

## 203 x 203 UC STANCHION WITH 350 x 215 x 20 THK BASEPLATE UNDERSIDE OF BASEPLATE TO BE

TOP OF FOUNDATION

TOP OF FOUNDATION

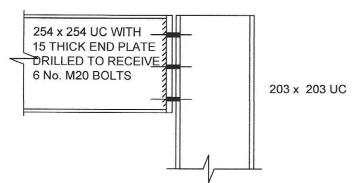
TOP OF MASSEPLATE TO BE GROUTED UP USING A NON-SHRINK SAND/CEMENT GROUT

2No. M20x200 LONG "REBAR" H.D.BOLTS

SET WITHIN
PREDRILLED HOLES
USING "SBD FIVE
STAR GROUT" OR
SIMILAR

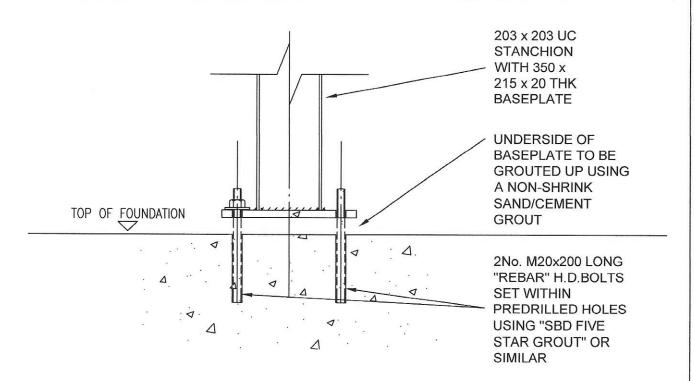
# 203 x 203 UC BASEPLATE AT FOUNDATION

Rev Description Dwn Date Chkd Drawing Status CONSTRUCTION **BEAM / STANCHION VERSI** & STANCHION BASEPLATE 14B DOWNSHIRE HILL ASSOCIATES LTD LONDON NW3 **DETAILS** STRUCTURAL ENGINEERS Date NOVEMBER 2021 Revision Drawing Number A4 Scale 7864922 / 22 Drawn RJV 6 VERWOOD ROAD, NORTH HARROW, MIDDLESEX, HA2 6LD: Tel: 07958 721212 email: riyaz@rversi.plus.com Checked



STANCHION TO BE KEMFIXED TO EXISTING BRICKWORK AT 600c/c VERTICALLY FOR RESTRAINT

## 254 x 254 UC BEAM / 203 x 203 UC STANCHION



### 203 x 203 UC BASEPLATE AT FOUNDATION



### Consulting Engineers

#### NOTES:-

- 1 WORK TO BE CARRIED OUT WITH PROPER MATERIALS AND IN A WORKMANLIKE & SAFE MANNER IN ACCORDANCE WITH CDM REGULATIONS.
- 2 MATERIALS TO COMPLY WITH RELEVANT BRITISH & EUROPEAN STANDARDS AND USED IN ACCORDANCE WITH APPROPRIATE AGREEMENT CERTIFICATES. INCLUDE TESTING OF STRUCTURE FOR SOUND SEPARATION AND MODIFICATION TO COMPLY WITH PART E.
- 3 ALL STRUCTURAL TIMBERS, ROOF TIMBERS AND EXTERNAL JOINERY TO BE TREATED WITH APPROPRIATE PRESERVATIVE.
- 4 PROVIDE STRUTTING AT MID-SPAN OF JOISTS SPANNING 2.5 4.5M AND AT THIRD SPAN FOR JOISTS OVER 4.5M.
- FLOOR AND ROOF JOISTS AND RAFTERS TO BE SECURLY FIXED, WITH TIMBER NOGGINGS AND RESTRAINED BY 30mm x 5mm MILD STEEL STRAPS, TAKEN ACROSS AND FIXED TO 3No JOISTS @ 1000mm c/c. STRAPS TO BE TURNED UP 100mm AND FIXED TO EXISTING WALL.
- TIMBER RAFTERS AND FLOOR JOISTS IN PAIRS OR THREES TO BE BOLTED TOGETHER WITH M10 BOLTS AT 500 CENTRES.
- 7 ALL TIMBER DESIGNED TO BS 5268 AND TO BE GRADE C24 TO BS 5268.
- ALL STEELWORK DESIGNED TO BS 5950 AND TO BE GRADE S275 TO BS 4360. ALL STEELWORK MUST BE SOURCED FROM A CE APPROVED SUPPLIER/FABRICATOR AND CARRY A CE MARKING.
- 9 ALL BOLTS TO BE GRADE 8.8 TO BS 4190 UNO.
- 10 ALL WELDS TO BE 6mm FULL PROFILE FILLET WELDS UNLESS NOTED.
- 11 ALL STEELWORK TO BE ENCASED WITH FIRE BOARD TO PROVIDE APPROPRIATE FIRE RESISTANCE.
- WHERE TWIN BEAMS ARE SPECIFIED SPACERS AND THROUGH BOLTS (M12 BOLTS)
  TO BE USED TO AT 500mm CENTRES TO TIE THE BEAMS TOGETHER.
- 13 ALL LOADBEARING MASONRY TO BE IN ACCORDANCE WITH BS 5628.
- 14 ALL BLOCKS TO BE MIN 7.3 N/mm<sup>2</sup> UNLESS NOTED OTHERWISE
- 15 MORTAR TO BE (iii) ABOVE DPC AND (i) BELOW DPC.
- WALL TIES TO BE TYPE 2 STAINLESS STEEL AND TO BE PROVIDED AT 450 CENTRES VERTICALLY AND 900 CENTRES HORIZONTALLY STAGGERED.
- 17 ALL LINTELS TO HAVE A MIN 150mm BEARING UNLESS NOTED OTHERWISE.

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Notes

Calculation sheet :-

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#### **NOTES CONTINUED:-**

- ALL FOUNDATIONS TO BE TAKEN DOWN TO GOOD GROUND CAPABLE OF SUPPORTING A MINIMUM BEARING PRESSURE OF 100 kN/m<sup>2</sup> TO THE SATISFACTION OF THE BUILDING INSPECTOR
- 19 ALL FOUNDATIONS TO BE SYMMETRICAL UNDER WALLS AND STANCHIONS UNLESS NOTED OTHERWISE
- 20 CONCRETE IN FOUNDATIONS TO BE GRADE FND2 UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT HIS OPERATIONS DO NOT IN ANY WAY IMPAIR THE SAFETY OR STABILITY OF THE EXISTING STRUCTURE. HE SHALL PROVIDE ANY TEMPORARY SUPPORTS REQUIRED FOR THIS PURPOSE, AND SHALL CAREFULLY INSPECT THE CONDITION OF THE STRUCTURE BOTH BEFORE AND AFTER THE EXECUTION OF THE WORK.

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LOADING							
22.00							
Sloping Roof	On Plan						
	Tiles	=	0.70				
	Battens, felt, insulation	=	0.10				
	Rafters	=	0.20				
	Ceiling	=	0.15				
			1.15		1.40	=	1.61
	Imposed	=	0.75		1.60	=	1.20
Flat Roof			<u>1.90</u>				2.81 kN/m <sup>2</sup>
<u>1 101 11001</u>							
	Roof sheeting	=	0.15				
	Roofing felt	=	0.15				
	Insulation	=	0.40				
	Firrings & joists	=	0.20				
	Plasterboard	=	0.15	page and the	27.0 <b>0</b> 1.00		
	lmnaaad	_	1.05	*	1.40	=	1.47
	Imposed	=	0.75	*	1.60	=	1.20
Ground Floor			<u>1.80</u>				2.67 kN/m <sup>2</sup>
<u> </u>							
	Flooring	=	0.25				
	Joists	=	0.25				
	Ceiling	=	0.25		The contracts		86 (CS)
	T		0.75	*	1.40	=	1.05
	Imposed	=	1.50	*	1.60	=	2.40
			2.25				3.45 kN/m <sup>2</sup>
External Solid	Brickwall						
	245 mana haistaan II		4.00				
	215mm brickwall Plaster	=	4.30				
	1 103101	=	0.25 4.55	*	1.40	=	6.37 kN/m <sup>2</sup>
External Cavity	/ Wall		7.00		1.40	<del>-</del>	0.01 KIN/III
	103mm brickwall	=	2.20				
	100mm blockwall	=	1.40				
	Plaster	=	0.25	5000	Signs Astronomics		3
Internal Black	wall		3.85	*	1.40	=	5.39 kN/m <sup>2</sup>
Internal Blockw	<u>vali</u>						
	100mm blockwall	=	1.50				
	Plaster	=	0.50				
			2.00	*	1.40	=	2.80 kN/m <sup>2</sup>
Stud Wall		=	0.50				<u>0.70</u> kN/m <sup>2</sup>

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#### Beam supporting roof

**B**1

Span (L) = 
$$8.3 \text{ m}$$

Sloping roof = 
$$2.81 \times 3.00$$
 =  $8.43$  Unfactored  $5.70$  TOTAL W1 =  $8.43 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W2 =  $1.70 \times 100$  W1 =  $1.70 \times 100$  W1

Bending Moment = 
$$W1 \times L^2/8$$
 = **73 kNm**

I required = 
$$5 \times W2 \times L^4 \times 10^9$$
 = **7452 cm<sup>4</sup>**  
 $384 \times 205 \times 10^3 \times (L/360) \times 10^4$ 

### Use Beam Section 254 x 254 UC 73, $I_{xx} = 11300 \text{ cm}^4$ , $M_b = 160 \text{ kNm}$

#### Reactions

Service Reaction R1 = 
$$W2 \times L/2$$
 = 23.7 kN  
Factored Reaction R2 =  $W1 \times L/2$  = 35.0 kN

#### For 215mm thick existing wall

Length of spreader required = 
$$(R1 \times 10^3) / (0.42 \times 215 \times 1.5)$$
 = 175 mm  
Provide 215 x 215 x 150 deep mass concrete padstone.

### **Consulting Engineers**

#### Beam in basement supporting post and floor B 2 Span (L) 3.9 m Beam loads (kN/m) **UDL Loads** Factored Unfactored Ground Floor 3.45 x 4.00 13.80 = 9.00 Stud wall 0.70 x 2.70 1.89 1.35 TOTAL 15.69 kN/m W1 = 10.35 kN/m W2 =- UDL **Point Load** 1.20 m a = b = 2.70 m Factored Unfactored Point Load from Beam B1+ P1 = 52.69 35.63 = P2 $W1xL^2/8 + P1xaxb/L$ Bending Moment 74 kNm $\frac{P2[3a/L - 4(a/L)^{3}]}{48} = \frac{L^{3}}{E(L/360)} =$ ∫ 5xW2xL + 3003 cm4 Use Beam Section 203 x 203 UC 46, $I_{xx} = 4560 \text{ cm}^4$ , $M_b = 100 \text{ kNm}$ Reactions Service Reaction at A $R1_a = W2 \times L/2 + P2 \times b/L$ 44.8 kN Factored Reaction at A R2<sub>a</sub> = W1xL/2 + P1xb/L67.1 kN = Service Reaction at B R<sub>1</sub><sub>b</sub> = W2xL/2 + P2xa/L31.1 kN Factored Reaction at B R2<sub>h</sub> = W1xL/2 + P1xa/L46.8 kN At A For 215mm thick existing wall $= (R1 \times 10^3) / (0.42 \times 215 \times 1.5) =$ Length of spreader required 331 mm Provide 350 x 215 x 150 deep mass concrete padstone. At B For 215mm thick existing wall $= (R1 \times 10^3) / (0.42 \times 215 \times 1.5)$ Length of spreader required 230 mm Provide 350 x 215 x 150 deep mass concrete padstone.

## **Consulting Engineers**

#### Stanchion supporting beams B2

ST 1

Load on stanchion (kN)

Beams B2 x 2 = 
$$\frac{\text{Factored}}{\text{94}}$$
 =  $\frac{94}{\text{M1}}$  =  $\frac{62}{\text{94 kN}}$  W2 =  $\frac{62}{\text{62 kN}}$ 

Moment in stanchion 
$$M_{xx}$$
 = 18.7 kNm

$$P_{cv} = 1250 \text{ kN} \quad M_b = 125 \text{ kNm}$$

Combining

$$\frac{W1}{P_{cy}} + \frac{M_{xx}}{M_b} = 0.225$$
 < 1.00 Okay

Use 203 x 203 UC 46 Stanchion

#### **Foundation**

Presumed Ground Bearing Capacity  $(P_b)$  = 100  $kN/m^2$ 

Size of foundation required =  $0.62 \text{ m}^2$ 

Provide 850 x 850 x 500 deep mass concrete pad foundation