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Network simulation/analysis Report

For

1:1 year, 1:30 year, 1:100year & 1:100+40%

Rainfall Events

For

Fitzroy Park Development

2nd June 2021

Rev.A 05th November 2021

Irish Office:
GFSC
Moneenageisha Road
Galway
Ireland

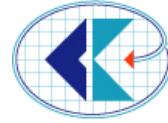
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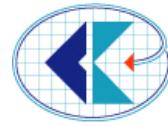
C

16-254-210602-01A

Revision A:

S1 & S2 drainage network simulation for the 1:30yr and 1:100yr + 40% storm events provided in Appendix B.

Revised attenuation calculations to include full site area is provided in Appendix C



INTRODUCTION

This report has been produced to support documentation already submitted in relation to the proposed development at Fitzroy Park. Refer to Coyle Kennedy report 16-254-190227-01RP rev C

This report, in conjunction with calculations and relevant drawings included in the appendices demonstrates that the surface water network is more than adequate to deal with the expected runoff from the various storm events - 1:1 year, 1:30 year, 1:100 year & 1:100 + 40% year rainfall events.

SURFACE WATER NETWORK ANALYSIS

For surface water drainage, the surface water networks have been divided up as follows;

- (i) Storm water network reference S1 & S2, taking the runoff from the paved carparking & road areas & discharging to the combined sewer running beneath Fitzroy Park via attenuation and a hydrocarbon interceptor.
- (ii) Storm water network reference S3, taking runoff from proposed blue/green roofs, paved areas & footpaths discharging through the attenuation/infiltration trench via percolation within the existing made ground to the Heath.

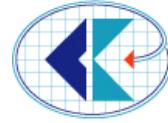
Refer to Figure 1 and Figure 2 on the following pages for diagrammatic locations of Storm Water Networks S1, S2 & S3.

Camden Co. Council have requested our client to demonstrate,

- the hydraulic performance of the proposed drainage network

We have used Autodesk Storm & Sanitary Analysis to run simulations for the 1:1yr, 1:30yr, 1:100yr & 1:100yr + 40% rainfall events for the surface water drainage networks. We have used rainfall data from the Met Office in our simulation model.

For the purposes of analysis for the green roofs Plot 1 to Plot 5 we have taken a figure of 0.05l/s runoff for the largest roof which is Plot 5. This figure is the equivalent of 2.0l/s per hectare and this value is applied to all the green roofs to take account of attenuation at source. Refer to Figure 2 for location of green roofs.



The resulting calculations show there is no surcharging of the surface water networks at any point during the simulation. We have included sections through the network at various times during the analysis for the 1:30yr and 1:100yr + 40% storm events. These sections and the calculations for all the rainfall events are included in Appendix A for S3 drainage network & Appendix B for S1 & S2 drainage networks. Revised attenuation calculations to include the full site area are provided in Appendix C,

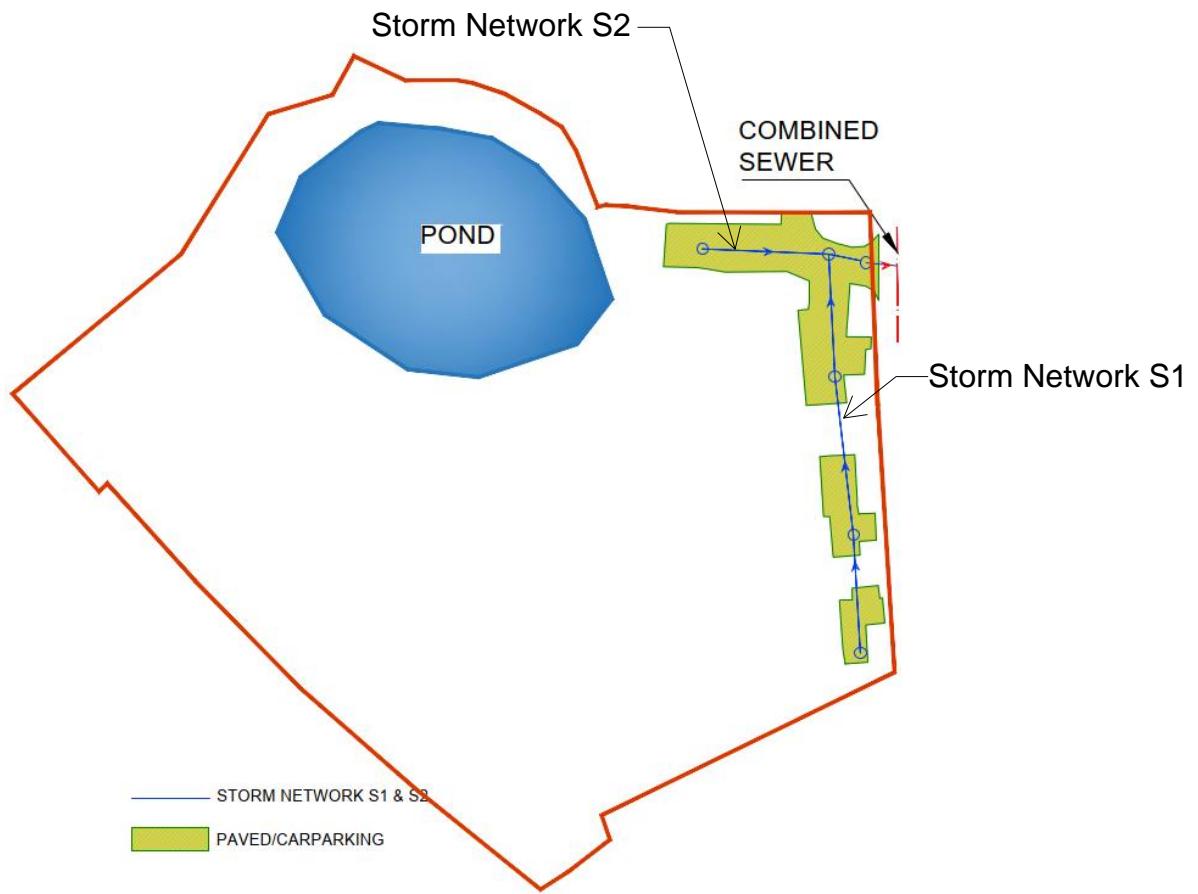
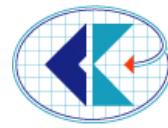


Figure 1.

Storm Network S1 & S2 with contributing paved carparking & road areas discharging to the combined sewer running beneath Fitzroy Park.

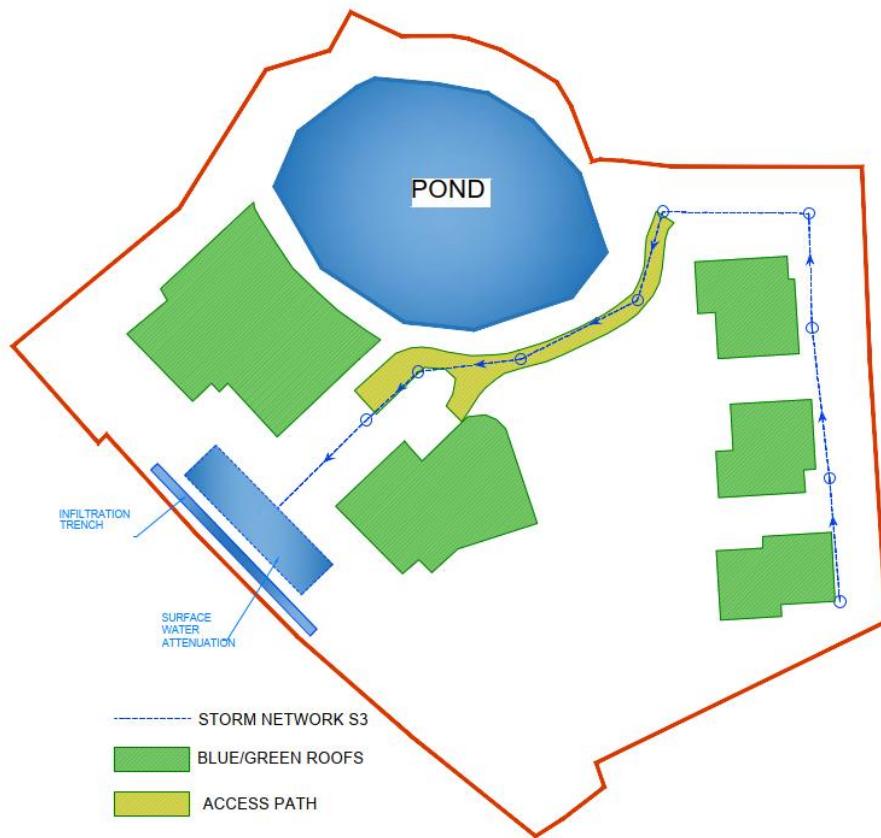
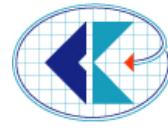
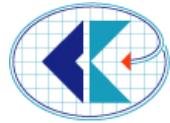


Figure 2.

Plan indicating Storm network S3 with contributing areas, Blue/Green Roofs, Access Path, Attenuation and Infiltration trench location.



Appendix A

Surface Water Analysis

Calculations and Sections

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy ParkS3 100+40.SPF
Description 16-254 Fitzroy Park Development
1:100YR + 40%
STORM NETWORK S3

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 100 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date JUN-01-2021 00:00:00
Ending Date JUN-02-2021 00:01:00
Report Time Step 00:00:10

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.008	86.100
Continuity Error (%)	1.000	

	Volume	Volume
Flow Routing Continuity	hectare-m	Mliters
External Inflow	0.000	0.000
External Outflow	0.002	0.021
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

Runoff Coefficient Computations Report

Subbasin Path-5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Path-6

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

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Subbasin Plot 5-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot 4-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Subbasin Plot 1

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.06
Composite Area & Weighted Runoff Coeff.	0.01		0.06

Subbasin Plot 2

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.06
Composite Area & Weighted Runoff Coeff.	0.01		0.06

Subbasin Plot 3

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.06
Composite Area & Weighted Runoff Coeff.	0.01		0.06

Subbasin Plot 4

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.06
Composite Area & Weighted Runoff Coeff.	0.02		0.06

Subbasin Plot 5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.06
Composite Area & Weighted Runoff Coeff.	0.02		0.06

Kirpich Time of Concentration Computations Report

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$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

Where:

Tc = Time of Concentration (min)
L = Flow length (m)
S = Slope (m/m)

Subbasin Path-5

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

Subbasin Path-6

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot 5-Paved

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot.4-Paved

Flow length (m):	20.00
Slope (%):	0.50
Computed TOC (minutes):	1.50

Subbasin Plot1

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot2

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot3

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

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Subbasin Plot4

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot5

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Path-5	86.10	14.35	77.49	0.18	0.900	0 00:05:00
Path-6	86.10	14.35	77.49	0.18	0.900	0 00:05:00
Plot 5-Paved	86.10	14.35	77.49	0.33	0.900	0 00:05:00
Plot .4-Paved	86.10	14.35	77.49	0.10	0.900	0 00:05:00
Plot1	86.10	14.35	5.17	0.03	0.060	0 00:05:00
Plot2	86.10	14.35	5.17	0.03	0.060	0 00:05:00
Plot3	86.10	14.35	5.17	0.03	0.060	0 00:05:00
Plot4	86.10	14.35	5.17	0.04	0.060	0 00:05:00
Plot5	86.10	14.35	5.17	0.05	0.060	0 00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S10	0.00	0.01	81.07	0 03:30	0	0	0:00:00
S11	0.00	0.01	80.31	0 03:34	0	0	0:00:00
S12	0.00	0.01	79.23	0 03:00	0	0	0:00:00
S13	0.00	0.01	78.43	0 03:00	0	0	0:00:00
S14	0.00	0.02	77.67	0 01:26	0	0	0:00:00
S15	0.02	0.08	77.58	0 06:00	0	0	0:00:00
S16(Outfall)	0.03	0.11	77.57	0 06:00	0	0	0:00:00
S6	0.00	0.00	82.80	0 00:56	0	0	0:00:00
S7	0.00	0.01	82.58	0 01:04	0	0	0:00:00
S8	0.00	0.01	82.42	0 02:55	0	0	0:00:00
S9	0.00	0.01	82.29	0 02:50	0	0	0:00:00
Outfall(Swale)	0.00	0.00	77.45	0 00:00	0	0	0:00:00
Stor-S3	0.01	0.03	77.58	0 06:01	0	0	0:00:00

Node Flow Summary

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Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Occurrence days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
S10	JUNCTION	0.00	0.08	0 02:38	0.00	
S11	JUNCTION	0.18	0.27	0 02:12	0.00	
S12	JUNCTION	0.00	0.27	0 03:22	0.00	
S13	JUNCTION	0.18	0.45	0 01:16	0.00	
S14	JUNCTION	0.00	0.45	0 01:08	0.00	
S15	JUNCTION	0.53	0.98	0 01:26	0.00	
S16(Outfall)	JUNCTION	0.00	0.91	0 06:01	0.00	
S6	JUNCTION	0.03	0.03	0 00:05	0.00	
S7	JUNCTION	0.03	0.06	0 00:56	0.00	
S8	JUNCTION	0.03	0.08	0 01:04	0.00	
S9	JUNCTION	0.00	0.08	0 05:13	0.00	
Outfall(Swale)	OUTFALL	0.00	0.91	0 06:00	0.00	
Stor-S3	STORAGE	0.00	0.17	0 00:23	0.00	

Storage Node Summary

Storage Node ID	Maximum Time of Max.	Maximum Total Ponded Volume 1000 m ³	Maximum Exfiltration Rate	Maximum Exfiltration Rate	Time of Max. Ponded Volume	Average Ponded Volume	Average Ponded Volume	Maximum Storage Node Outflow LPS
cmm	hh:mm:ss	1000 m ³	hh:mm:ss	hh:mm:ss	days hh:mm	1000 m ³	(%)	
Stor-S3	0.00 0:00:00	0.002 0.000	4	0 06:01	0 06:01	0.001	1	0.70

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Swale)	39.81	0.60	0.91
System	39.81	0.60	0.91

Link Flow Summary

Link ID Ratio of	Element Total	Element Reported	Time of Maximum Inflow	Length	Peak Flow	Design Flow	Ratio of

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Maximum Flow Depth	Time Surcharged minutes	Type Condition	Peak Occurrence	Flow hh:mm	Velocity Attained	Factor	during Analysis	Flow LPS	Capacity LPS	Maximum /Design Flow
			days	hh:mm	m/sec		LPS	LPS		
S3.000		CONDUIT	0	00:56	0.13	1.00	0.03	17.15	0.00	
0.04	0	Calculated								
S3.001		CONDUIT	0	01:04	0.15	1.00	0.06	13.28	0.00	
0.05	0	Calculated								
S3.002		CONDUIT	0	05:13	0.26	1.00	0.08	13.42	0.01	
0.05	0	Calculated								
S3.003		CONDUIT	0	02:38	0.45	1.00	0.08	36.85	0.00	
0.03	0	Calculated								
S3.004		CONDUIT	0	02:12	0.36	1.00	0.08	36.60	0.00	
0.05	0	Calculated								
S3.005		CONDUIT	0	03:22	0.60	1.00	0.27	36.73	0.01	
0.06	0	Calculated								
S3.006		CONDUIT	0	01:16	0.52	1.00	0.27	35.61	0.01	
0.07	0	Calculated								
S3.007		CONDUIT	0	01:08	0.64	1.00	0.45	41.88	0.01	
0.10	0	Calculated								
S3.008		CONDUIT	0	01:26	0.21	1.00	0.45	13.53	0.03	
0.31	0	Calculated								
S3.009		CONDUIT	0	06:01	0.37	1.00	0.91	10.98	0.08	
0.63	0	Calculated								
Storage Link		CONDUIT	0	06:07	0.29	1.00	0.70	13.20	0.05	
0.33	0	Calculated								
Outfall Orifice		ORIFICE	0	06:00			0.91			
1.00										

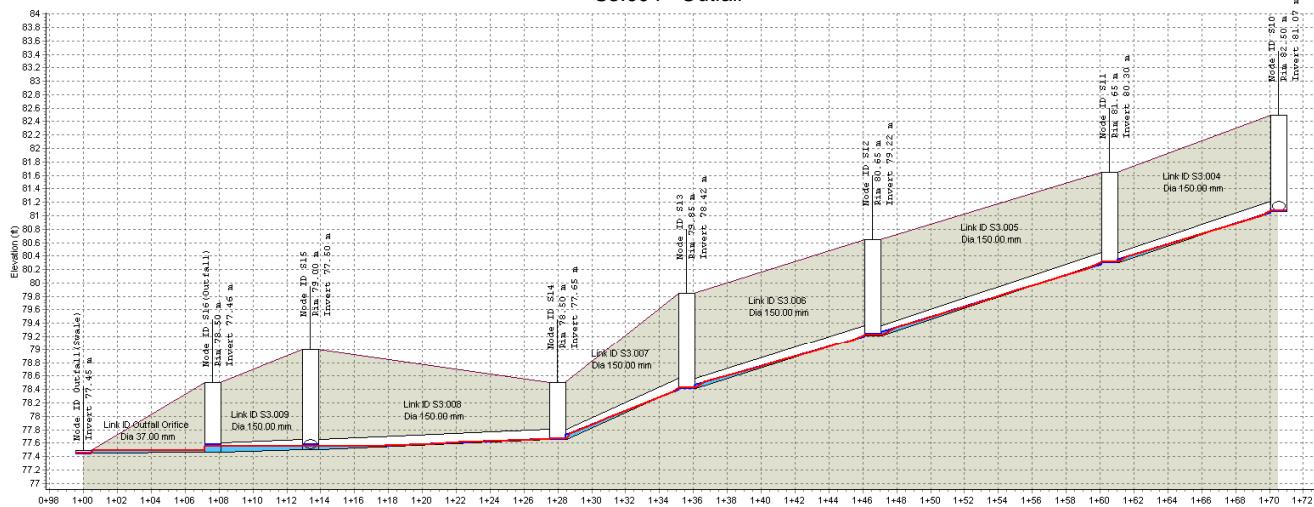
Highest Flow Instability Indexes

All links are stable.

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 Analysis ended on: Wed Jun 2 10:17:15 2021
 Total elapsed time: 00:00:04

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S3.004 - Outfall

06/01/2021 01:00:00

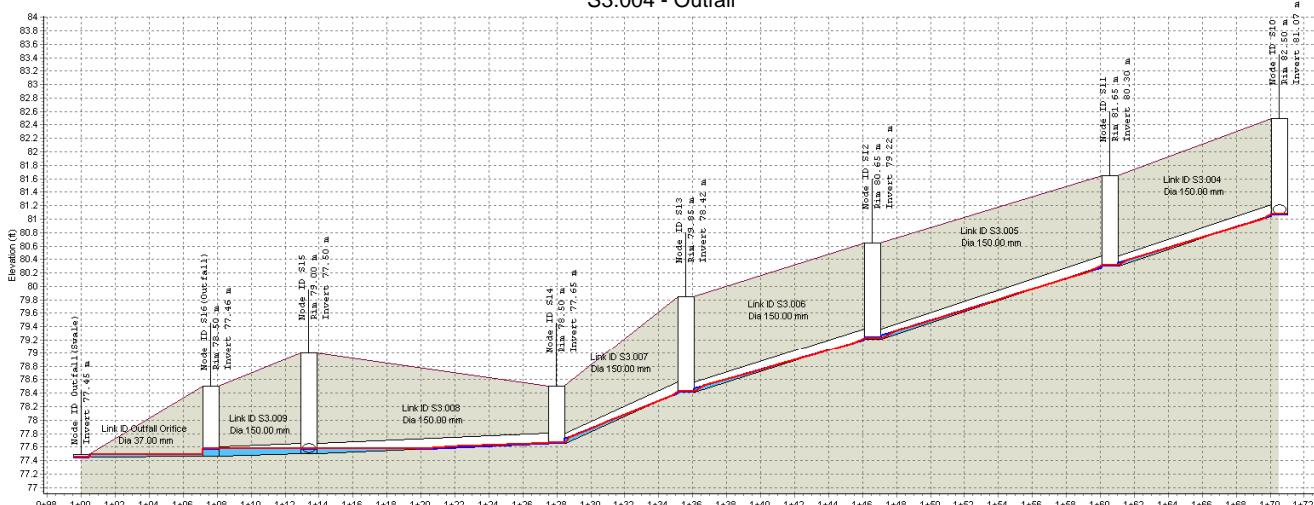


Node ID:	S16(Outfall)	S15	S14	S13	S12	S11	S10
Rim (m):	78.50	79.00	78.50	79.85	80.65	81.65	82.50
Invert (m):	77.45	77.46	77.50	77.65	78.42	79.22	80.30
Min Pipe Cover (m):	0.00	1.35	0.70	1.28	1.28	1.20	1.28
Max HGL (m):	77.45	77.57	77.58	77.67	78.43	79.23	80.31
Link ID:	Outfall Orifice	S3.009	S3.008	S3.007	S3.006	S3.005	S3.004
Length (m):	5.78	14.55	7.61	10.99	14.01	9.95	
Dia (mm):	150.00	150.00	150.00	150.00	150.00	150.00	
Slope (m/m):	0.0069	0.0105	0.1007	0.0728	0.0774	0.0769	
Up Invert (m):	77.46	77.50	77.65	78.42	79.22	80.30	81.07
Dn Invert (m):	77.45	77.46	77.50	77.65	78.42	79.22	80.30
Max Q (lps):	0.91	0.91	0.45	0.45	0.27	0.27	0.09
Max Vel (m/s):	0.00	0.37	0.21	0.64	0.52	0.60	0.36
Max Depth (m):	0.00	0.09	0.05	0.01	0.01	0.01	0.01

1:100yr +40% 6hr Storm Analysis - Elapsed Time 01:00:00

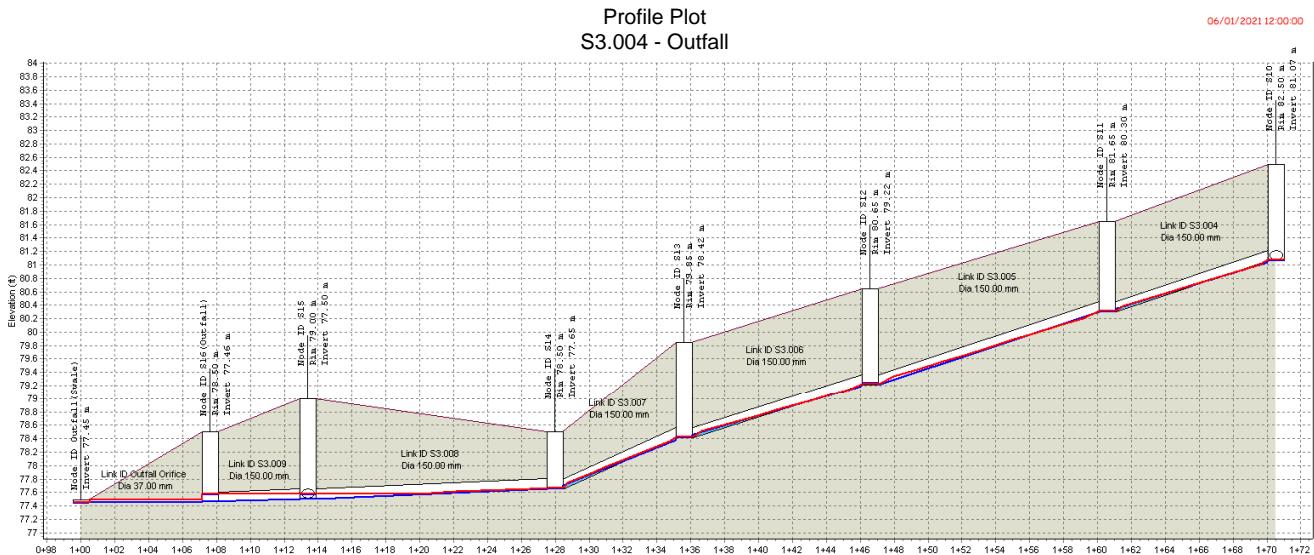
Profile Plot
S3.004 - Outfall

06/01/2021 06:00:00

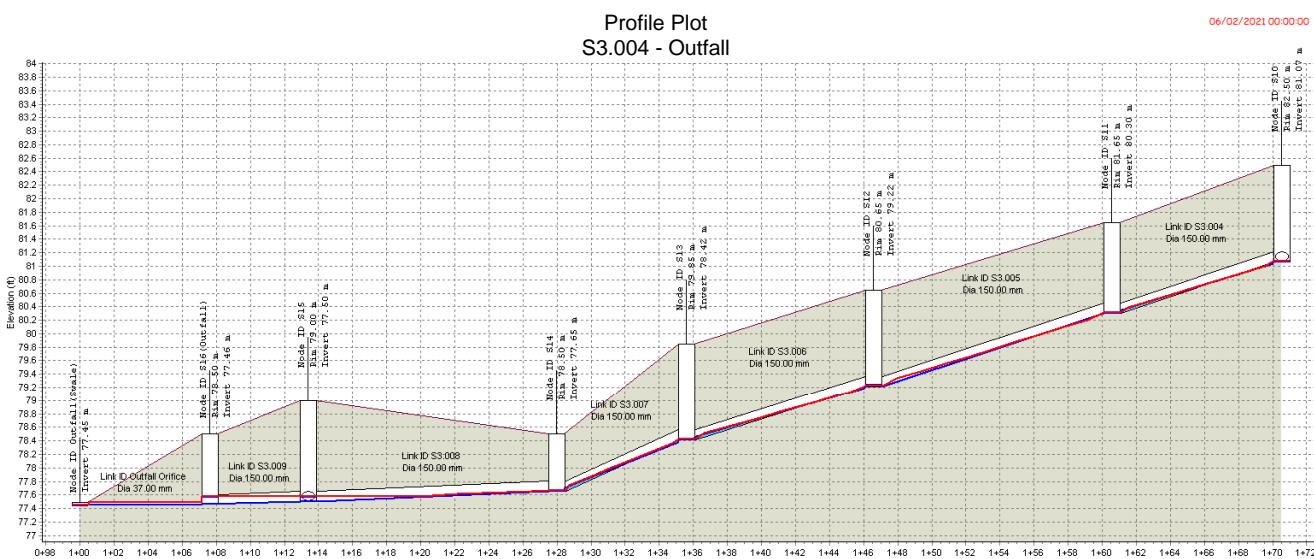


Node ID:	S16(Outfall)	S15	S14	S13	S12	S11	S10
Rim (m):	78.50	79.00	78.50	79.85	80.65	81.65	82.50
Invert (m):	77.45	77.46	77.50	77.65	78.42	79.22	80.30
Min Pipe Cover (m):	0.00	1.35	0.70	1.28	1.28	1.20	1.28
Max HGL (m):	77.45	77.57	77.58	77.67	78.43	79.23	80.31
Link ID:	Outfall Orifice	S3.009	S3.008	S3.007	S3.006	S3.005	S3.004
Length (m):	5.78	14.55	7.61	10.99	14.01	9.95	
Dia (mm):	150.00	150.00	150.00	150.00	150.00	150.00	
Slope (m/m):	0.0069	0.0105	0.1007	0.0728	0.0774	0.0769	
Up Invert (m):	77.46	77.50	77.65	78.42	79.22	80.30	81.07
Dn Invert (m):	77.45	77.46	77.50	77.65	78.42	79.22	80.30
Max Q (lps):	0.91	0.91	0.45	0.45	0.27	0.27	0.09
Max Vel (m/s):	0.00	0.37	0.21	0.64	0.52	0.60	0.36
Max Depth (m):	0.00	0.09	0.05	0.01	0.01	0.01	0.01

1:100yr +40% 6hr Storm Analysis - Elapsed Time 06:00:00



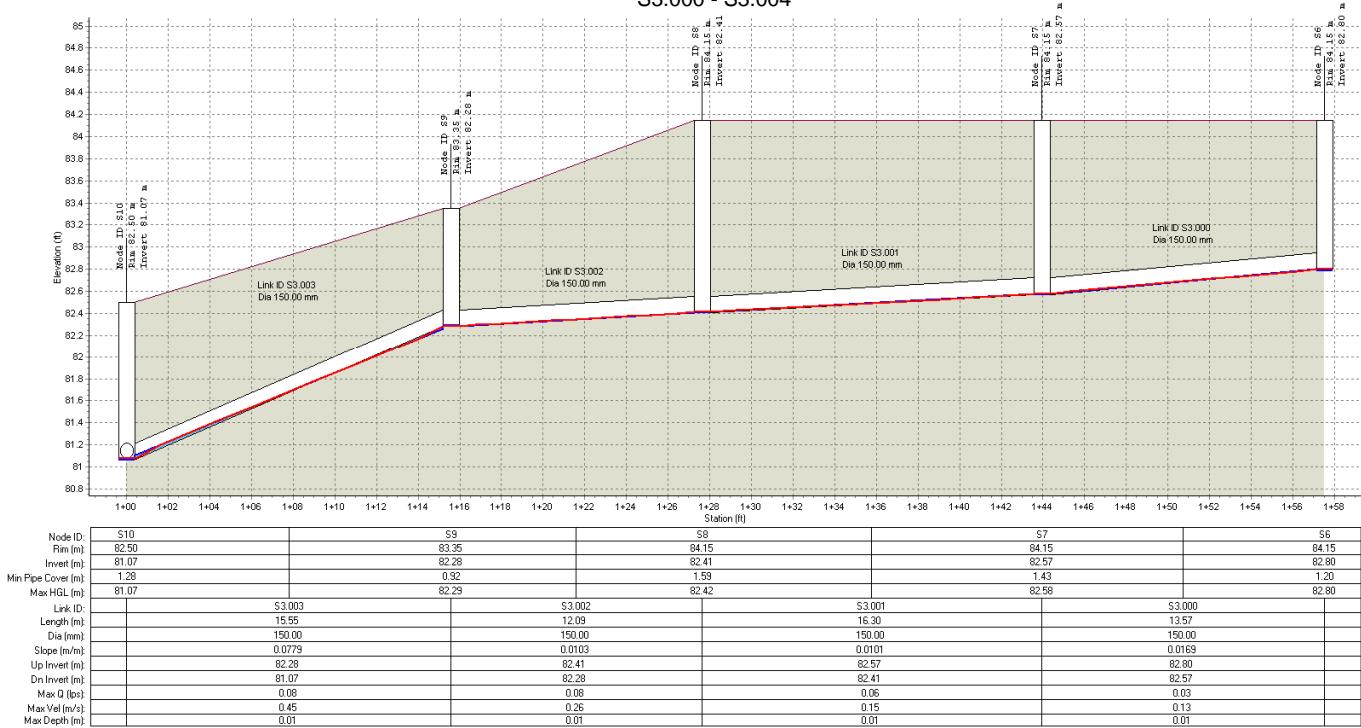
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1:100yr +40% 6hr Storm Analysis - Elapsed Time 24:00:00

Profile Plot
S3.000 - S3.004

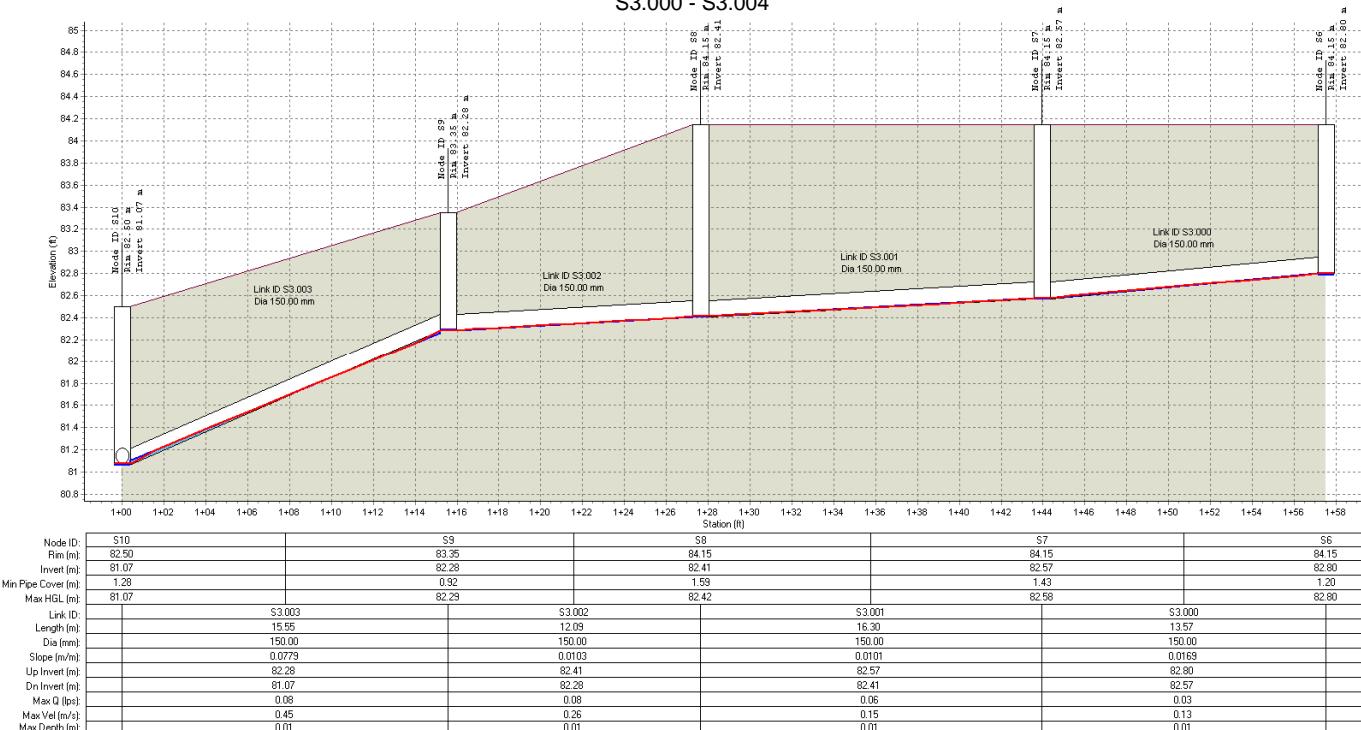
06/01/2021 01:00:00



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Profile Plot
S3.000 - S3.004

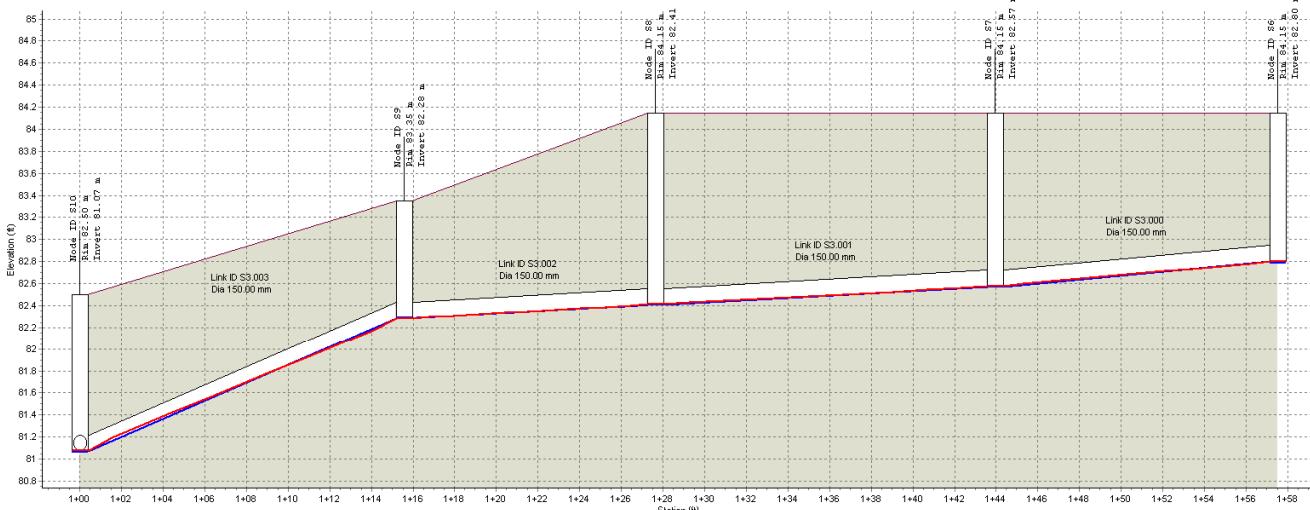
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1:100yr +40% 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S3.000 - S3.004

06/01/2021 12:00:00

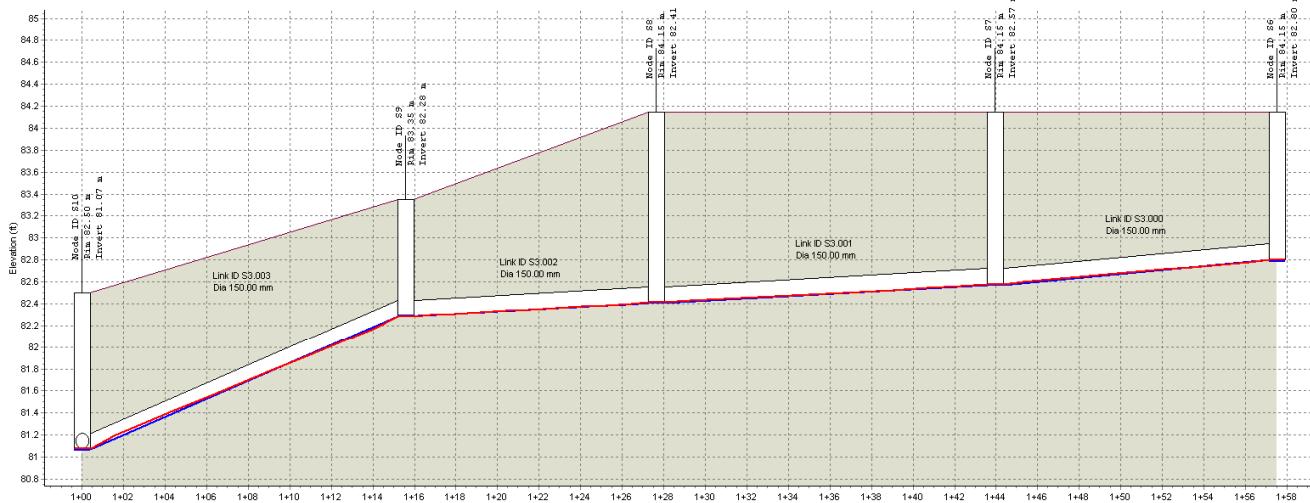


Node ID:	S10	S9	S8	S7	S6
Rim (m):	82.50	83.35	84.15	84.15	84.15
Invert (m):	81.07	82.28	82.41	82.57	82.80
Min Pipe Cover (m):	1.28	0.92	1.59	1.43	1.20
Max HGL (m):	81.07	82.29	82.42	82.58	82.80
Link ID:	S3.003	S3.002	S3.001	S3.000	
Length (m):	15.55	12.09	16.30		13.57
Di (mm):	150.00	150.00	150.00		150.00
Slope (m/m):	0.0779	0.0103	0.0101		0.0169
Up Invert (m):	82.28	82.41	82.57		82.80
Dn Invert (m):	81.07	82.28	82.41		82.57
Max Q (lps):	0.08	0.08	0.06		0.03
Max Vel (m/s):	0.45	0.26	0.15		0.13
Max Depth (m):	0.01	0.01	0.01		0.01

1:100yr +40% 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S3.000 - S3.004

06/02/2021 00:00:00



Node ID:	S10	S9	S8	S7	S6
Rim (m):	82.50	83.35	84.15	84.15	84.15
Invert (m):	81.07	82.28	82.41	82.57	82.80
Min Pipe Cover (m):	1.28	0.92	1.59	1.43	1.20
Max HGL (m):	81.07	82.29	82.42	82.58	82.80
Link ID:	S3.003	S3.002	S3.001	S3.000	
Length (m):	15.55	12.09	16.30		13.57
Di (mm):	150.00	150.00	150.00		150.00
Slope (m/m):	0.0779	0.0103	0.0101		0.0169
Up Invert (m):	82.28	82.41	82.57		82.80
Dn Invert (m):	81.07	82.28	82.41		82.57
Max Q (lps):	0.08	0.08	0.06		0.03
Max Vel (m/s):	0.45	0.26	0.15		0.13
Max Depth (m):	0.01	0.01	0.01		0.01

1:100yr +40% 6hr Storm Analysis - Elapsed Time 24:00:00

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy ParkS3 100.SPF
Description 16-254 Fitzroy Park Development
1:100YR
STORM NETWORK S3

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 100 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date JUN-01-2021 00:00:00
Ending Date JUN-02-2021 00:01:00
Report Time Step 00:00:10

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.006	61.500
Continuity Error (%)	1.000	

	Volume	Volume
Flow Routing Continuity	hectare-m	Mliters
External Inflow	0.000	0.000
External Outflow	0.002	0.016
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

Runoff Coefficient Computations Report

Subbasin Path-5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Path-6

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

16-254 Fitzroy Park Development

Subbasin Plot 5-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot 4-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Subbasin Plot 1

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.09
Composite Area & Weighted Runoff Coeff.	0.01		0.09

Subbasin Plot 2

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.09
Composite Area & Weighted Runoff Coeff.	0.01		0.09

Subbasin Plot 3

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.09
Composite Area & Weighted Runoff Coeff.	0.01		0.09

Subbasin Plot 4

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.09
Composite Area & Weighted Runoff Coeff.	0.02		0.09

Subbasin Plot 5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.09
Composite Area & Weighted Runoff Coeff.	0.02		0.09

Kirpich Time of Concentration Computations Report

16-254 Fitzroy Park Development

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

Where:

Tc = Time of Concentration (min)
L = Flow length (m)
S = Slope (m/m)

Subbasin Path-5

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

Subbasin Path-6

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot 5-Paved

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot.4-Paved

Flow length (m):	20.00
Slope (%):	0.50
Computed TOC (minutes):	1.50

Subbasin Plot1

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot2

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot3

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

16-254 Fitzroy Park Development

Subbasin Plot4

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot5

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Path-5	61.50	10.25	55.35	0.13	0.900	0 00:05:00
Path-6	61.50	10.25	55.35	0.13	0.900	0 00:05:00
Plot 5-Paved	61.50	10.25	55.35	0.24	0.900	0 00:05:00
Plot .4-Paved	61.50	10.25	55.35	0.07	0.900	0 00:05:00
Plot1	61.50	10.25	5.53	0.03	0.090	0 00:05:00
Plot2	61.50	10.25	5.53	0.03	0.090	0 00:05:00
Plot3	61.50	10.25	5.53	0.03	0.090	0 00:05:00
Plot4	61.50	10.25	5.53	0.05	0.090	0 00:05:00
Plot5	61.50	10.25	5.53	0.05	0.090	0 00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S10	0.00	0.01	81.07	0 03:07	0	0	0:00:00
S11	0.00	0.01	80.31	0 01:34	0	0	0:00:00
S12	0.00	0.01	79.23	0 01:13	0	0	0:00:00
S13	0.00	0.01	78.43	0 01:20	0	0	0:00:00
S14	0.00	0.02	77.67	0 01:30	0	0	0:00:00
S15	0.01	0.05	77.55	0 06:00	0	0	0:00:00
S16(Outfall)	0.02	0.09	77.55	0 05:59	0	0	0:00:00
S6	0.00	0.00	82.80	0 00:56	0	0	0:00:00
S7	0.00	0.01	82.58	0 01:21	0	0	0:00:00
S8	0.00	0.01	82.42	0 02:15	0	0	0:00:00
S9	0.00	0.01	82.29	0 04:49	0	0	0:00:00
Outfall(Swale)	0.00	0.00	77.45	0 00:00	0	0	0:00:00
Stor-S3	0.00	0.00	77.55	0 00:00	0	0	0:00:00

Node Flow Summary

16-254 Fitzroy Park Development

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Occurrence days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
S10	JUNCTION	0.00	0.09	0 02:49	0.00	
S11	JUNCTION	0.13	0.22	0 01:25	0.00	
S12	JUNCTION	0.00	0.22	0 01:42	0.00	
S13	JUNCTION	0.13	0.35	0 01:09	0.00	
S14	JUNCTION	0.00	0.35	0 01:08	0.00	
S15	JUNCTION	0.41	0.76	0 01:11	0.00	
S16(Outfall)	JUNCTION	0.00	0.76	0 05:48	0.00	
S6	JUNCTION	0.03	0.03	0 00:05	0.00	
S7	JUNCTION	0.03	0.06	0 00:56	0.00	
S8	JUNCTION	0.03	0.09	0 01:20	0.00	
S9	JUNCTION	0.00	0.09	0 01:28	0.00	
Outfall(Swale)	OUTFALL	0.00	0.76	0 05:59	0.00	
Stor-S3	STORAGE	0.00	0.00	0 00:00	0.00	

Storage Node Summary

Storage Node ID	Maximum Time of Max.	Maximum Total Ponded Volume 1000 m³	Maximum Exfiltration Rate	Maximum Exfiltration Rate	Time of Max. Ponded Volume	Average Ponded Volume	Average Ponded Volume	Maximum Storage Node Outflow LPS
cmm	hh:mm:ss	1000 m³	hh:mm:ss	hh:mm:ss	days hh:mm	1000 m³	(%)	
Stor-S3	0.00 0:00:00	0.000 0.000	0	0 00:00	0	0.000	0	0.00

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Swale)	26.74	0.71	0.76
System	26.74	0.71	0.76

Link Flow Summary

Link ID Ratio of	Total	Element Reported	Time of Maximum Flow	Length	Peak Flow	Design Flow	Ratio of

16-254 Fitzroy Park Development

Maximum Flow Depth	Time Surcharged minutes	Type Condition	Peak	Flow	Velocity	Factor	during Analysis	Flow	Maximum
			Occurrence	Attained	m/sec	LPS		Capacity	/Design Flow
days	hh:mm						LPS		
S3.000 0.04	0	CONDUIT Calculated	0 00:56	0.13	1.00	0.03	17.15	0.00	
S3.001 0.05	0	CONDUIT Calculated	0 01:20	0.15	1.00	0.06	13.28	0.00	
S3.002 0.05	0	CONDUIT Calculated	0 01:28	0.27	1.00	0.09	13.42	0.01	
S3.003 0.04	0	CONDUIT Calculated	0 02:49	0.45	1.00	0.09	36.85	0.00	
S3.004 0.05	0	CONDUIT Calculated	0 01:25	0.37	1.00	0.09	36.60	0.00	
S3.005 0.06	0	CONDUIT Calculated	0 01:42	0.56	1.00	0.22	36.73	0.01	
S3.006 0.06	0	CONDUIT Calculated	0 01:09	0.50	1.00	0.22	35.61	0.01	
S3.007 0.09	0	CONDUIT Calculated	0 01:08	0.59	1.00	0.35	41.88	0.01	
S3.008 0.21	0	CONDUIT Calculated	0 01:11	0.20	1.00	0.35	13.53	0.03	
S3.009 0.44	0	CONDUIT Calculated	0 05:48	0.35	1.00	0.76	10.98	0.07	
Storage Link 0.16	0	CONDUIT Calculated	0 00:00	0.00	1.00	0.00	13.20	0.00	
Outfall Orifice 1.00	0	ORIFICE	0 05:59			0.76			

 Highest Flow Instability Indexes

 All links are stable.

Analysis began on: Wed Jun 2 11:44:11 2021
 Analysis ended on: Wed Jun 2 11:44:14 2021
 Total elapsed time: 00:00:03

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy ParkS3 100+40.SPF
Description 16-254 Fitzroy Park Development
1:30YR
STORM NETWORK S3

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 30 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date JUN-01-2021 00:00:00
Ending Date JUN-02-2021 00:01:00
Report Time Step 00:00:10

Runoff Quantity Continuity Volume Depth
hectare-m mm

Total Precipitation 0.004 44.580
Continuity Error (%) 1.000

Flow Routing Continuity Volume Volume
hectare-m Mliters

External Inflow 0.000 0.000
External Outflow 0.001 0.013
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.000
Continuity Error (%) 0.000

Runoff Coefficient Computations Report

Subbasin Path-5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Path-6

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

16-254 Fitzroy Park Development

Subbasin Plot 5-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot 4-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Subbasin Plot 1

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.13
Composite Area & Weighted Runoff Coeff.	0.01		0.13

Subbasin Plot 2

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.13
Composite Area & Weighted Runoff Coeff.	0.01		0.13

Subbasin Plot 3

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.13
Composite Area & Weighted Runoff Coeff.	0.01		0.13

Subbasin Plot 4

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.13
Composite Area & Weighted Runoff Coeff.	0.02		0.13

Subbasin Plot 5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.13
Composite Area & Weighted Runoff Coeff.	0.02		0.13

Kirpich Time of Concentration Computations Report

16-254 Fitzroy Park Development

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

Where:

Tc = Time of Concentration (min)
L = Flow length (m)
S = Slope (m/m)

Subbasin Path-5

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

Subbasin Path-6

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot 5-Paved

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot.4-Paved

Flow length (m):	20.00
Slope (%):	0.50
Computed TOC (minutes):	1.50

Subbasin Plot1

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot2

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot3

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

16-254 Fitzroy Park Development

Subbasin Plot4

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot5

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Path-5	44.58	7.43	40.12	0.10	0.900	0 00:05:00
Path-6	44.58	7.43	40.12	0.10	0.900	0 00:05:00
Plot 5-Paved	44.58	7.43	40.12	0.17	0.900	0 00:05:00
Plot .4-Paved	44.58	7.43	40.12	0.05	0.900	0 00:05:00
Plot1	44.58	7.43	5.80	0.03	0.130	0 00:05:00
Plot2	44.58	7.43	5.80	0.03	0.130	0 00:05:00
Plot3	44.58	7.43	5.80	0.03	0.130	0 00:05:00
Plot4	44.58	7.43	5.80	0.05	0.130	0 00:05:00
Plot5	44.58	7.43	5.80	0.05	0.130	0 00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S10	0.00	0.01	81.07	0 02:09	0	0	0:00:00
S11	0.00	0.01	80.31	0 05:00	0	0	0:00:00
S12	0.00	0.01	79.23	0 01:58	0	0	0:00:00
S13	0.00	0.01	78.43	0 04:17	0	0	0:00:00
S14	0.00	0.01	77.67	0 01:33	0	0	0:00:00
S15	0.01	0.02	77.52	0 05:58	0	0	0:00:00
S16(Outfall)	0.02	0.06	77.52	0 04:14	0	0	0:00:00
S6	0.00	0.00	82.80	0 01:09	0	0	0:00:00
S7	0.00	0.01	82.58	0 01:21	0	0	0:00:00
S8	0.00	0.01	82.42	0 01:48	0	0	0:00:00
S9	0.00	0.01	82.29	0 01:23	0	0	0:00:00
Outfall(Swale)	0.00	0.00	77.45	0 00:00	0	0	0:00:00
Stor-S3	0.00	0.00	77.55	0 00:00	0	0	0:00:00

Node Flow Summary

16-254 Fitzroy Park Development

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Occurrence days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
S10	JUNCTION	0.00	0.09	0 01:38	0.00	
S11	JUNCTION	0.10	0.19	0 01:21	0.00	
S12	JUNCTION	0.00	0.19	0 02:41	0.00	
S13	JUNCTION	0.10	0.28	0 01:13	0.00	
S14	JUNCTION	0.00	0.28	0 01:06	0.00	
S15	JUNCTION	0.33	0.61	0 01:26	0.00	
S16(Outfall)	JUNCTION	0.00	0.61	0 03:52	0.00	
S6	JUNCTION	0.03	0.03	0 00:05	0.00	
S7	JUNCTION	0.03	0.06	0 01:09	0.00	
S8	JUNCTION	0.03	0.09	0 01:19	0.00	
S9	JUNCTION	0.00	0.09	0 01:36	0.00	
Outfall(Swale)	OUTFALL	0.00	0.61	0 04:13	0.00	
Stor-S3	STORAGE	0.00	0.00	0 00:00	0.00	

Storage Node Summary

Storage Node ID	Maximum Time of Max.	Maximum Total Ponded Volume 1000 m ³	Maximum Exfiltration Rate	Maximum Exfiltration Rate	Time of Max. Ponded Volume	Average Ponded Volume	Average Ponded Volume	Maximum Storage Node Outflow LPS
cmm	hh:mm:ss	1000 m ³	hh:mm:ss	hh:mm:ss	days hh:mm	1000 m ³	(%)	
Stor-S3	0.00 0:00:00	0.000 0.000	0	0 00:00	0 00:00	0.000	0	0.00

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Swale)	26.69	0.57	0.61
System	26.69	0.57	0.61

Link Flow Summary

Link ID Ratio of	Element Total	Element Reported	Time of Maximum Inflow	Length	Peak Flow	Design Flow	Ratio of

16-254 Fitzroy Park Development

Maximum Flow Depth	Time minutes	Type Condition	Peak Occurrence	Flow hh:mm	Velocity Attained	Factor	during Analysis	Flow LPS	Capacity LPS	Maximum /Design Flow
				days hh:mm	m/sec			LPS		
S3.000 0.04	0	CONDUIT Calculated	0	01:09	0.14	1.00	0.03	17.15		0.00
S3.001 0.06	0	CONDUIT Calculated	0	01:19	0.16	1.00	0.06	13.28		0.00
S3.002 0.05	0	CONDUIT Calculated	0	01:36	0.27	1.00	0.09	13.42		0.01
S3.003 0.04	0	CONDUIT Calculated	0	01:38	0.46	1.00	0.09	36.85		0.00
S3.004 0.04	0	CONDUIT Calculated	0	01:21	0.38	1.00	0.09	36.60		0.00
S3.005 0.05	0	CONDUIT Calculated	0	02:41	0.54	1.00	0.19	36.73		0.01
S3.006 0.06	0	CONDUIT Calculated	0	01:13	0.49	1.00	0.19	35.61		0.01
S3.007 0.08	0	CONDUIT Calculated	0	01:06	0.56	1.00	0.28	41.88		0.01
S3.008 0.13	0	CONDUIT Calculated	0	01:26	0.21	1.00	0.28	13.53		0.02
S3.009 0.29	0	CONDUIT Calculated	0	03:52	0.34	1.00	0.61	10.98		0.06
Storage Link 0.08	0	CONDUIT Calculated	0	00:00	0.00	1.00	0.00	13.20		0.00
Outfall Orifice 1.00	0	ORIFICE	0	04:13			0.61			

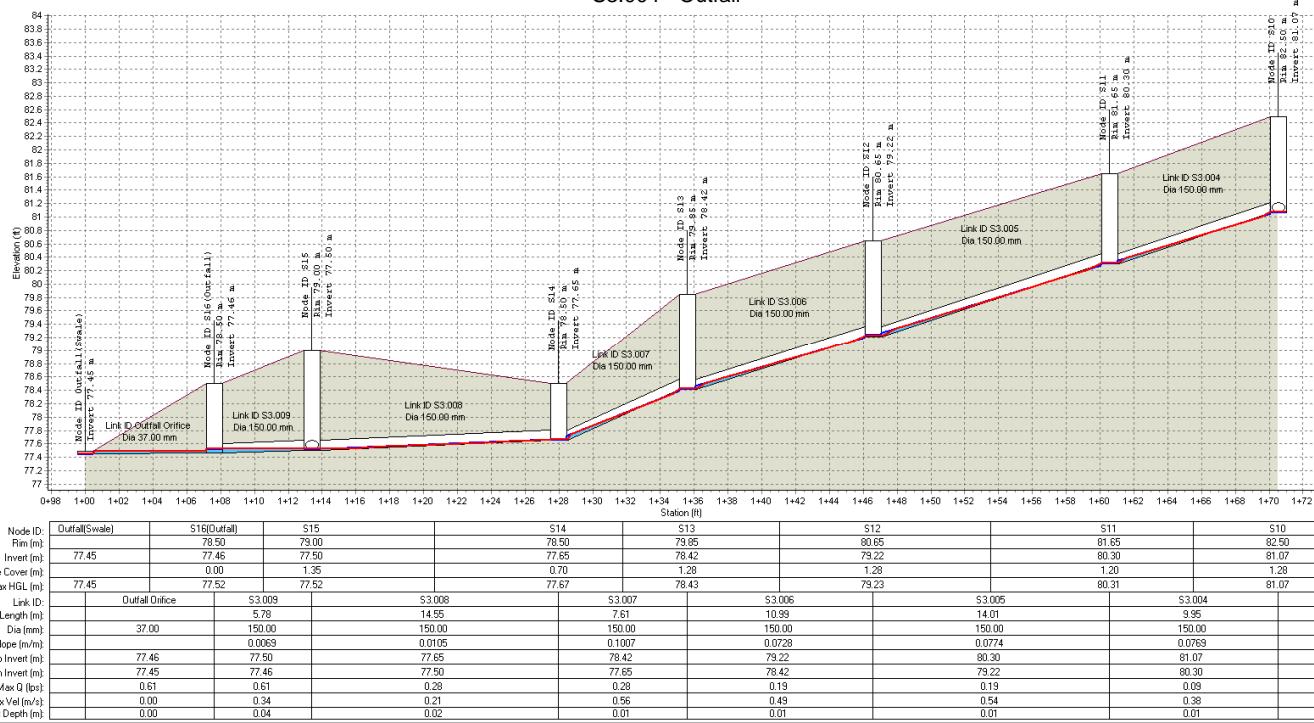
Highest Flow Instability Indexes

All links are stable.

Analysis began on: Wed Jun 2 10:49:05 2021
Analysis ended on: Wed Jun 2 10:49:08 2021
Total elapsed time: 00:00:03

Profile Plot
S3.004 - Outfall

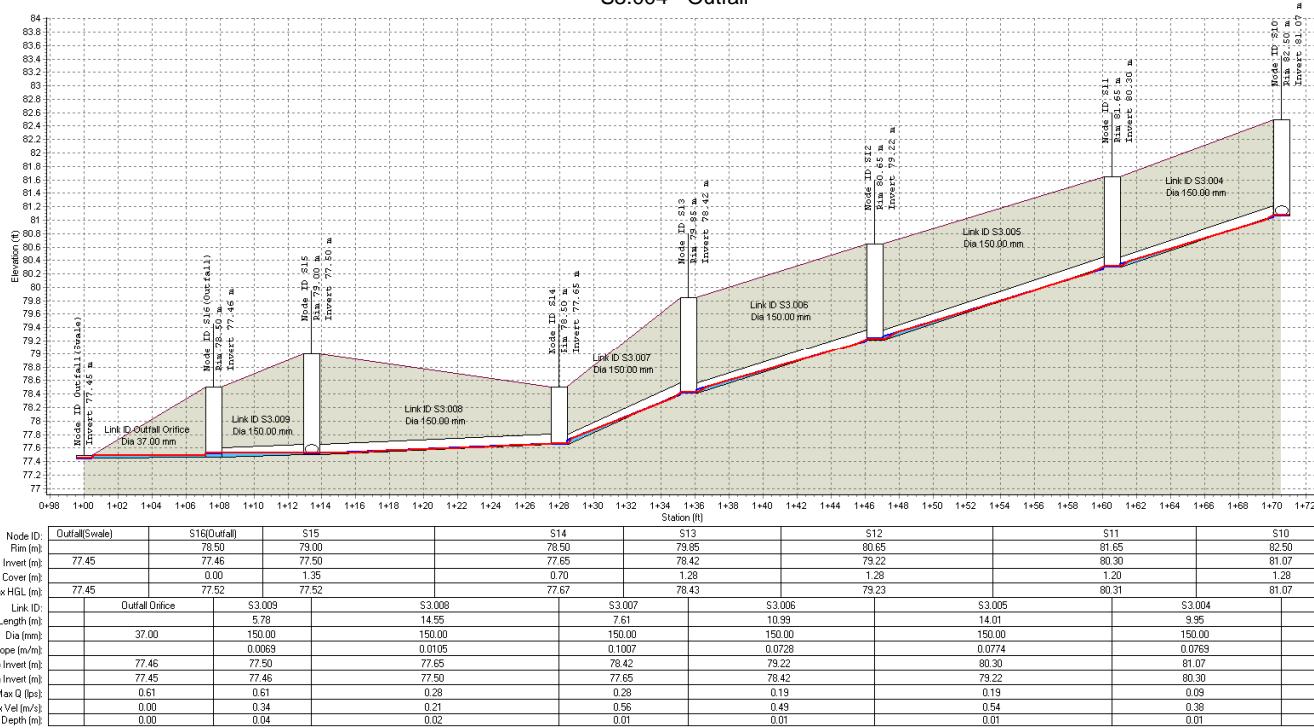
06/01/2021 01:00:00



1:30yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S3.004 - Outfall

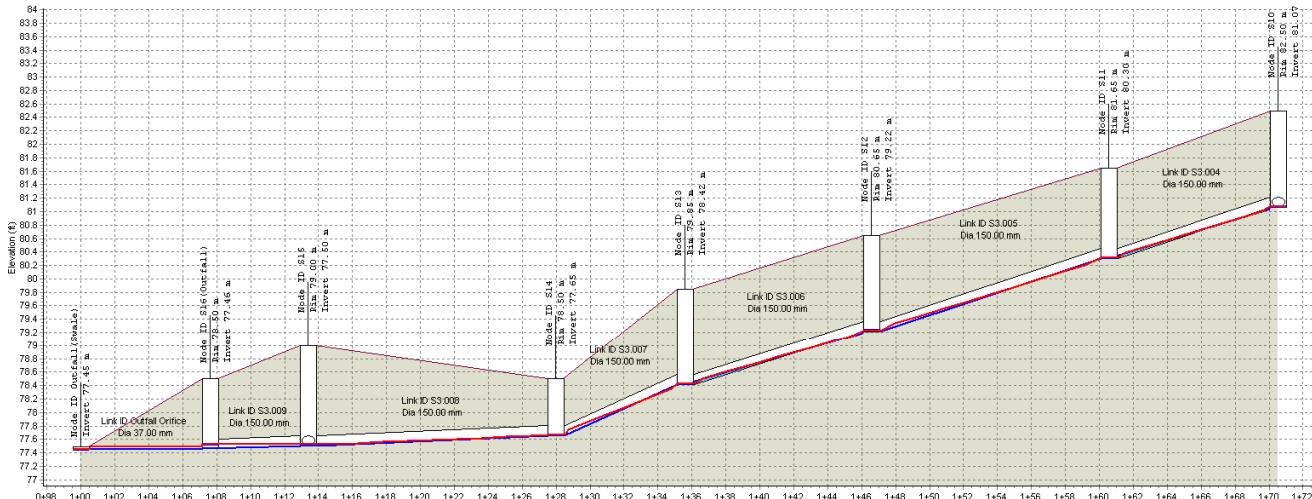
06/01/2021 06:00:00



1:30yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot S3.004 - Outfall

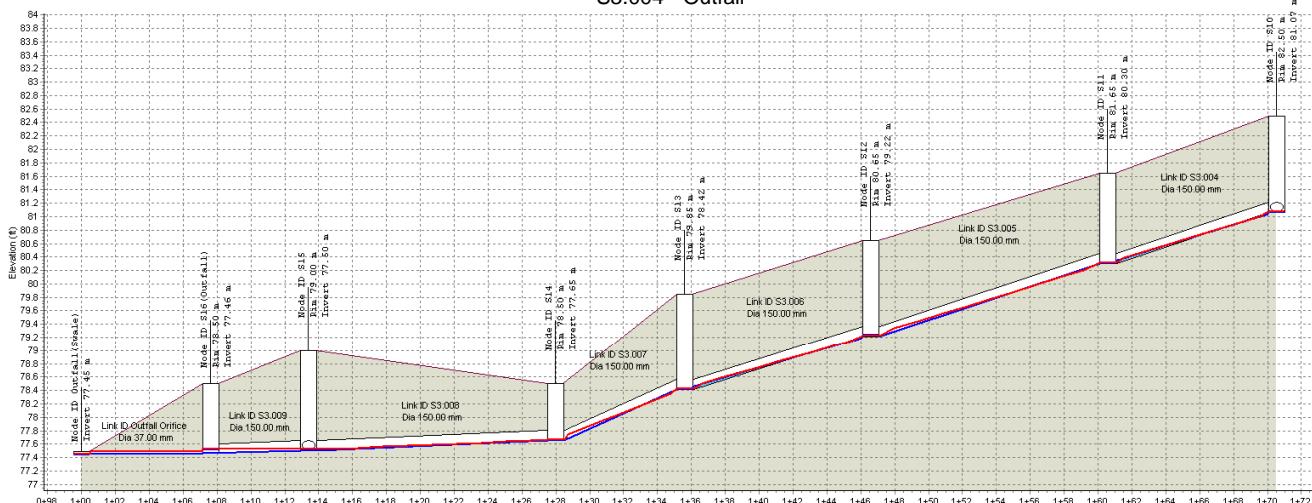
06/01/2021 12:00:00



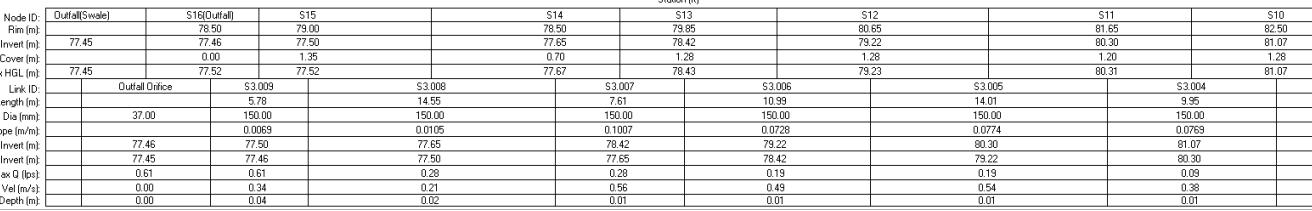
1:30yr 6hr Storm Analysis - Elapsed Time 12:00:00

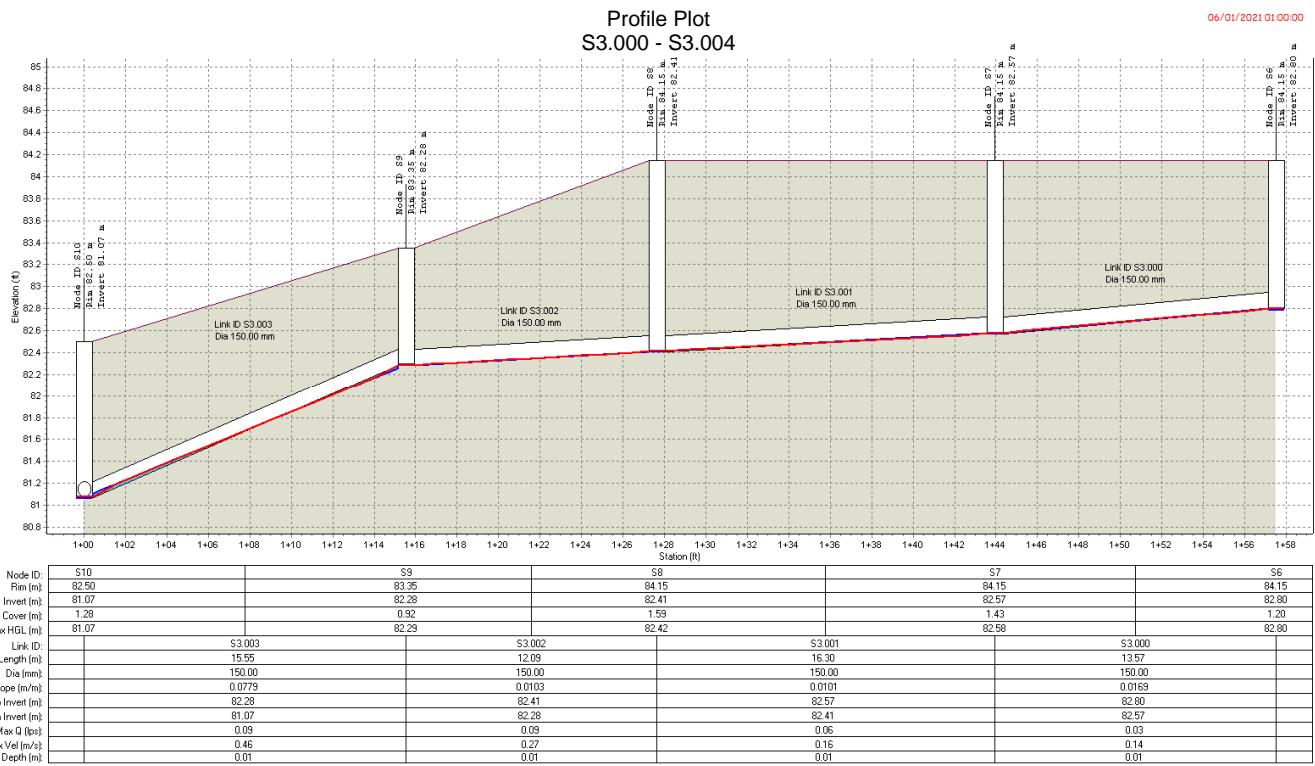
Profile Plot S3.004 - Outfall

06/02/2021 00:00:00

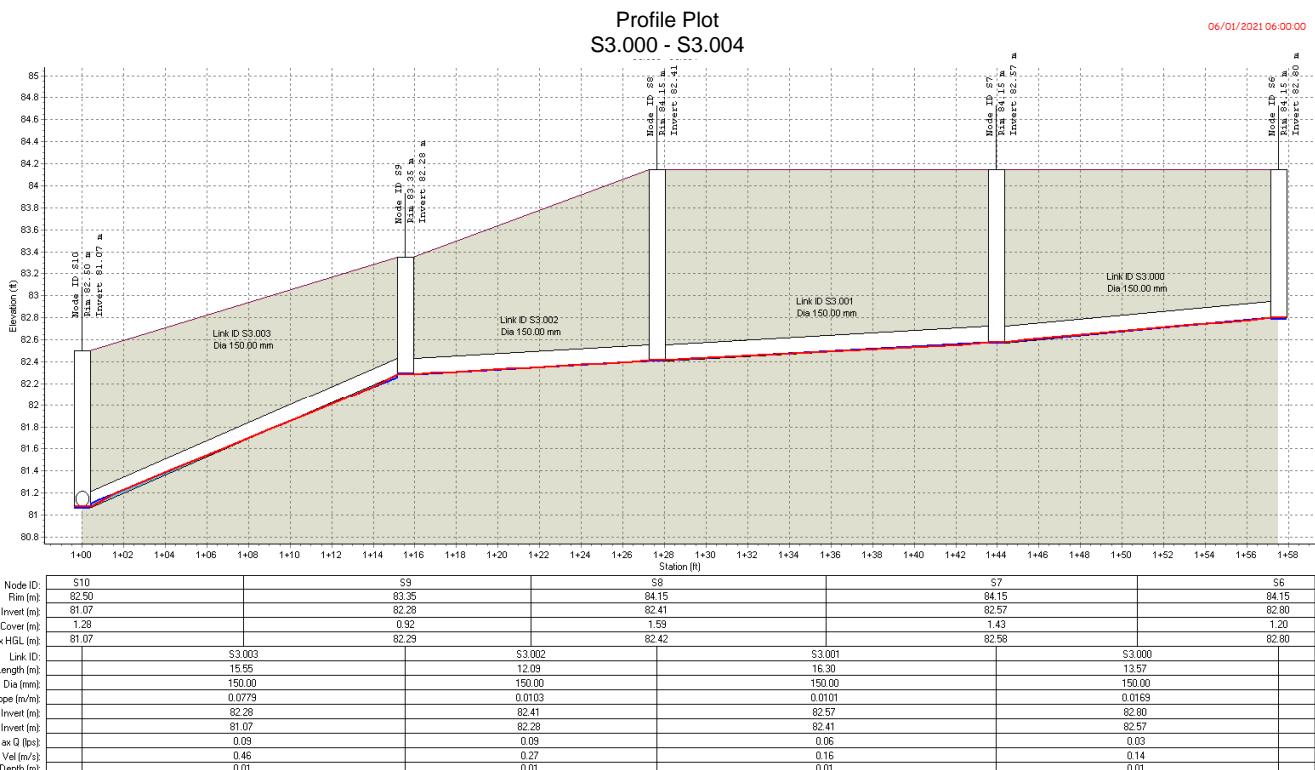


1:30yr 6hr Storm Analysis - Elapsed Time 24:00:00





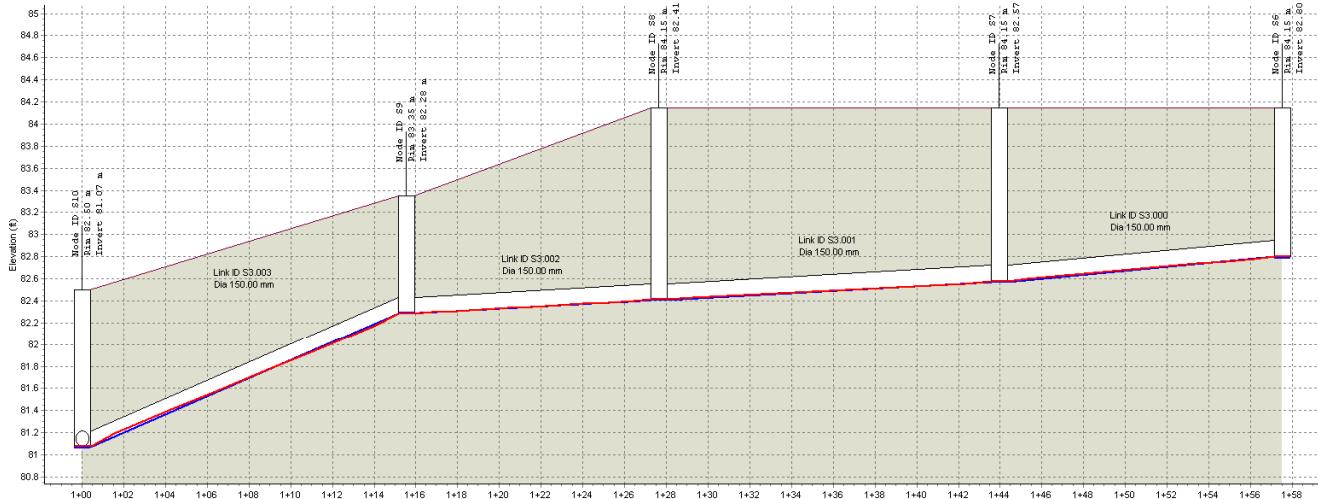
1:30yr 6hr Storm Analysis - Elapsed Time 01:00:00



1:30yr 6hr Storm Analysis - Elapsed Time 06:00:00

06/01/2021 12:00:00

Profile Plot S3.000 - S3.004

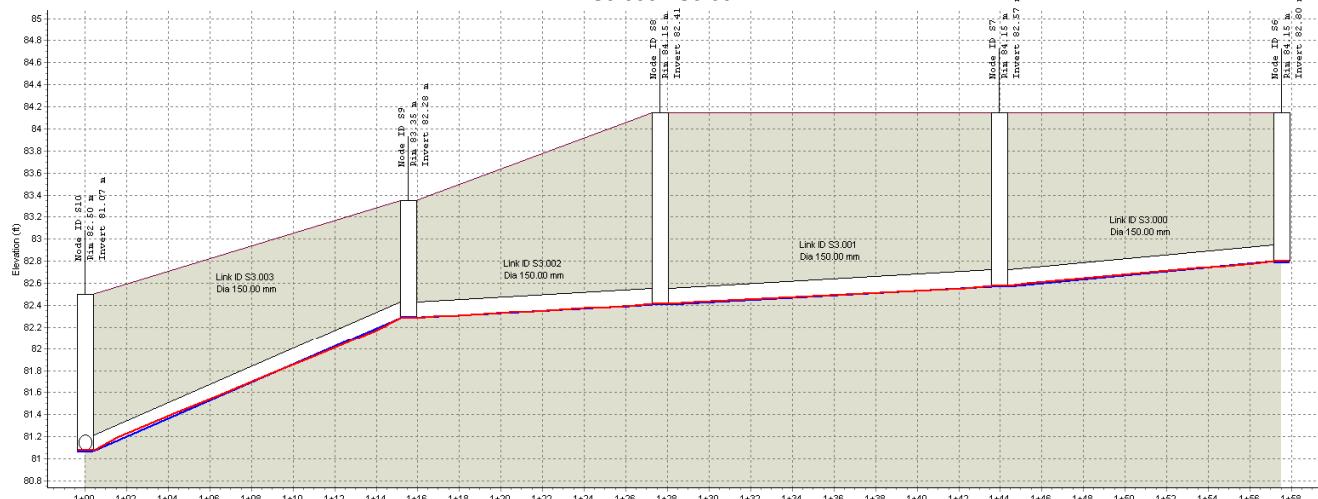


Node ID:	S10	S9	S8	S7	S6
Rim (m):	82.50	83.35	84.15	84.15	84.15
Invert (m):	81.07	82.28	82.41	82.57	82.80
Min Pipe Cover (m):	1.28	0.92	1.59	1.43	1.20
Max HGL (m):	81.07	82.29	82.42	82.58	82.80
Link ID:	S3.003	S3.002	S3.001	S3.000	
Length (m):	15.55	12.09	16.30	13.57	
Dia (mm):	150.00	150.00	150.00	150.00	
Slope (m/m):	0.0779	0.0103	0.0101	0.0169	
Up Invert (m):	82.28	82.41	82.57	82.80	
Dn Invert (m):	81.07	82.28	82.41	82.57	
Max Q (lps):	0.09	0.09	0.06	0.03	
Max Vel (m/s):	0.46	0.27	0.16	0.14	
Max Depth (m):	0.01	0.01	0.01	0.01	

1:30yr 6hr Storm Analysis - Elapsed Time 12:00:00

06/02/2021 00:00:00

Profile Plot S3.000 - S3.004



Node ID:	S10	S9	S8	S7	S6
Rim (m):	82.50	83.35	84.15	84.15	84.15
Invert (m):	81.07	82.28	82.41	82.57	82.80
Min Pipe Cover (m):	1.28	0.92	1.59	1.43	1.20
Max HGL (m):	81.07	82.29	82.42	82.58	82.80
Link ID:	S3.003	S3.002	S3.001	S3.000	
Length (m):	15.55	12.09	16.30	13.57	
Dia (mm):	150.00	150.00	150.00	150.00	
Slope (m/m):	0.0779	0.0103	0.0101	0.0169	
Up Invert (m):	82.28	82.41	82.57	82.80	
Dn Invert (m):	81.07	82.28	82.41	82.57	
Max Q (lps):	0.09	0.09	0.06	0.03	
Max Vel (m/s):	0.46	0.27	0.16	0.14	
Max Depth (m):	0.01	0.01	0.01	0.01	

1:30yr 6hr Storm Analysis - Elapsed Time 24:00:00

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy ParkS3 100+40.SPF
Description 16-254 Fitzroy Park Development
1:1YR
STORM NETWORK S3

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 1 year
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date JUN-01-2021 00:00:00
Ending Date JUN-02-2021 00:01:00
Report Time Step 00:00:10

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.002	19.500
Continuity Error (%)	1.000	

	Volume	Volume
Flow Routing Continuity	hectare-m	Mliters
External Inflow	0.000	0.000
External Outflow	0.001	0.008
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

Runoff Coefficient Computations Report

Subbasin Path-5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Path-6

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

16-254 Fitzroy Park Development

Subbasin Plot 5-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot 4-Paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Subbasin Plot 1

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.30
Composite Area & Weighted Runoff Coeff.	0.01		0.30

Subbasin Plot 2

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.30
Composite Area & Weighted Runoff Coeff.	0.01		0.30

Subbasin Plot 3

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.30
Composite Area & Weighted Runoff Coeff.	0.01		0.30

Subbasin Plot 4

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.30
Composite Area & Weighted Runoff Coeff.	0.02		0.30

Subbasin Plot 5

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.30
Composite Area & Weighted Runoff Coeff.	0.02		0.30

Kirpich Time of Concentration Computations Report

16-254 Fitzroy Park Development

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

Where:

Tc = Time of Concentration (min)
L = Flow length (m)
S = Slope (m/m)

Subbasin Path-5

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

Subbasin Path-6

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot 5-Paved

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

Subbasin Plot.4-Paved

Flow length (m):	20.00
Slope (%):	0.50
Computed TOC (minutes):	1.50

Subbasin Plot1

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot2

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot3

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

16-254 Fitzroy Park Development

Subbasin Plot4

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Plot5

Flow length (m):	15.00
Slope (%):	0.50
Computed TOC (minutes):	1.20

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Path-5	19.50	3.25	17.55	0.04	0.900	0 00:05:00
Path-6	19.50	3.25	17.55	0.04	0.900	0 00:05:00
Plot 5-Paved	19.50	3.25	17.55	0.08	0.900	0 00:05:00
Plot .4-Paved	19.50	3.25	17.55	0.02	0.900	0 00:05:00
Plot1	19.50	3.25	5.85	0.03	0.300	0 00:05:00
Plot2	19.50	3.25	5.85	0.03	0.300	0 00:05:00
Plot3	19.50	3.25	5.85	0.03	0.300	0 00:05:00
Plot4	19.50	3.25	5.85	0.05	0.300	0 00:05:00
Plot5	19.50	3.25	5.85	0.05	0.300	0 00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S10	0.00	0.01	81.07	0 03:12	0	0	0:00:00
S11	0.00	0.01	80.31	0 04:38	0	0	0:00:00
S12	0.00	0.01	79.23	0 03:39	0	0	0:00:00
S13	0.00	0.01	78.43	0 04:35	0	0	0:00:00
S14	0.00	0.01	77.66	0 05:00	0	0	0:00:00
S15	0.01	0.02	77.52	0 02:17	0	0	0:00:00
S16(Outfall)	0.01	0.04	77.50	0 05:00	0	0	0:00:00
S6	0.00	0.00	82.80	0 00:54	0	0	0:00:00
S7	0.00	0.01	82.58	0 01:07	0	0	0:00:00
S8	0.00	0.01	82.42	0 05:39	0	0	0:00:00
S9	0.00	0.01	82.29	0 05:01	0	0	0:00:00
Outfall(Swale)	0.00	0.00	77.45	0 00:00	0	0	0:00:00
Stor-S3	0.00	0.00	77.55	0 00:00	0	0	0:00:00

Node Flow Summary

16-254 Fitzroy Park Development

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Occurrence days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
S10	JUNCTION	0.00	0.09	0 05:39	0.00	
S11	JUNCTION	0.04	0.13	0 01:07	0.00	
S12	JUNCTION	0.00	0.13	0 03:12	0.00	
S13	JUNCTION	0.04	0.18	0 01:09	0.00	
S14	JUNCTION	0.00	0.18	0 01:09	0.00	
S15	JUNCTION	0.20	0.38	0 01:12	0.00	
S16(Outfall)	JUNCTION	0.00	0.38	0 01:12	0.00	
S6	JUNCTION	0.03	0.03	0 00:05	0.00	
S7	JUNCTION	0.03	0.06	0 00:54	0.00	
S8	JUNCTION	0.03	0.09	0 01:06	0.00	
S9	JUNCTION	0.00	0.09	0 02:19	0.00	
Outfall(Swale)	OUTFALL	0.00	0.38	0 01:21	0.00	
Stor-S3	STORAGE	0.00	0.00	0 00:00	0.00	

Storage Node Summary

Storage Node ID	Maximum Time of Max.	Maximum Total Ponded Volume 1000 m³	Maximum Exfiltration Rate	Maximum Exfiltration Rate	Time of Max. Ponded Volume	Average Ponded Volume	Average Ponded Volume	Maximum Storage Node Outflow LPS
cmm	hh:mm:ss	1000 m³	hh:mm:ss	hh:mm:ss	days hh:mm	1000 m³	(%)	
Stor-S3	0.00 0:00:00	0.000 0.000	0	0 00:00	0	0.000	0	0.00

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Swale)	26.61	0.35	0.38
System	26.61	0.35	0.38

Link Flow Summary

Link ID Ratio of	Element Total	Element Reported	Time of Maximum Inflow	Length	Peak Flow	Design Flow	Ratio of

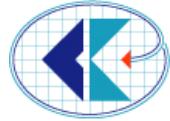
16-254 Fitzroy Park Development

Maximum Flow Depth	Time minutes	Type Condition	Peak Occurrence days hh:mm	Velocity Attained m/sec	Factor during Analysis	Flow Capacity LPS	Maximum /Design Flow LPS	
S3.000 0.04	0	CONDUIT Calculated	0 00:54	0.14	1.00	0.03	17.15	0.00
S3.001 0.06	0	CONDUIT Calculated	0 01:06	0.16	1.00	0.06	13.28	0.00
S3.002 0.05	0	CONDUIT Calculated	0 02:19	0.27	1.00	0.09	13.42	0.01
S3.003 0.04	0	CONDUIT Calculated	0 05:39	0.46	1.00	0.09	36.85	0.00
S3.004 0.04	0	CONDUIT Calculated	0 01:07	0.40	1.00	0.09	36.60	0.00
S3.005 0.04	0	CONDUIT Calculated	0 03:12	0.48	1.00	0.13	36.73	0.00
S3.006 0.05	0	CONDUIT Calculated	0 01:09	0.46	1.00	0.13	35.61	0.00
S3.007 0.06	0	CONDUIT Calculated	0 01:09	0.46	1.00	0.18	41.88	0.00
S3.008 0.10	0	CONDUIT Calculated	0 01:12	0.20	1.00	0.18	13.53	0.01
S3.009 0.18	0	CONDUIT Calculated	0 01:12	0.30	1.00	0.38	10.98	0.03
Storage Link 0.06	0	CONDUIT Calculated	0 00:00	0.00	1.00	0.00	13.20	0.00
Outfall Orifice 0.96		ORIFICE	0 01:21			0.38		

 Highest Flow Instability Indexes

 All links are stable.

Analysis began on: Wed Jun 2 11:31:05 2021
 Analysis ended on: Wed Jun 2 11:31:08 2021
 Total elapsed time: 00:00:03



Appendix B

Surface Water Analysis Calculations and Sections for S1 & S2 network

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy Park S1 S2 100yr+40.SPF
Description 16-254 Fitzroy Park Development
100yr Return + 40%
Storm Network S1 & S2

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 100 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date NOV-04-2021 00:00:00
Ending Date NOV-05-2021 00:00:00
Report Time Step 00:00:10

Runoff Quantity Continuity Volume Depth
Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.012 86.100
Continuity Error (%) 1.000

Flow Routing Continuity Volume Volume
Flow Routing Continuity hectare-m MLiters

External Inflow 0.000 0.000
External Outflow 0.011 0.110
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.000
Continuity Error (%) 0.000

Runoff Coefficient Computations Report

Subbasin Entrance

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.06	-	0.90
Composite Area & Weighted Runoff Coeff.	0.06		0.90

Subbasin Plot1-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot2-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.90
Composite Area & Weighted Runoff Coeff.	0.02		0.90

Subbasin Plot3-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.90
Composite Area & Weighted Runoff Coeff.	0.02		0.90

Kirpich Time of Concentration Computations Report

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

16-254 Fitzroy Park Development

Where:

Tc = Time of Concentration (min)
 L = Flow length (m)
 S = Slope (m/m)

 Subbasin Entrance

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

 Subbasin Plot1-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot2-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot3-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Entrance	86.10	14.35	77.49	2.13	0.900	0 00:05:00
Plot1-paved	86.10	14.35	77.49	1.25	0.900	0 00:05:00
Plot2-paved	86.10	14.35	77.49	0.89	0.900	0 00:05:00
Plot3-paved	86.10	14.35	77.49	0.82	0.900	0 00:05:00

 Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S Outfall	0.02	0.08	81.23	0 00:45	0	0	0:00:00
S1	0.01	0.02	82.82	0 00:16	0	0	0:00:00
S2	0.01	0.04	82.59	0 00:21	0	0	0:00:00
S3	0.01	0.05	82.46	0 00:29	0	0	0:00:00
S4	0.00	0.00	81.65	0 00:00	0	0	0:00:00
S5	0.01	0.04	81.56	0 00:57	0	0	0:00:00
Outfall(Combined)	0.00	0.00	81.14	0 00:00	0	0	0:00:00
Stor-S1	0.00	0.00	81.60	0 00:00	0	0	0:00:00

 Node Flow Summary

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Inflow days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding days hh:mm
S Outfall	JUNCTION	0.00	5.09	0 00:22	0.00	
S1	JUNCTION	0.82	0.82	0 00:05	0.00	
S2	JUNCTION	0.89	1.71	0 00:16	0.00	
S3	JUNCTION	1.25	2.95	0 00:21	0.00	
S4	JUNCTION	0.00	0.00	0 00:00	0.00	
S5	JUNCTION	2.13	5.09	0 00:25	0.00	
Outfall(Combined)	OUTFALL	0.00	5.09	0 00:53	0.00	
Stor-S1	STORAGE	0.00	0.00	0 00:00	0.00	

16-254 Fitzroy Park Development

 Storage Node Summary

Storage Node ID	Maximum Total Exfiltrated Volume m³	Maximum Ponded Volume	Time of Max Ponded Volume	Average Ponded Volume	Average Ponded Volume	Maximum Storage Node Outflow LPS	Maximum Exfiltration Rate cmm	Time of Max. Exfiltration Rate hh:mm:ss	1000
Stor-S1 0.000	0.000	0	0 00:00	0.000	0	0.00	0.00	0:00:00	

 Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Combined)	26.28	4.84	5.09
System	26.28	4.84	5.09

 Link Flow Summary

Link ID	Element Type	Time of Occurrence days hh:mm	Maximum Velocity m/sec	Length Factor	Peak Flow during Analysis LPS	Design Capacity LPS	Ratio of Maximum Flow /Design Flow	Ratio of Maximum Flow Depth	Total Surcharged Time minutes	Reported Condition
S1.000	CONDUIT	0 00:16	0.33	1.00	0.82	17.99	0.05	0.20	0	Calculated
S1.001	CONDUIT	0 00:21	0.40	1.00	1.71	11.92	0.14	0.29	0	Calculated
S1.002	CONDUIT	0 00:25	0.60	1.00	2.95	13.74	0.21	0.32	0	Calculated
S1.003	CONDUIT	0 00:22	0.99	1.00	5.09	29.97	0.17	0.40	0	Calculated
S2.000	CONDUIT	0 00:00	0.00	1.00	0.00	13.20	0.00	0.14	0	Calculated
Storage Link	CONDUIT	0 00:00	0.00	1.00	0.00	17.46	0.00	0.14	0	Calculated
Orifice-S1	ORIFICE	0 00:53			5.09			0.53		

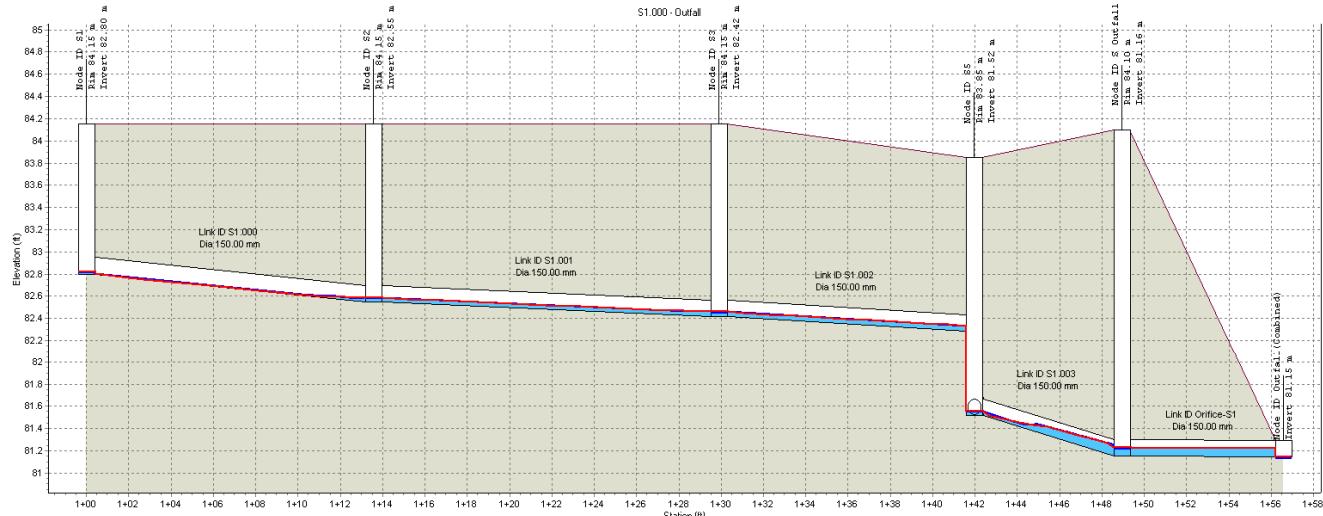
 Highest Flow Instability Indexes

All links are stable.

Analysis began on: Fri Nov 5 15:04:06 2021
 Analysis ended on: Fri Nov 5 15:04:07 2021
 Total elapsed time: 00:00:01

Profile Plot
S1.000 - Outfall

04/11/2021 01:00:00



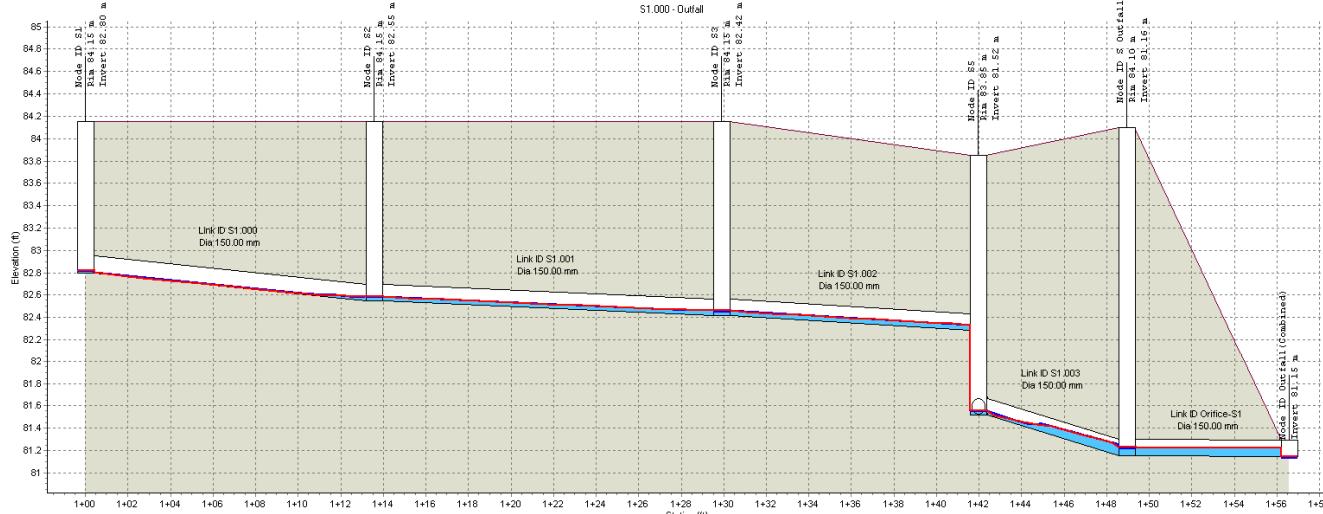
Node ID	S1	S2	S3	S5	S Outfall	Outfall[Combined]
Rim (m)	84.15	84.15	84.15	83.85	84.10	
Invert (m)	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m)	1.20	1.45	1.59	1.42	0.00	
Max HGL (m)	82.82	82.59	82.46	81.56	81.23	81.14

Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1
Length (m):	13.57	16.30	12.09	6.99	
Dia (mm):	150.00	150.00	150.00	150.00	150.00
Slope (m/m):	0.0186	0.0092	0.0108	0.0515	
Up Invert (m):	82.80	82.55	82.42	81.52	81.16
Down Invert (m):	82.55	82.42	82.28	81.16	81.15
Max Q (lps):	0.82	1.71	2.95	5.09	5.09
Max Vel (m/s):	0.33	0.40	0.60	0.99	0.00
Max Depth (m):	0.03	0.04	0.05	0.06	0.00

1:100yr +40% 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S1.000 - Outfall

04/11/2021 06:00:00



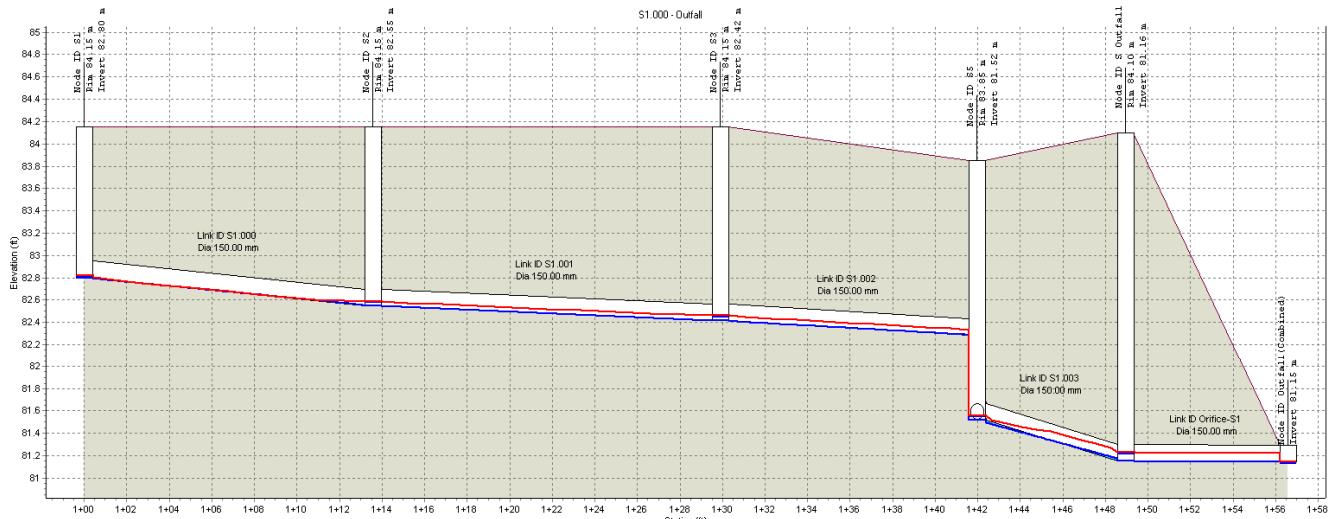
Node ID	S1	S2	S3	S5	S Outfall	Outfall[Combined]
Rim (m)	84.15	84.15	84.15	83.85	84.10	
Invert (m)	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m)	1.20	1.45	1.59	1.42	0.00	
Max HGL (m)	82.82	82.59	82.46	81.56	81.23	81.14

Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1
Length (m):	13.57	16.30	12.09	6.99	
Dia (mm):	150.00	150.00	150.00	150.00	150.00
Slope (m/m):	0.0186	0.0092	0.0108	0.0515	
Up Invert (m):	82.80	82.55	82.42	81.52	81.16
Down Invert (m):	82.55	82.42	82.28	81.16	81.15
Max Q (lps):	0.82	1.71	2.95	5.09	5.09
Max Vel (m/s):	0.33	0.40	0.60	0.99	0.00
Max Depth (m):	0.03	0.04	0.05	0.06	0.00

1:100yr +40% 6hr Storm Analysis - Elapsed Time 06:00:00

**Profile Plot
S1.000 - Outfall**

04/11/2021 12:00:00

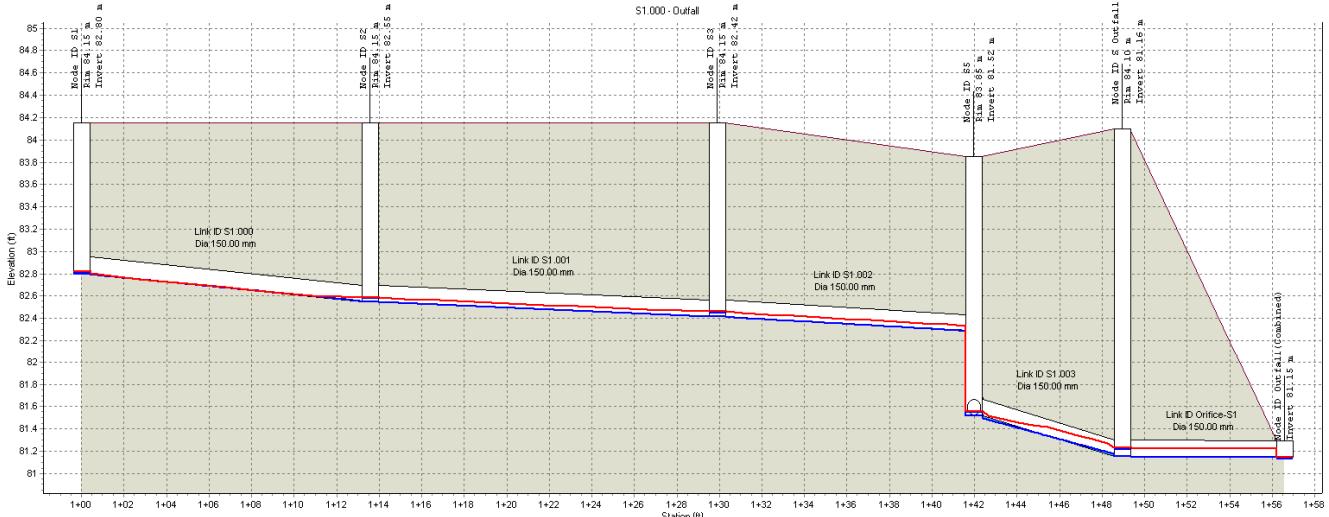


Node ID:	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.59	82.46	81.56	81.23	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Office-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0092	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.82	1.71	2.95	5.09	5.09	
Max Vel (m/s):	0.33	0.40	0.60	0.99	0.00	
Max Depth (m):	0.03	0.04	0.05	0.06	0.00	

1:100yr +40% 6hr Storm Analysis - Elapsed Time 12:00:00

**Profile Plot
S1.000 - Outfall**

05/11/2021 00:00:00

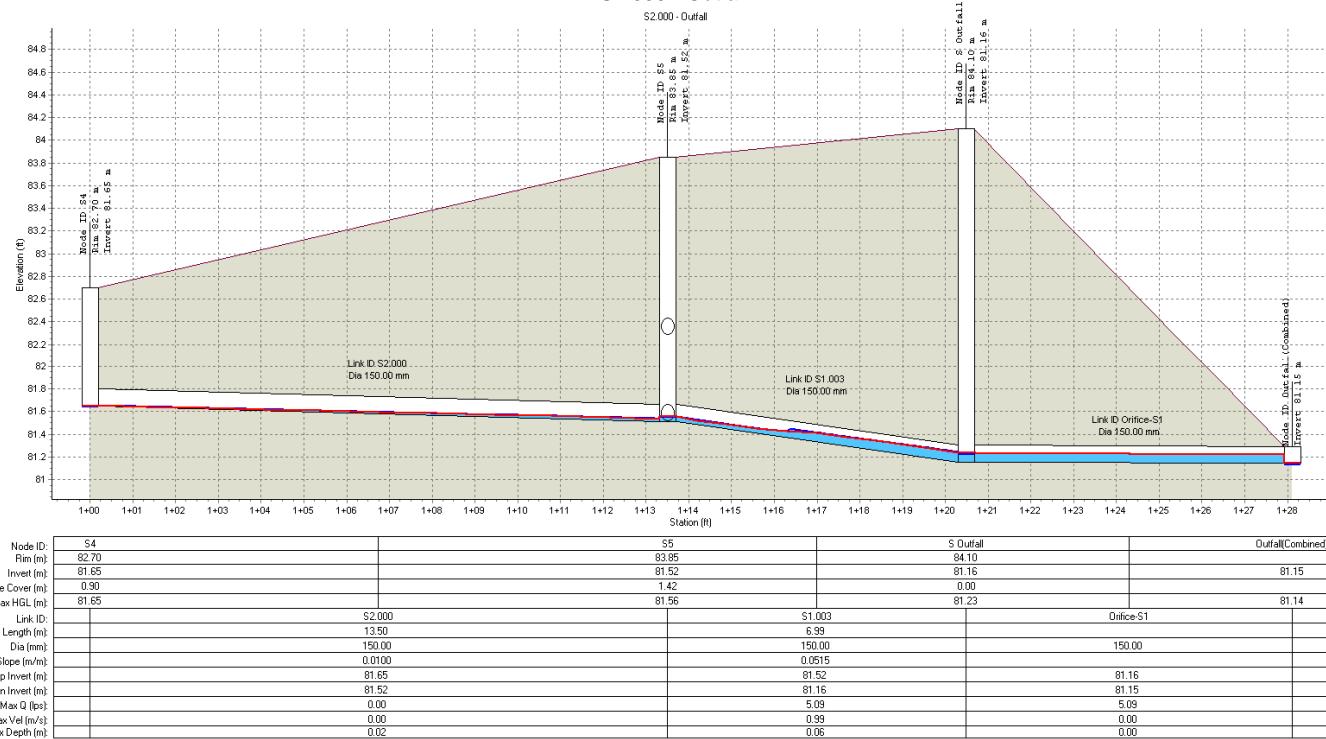


Node ID:	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.59	82.46	81.56	81.23	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Office-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0092	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.82	1.71	2.95	5.09	5.09	
Max Vel (m/s):	0.33	0.40	0.60	0.99	0.00	
Max Depth (m):	0.03	0.04	0.05	0.06	0.00	

1:100yr +40% 6hr Storm Analysis - Elapsed Time 24:00:00

Profile Plot
S2.000 - Outfall

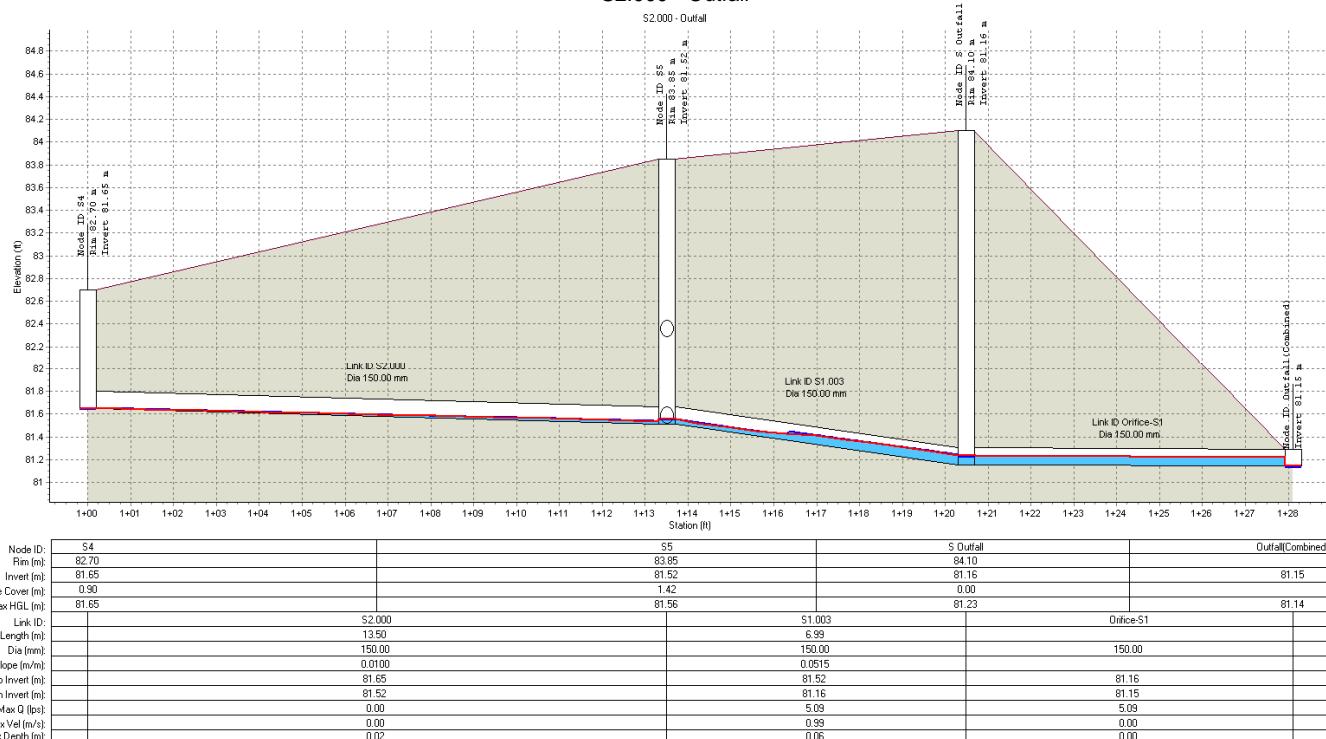
04/11/2021 01:00:00



1:100yr +40% 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S2.000 - Outfall

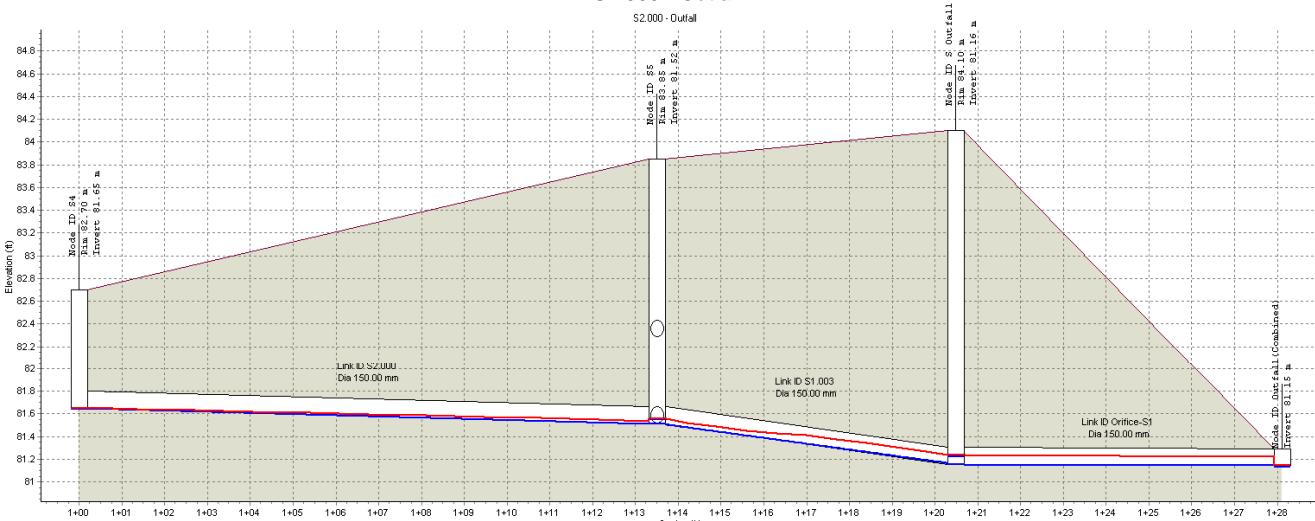
04/11/2021 06:00:00



1:100yr +40% 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 12:00:00

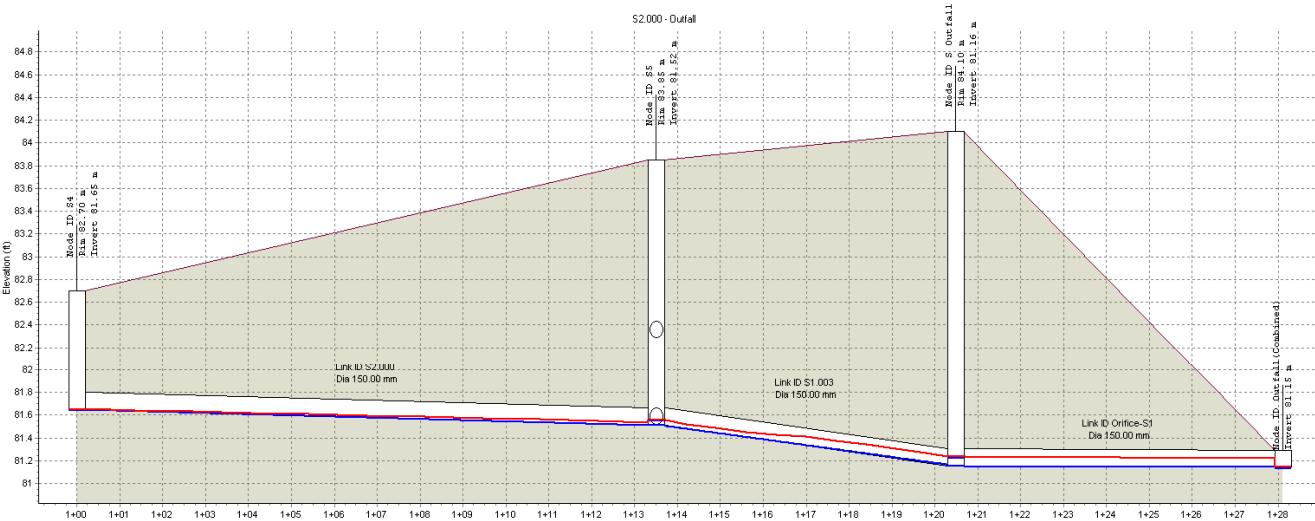


Node ID:	S4	Link ID:	S2.000	Node ID:	S5	Link ID:	S1.003	Node ID:	S Outfall	Link ID:	Orifice-S1	Outfall(Combined):
Rim (m):	82.70	Length (m):	13.50	Invert (m):	83.95	Dia (mm):	150.00	Rim (m):	84.10	Up Invert (m):	81.15	
Invert (m):	81.65	Dia (mm):	150.00	Min Pipe Cover (m):	0.90	Slope (m/m):	0.0100	Invert (m):	81.16	Dn Invert (m):	81.16	
Min Pipe Cover (m):	0.90	Up Invert (m):	81.65	Max HGL (m):	81.65	Dn Invert (m):	81.52	0.00	0.00	Max Q (lps):	5.09	
Max HGL (m):	81.65	Dn Invert (m):	81.52	Max Vel (m/s):	0.00	Max Q (lps):	5.09	0.00	0.00	Max Depth (m):	0.02	
						Max Vel (m/s):	0.99				0.06	
						Max Depth (m):	0.06					

1:100yr +40% 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S2.000 - Outfall

05/11/2021 00:00:00



Node ID:	S4	Link ID:	S2.000	Node ID:	S5	Link ID:	S1.003	Node ID:	S Outfall	Link ID:	Orifice-S1	Outfall(Combined):
Rim (m):	82.70	Length (m):	13.50	Invert (m):	83.95	Dia (mm):	150.00	Rim (m):	84.10	Up Invert (m):	81.15	
Invert (m):	81.65	Dia (mm):	150.00	Min Pipe Cover (m):	0.90	Slope (m/m):	0.0100	Invert (m):	81.16	Dn Invert (m):	81.16	
Min Pipe Cover (m):	0.90	Up Invert (m):	81.65	Max HGL (m):	81.65	Dn Invert (m):	81.52	0.00	0.00	Max Q (lps):	5.09	
Max HGL (m):	81.65	Dn Invert (m):	81.52	Max Vel (m/s):	0.00	Max Q (lps):	5.09	0.00	0.00	Max Depth (m):	0.02	
						Max Vel (m/s):	0.99				0.06	
						Max Depth (m):	0.06					

1:100yr +40% 6hr Storm Analysis - Elapsed Time 24:00:00

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

***** Project Description *****

File Name 16-254-Fitzroy Park S1 S2 100yr.SPF
Description 16-254 Fitzroy Park Development
100yr Return
Storm Network S1 & S2

***** Analysis Options *****

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 100 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date NOV-04-2021 00:00:00
Ending Date NOV-05-2021 00:00:00
Report Time Step 00:00:10

Runoff Quantity Continuity Volume Depth
Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.009 61.500
Continuity Error (%) 1.000

Flow Routing Continuity Volume Volume
Flow Routing Continuity hectare-m Mliters

External Inflow 0.000 0.000
External Outflow 0.008 0.078
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.000
Continuity Error (%) 0.000

***** Runoff Coefficient Computations Report *****

----- Subbasin Entrance -----

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

----- Subbasin Plot1-paved -----

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

----- Subbasin Plot2-paved -----

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

----- Subbasin Plot3-paved -----

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.90

16-254 Fitzroy Park Development

Composite Area & Weighted Runoff Coeff. 0.02 0.90

Kirpich Time of Concentration Computations Report

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

Where:

Tc = Time of Concentration (min)
L = Flow length (m)
S = Slope (m/m)

Subbasin Entrance

Flow length (m): 10.00
Slope (%): 0.50
Computed TOC (minutes): 0.88

Subbasin Plot1-paved

Flow length (m): 5.00
Slope (%): 0.50
Computed TOC (minutes): 0.52

Subbasin Plot2-paved

Flow length (m): 5.00
Slope (%): 0.50
Computed TOC (minutes): 0.52

Subbasin Plot3-paved

Flow length (m): 5.00
Slope (%): 1.50
Computed TOC (minutes): 0.34

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days hh:mm:ss
Entrance	61.50	10.25	55.35	1.52	0.900	0 00:05:00
Plot1-paved	61.50	10.25	55.35	0.89	0.900	0 00:05:00
Plot2-paved	61.50	10.25	55.35	0.64	0.900	0 00:05:00
Plot3-paved	61.50	10.25	55.35	0.58	0.900	0 00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Flooded Time minutes	Retention Time hh:mm:ss
S Outfall	0.02	0.06	81.22	0 01:47	0	0	0:00:00
S1	0.00	0.02	82.82	0 00:16	0	0	0:00:00
S2	0.01	0.03	82.58	0 01:53	0	0	0:00:00
S3	0.01	0.04	82.46	0 00:24	0	0	0:00:00
S4	0.00	0.00	81.65	0 00:00	0	0	0:00:00
S5	0.01	0.04	81.55	0 02:44	0	0	0:00:00
Outfall(Combined)	0.00	0.00	81.14	0 00:00	0	0	0:00:00
Stor-S1	0.00	0.00	81.60	0 00:00	0	0	0:00:00

16-254 Fitzroy Park Development

Node Flow Summary

Node ID	Element Type	Maximum Lateral Inflow	Peak Inflow	Time of Peak Occurrence	Maximum Flooding Overflow	Time of Peak Flooding Occurrence
		LPS	LPS	days hh:mm	LPS	days hh:mm
S Outfall	JUNCTION	0.00	3.63	0 00:25	0.00	
S1	JUNCTION	0.58	0.58	0 00:05	0.00	
S2	JUNCTION	0.64	1.22	0 00:16	0.00	
S3	JUNCTION	0.89	2.11	0 01:45	0.00	
S4	JUNCTION	0.00	0.00	0 00:00	0.00	
S5	JUNCTION	1.52	3.63	0 00:24	0.00	
Outfall(Combined)	OUTFALL	0.00	3.63	0 00:49	0.00	
Stor-S1	STORAGE	0.00	0.00	0 00:00	0.00	

Storage Node Summary

Storage Node ID	Maximum Ponded Volume	Maximum Ponded Volume (%)	Time of Max Volume	Average Ponded Volume	Average Ponded Volume (1000 m³)	Storage Node Outflow	Maximum Exfiltration Rate	Maximum Exfiltration Rate (LPS)	Time of Max. Exfiltration	Total Exfiltrated Volume
Stor-S1	0.000	0	0 00:00	0.000	0	0.00	0.00	0.00	0:00:00	0.000

Outfall Loading Summary

Outfall Node ID	Flow Frequency	Average Flow (%)	Peak Inflow
Outfall(Combined)	26.24	3.46	3.63
System	26.24	3.46	3.63

Link Flow Summary

Link ID	Element Type	Time of Peak Flow Occurrence	Maximum Velocity Attained	Length Factor	Peak Flow during Analysis	Design Capacity	Ratio of Maximum Flow /Design	Ratio of Maximum Flow	Total Time	Reported Condition
		days hh:mm	m/sec		LPS	LPS		Depth	Surcharged minutes	
S1.000	CONDUIT	0 00:16	0.30	1.00	0.58	17.99	0.03	0.17	0	Calculated
S1.001	CONDUIT	0 01:45	0.37	1.00	1.22	11.92	0.10	0.25	0	Calculated
S1.002	CONDUIT	0 00:24	0.55	1.00	2.11	13.74	0.15	0.27	0	Calculated
S1.003	CONDUIT	0 00:25	0.93	1.00	3.63	29.97	0.12	0.33	0	Calculated
S2.000	CONDUIT	0 00:00	0.00	1.00	0.00	13.20	0.00	0.12	0	Calculated
Storage Link	CONDUIT	0 00:00	0.00	1.00	0.00	17.46	0.00	0.12	0	Calculated
Orifice-S1	ORIFICE	0 00:49			3.63			0.42		

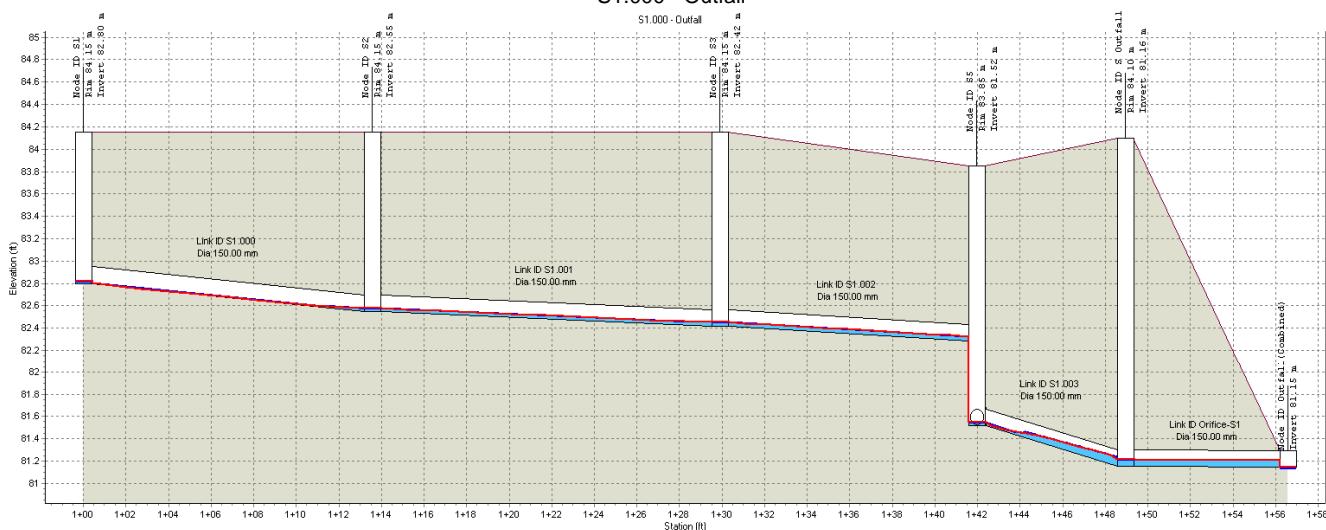
Highest Flow Instability Indexes

All links are stable.

Analysis began on: Fri Nov 5 15:30:59 2021
 Analysis ended on: Fri Nov 5 15:31:00 2021
 Total elapsed time: 00:00:01

Profile Plot S1.000 - Outfall

04/11/2021 01:00:00

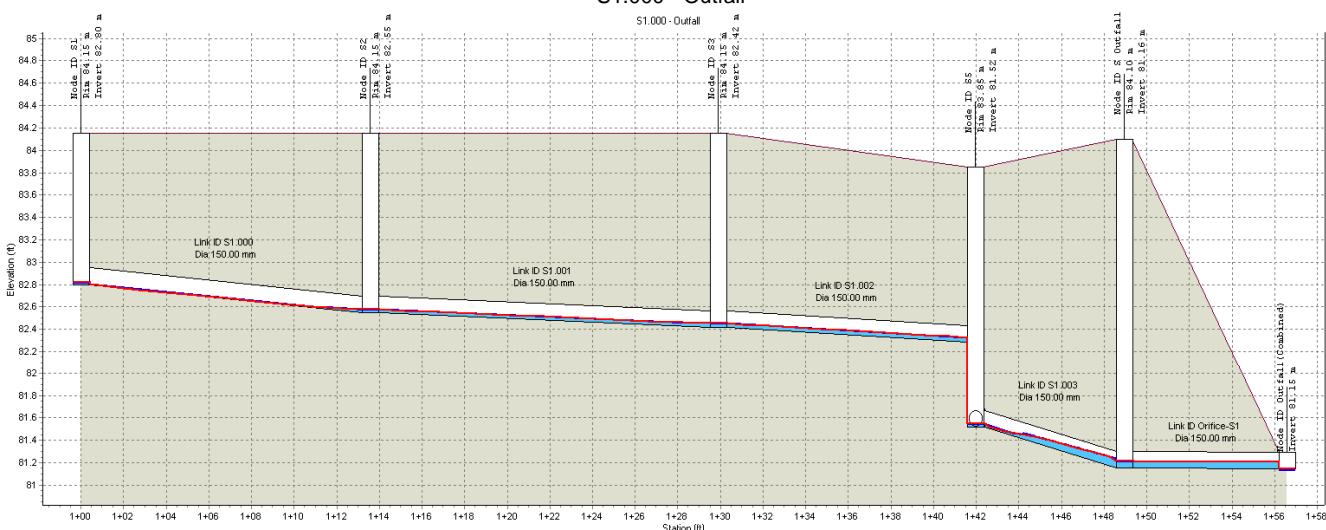


Node ID:	S1	S2	S3	S4	S5	Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	82.42	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15	
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00		
Max HGL (m):	82.82	82.58	82.46	81.55	81.22	81.14	
Link ID:	S1.000	S1.001	S1.002	S1.003	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99			
Dia (mm):	150.00	150.00	150.00	150.00		150.00	
Slope (m/m):	0.0186	0.0082	0.0108	0.0515			
Up Invert (m):	82.80	82.55	82.42	81.52		81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16		81.15	
Max Q (lps):	0.58	1.22	2.11	3.63		3.63	
Max Vel (m/s):	0.30	0.37	0.55	0.93		0.00	
Max Depth (m):	0.03	0.04	0.04	0.05		0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot S1.000 - Outfall

04/11/2021 06:00:00

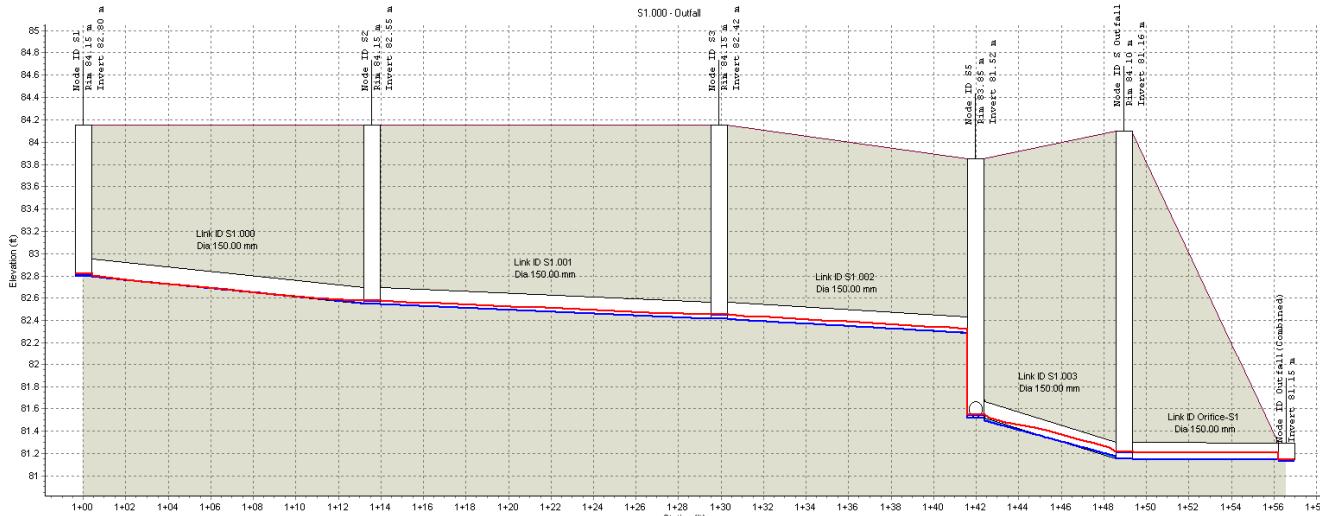


Node ID:	S1	S2	S3	S4	S5	Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	82.42	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15	
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00		
Max HGL (m):	82.82	82.58	82.46	81.55	81.22	81.14	
Link ID:	S1.000	S1.001	S1.002	S1.003	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99			
Dia (mm):	150.00	150.00	150.00	150.00		150.00	
Slope (m/m):	0.0186	0.0082	0.0108	0.0515			
Up Invert (m):	82.80	82.55	82.42	81.52		81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16		81.15	
Max Q (lps):	0.58	1.22	2.11	3.63		3.63	
Max Vel (m/s):	0.30	0.37	0.55	0.93		0.00	
Max Depth (m):	0.03	0.04	0.04	0.05		0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S1.000 - Outfall

04/11/2021 12:00:00

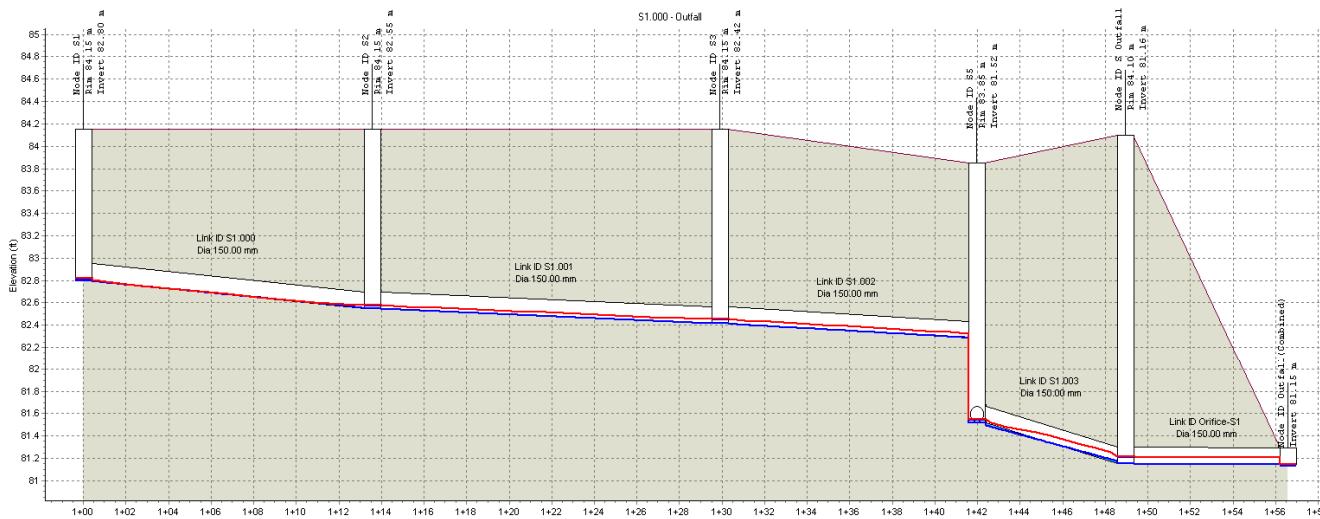


Node ID:	S1	S2	S3	S5	S Outfall	Outfall[Combined]
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.58	82.46	81.55	81.22	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0082	0.0109	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.58	1.22	2.11	3.63	3.63	
Max Vel (m/s):	0.30	0.37	0.55	0.93	0.00	
Max Depth (m):	0.03	0.04	0.04	0.05	0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S1.000 - Outfall

05/11/2021 00:00:00

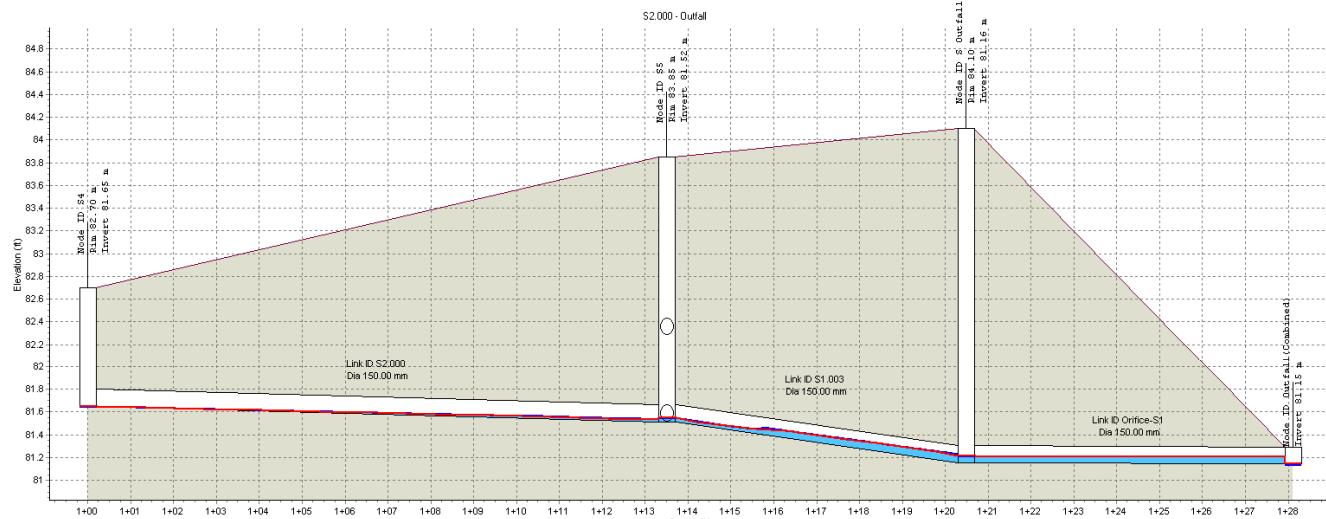


Node ID:	S1	S2	S3	S5	S Outfall	Outfall[Combined]
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.58	82.46	81.55	81.22	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0082	0.0109	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.58	1.22	2.11	3.63	3.63	
Max Vel (m/s):	0.30	0.37	0.55	0.93	0.00	
Max Depth (m):	0.03	0.04	0.04	0.05	0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 24:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 01:00:00

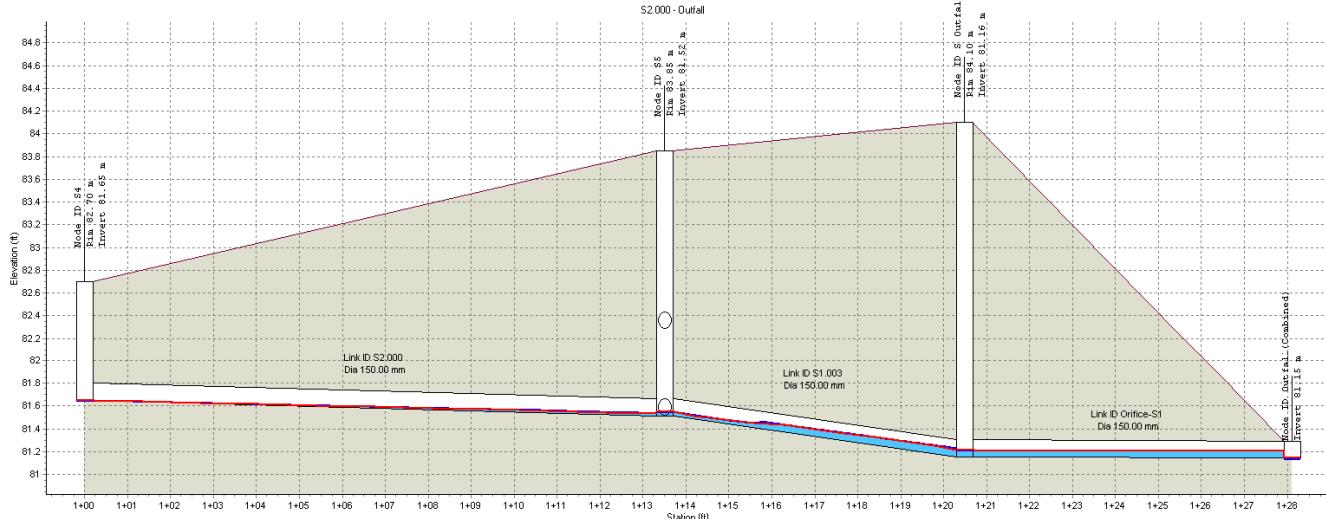


S4		S5		S Outfall		Outfall[Combined]
Node ID:	S4	Node ID:	S5	Node ID:	S Outfall	
Rim (m):	82.70	Invert (m):	81.65	Rim (m):	83.95	
Invert (m):	81.65		81.52	Invert (m):	84.10	
0.90			1.42	0.00	81.15	
Min Pipe Cover (m):	0.90		81.55	81.22		
Max HGL (m):	81.65					81.14
Link ID:	S2.000		Link ID:	S1.003		Orifice-S1
Length (m):	13.50		Dis (mm):	150.00		
Dis (mm):	150.00		Slope (m/m):	0.0100		
Slope (m/m):	0.0100		Up Invert (m):	81.65		150.00
Up Invert (m):	81.65		Dn Invert (m):	81.52		81.16
Dn Invert (m):	81.52		Max Q (lps):	0.00		81.15
Max Q (lps):	0.00		Max Vel (m/s):	3.63		3.63
Max Vel (m/s):	0.00		Max Depth (m):	0.93		0.00
Max Depth (m):	0.02			0.05		

1:100yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 06:00:00

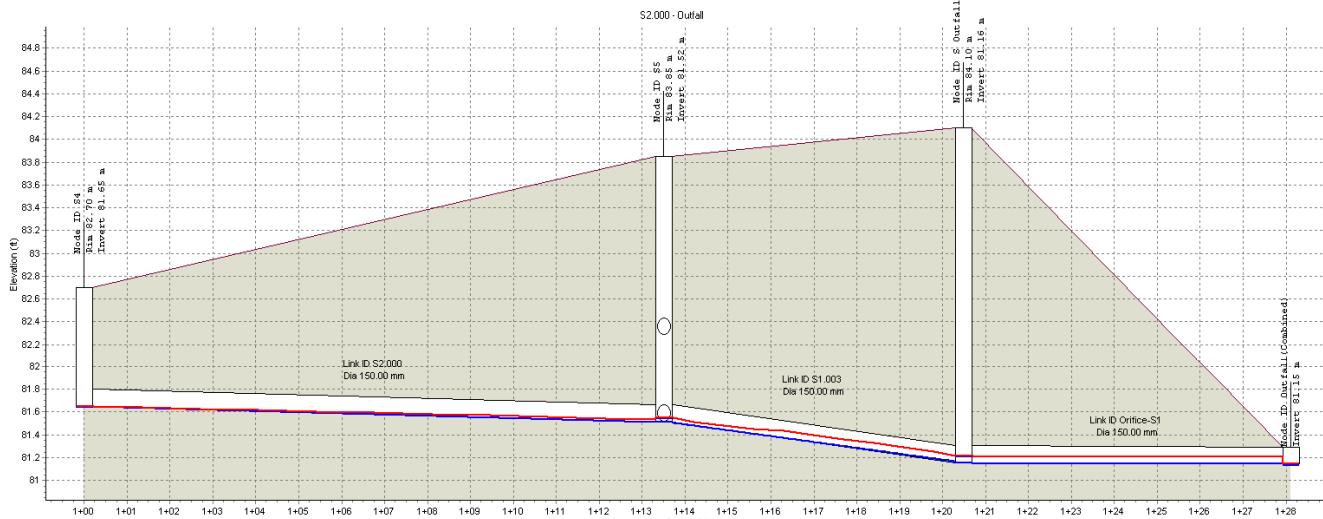


S4		S5		S Outfall		Outfall[Combined]
Node ID:	S4	Node ID:	S5	Node ID:	S Outfall	
Rim (m):	82.70	Invert (m):	81.65	Rim (m):	83.95	
Invert (m):	81.65		81.52	Invert (m):	84.10	
0.90			1.42	0.00	81.15	
Min Pipe Cover (m):	0.90		81.55	81.22		
Max HGL (m):	81.65					81.14
Link ID:	S2.000		Link ID:	S1.003		Orifice-S1
Length (m):	13.50		Dis (mm):	150.00		
Dis (mm):	150.00		Slope (m/m):	0.0100		150.00
Slope (m/m):	0.0100		Up Invert (m):	81.65		81.16
Up Invert (m):	81.65		Dn Invert (m):	81.52		81.15
Dn Invert (m):	81.52		Max Q (lps):	0.00		3.63
Max Q (lps):	0.00		Max Vel (m/s):	3.63		0.00
Max Vel (m/s):	0.00		Max Depth (m):	0.93		
Max Depth (m):	0.02			0.05		

1:100yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 12:00:00

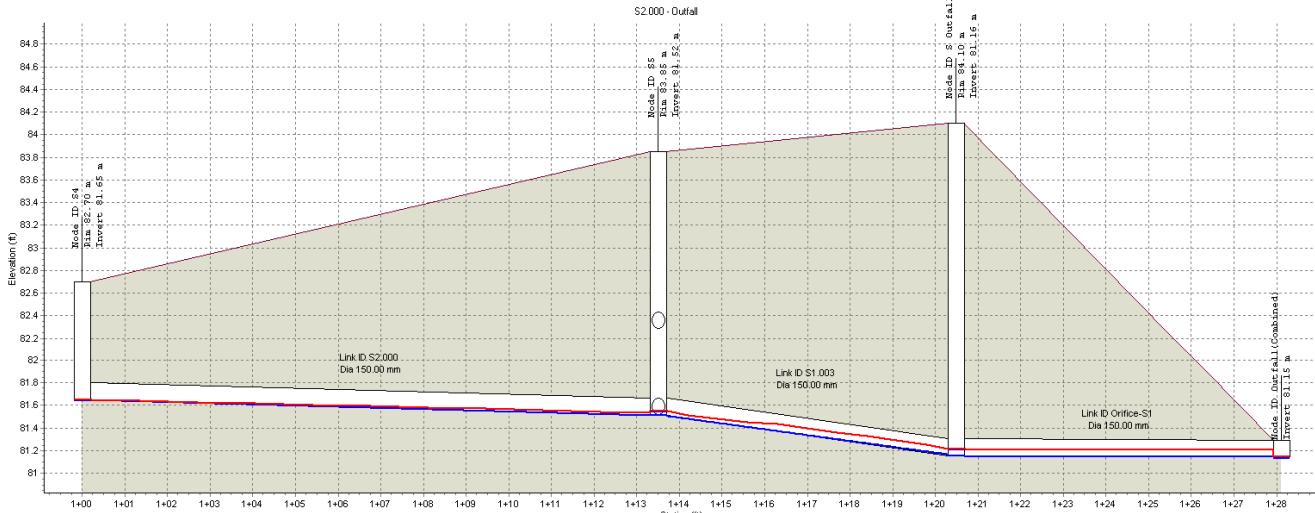


Node ID:	S4	Link ID:	S2.000	Node ID:	S5	Link ID:	S1.003	Node ID:	Outfall	Link ID:	Orifice-S1	Outfall(Combined)
Rim (m):	82.70				83.85			84.10			81.15	
Invert (m):	81.65				81.52			81.16				
Min Pipe Cover (m):	0.90				1.42			0.00				
Max HGL (m):	81.65				81.55			81.22			81.14	
Link ID:												
Length (m):		13.50				6.99						
Dia (mm):		150.00				150.00					150.00	
Slope (m/m):		0.0100				0.0515						
Up Invert (m):		81.65				81.52					81.16	
Dn Invert (m):		81.52				81.16					81.15	
Max Q (lps):		0.00				3.63					3.63	
Max Vel (m/s):		0.00				0.93					0.00	
Max Depth (m):		0.02				0.05					0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S2.000 - Outfall

05/11/2021 00:00:00



Node ID:	S4	Link ID:	S2.000	Node ID:	S5	Link ID:	S1.003	Node ID:	Outfall	Link ID:	Orifice-S1	Outfall(Combined)
Rim (m):	82.70				83.85			84.10			81.15	
Invert (m):	81.65				81.52			81.16				
Min Pipe Cover (m):	0.90				1.42			0.00				
Max HGL (m):	81.65				81.55			81.22			81.14	
Link ID:												
Length (m):		13.50				6.99					150.00	
Dia (mm):		150.00				150.00						
Slope (m/m):		0.0100				0.0515						
Up Invert (m):		81.65				81.52					81.16	
Dn Invert (m):		81.52				81.16					81.15	
Max Q (lps):		0.00				3.63					3.63	
Max Vel (m/s):		0.00				0.93					0.00	
Max Depth (m):		0.02				0.05					0.00	

1:100yr 6hr Storm Analysis - Elapsed Time 24:00:00

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy Park S1 S2 30yr.SPF
Description 16-254 Fitzroy Park Development
30yr Return
Storm Network S1 & S2

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 30 years
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date NOV-04-2021 00:00:00
Ending Date NOV-05-2021 00:00:00
Report Time Step 00:00:10

Runoff Quantity Continuity Volume Depth
Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.006 44.580
Continuity Error (%) 1.000

Flow Routing Continuity Volume Volume
Flow Routing Continuity hectare-m Mliters

External Inflow 0.000 0.000
External Outflow 0.006 0.057
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.000
Continuity Error (%) 0.000

Runoff Coefficient Computations Report

Subbasin Entrance

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot1-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot2-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.03	-	0.90
Composite Area & Weighted Runoff Coeff.	0.03		0.90

Subbasin Plot3-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Kirpich Time of Concentration Computations Report

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

16-254 Fitzroy Park Development

Where:

Tc = Time of Concentration (min)
 L = Flow length (m)
 S = Slope (m/m)

 Subbasin Entrance

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

 Subbasin Plot1-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot2-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot3-paved

Flow length (m):	5.00
Slope (%):	1.50
Computed TOC (minutes):	0.34

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days	Time of Concentration hh:mm:ss
Entrance	44.58	7.43	40.12	1.11	0.900	0	00:05:00
Plot1-paved	44.58	7.43	40.12	0.64	0.900	0	00:05:00
Plot2-paved	44.58	7.43	40.12	0.46	0.900	0	00:05:00
Plot3-paved	44.58	7.43	40.12	0.42	0.900	0	00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Time Flooded minutes	Retention Time hh:mm:ss
S Outfall	0.01	0.05	81.21	0 00:45	0	0	0:00:00
S1	0.00	0.02	82.82	0 00:19	0	0	0:00:00
S2	0.01	0.03	82.58	0 00:25	0	0	0:00:00
S3	0.01	0.03	82.45	0 00:36	0	0	0:00:00
S4	0.00	0.00	81.65	0 00:00	0	0	0:00:00
S5	0.01	0.03	81.55	0 00:54	0	0	0:00:00
Outfall(Combined)	0.00	0.00	81.14	0 00:00	0	0	0:00:00
Stor-S1	0.00	0.00	81.60	0 00:00	0	0	0:00:00

Node Flow Summary

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Inflow days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding days hh:mm
S Outfall	JUNCTION	0.00	2.63	0 00:26	0.00	
S1	JUNCTION	0.42	0.42	0 00:05	0.00	
S2	JUNCTION	0.46	0.88	0 00:19	0.00	
S3	JUNCTION	0.64	1.53	0 00:25	0.00	
S4	JUNCTION	0.00	0.00	0 00:00	0.00	
S5	JUNCTION	1.11	2.63	0 00:37	0.00	
Outfall(Combined)	OUTFALL	0.00	2.63	0 00:45	0.00	
Stor-S1	STORAGE	0.00	0.00	0 00:00	0.00	

16-254 Fitzroy Park Development

Storage Node Summary

Storage Node ID	Maximum Ponded Volume	Maximum Ponded Volume (%)	Time of Max Ponded Volume	Average Ponded Volume	Average Ponded Volume (%)	Maximum Storage Node Outflow LPS	Maximum Exfiltration Rate cmm	Time of Max. Exfiltration hh:mm:ss	Total Exfiltrated Volume 1000 m³
Stor-S1	0.000	0	0 00:00	0.000	0	0.00	0.00	0:00:00	0.000

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Combined)	26.20	2.51	2.63
System	26.20	2.51	2.63

Link Flow Summary

Link ID	Element Type	Time of Peak Flow Occurrence	Maximum Velocity Attained	Length Factor	Peak Flow during Analysis	Design Capacity LPS	Ratio of Maximum Flow /Design LPS	Ratio of Maximum Flow Depth	Total Time Surcharged minutes	Reported Condition
S1.000	CONDUIT	0 00:19	0.28	1.00	0.42	17.99	0.02	0.15	0	Calculated
S1.001	CONDUIT	0 00:25	0.33	1.00	0.88	11.92	0.07	0.21	0	Calculated
S1.002	CONDUIT	0 00:37	0.50	1.00	1.53	13.74	0.11	0.23	0	Calculated
S1.003	CONDUIT	0 00:26	0.88	1.00	2.63	29.97	0.09	0.27	0	Calculated
S2.000	CONDUIT	0 00:00	0.00	1.00	0.00	13.20	0.00	0.10	0	Calculated
Storage Link	CONDUIT	0 00:00	0.00	1.00	0.00	17.46	0.00	0.10	0	Calculated
Orifice-S1	ORIFICE	0 00:45			2.63			0.34		

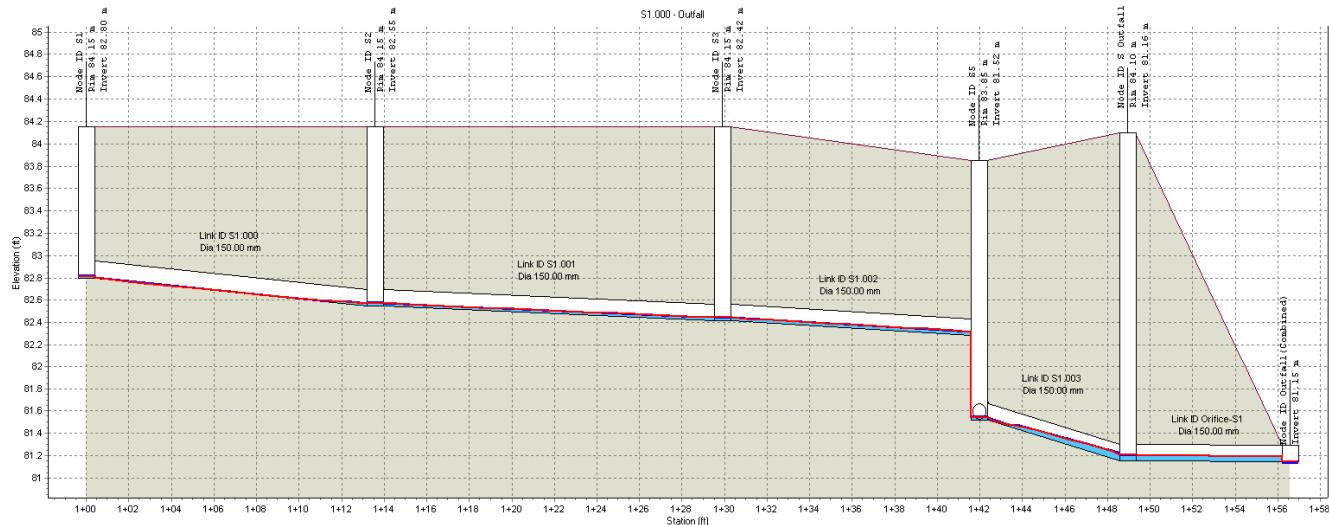
Highest Flow Instability Indexes

All links are stable.

Analysis began on: Fri Nov 5 15:55:49 2021
 Analysis ended on: Fri Nov 5 15:55:50 2021
 Total elapsed time: 00:00:01

Profile Plot
S1.000 - Outfall

04/11/2021 01:00:00

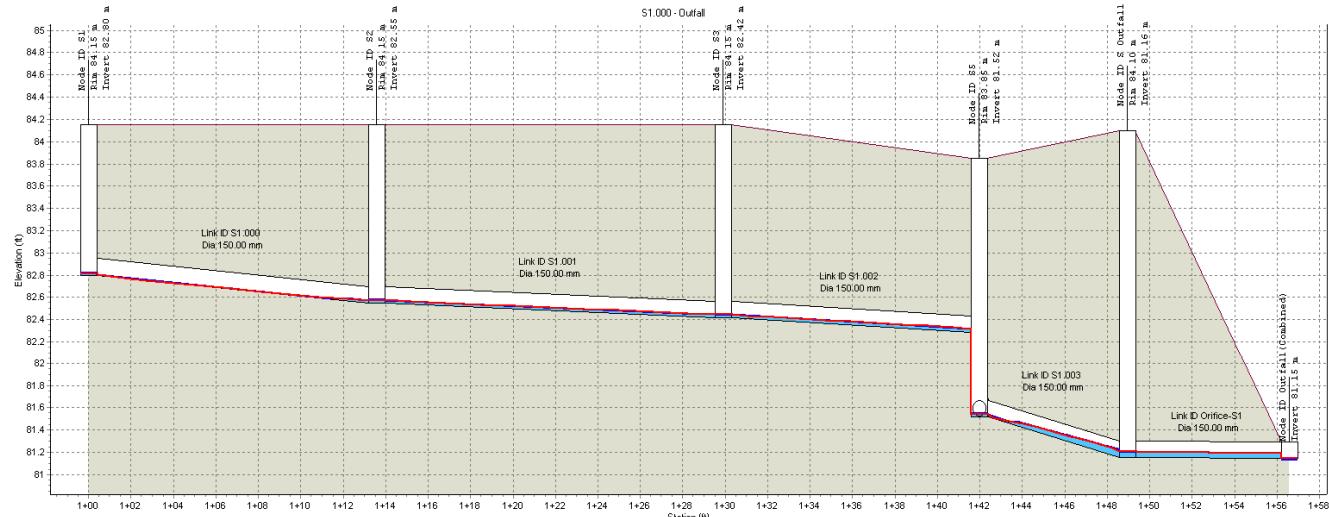


Node ID:	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.58	82.45	81.55	81.21	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00	150.00	150.00
Slope (m/m):	0.0186	0.0082	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Down Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.42	0.88	1.53	2.63	2.63	
Max Vel (m/s):	0.28	0.33	0.50	0.88	0.00	
Max Depth (m):	0.02	0.03	0.03	0.04	0.04	0.00

1:30yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S1.000 - Outfall

04/11/2021 06:00:00

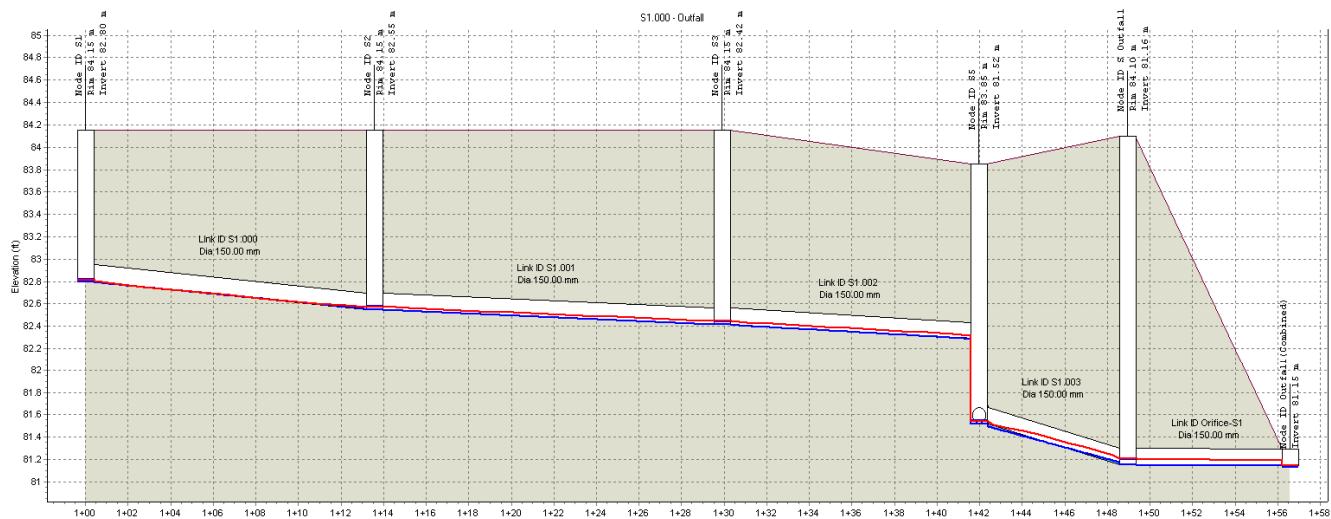


Node ID:	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.82	82.58	82.45	81.55	81.21	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00	150.00	150.00
Slope (m/m):	0.0186	0.0082	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Down Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.42	0.88	1.53	2.63	2.63	
Max Vel (m/s):	0.28	0.33	0.50	0.88	0.00	
Max Depth (m):	0.02	0.03	0.03	0.04	0.04	0.00

1:30yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S1.000 - Outfall

04/11/2021 12:00:00

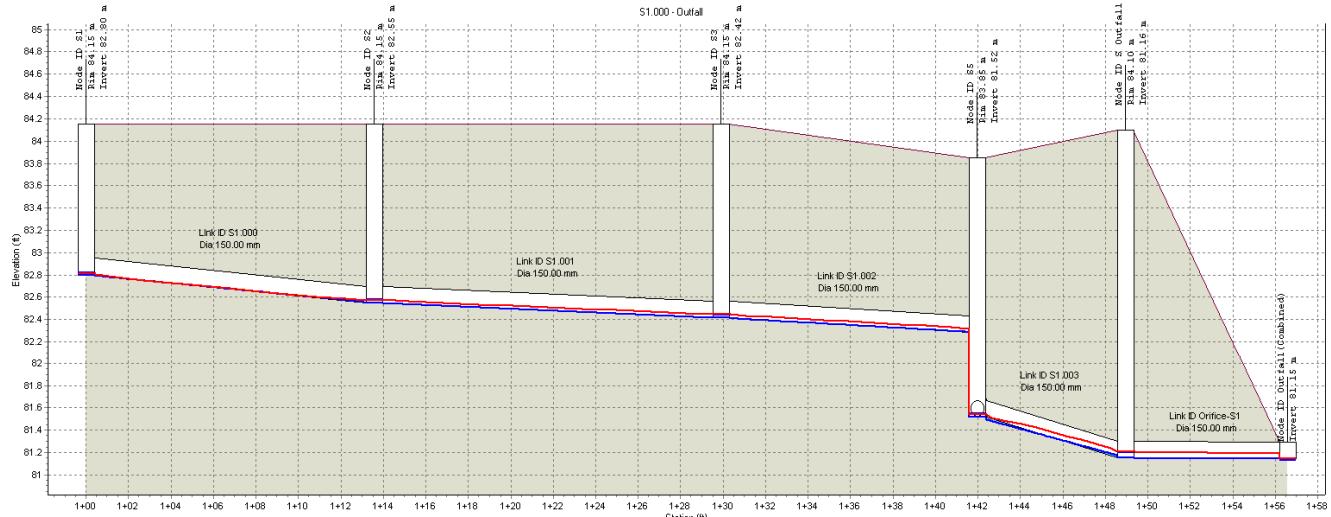


Node ID	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m)	84.15	84.15	84.15	83.85	84.10	
Invert (m)	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m)	1.20	1.45	1.59	1.42	0.00	
Max HGL (m)	82.82	82.58	82.45	81.55	81.21	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m)	13.57	16.30	12.09	6.99		
Dia (mm)	150.00	150.00	150.00	150.00		150.00
Slope (m/m)	0.0186	0.0082	0.0108	0.0515		
Up Invert (m)	82.80	82.55	82.42	81.52	81.16	
DN Invert (m)	82.55	82.42	82.28	81.16	81.15	
Max Q (lps)	0.42	0.88	1.53	2.63	2.63	
Max Vel (m/s)	0.28	0.33	0.50	0.88	0.00	
Max Depth (m)	0.02	0.03	0.03	0.04	0.00	

1:30yr 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S1.000 - Outfall

05/11/2021 00:00:00

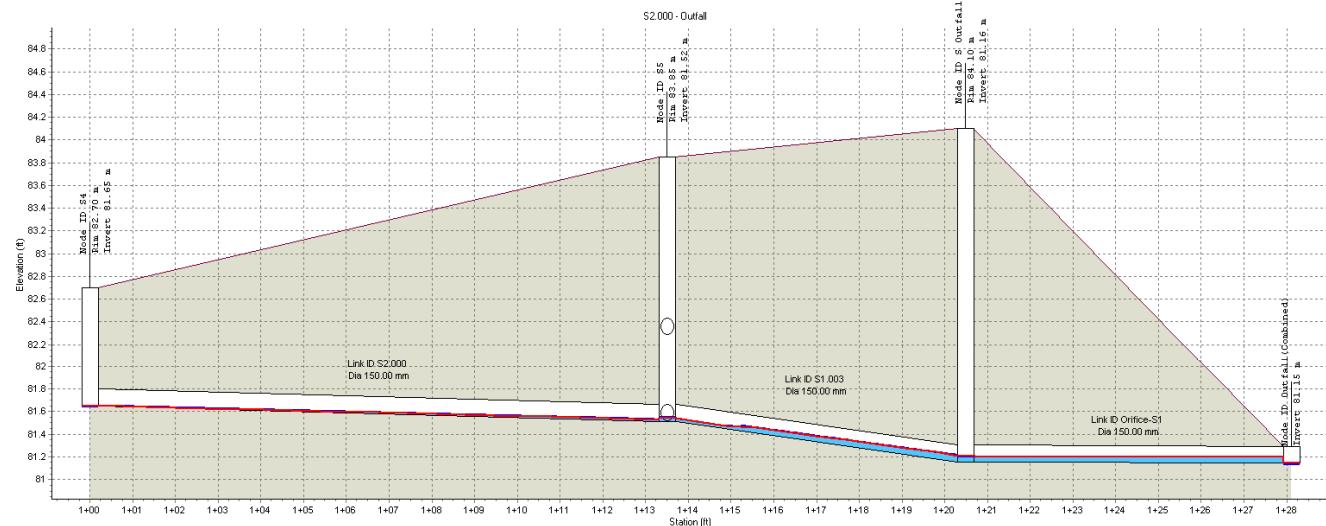


Node ID	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m)	84.15	84.15	84.15	83.85	84.10	
Invert (m)	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m)	1.20	1.45	1.59	1.42	0.00	
Max HGL (m)	82.82	82.58	82.45	81.55	81.21	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m)	13.57	16.30	12.09	6.99		
Dia (mm)	150.00	150.00	150.00	150.00		150.00
Slope (m/m)	0.0186	0.0082	0.0108	0.0515		
Up Invert (m)	82.80	82.55	82.42	81.52	81.16	
DN Invert (m)	82.55	82.42	82.28	81.16	81.15	
Max Q (lps)	0.42	0.88	1.53	2.63	2.63	
Max Vel (m/s)	0.28	0.33	0.50	0.88	0.00	
Max Depth (m)	0.02	0.03	0.03	0.04	0.00	

1:30yr 6hr Storm Analysis - Elapsed Time 24:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 01:00:00

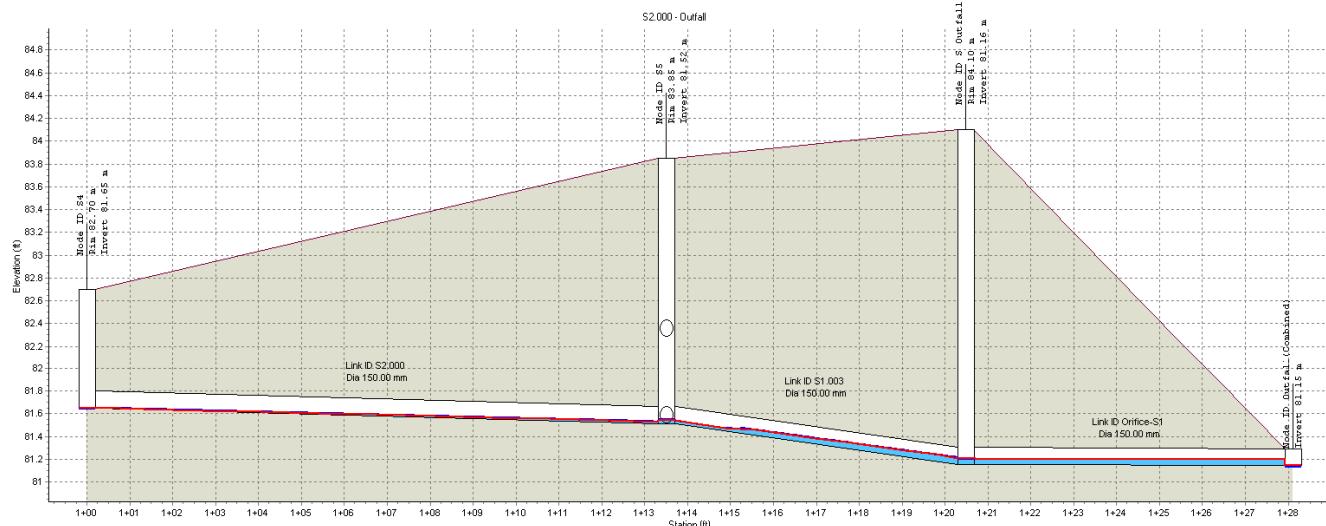


Node ID:	S4	Node ID:	S5	Node ID:	S Outfall	Outfall(Combined)
Rim (m):	82.70	Elev (m):	83.95	Rim (m):	84.10	
Invert (m):	81.65	Invert (m):	81.52	Invert (m):	81.16	81.15
Min Pipe Cover (m):	0.90			1.42	0.00	
Max HGL (m):	81.65			81.55	81.21	81.14
Link ID:	S2.000			S1.003		Office-S1
Length (m):	13.50			6.99		
Dia (mm):	150.00			150.00		150.00
Slope (m/m):	0.0100			0.0515		
Up Invert (m):	81.65			81.52		81.16
Dn Invert (m):	81.52			81.16		81.15
Max Q (lps):	0.00			2.63		2.63
Max Vel (m/s):	0.00			0.88		0.00
Max Depth (m):	0.02			0.04		0.00

1:30yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 06:00:00

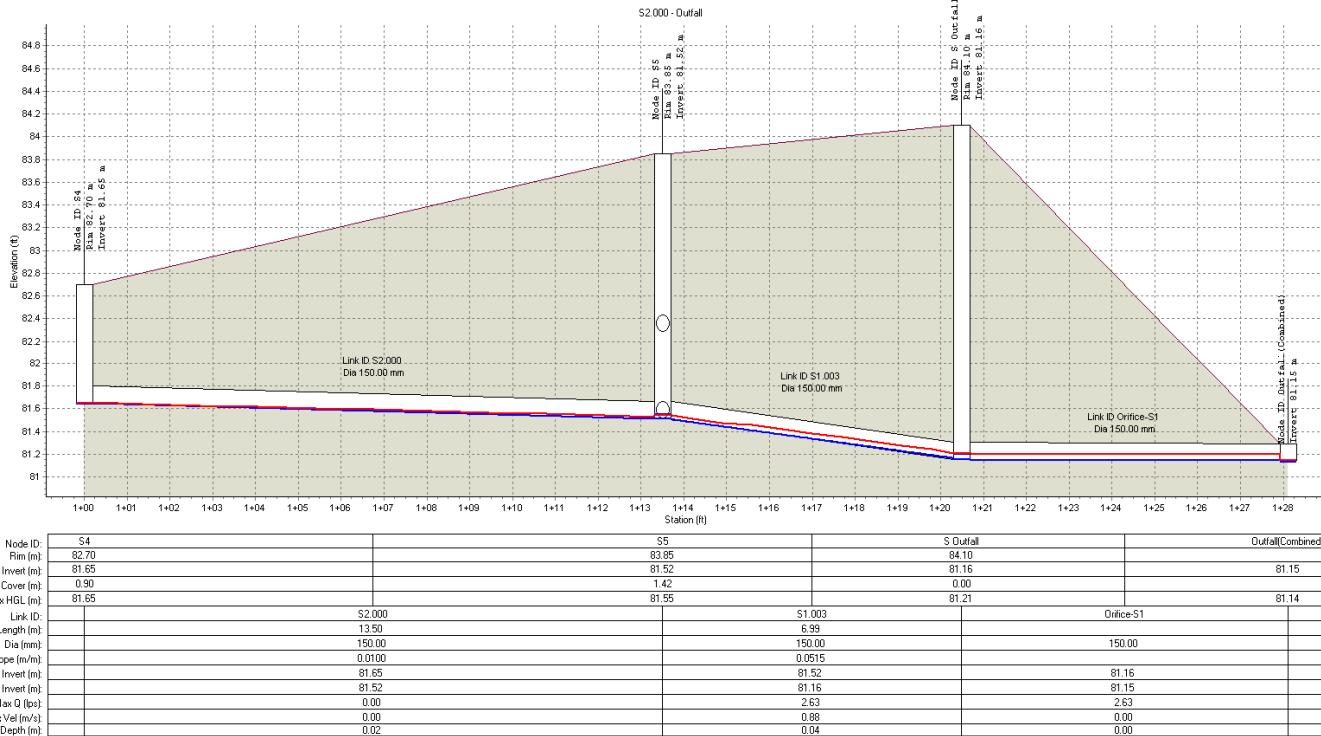


Node ID:	S4	Node ID:	S5	Node ID:	S Outfall	Outfall(Combined)
Rim (m):	82.70	Elev (m):	83.95	Rim (m):	84.10	
Invert (m):	81.65	Invert (m):	81.52	Invert (m):	81.16	81.15
Min Pipe Cover (m):	0.90			1.42	0.00	
Max HGL (m):	81.65			81.55	81.21	81.14
Link ID:	S2.000			S1.003		Office-S1
Length (m):	13.50			6.99		
Dia (mm):	150.00			150.00		150.00
Slope (m/m):	0.0100			0.0515		
Up Invert (m):	81.65			81.52		81.16
Dn Invert (m):	81.52			81.16		81.15
Max Q (lps):	0.00			2.63		2.63
Max Vel (m/s):	0.00			0.88		0.00
Max Depth (m):	0.02			0.04		0.00

1:30yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S2.000 - Outfall

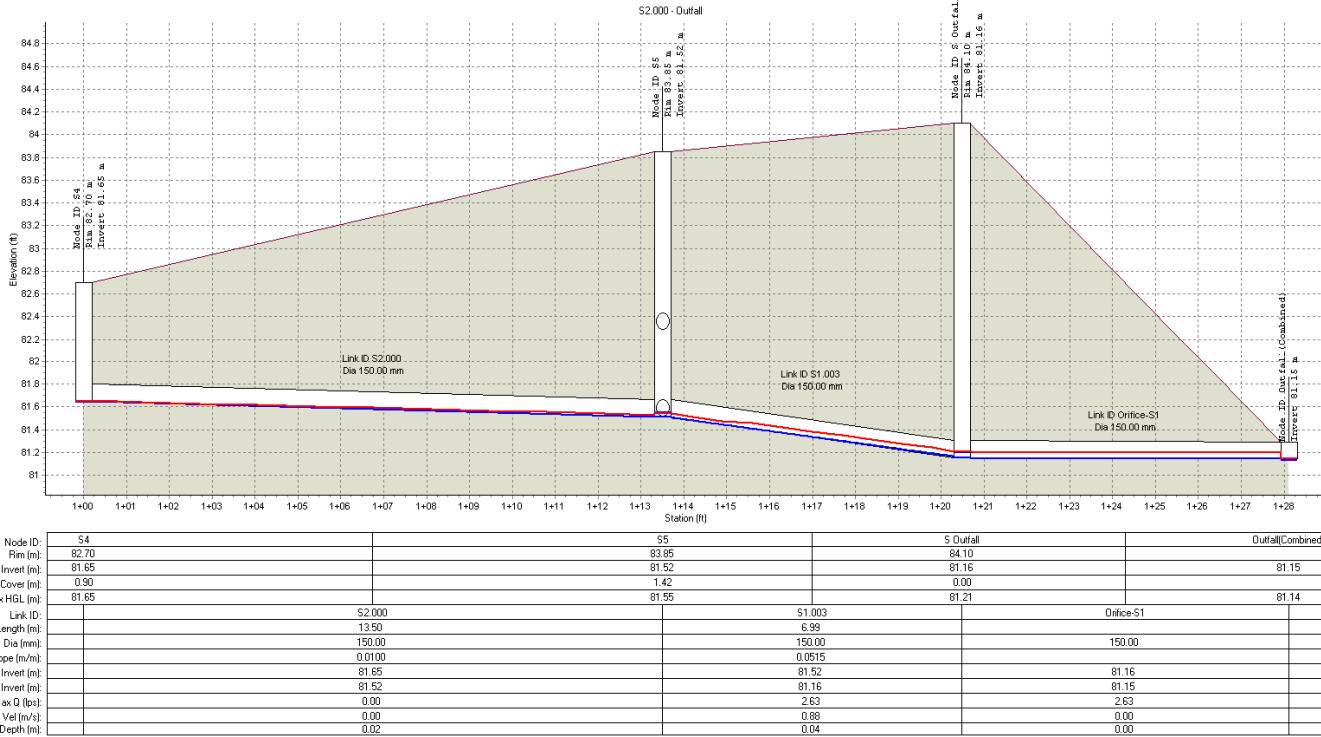
04/11/2021 12:00:00



1:30yr 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S2.000 - Outfall

05/11/2021 00:00:00



1:30yr 6hr Storm Analysis - Elapsed Time 24:00:00

16-254 Fitzroy Park Development

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0)

Project Description

File Name 16-254-Fitzroy Park S1 S2 lyr.SPF
Description 16-254 Fitzroy Park Development
lyr Return
Storm Network S1 & S2

Analysis Options

Flow Units LPS
Subbasin Hydrograph Method. Modified Rational
Time of Concentration..... Kirpich
Return Period..... 1 year
Storm Duration..... 360 min
Link Routing Method Hydrodynamic
Storage Node Exfiltration.. Constant rate, wetted area
Starting Date NOV-04-2021 00:00:00
Ending Date NOV-05-2021 00:00:00
Report Time Step 00:00:10

Runoff Quantity Continuity Volume Depth
Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.003 19.500
Continuity Error (%) 1.000

Flow Routing Continuity Volume Volume
Flow Routing Continuity hectare-m Mliters

External Inflow 0.000 0.000
External Outflow 0.002 0.025
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.000
Continuity Error (%) 0.000

Runoff Coefficient Computations Report

Subbasin Entrance

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot1-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.01	-	0.90
Composite Area & Weighted Runoff Coeff.	0.01		0.90

Subbasin Plot2-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.00	-	0.90
Composite Area & Weighted Runoff Coeff.	0.00		0.90

Subbasin Plot3-paved

Soil/Surface Description	Area (ha)	Soil Group	Runoff Coeff.
-	0.02	-	0.90
Composite Area & Weighted Runoff Coeff.	0.02		0.90

Kirpich Time of Concentration Computations Report

$$Tc = (0.01947 * (L^{0.77}) * (S^{-0.385}))$$

16-254 Fitzroy Park Development

Where:

Tc = Time of Concentration (min)
 L = Flow length (m)
 S = Slope (m/m)

 Subbasin Entrance

Flow length (m):	10.00
Slope (%):	0.50
Computed TOC (minutes):	0.88

 Subbasin Plot1-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot2-paved

Flow length (m):	5.00
Slope (%):	0.50
Computed TOC (minutes):	0.52

 Subbasin Plot3-paved

Flow length (m):	5.00
Slope (%):	1.50
Computed TOC (minutes):	0.34

Subbasin Runoff Summary

Subbasin ID	Accumulated Precip mm	Rainfall Intensity mm/hr	Total Runoff mm	Peak Runoff LPS	Weighted Runoff Coeff	Time of Concentration days	Time of Concentration hh:mm:ss
Entrance	19.50	3.25	17.55	0.48	0.900	0	00:05:00
Plot1-paved	19.50	3.25	17.55	0.28	0.900	0	00:05:00
Plot2-paved	19.50	3.25	17.55	0.20	0.900	0	00:05:00
Plot3-paved	19.50	3.25	17.55	0.19	0.900	0	00:05:00

Node Depth Summary

Node ID	Average Depth Attained m	Maximum Depth Attained m	Maximum HGL Attained m	Time of Max Occurrence days hh:mm	Total Flooded Volume ha-mm	Total Time Flooded minutes	Retention Time hh:mm:ss
S Outfall	0.01	0.03	81.18	0 00:59	0	0	0:00:00
S1	0.00	0.01	82.81	0 00:25	0	0	0:00:00
S2	0.00	0.02	82.57	0 00:33	0	0	0:00:00
S3	0.01	0.02	82.44	0 00:38	0	0	0:00:00
S4	0.00	0.00	81.65	0 00:00	0	0	0:00:00
S5	0.01	0.02	81.54	0 00:37	0	0	0:00:00
Outfall(Combined)	0.00	0.00	81.14	0 00:00	0	0	0:00:00
Stor-S1	0.00	0.00	81.60	0 00:00	0	0	0:00:00

Node Flow Summary

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Time of Peak Inflow days hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding days hh:mm
S Outfall	JUNCTION	0.00	1.15	0 00:36	0.00	
S1	JUNCTION	0.19	0.19	0 00:05	0.00	
S2	JUNCTION	0.20	0.39	0 00:25	0.00	
S3	JUNCTION	0.28	0.67	0 00:33	0.00	
S4	JUNCTION	0.00	0.00	0 00:00	0.00	
S5	JUNCTION	0.48	1.15	0 00:38	0.00	
Outfall(Combined)	OUTFALL	0.00	1.15	0 00:52	0.00	
Stor-S1	STORAGE	0.00	0.00	0 00:00	0.00	

16-254 Fitzroy Park Development

Storage Node Summary

Storage Node ID	Maximum Ponded Volume	Maximum Ponded Volume (%)	Time of Max. Ponded Volume	Average Ponded Volume	Average Ponded Volume (%)	Maximum Storage Node Outflow LPS	Maximum Exfiltration Rate cmm	Time of Max. Exfiltration Rate hh:mm:ss	Total Exfiltrated Volume 1000 m³
Stor-S1	0.000	0	0 00:00	0.000	0	0.00	0.00	0:00:00	0.000

Outfall Loading Summary

Outfall Node ID	Flow Frequency (%)	Average Flow LPS	Peak Inflow LPS
Outfall(Combined)	26.07	1.10	1.15
System	26.07	1.10	1.15

Link Flow Summary

Link ID	Element Type	Time of Peak Flow Occurrence	Maximum Velocity Attained	Length Factor	Peak Flow during Analysis	Design Capacity LPS	Ratio of Maximum Flow LPS	Ratio of Maximum Flow	Total Time Surcharged	Reported Condition
S1.000	CONDUIT	0 00:25	0.23	1.00	0.19	17.99	0.01	0.10	0	Calculated
S1.001	CONDUIT	0 00:33	0.26	1.00	0.39	11.92	0.03	0.14	0	Calculated
S1.002	CONDUIT	0 00:38	0.40	1.00	0.67	13.74	0.05	0.15	0	Calculated
S1.003	CONDUIT	0 00:36	0.76	1.00	1.15	29.97	0.04	0.17	0	Calculated
S2.000	CONDUIT	0 00:00	0.00	1.00	0.00	13.20	0.00	0.07	0	Calculated
Storage Link	CONDUIT	0 00:00	0.00	1.00	0.00	17.46	0.00	0.07	0	Calculated
Orifice-S1	ORIFICE	0 00:52			1.15			0.20		

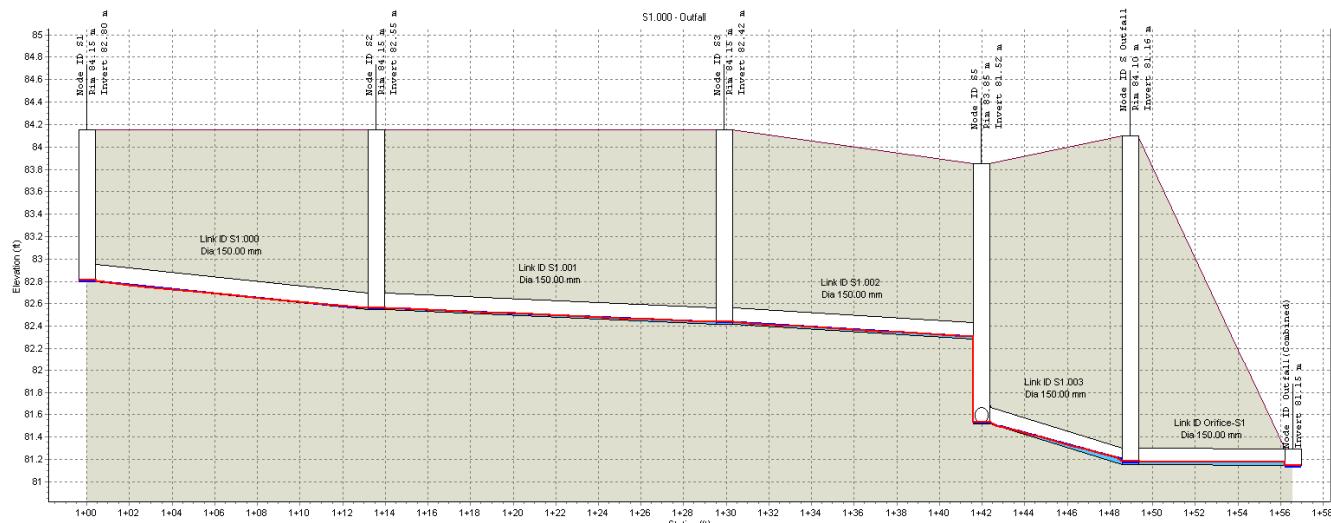
Highest Flow Instability Indexes

All links are stable.

Analysis began on: Fri Nov 5 16:08:32 2021
 Analysis ended on: Fri Nov 5 16:08:33 2021
 Total elapsed time: 00:00:01

Profile Plot S1.000 - Outfall

04/11/2021 01:00:00

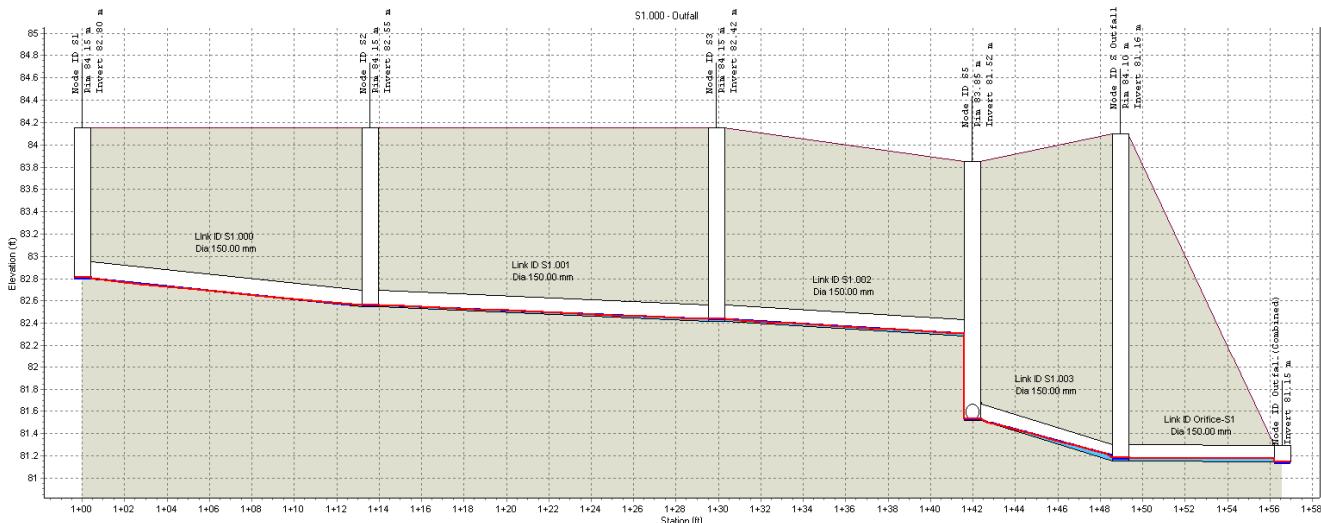


Node ID:	S1	S2	S3	S5	S Outfall	Outfall(Combined)
Rim (m):	84.15	84.15	84.15	83.85	84.10	
Invert (m):	82.80	82.55	82.42	81.52	81.16	81.15
Min Pipe Cover (m):	1.20	1.45	1.59	1.42	0.00	
Max HGL (m):	82.81	82.57	82.44	81.54	81.18	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Office-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0082	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.19	0.39	0.67	1.15	1.15	
Max Vel (m/s):	0.23	0.26	0.40	0.76	0.00	
Max Depth (m):	0.01	0.02	0.02	0.02	0.00	

1:1yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot S1.000 - Outfall

04/11/2021 06:00:00

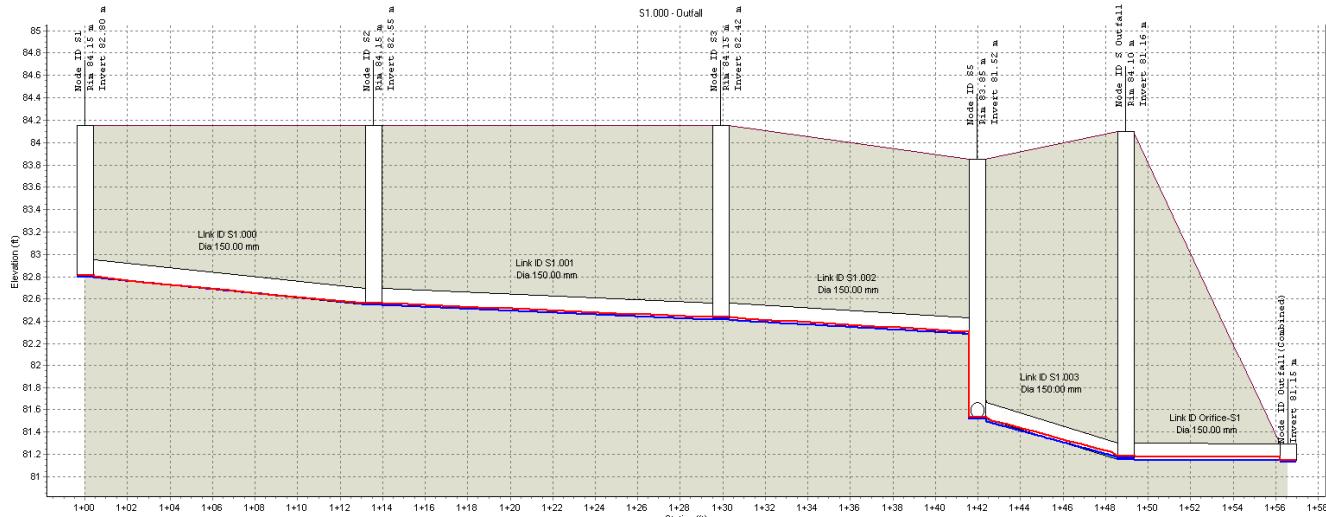


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Max Depth (m):	0.01	0.02	0.02	0.02	0.00	

1:1yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S1.000 - Outfall

04/11/2021 12:00:00

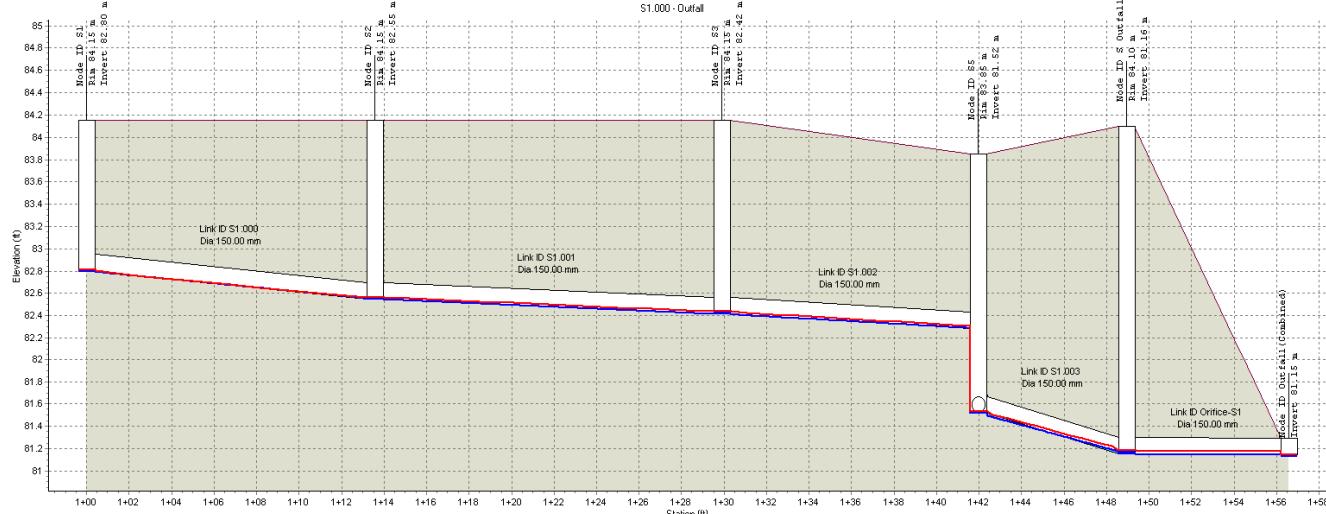


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Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
Length (m):	13.57	16.30	12.09	6.99		
Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0082	0.0108	0.0515		
Up Invert (m):	82.80	82.55	82.42	81.52	81.16	
Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.19	0.39	0.67	1.15	1.15	
Max Vel (m/s):	0.23	0.26	0.40	0.76	0.00	
Max Depth (m):	0.01	0.02	0.02	0.02	0.00	

1:1yr 6hr Storm Analysis - Elapsed Time 12:00:00

Profile Plot
S1.000 - Outfall

05/11/2021 00:00:00

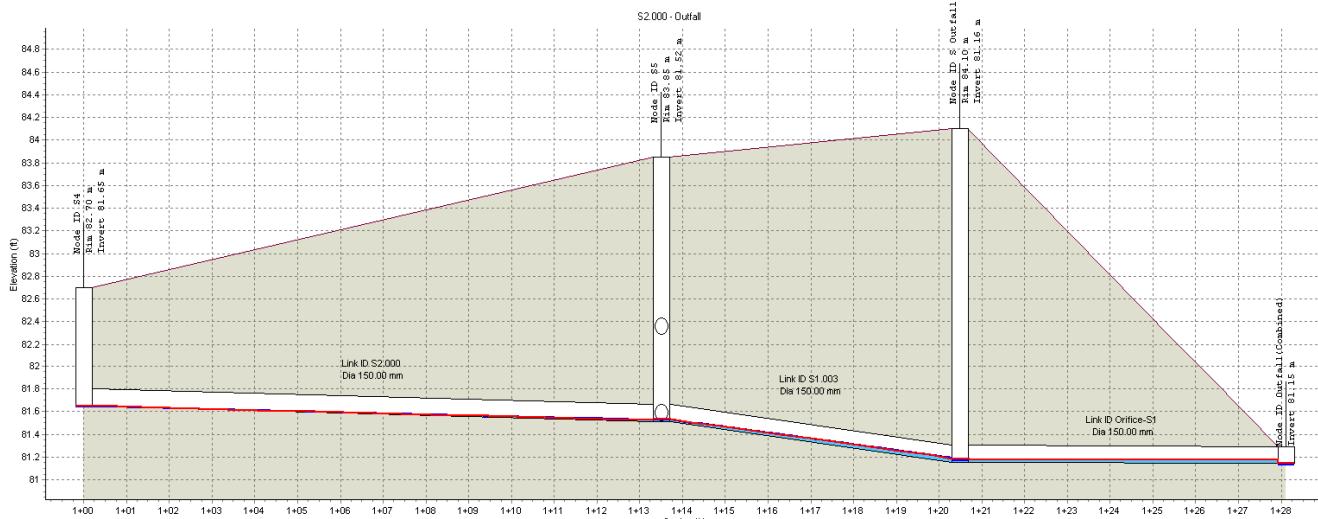


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Max HGL (m):	82.81	82.57	82.44	81.54	81.18	81.14
Link ID:	S1.000	S1.001	S1.002	S1.003	Orifice-S1	
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Dia (mm):	150.00	150.00	150.00	150.00		150.00
Slope (m/m):	0.0186	0.0082	0.0108	0.0515		
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Dn Invert (m):	82.55	82.42	82.28	81.16	81.15	
Max Q (lps):	0.19	0.39	0.67	1.15	1.15	
Max Vel (m/s):	0.23	0.26	0.40	0.76	0.00	
Max Depth (m):	0.01	0.02	0.02	0.02	0.00	

1:1yr 6hr Storm Analysis - Elapsed Time 24:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 01:00:00

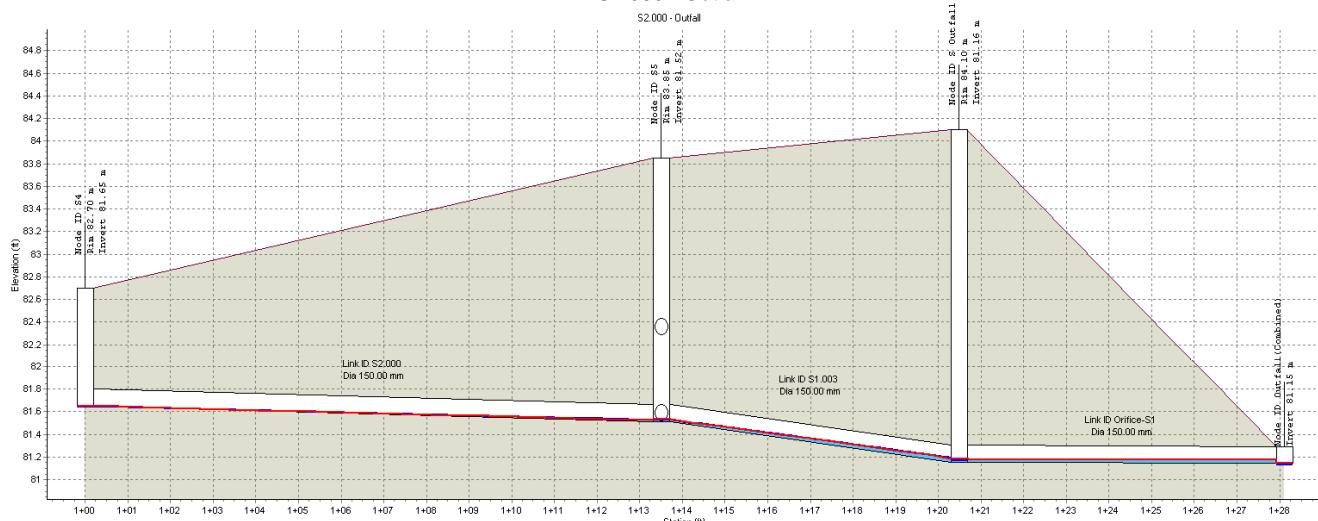


Node ID:	S4	Link ID:	S5	Link ID:	S Outfall	Outfall(Combined):
Rim (m):	82.70		83.95	84.10		
Invert (m):	81.65		81.52	81.16		81.15
Min Pipe Cover (m):	0.90		1.42	0.00		
Max HGL (m):	81.65		81.54	81.18		81.14
Link ID:	S.2.000		S1.003		Orifice-S1	
Length (m):	13.50		6.99			
Dia (mm):	150.00		150.00			150.00
Slope (m/m):	0.0100		0.0515			
Up Invert (m):	81.65		81.52			81.16
Dn Invert (m):	81.52		81.16			81.15
Max Q (lps):	0.00		1.15			1.15
Max Vel (m/s):	0.00		0.76			0.00
Max Depth (m):	0.01		0.02			0.00

1:1yr 6hr Storm Analysis - Elapsed Time 01:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 06:00:00

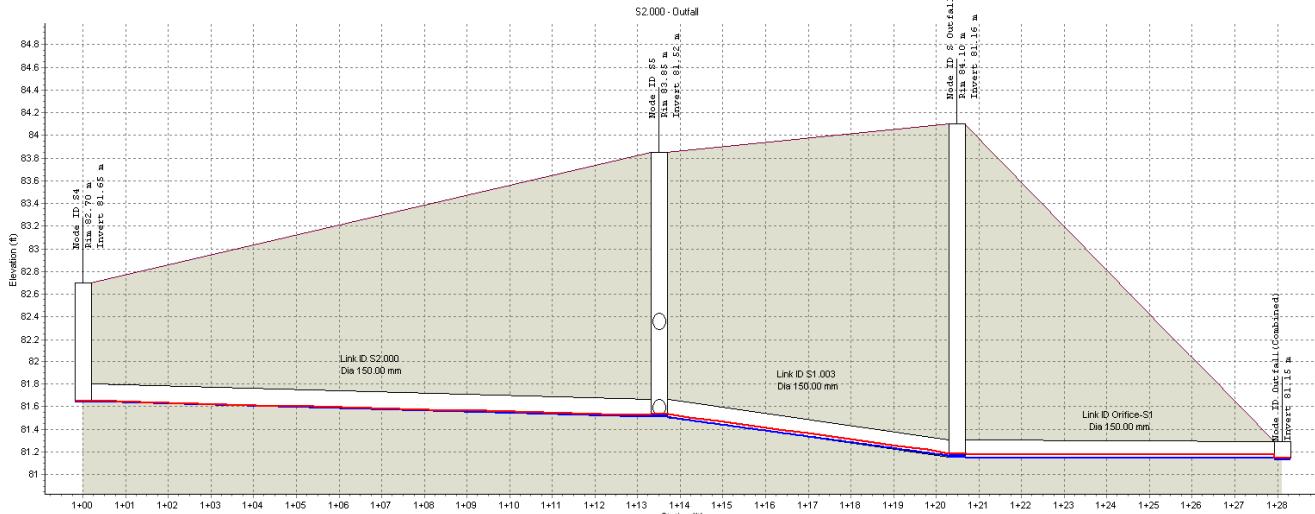


Node ID:	S4	Link ID:	S5	Link ID:	S Outfall	Outfall(Combined):
Rim (m):	82.70		83.95	84.10		
Invert (m):	81.65		81.52	81.16		81.15
Min Pipe Cover (m):	0.90		1.42	0.00		
Max HGL (m):	81.65		81.54	81.18		81.14
Link ID:	S.2.000		S1.003		Orifice-S1	
Length (m):	13.50		6.99			
Dia (mm):	150.00		150.00			150.00
Slope (m/m):	0.0100		0.0515			
Up Invert (m):	81.65		81.52			81.16
Dn Invert (m):	81.52		81.16			81.15
Max Q (lps):	0.00		1.15			1.15
Max Vel (m/s):	0.00		0.76			0.00
Max Depth (m):	0.01		0.02			0.00

1:1yr 6hr Storm Analysis - Elapsed Time 06:00:00

Profile Plot
S2.000 - Outfall

04/11/2021 12:00:00

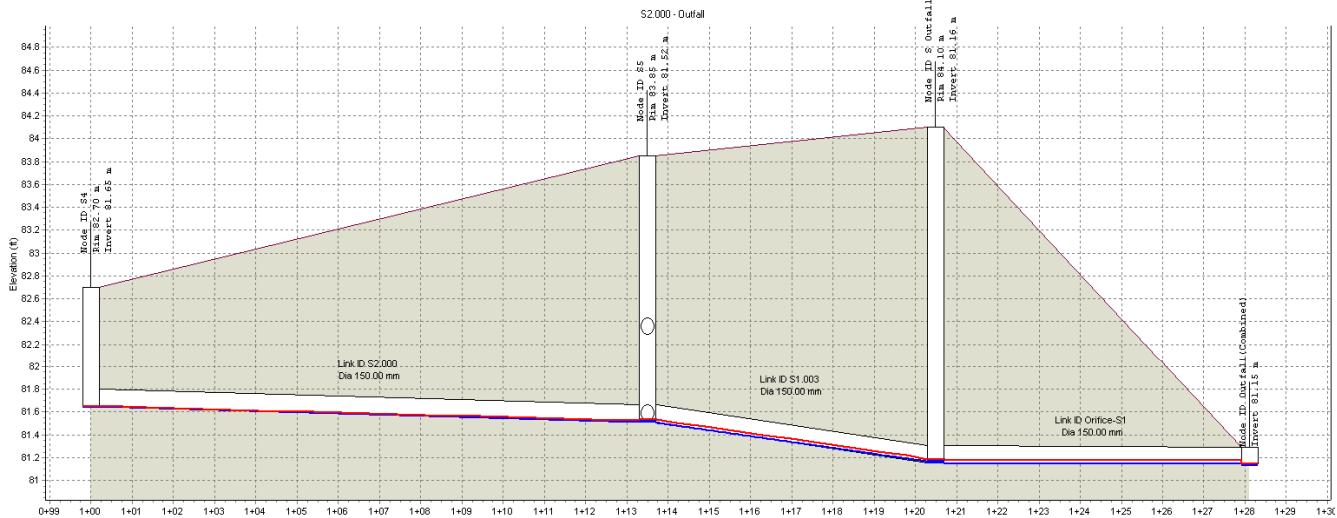


Node ID:	S4	Link ID:	S5	Node ID:	S Outfall	Outfall[Combined]:
Rim (m):	82.70		83.95	84.10		
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Link ID:	S2.000		S1.003		Orifice-S1	
Length (m):	13.50		6.99			
Dia (mm):	150.00		150.00		150.00	
Slope (m/m):	0.0100		0.0515			
Up Invert (m):	81.65		81.52		81.16	
Dn Invert (m):	81.52		81.16		81.15	
Max Q (lps):	0.00		1.15		1.15	
Max Vel (m/s):	0.00		0.76		0.00	
Max Depth (m):	0.01		0.02			

1:1yr 6hr Storm Analysis - Elapsed Time 12:00:00

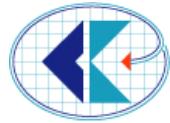
Profile Plot
S2.000 - Outfall

05/11/2021 00:00:00



Node ID:	S4	Link ID:	S5	Node ID:	S Outfall	Outfall[Combined]:
Rim (m):	82.70		83.95	84.10		
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Min Pipe Cover (m):	0.90		1.42	0.00		
Max HGL (m):	81.65		81.54	81.18		81.14
Link ID:	S2.000		S1.003		Orifice-S1	
Length (m):	13.50		6.99			
Dia (mm):	150.00		150.00		150.00	
Slope (m/m):	0.0100		0.0515			
Up Invert (m):	81.65		81.52		81.16	
Dn Invert (m):	81.52		81.16		81.15	
Max Q (lps):	0.00		1.15		1.15	
Max Vel (m/s):	0.00		0.76		0.00	
Max Depth (m):	0.01		0.02			

1:1yr 6hr Storm Analysis - Elapsed Time 24:00:00



Appendix C

Attenuation Calculations



Calculated by:	Alan Clancy
Site name:	Fitzroy Park Development
Site location:	Fitzroy Park N6 6JA

Parking area

This is an estimation of the storage volume requirements that are needed to meet normal best practice criteria in line with Environment Agency guidance "Preliminary rainfall runoff management for developments", W5-074/A/TR1/1 rev. E (2012) and the SuDS Manual, C753 (Ciria, 2015). It is not to be used for detailed design of drainage systems. It is recommended that hydraulic modelling software is used to calculate volume requirements and design details before finalising the drainage scheme.

Site coordinates

Latitude: 51.56695° N

Longitude: 0.15783° W

Reference: 6530830

Date: 2021-11-05T16:10:02

Methodology	IH124
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Site characteristics

Total site area (ha)	0.5041
Significant public open space (ha)	0.08
Area positively drained (ha)	0.0475
Pervious area contribution (%)	30
Impermeable area (ha)	0.026
Percentage of drained area that is impermeable (%)	55
Impervious area drained via infiltration (ha)	0
Return period for infiltration system design (year)	10
Impervious area drained to rainwater harvesting systems (ha)	0
Return period for rainwater harvesting system design (year)	10
Compliance factor for rainwater harvesting system design (%)	66
Net site area for storage volume design (ha)	0.05
Net impermeable area for storage volume design (ha)	0.03

Design criteria

Volume control approach	controlled discharge	
Climate change allowance factor	Default	Edited
Urban creep allowance factor	1.4	1.4
Interception rainfall depth (mm)	5	5
Minimum flow rate (l/s)	5	5
Qbar estimation method	Calculate from SPR and SAAR	
SPR estimation method	Calculate from SOIL type	
	Default	Edited
Qbar total site area (l/s)	0.57	--
SOIL type	4	4
HOST class	N/A	N/A
SPR	0.47	0.47
Hydrology	Default	Edited
SAAR (mm)	659	659
M5-60 Rainfall Depth (mm)	20	20
'r' Ratio M5-60/M5-2 day	0.4	0.4
Rainfall 100 yrs 6 hrs	63	
Rainfall 100 yrs 12 hrs	98.56	
FEH/FSR conversion factor	1.28	1.28
Hydrological region	6	
Growth curve factor: 1 year	0.85	0.85
Growth curve factor: 10 year	1.62	1.62
Growth curve factor: 30 year	2.3	2.3
Growth curve factor: 100 year	3.19	3.19

* Where rainwater harvesting or infiltration has been used for managing surface water runoff such that the effective impermeable area is less than 50 % of the 'area positively drained', the 'net site area' and the estimates of Qbar and other flow rates will have been reduced accordingly.

	Default	Edited
Qbar total site area (l/s)	0.57	0.57
Qbar net site area (l/s)	0.21	0.21
1 in 1 year (l/s)	5	5
1 in 30 years (l/s)	5	5
1 in 100 years (l/s)	5	5

	Default	Edited
Interception storage (m ³)	1	1
Attenuation storage (m ³)	2	2
Long term storage (m ³)	0	0
Treatment storage (m ³)	3	3
Total storage (excluding treatment) (m ³)	3	3

