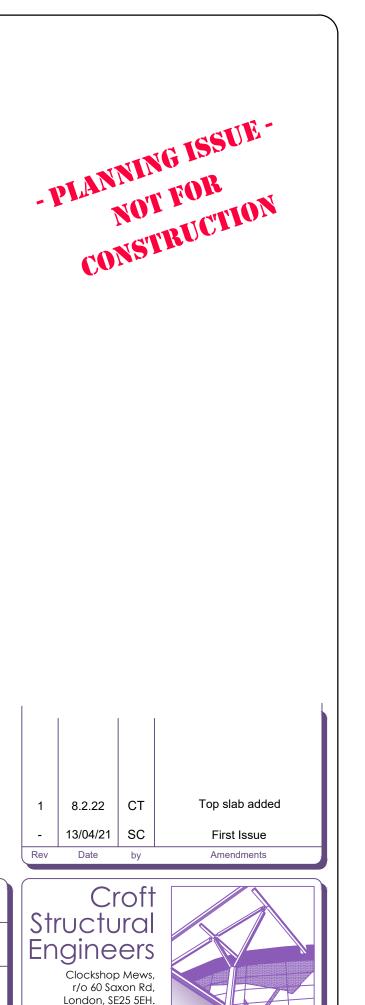




Section 1-1 : Construction Sequence

FOR LOCATION OF SECTION, REFER TO DRAWING SL-100

	(1:100)	
Job Number	Dwg Number	Client Ground and Project
210221	TW-100	Consultants Ltd
		Project 40 Leighton Road,
	Rev 1	NW5 2QE
1:100 @A3	1	Drawing Title Temporary Works
Ву	Approved by	Construction Sequence
SC	GW	Issued for INFORMATION ONLY



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Appendix E : Utilities Searches



CROFT STRUCTURAL ENGINEERS



Network Records NetMAP Symbols Booklet - London

This symbol booklet is intended as a general guide only - some local variations of these symbols may be found.

Version 1.2

Released October 2010

Always check with your local Network Records office or the UK Power Networks server to ensure that you are using the most up to date copy of this booklet.Tel: 08000 565866

Index:-

Page no:

Contents:

Guidance notes. 1 2 The area covered by this guide. 3 Scenerv. Scenery (UK Power Networks use only-boxed red) 4 7 Primary distribution cables (EHV). 8 Secondary distribution cables (LV/HV). 9 Cable terminology. 10 Cable size abbreviations. 11 Cable ducts. Other NetMAP symbols. 12 15 Services. 17 Symbols used in cross sections. Abbreviations used in cross sections. 19 20 Typical plan and cross section representations: All areas: NetMAP/vector. All areas: composite raster style 1. Ex-Western area and Holborn: main and wavs. The City of London: single line. Finsbury and Shoreditch: multi-single line style 1. Ex-North Eastern area: HV/LV. Ex-North Eastern area: multi-single line style 2. Ex-North Eastern area: composite raster style 2. Regional NetMAP anomalies - general overview. 23 24 Region 1: ex-Western area. 25 Region 2: ex-Northern area. 27 Region 3: ex-North eastern area. 29 Region 4: ex-South Eastern area. 30 Region 5: ex Southern area.

Guidance notes.

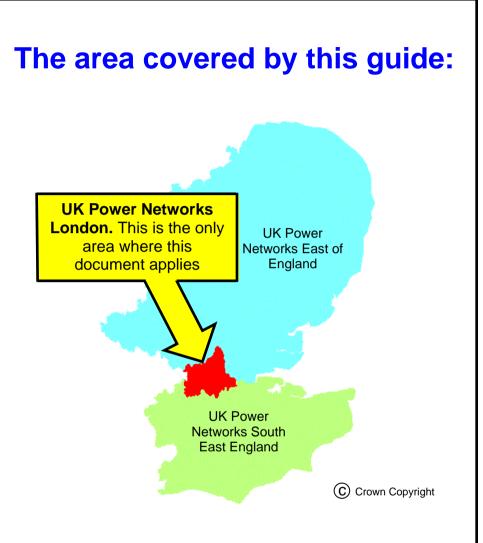
Important notice:

If you do not understand the NetMAP record that you are using, please contact UK Power Networks Network Records for guidance **Tel: 08000 565866.**

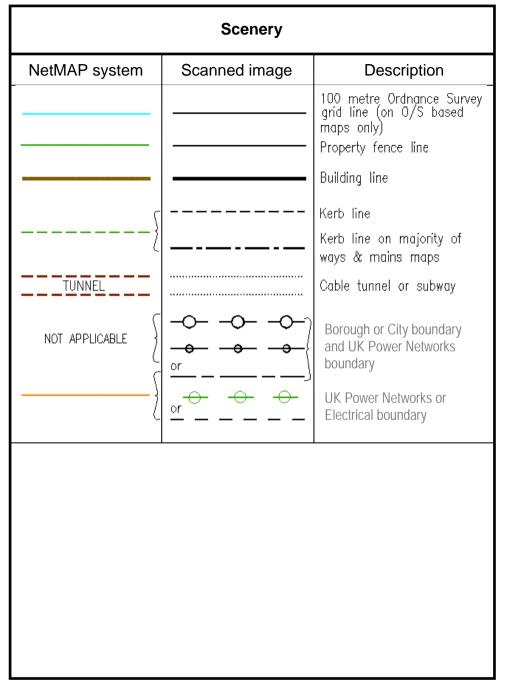
- The position of apparatus shown on NetMAP is believed to be correct, but the original landmarks may have altered since the apparatus was installed.
- It must be assumed that there is at least one service to each property, lamp column, street sign etc. A separate record may be available.
- When excavations are to be carried out near Extra High Voltage (EHV) cables, further details must be obtained before commencement of work.
- Third party cables are not usually shown.
- When two or more maps are supplied for the same area, the maps must be read in conjunction with each other and with this symbol booklet.
- All LV cables are assumed to be 4 core, and all HV cables assumed to be 3 core unless otherwise stated.
- All Imperial cable sizes are assumed to be copper and all metric cable sizes are assumed to be aluminium unless otherwise stated.



Plan Provision Team Fore Hamlet Ipswich Suffolk IP3 8AA Tel: 08000 565866



Please see the anomalies map at the end of this safety booklet for greater map area detail, and a breakdown of the more significant anomalies within the London area.



Scenery for UK Power Networks use only - boxed in red								
NetMAP system	Scanned image	Description						
Inset Network – Contact xxxx IDNO for further information	Not applicable	Area of inset network - not the asset of UK Power Networks (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Proposed Cross Rail route (only visible to of UK Power Networks and their immediate contractors)						
	Not applicable	High pressure pipelines in the general vicinity (only visible to of UK Power Networks and their immediate contractors)						
Note: Pipelines are only viewable on NetMAP by UK Power Networks staff and their immediate contractors. Do not carry out any excavation without consent from the relevant agency - legally protected high pressure petroleum products pipeline route in the general vicinity - consult www.linewatch.co.uk for contacts and guidance. Pipeline contact numbers can also be found on the intranet – out of hours, contact our Control Centre.								
	Not applicable	Water - surface water (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Water - Source Protection Zone 1 (only visible to UK Power Networks and their immediate contractors)						
\bigcirc	Not applicable	Water - Source Protection Zone 2 (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Water - Source Protection Zone 3 (only visible to UK Power Networks and their immediate contractors)						
section continued on next page								

Scenery for UK Power Networks use only - boxed in red							
NetMAP system	Scanned image	Description					
	Not applicable	Historical - Scheduled Monuments (only visible to UK Power Networks and their immediate contractors)					
	Not applicable	Historical - Parks and Gardens (only visible to UK Power Networks and their immediate contractors)					
	Not applicable	Historical - Areas of Archaeological Potential (AAP) (only visible to UK Power Networks and their Immediate contractors)					
	Not applicable	Nature - Ramsar Wetlands of International Importance (only visible to UK Power Networks and their immediate contractors)					
	Not applicable	Nature - Special Area of Conservation (SAC) (only visible to UK Power Networks and their immediate contractors)					
	Not applicable	Nature - Special Protected Area (SPA) (only visible UK Power Networks and their immediate contractors)					
<pre>sect</pre>	Not applicable	Nature - Site of Special and Scientific Interest (SSSI) (only visible to UK Power Networks and their immediate contractors) ext page					

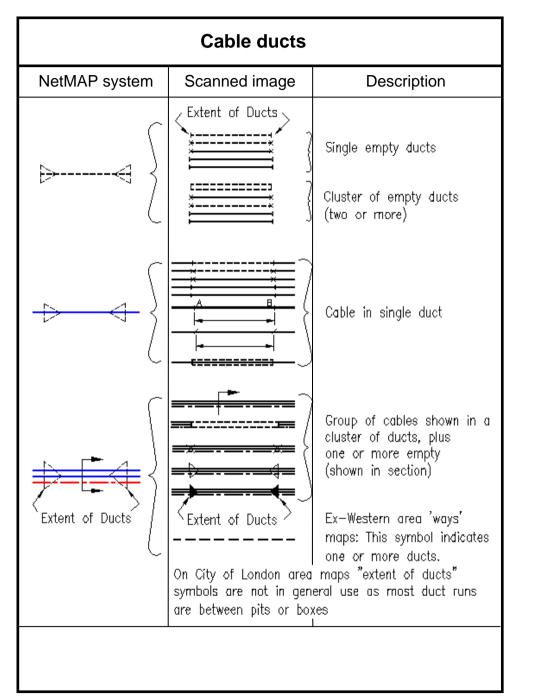
Scenery for UK Power Networks use only - boxed in red								
NetMAP system	Scanned image	Description						
	Not applicable	Nature - Local Nature Reserve (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Nature - National Nature Reserve (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Nature - Area of Outstanding Natural Beauty (AONB) (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Nature - National Park (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Fluid filled cables - very high sensitivity (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Fluid filled cables - high sensitivity (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Fluid filled cables - medium sensitivity (only visible to UK Power Networks and their immediate contractors)						
	Not applicable	Fluid filled cables - low sensitivity (only visible to UK Power Networks and their immediate contractors)						

Prir	mary distributior	n cables
NetMAP system	Scanned image	Description
EHY CABLE Solid BHY CABLE Gas EHY CABLE Gas Oil Cable stop Cable stop Shallow	—— EHV Coble Route 259 —— Not applicable —— 5—— 5—— 5—— 5—— 5	UK Power Networks route (11,000 , 22,000 to 132,000 volts) Oil/gas cable stop Part of UK Power Networks cable route where cover is less than normal
		1

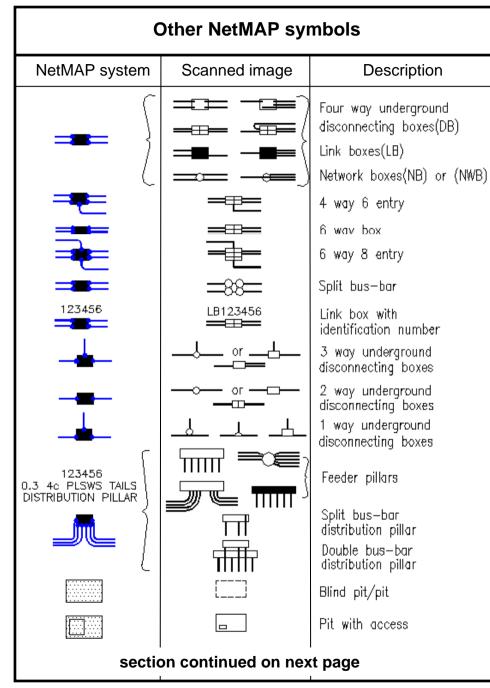
Seco	ndary distributio	on cables
NetMAP system	Scanned image	Description
(20kV) (11kV) (6.6kV)		HV cable (up to 20kV) 3 phase LV cable (230V or 400/230V) 1 or 2 phase LV cable (230V or 400/230V) Pilot or Telephone cable, often not shown in plan if running with other cables Fibre-optic cable Earth cable HV or LV cable in duct Duct route(s) not containing live cables
	8	

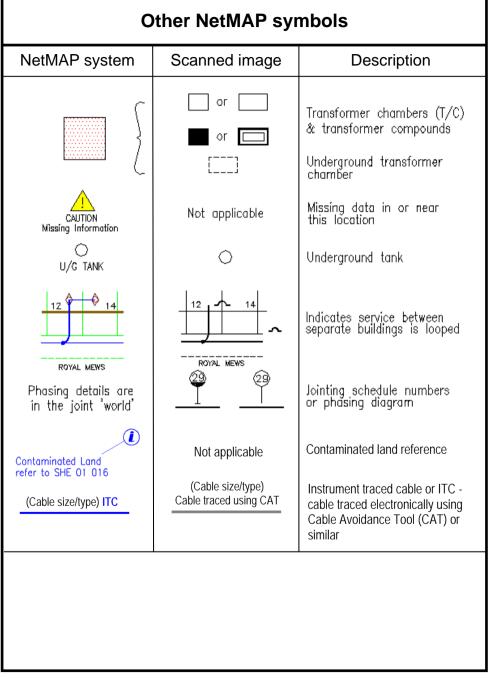
	Cable terminol	ogy
NetMAP system	Scanned image	Description
PL PLST or PLSW PLSTS PLSTS PLSWS PLSW PLS PLST or PLSW PLST PLST PLST PLSW AI Cu WV CS PVC EPR XLPE SOL ax cx	PL PLS PLA PLTS PLDT PLWS PLBW LC & H LC & A LC & BA DSTA STA SWA AI Cu WV CS PVC EPR XLPE SOLIDAL TRIPLEX TRIPLEX	Paper Lead Paper Lead Served Paper Lead Armoured Paper Lead Steel Tape Served Paper Lead Steel Wire Served Paper Lead Bright Wire Lead Covered & Hessian Lead Covered & Armoured Lead Covered & Armoured Dauble steel tape armoured Steel Tape Armoured Steel Wire Armoured Aluminium Copper Waveconal Consac Polyvinyl Chloride Ethylene Propylene Rubber Cross Linked Polyethylene Solid Aluminium Triplex (aluminium) Triplex (copper)

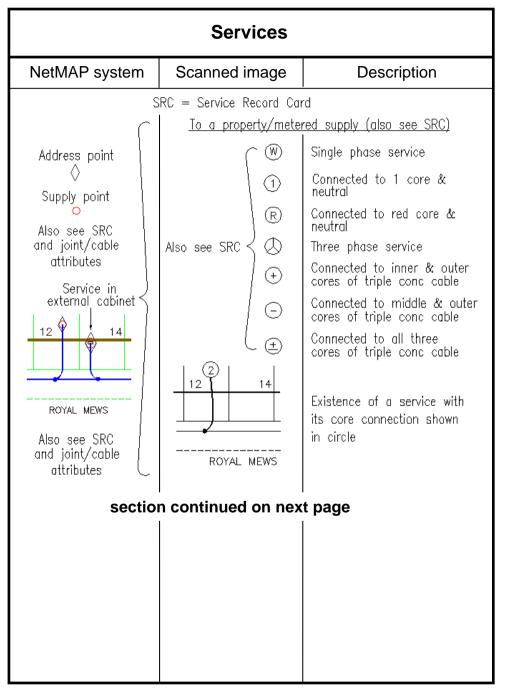
Ca	able size abbrevi	ations
NetMAP system	Scanned image	Description
1c c/c t/c 4c 3c CNE	Ус 96 Геог T/сс 96 (см)	Single core. Concentric cores Triple concentric cores Four cores Three cores and concentric neutral — not of the Waveconal type
2c s/c 3c DC P Pr	% (or Tw) % 死 DC P Pr	Two cores (or twin) Split concentric cores Three cores Direct current Pilot Number of telephone pairs

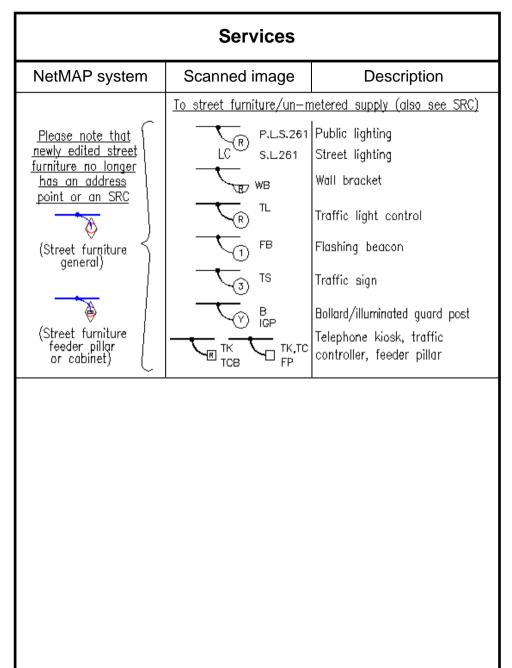


O	ther NetMAP syn	nbols
NetMAP system	Scanned image	Description
0.3 4c AL PLSWS (Details also in cable attributes and/or section)	.3%(59)	Cable size (and year laid)
4		Cable capped end
	se Pe	Cable pressure (or pot) end or signal end
	─── ₽⁄ ^E ── ₽ ^E	Pressure/pot end & earth cable/electrode
	+ +- ⊑	Earth rod (vertical) Earth rod (horizontal) Earth plate Earth plate or end
MAIN SERV		Bottle or trouser joint or combined crutch & pressure end – (CPE)
		Straight joints
		Tee joints Crutch (or spur) joints (CJ) straight & crutch joints combined (S&CJ)
UT (Disconnected universal tee)		Double crutch (or spur) joint Sleeve
	n continued on next	page





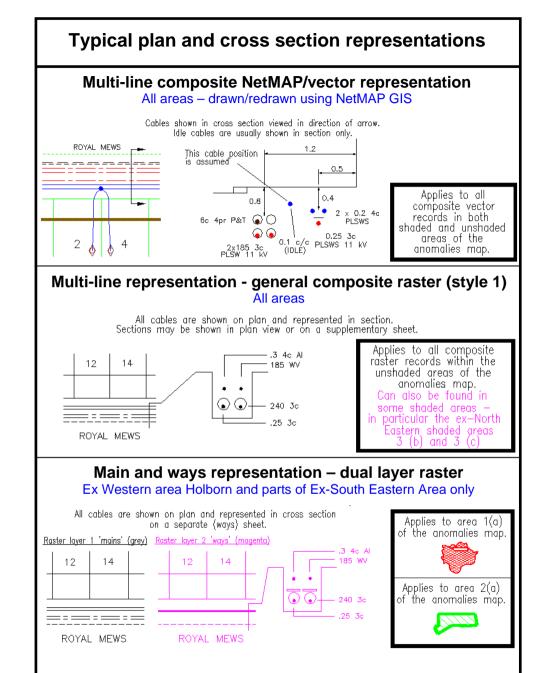


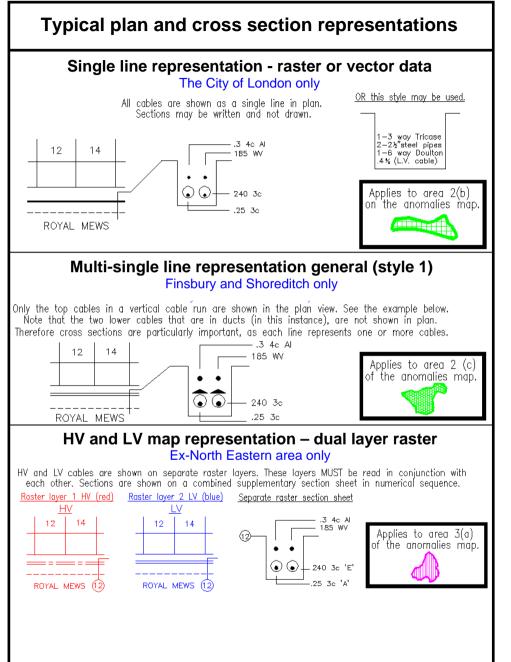


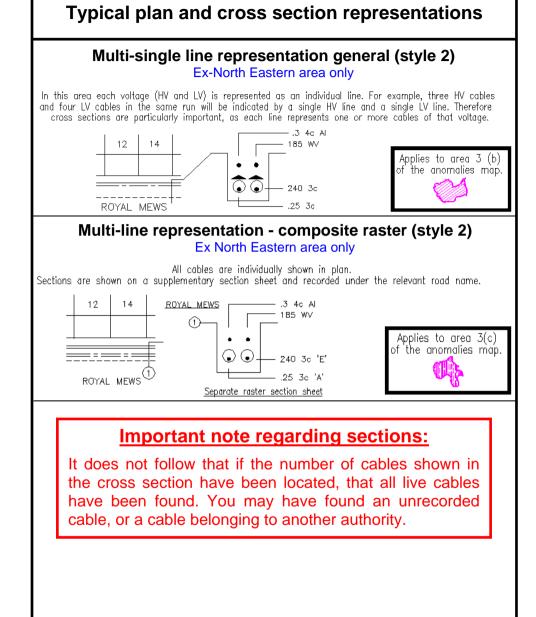
Symb	ols used in cros	s sections
NetMAP system	Scanned image	Description
•	• •	Cable laid direct
۲	، ک	Cable laid in duct
⊗	$\oslash \otimes$	Blocked duct (sometimes used for unidentified cables)
0	$\circ \circ$	Single earthenware duct
○ 2¥" S	0	Single steel pipe
		Square cable duct
00	88	Group of circular ducts
83	88	Group of circular ducts (Sykes)
		Group of square ducts (Doulton)
C	ᄑᅋ ^て ᄆ᠊ᢦ	Cable trough
	000	Bitumen casing (Crompton)
L	<u></u>	Bitumen filled iron trough (Trunks)
\otimes	63	Bitumen casing (Tri-case)
sectio	n continued on nex	kt page

Symbols used in cross sections NetMAP system Scanned image Description Protective slab Tiles. \frown Concrete slabs Steel plate Plastic tile tape — т/т Timber Timber 👝 777

NetMAP systemScanned imageDescriptionEWE.W.D(s) or EW.Earthenware ductsFF.P or F or F.DFibre ductAASB or AAsbestosPPPlastic or pitch fibreSS.P or SSteelCC.I or C or C.I.PCast ironWIW.IWrought iron pipeFF or F.DFibre ductPPPlastic cipital ci
F F.P or F or F.D Fibre duct A ASB or A Asbestos P P P Plastic or pitch fibre S S.P or S Steel C C.I or C or C.I.P Cast iron WI W.I Wrought iron pipe F F or F.D Fibre duct
Left blank – means NR







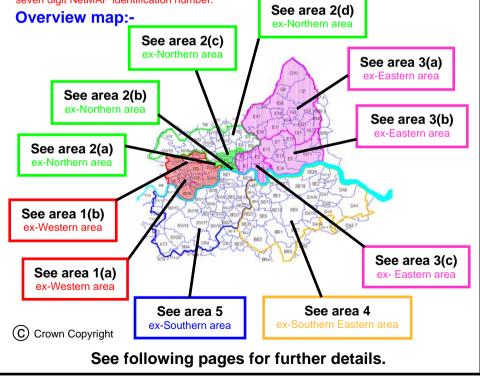
Regional NetMAP Anomalies - general overview:

The following pages explain the various major map style anomalies found within the London area. These styles are a legacy from the five individual London Electricity areas which were again formed from seventeen separately organised LEB districts. Areas with significant anomalies are shown in the following pages as cross-hatched areas. Areas with standard composite vector and raster layer information are shown as un-hatched areas.

<u>Cautionary note</u>: - any region or sub-region, either shaded or un-shaded, may contain some local anomalies not mentioned in the following pages – if in doubt, please contact the UK Power Networks Plan Provision team on telephone number 08701 963797.

All regions (1-5) will contain recently created composite vector (NetMAP/AutoCAD) data.

Recent work created using the NetMAP system and previously created using the AutoCAD system (as opposed to raster/scanned data) are recorded in the composite vector style shown on the UK Power Networks London area symbol sheet - see the first example on page 18 of this document. Recent data will be indicated by the existence of multi-coloured cables on the NetMAP system, but this may not be reflected on printed matter produced with a black and white printer. AutoCAD data looks similar to the coloured NetMAP data, but does not hold any cable 'attributes' when selected using the NetMAP system. These cables will be represented individually (multi-line representation). New NetMAP cross sections may be accessed electronically on the NetMAP system and are presented in printed format accompanied by a seven digit NetMAP identification number.



Region 1 ex-Western area

This region includes Westminster, Kensington, Chelsea, Hammersmith and Fulham. The region is covered by two map layer systems – **region 1(a)** mains and ways dual layer raster, and **region 1(b)** composite raster. The following explains this in greater detail.

Region 1(a) (hatched)

Mains and ways representation:

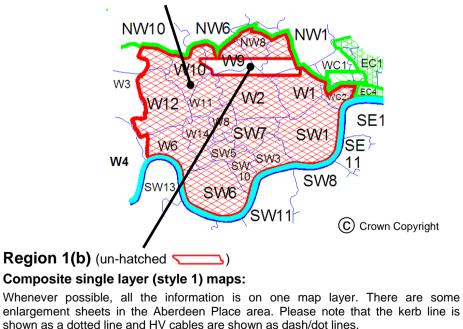
This system consists of two maps layers for the same area.

- i) The mains map shows all cable routes.
- ii) The ways map shows pipe and duct routes with cross sections.

There are some enlargement sheets, cross sections and jointing details. EHV routes are shown on either the mains or the ways map.

It is important that all these maps are read in conjunction with each other.

Caution: - It is also important to note that the kerb line detail on these maps is a dash/dot line, which on the majority of UK Power Networks Central (London) records would refer to an HV cable route. HV cables are shown as a solid line when laid direct and a dashed line when in a duct.



Region 2 ex-Northern area

This region includes Islington, Hackney, the City of London and parts of Brent, Camden and Ealing. The region is covered by four map layer systems - **Region 2(a)** - mains and ways dual layer raster (Holborn area), **Region 2(b)** - single line representation (City of London), **Region 2(c)** - multi-single line representation (Finsbury and Shoreditch) and **Region 2(d)** - composite multi-line maps (all other areas). This following explains this in greater detail.

Region 2(a) (hatched)

Covers part of WC1 and WC2 (Holborn).

Mains and ways representation:

This system consists of two maps layers for the same area.

- i) The mains map shows all cable routes.
- ii) The ways map shows pipe and duct routes with cross sections.

Where needed, extra sheets have been added for enlargements, cross sections and jointing details. EHV routes are shown on the mains map layer.

It is important that all these maps are read in conjunction with each other.

Caution: - It is also important to note that the kerb line detail on these maps is a dash/dot line, which on the majority of UK Power Networks Central (London) records would refer to an HV cable route. HV cables are shown as a solid line when laid direct and a dashed line when in a duct.



Region 2(b) (hatched

Covers parts of postal areas EC1, EC2 and all of postal areas EC3 and EC4.

Single line representation maps:

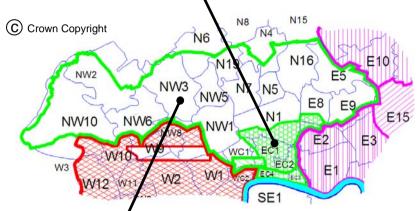
Whenever possible, all the information is on one map layer .One line can represent any number of cables or ducts. It is therefore very important to use cross sections. In some cross sections details may be written and not drawn. In complex and redrawn areas, some detail may be drawn using multi-line representation. There are some enlargement sheets.

Region 2(c) (hatched 1)



Multi-single line representation (style 1) maps:

Whenever possible, all the information is on one map layer. When cables lay immediately above/below each other, it is shown as a single line. For example if six cables lay three on three, only three lines would indicate the six cables. If the cables were laid flat, six separate lines would be shown. It is therefore important not to assume that the lines drawn indicate the number of cables, at any point. **Cross sections must be used.**



Region 2(d) (un-hatched)

Covers all other postal areas in this region

Composite single layer (style 1) maps:

Whenever possible, all the information is on one map layer. There are some enlargement sheets.

Region 3 ex-North Eastern area

This region includes Tower Hamlets, Newham, Redbridge, Waltham Forest, Loughton (Epping) and Barking and Dagenham. This region is covered by three mapping systems.

Region 3(a) (hatched

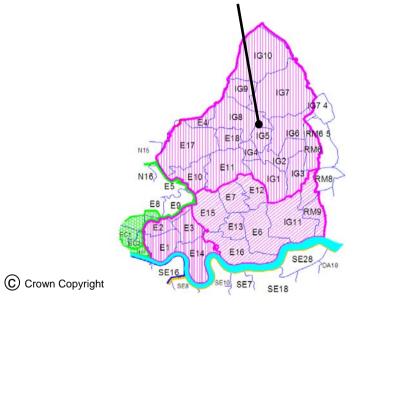
Separate HV and LV representation maps:

This system consists of two maps layers for the same area.

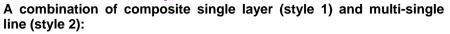
- i) The HV map layer showing HV cables and duct routes.
- ii) The LV map layer showing LV cables and duct routes.

Cross sections for both HV and LV cable routes are shown on a separate sheet. EHV cable routes are shown on the HV map layer.

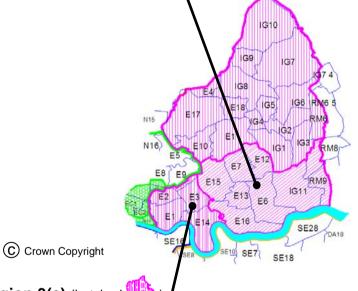
It is important that all these maps are read in conjunction with each other.



Region 3(b) (hatched



Whenever possible, all the information is on one map layer. There are some enlargement sheets. There is a combination of map styles used in this area. Some areas may be conventional multi-line line representation with many areas of multisingle line representation. In the multi-line areas each (live) cable is shown individually in plan. In the multi-single line map areas, there is a single line for each voltage type, with a single HV line and a single LV line representing more than one cable run of each voltage (when applicable). Therefore a cable run containing three HV cable and four LV cables will be represented by one HV line and one LV line.



Region 3(c) (hatched

A combination of composite single layer (style 2) and multi-single line (style 2):

Whenever possible, all the information is on one map layer. There are some enlargement sheets. In this area (postal code areas E1, E2, E3, E14 and part of E9), the cross sections are listed under each road name. It is therefore extremely important that you have the correct cross sections for the road you are working in.

There is a combination of map styles used in this area. Most areas are composite single layer (style 2) with some areas of multi-single line representation, as described in region 3(b).

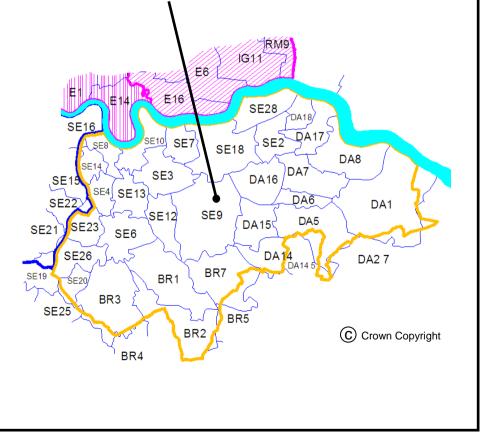
Region 4 ex-South Eastern area

This region includes Lewisham, Greenwich, Bromley, Bexley and Dartford. Nearly all maps are drawn in one style – single layer composite raster/vector.

Region 4 (un-hatched)

Composite single layer (style 1) with a small number of mains and ways representation maps :

Mainly composite maps - whenever possible, all the information is on one map layer. There are some enlargement and cross section sheets. Some maps do not show single phase services unless they are long and deviating. There are however some maps drawn using the mains and ways style. These are rare, but please be aware that they exist.



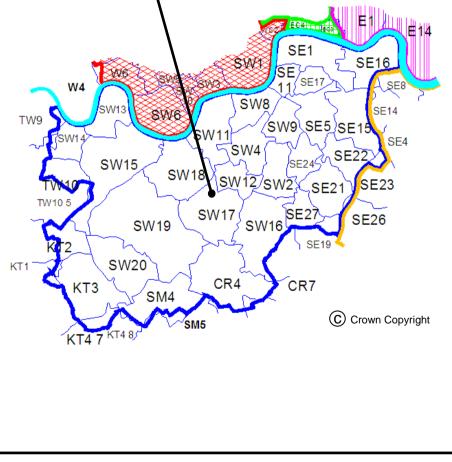
Region 5 ex-Southern area

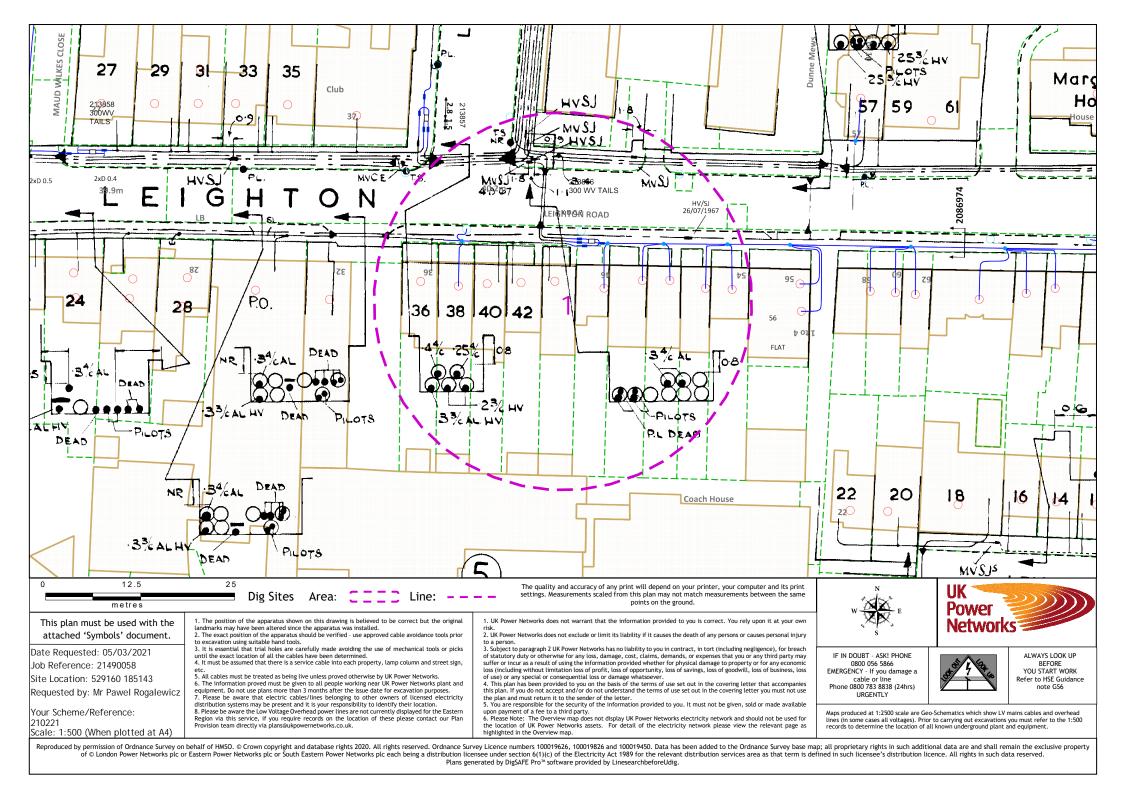
This region includes Southwark, Lambeth, Wandsworth, Merton, Kingston upon Thames and Richmond upon Thames. All maps are drawn to one style - single layer composite raster/vector.

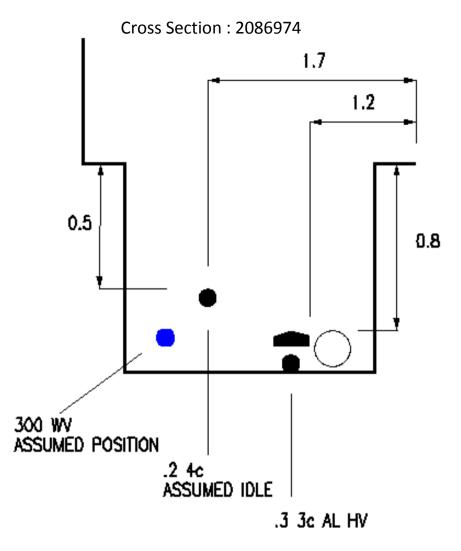
Region 5 (un-hatched)

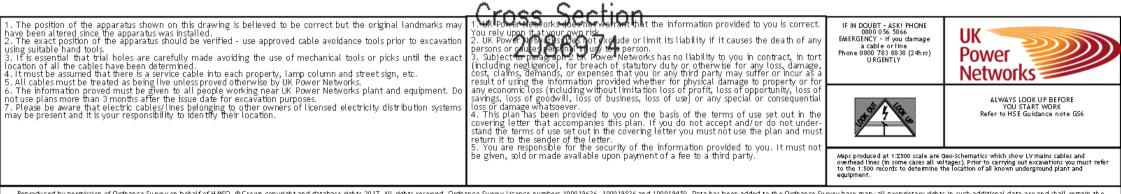
Composite single layer (style 1) maps:

Composite maps - whenever possible, all the information is on one map layer. There are some enlargement and cross section sheets. A small number of maps may not show services.









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