

SITE INVESTIGATION FACTUAL REPORT

Report No: [REDACTED]
Client: Crawford Claims Management
Site: 14 Lindfield Gardens
Client Ref: [REDACTED]
Date of Visit: 12/05/2020



Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys



Investigation Layout Plan			Sheet: 1 of 1	Site: 14 LINDFIELD GARDENS																																		
			Job No: [REDACTED]																																			
			Date: 12/05/20	Work carried out for: Crawford Claims Management																																		
DB (SI)	SA (Checked)	DVC (Drawn)	Weather: Dry																																			
<div><div></div><div><div>TP1</div><div>SHRUB</div><div>BH1</div><div>SHRUBS</div><div>TREE HT=8m D=2.7m</div><div>TREE HT=12m D=9m</div><div>BLOCK PAVING</div></div></div> <div>ON SITE TREE IDENTIFICATION FOR GUIDANCE ONLY. NOT AUTHENTICATED.</div> <table><tr><td rowspan="7">Remarks: Trial Pit 1 abandoned</td><td>Key:</td><td>Surface Water Drain</td><td>---</td></tr><tr><td>Combined Gully</td><td>RWWG</td><td>Foul Water Drain</td><td>---</td></tr><tr><td>Manhole</td><td>MH</td><td>Tree / Bush</td><td>(approx. ht in m)</td></tr><tr><td>Rain Water Pipe</td><td>RWP</td><td>Trial Pit</td><td></td></tr><tr><td>Rain Water Gully</td><td>RWG</td><td>Borehole</td><td></td></tr><tr><td>Soil Vent Pipe</td><td>SVP</td><td>O/D - Open Discharge</td><td></td></tr><tr><td>Waste Gully</td><td>WG</td><td></td><td></td></tr><tr><td>Waste Pipe</td><td>WP</td><td></td><td></td></tr></table> <table><tr><td>Scale:</td><td>N.T.S.</td></tr></table>					Remarks: Trial Pit 1 abandoned	Key:	Surface Water Drain	---	Combined Gully	RWWG	Foul Water Drain	---	Manhole	MH	Tree / Bush	(approx. ht in m)	Rain Water Pipe	RWP	Trial Pit		Rain Water Gully	RWG	Borehole		Soil Vent Pipe	SVP	O/D - Open Discharge		Waste Gully	WG			Waste Pipe	WP			Scale:	N.T.S.
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TEST REPORT: Trial Pit

REPORT NUMBER: [REDACTED]

TRIAL PIT REF: TP1

DATE: 12/05/2020

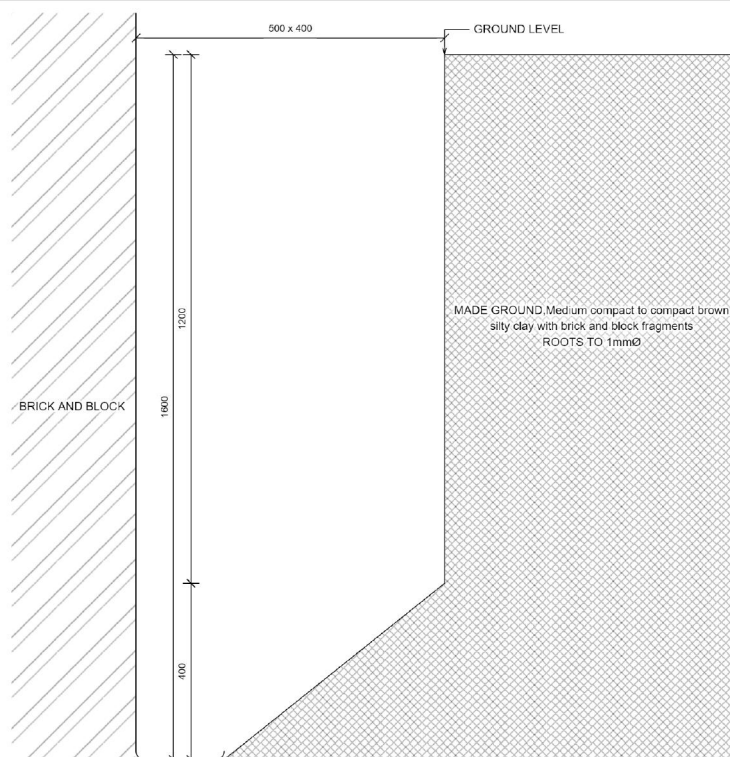
CLIENT: Crawford & Co

SITE: 14 Lindfield Gardens

JOB NO: [REDACTED]

WEATHER: Dry

EXCAVATION METHOD: Hand tools



U/S of foundation not found no soil sample taken. TP excavated to 1200mm Mackintosh probe used to find possible top of foundation at 1600mm

Key:
D Small disturbed sample J Jar sample
B Bulk disturbed sample V Pilcon vane (kPa)
W Water sample M Mackintosh probe
TDTD Too dense to drive

Remarks:
Test results reported relate only to the items tested.
This report shall not be reproduced except in full without approval of the Laboratory.
Amended report. This test report supersedes test report version 1

For and on behalf of CET
Scott Alger - Lab

Report Format:

Approved Signatory
12-May-20

[REDACTED]

[REDACTED]

[REDACTED]

Report version 2

Page 1 of 1

Borehole		1	Sheet: 1 of 2 Job No: Date: 12/05/2020 Ground Level:		Site: 14 Lindfield Gardens Client: Crawford Claims Management
Boring Method:	Rotary Auger				
Diameter (mm):	100	Weather:	Dry		
Depth	Soil Description				Samples and Tests
(m)		Thickness	Legend	Depth	Type Result
0.00	MADEGROUND medium compact brown silty sandy clay with brick and concrete	2.00			
2.00	Stiff orange-brown silty CLAY	1.00		2.00	DV 130
					140
				2.50	D
3.00	Very stiff orange-brown silty CLAY	3.00		3.00	DV 140+
					140+
				3.50	D
				4.00	DV 140+
					140+
				4.50	D
Remarks:		Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive		To Max Depth Dia (m) (mm)	
Logged:	Db	SA	Checked:	Approved:	Version V1.0 28/01/16 N.T.S.

Borehole		1	Sheet: 2 of 2 Job No: Date: 12/05/2020		Site: 14 Lindfield Gardens Client: Crawford Claims Management
Boring Method: Rotary Auger		Ground Level:			
Diameter (mm):	100	Weather:	Dry		
Depth	Soil Description				Thickness Legend Depth Type Results 5.00 DV 140+ 140+
(m)					
6.00	End of BH				6.00 DV 140+ 140+
Remarks: BH ends at 6m.BH dry and open on completion,no roots observed below 3.2m.					Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive
Logged:	Db	SA	Checked:	Approved:	To Max Depth Dia (m) (mm) 2.00 10 3.20 1 N.T.S.
Version		V1.0 28/01/16			

Laboratory Summary Results

Our Ref: [REDACTED]

Location: 14 Lindfield Gardens

Client: Crawford Claims Management

Address: [REDACTED]

Date Sampled: 12/05/2020

Date Received: 14/05/2020

Date Tested: 14/05/2020

Date of Report: 22/05/2020

Sample Ref TP/BH No	Depth (m)	Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity Index (%) [6]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (h)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated * Heave Potential (mm) [10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH * Value [13]	Sulphate Content * (g/l)		* Class [16]
																		SO ₃	SO ₄	
1	U/S 2.00	D	31	<5	69	27	42	0.11	42	CH	168	535			135					
	2.5	D	28	<5							168	1130								
	3.0	D	27	<5							168	1230			140					
	3.5	D																		
	4.0	D	31	<5	76	28	48	0.06	48	CV	168	1280			140					
	4.5	D																		
	5.0	D	34	<5							168	881			140					
	6.0	D	32	<5	74	29	45	0.07	45	CV	168	696			140					

Test Methods / Notes

[1] BS 1377: Part 2: 1990, Test No 3.2

[2] Test method 11-55, laboratory measured

[3] BS 1377: Part 2: 1990, Test No 4.4

[4] BS 1377: Part 2: 1990, Test No 5.1

[5] BS 1377: Part 2: 1990, Test No 5.4

[6] BS 1377: Part 2: 1990, Test No 5.4

[7] BS 5930: 2018, Figure 8 - Plasticity Chart for the classification

of fine soils

[8] In-house method S16 adapted from BS 1377: Part 2: 1990

[9] In-house Test Procedure S17, One Dimensional Swell/Shrink Test

[10] Estimated Heave Potential

[11] Values of shear strength were determined in situ by CPT using

a Platon hand vane or cone vane (GV).

[12] BS 1377: Part 2: 1990, Test No 4

[13] BS 1377: Part 2: 1990, Test No 9

[14] BS 1377: Part 2: 1990, Test No 5.6

[15] SO₃ = 1.2 x SO₄

[16] BS 5930: 2018, Figure 8 - Plasticity Chart for the classification

Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be

prudent to consider the sample as falling into the DS-4 or DS-5

class respectively unless water soluble magnesium testing is undertaken

to prove otherwise.

* These tests are not UKAS accredited

Full reports can be provided upon request.

Key

- D Disturbed sample (small)
- B Disturbed sample (bulk)
- U Undisturbed sample
- W Groundwater sample
- FNP Potentially Non-Plastic by inspection
- U/S Underside of Foundation

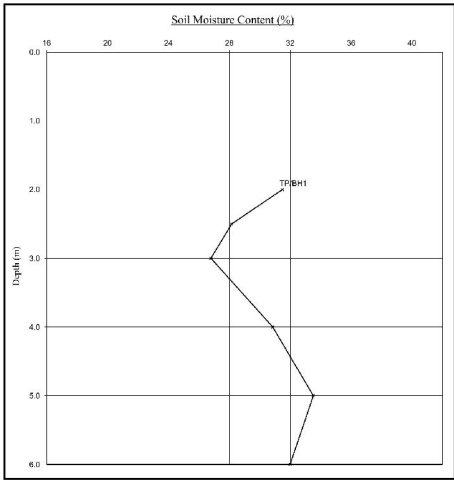


Version: SBH V1.1 - 13.01.2020

4161

Moisture Content Profiles

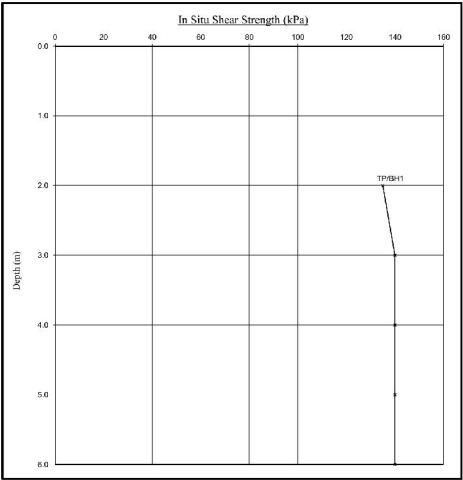
Our Ref: [redacted]
Location: 14 Linfield Gardens
Work carried out for: Crawford Claims Management



Notes:
1. If plotted, 0.4 LL and PI-2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Shear Strength Profiles

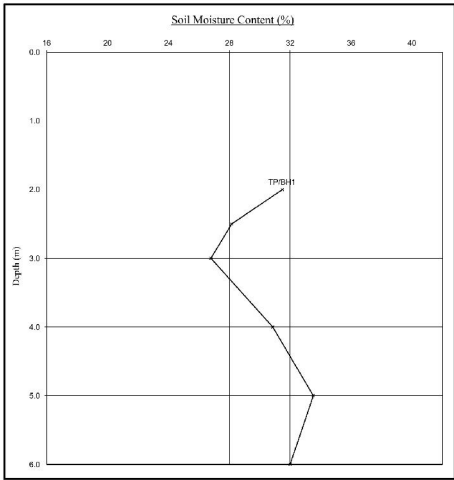
Date Sampled: 12/05/2020
Date Received: 14/05/2020
Date Tested: 14/05/2020
Date of Report: 22/05/2020



Note:
1. Unless otherwise stated, values of Shear Strength were determined in situ by CPT using a Phoenix Hand Vane the calibration of which is limited to a maximum reading of 140 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

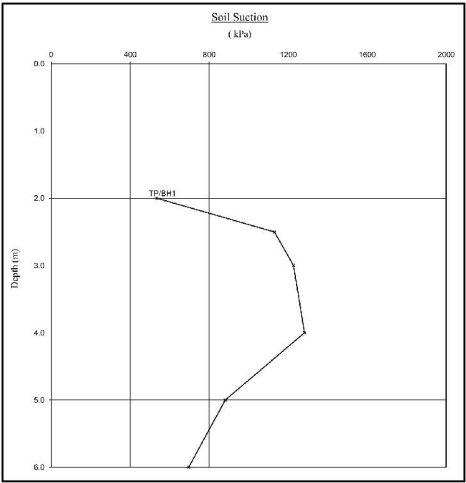
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Notes:
1. Triplotok, 0.4 TL and PI-2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Soil Suction Profiles

Date Sampled: 12/05/2020
Date Received: 14/05/2020
Date Tested: 14/05/2020
Date of Report: 22/05/2020



Note:
When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BS7 on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

Consultant: Dr M P Denne B.Sc. (Hons), M.Sc., Ph.D