

SITE INVESTIGATION FACTUAL REPORT

Report No: [REDACTED]
Client: Crawford Claims Management
Site: 151 Gloucester Avenue, London
Client Ref: [REDACTED]
Date of Visit: 18/11/2019



Investigation Layout Plan

Sheet: 1 of 1

Job No: [REDACTED]

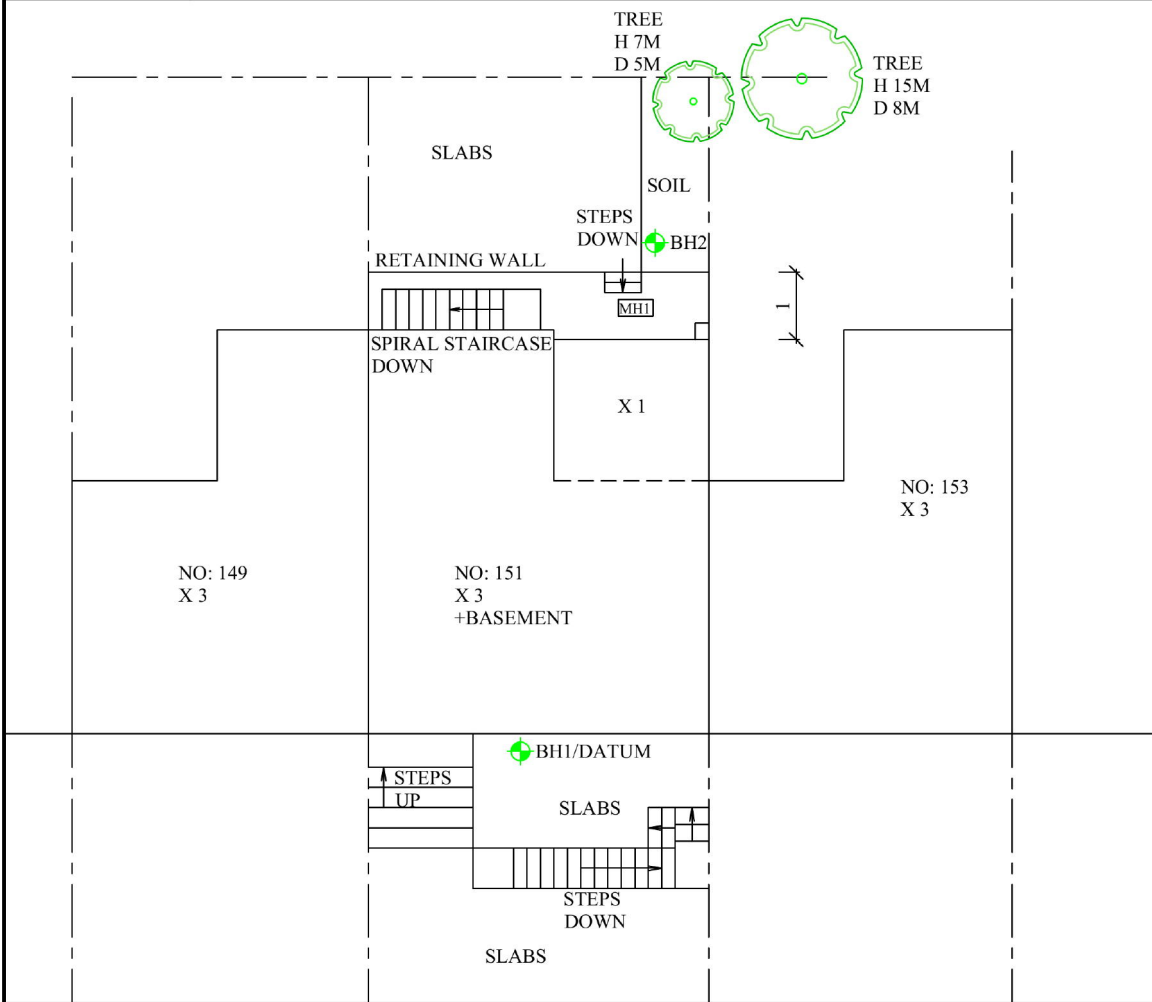
Date: 18/11/2019

Site: 151 Gloucester Ave NW1

Work carried out for: Crawford Claims Management

(SI) SA (Checked) CFT (Drawn)

Weather: DRY



FOOTPATH

GLOUCESTER AVE

ON SITE TREE IDENTIFICATION FOR GUIDANCE ONLY. NOT AUTHENTICATED.

Remarks:





Key:

- Combined Gully RWWG
- Manhole MH
- Rain Water Pipe RWP
- Rain Water Gully RWG
- Soil Vent Pipe SVP
- Waste Gully WG
- Waste Pipe WP

- Surface Water Drain
- Foul Water Drain
- Tree / Bush (approx. ht in m)
- Trial Pit
- Borehole
- O/D - Open Discharge

Scale: N.T.S.

Borehole		1 +Datum		Sheet: 1 of 2	Site: 151 Gloucester Avenue	
Boring Method: Hand Auger		Weather: Dry		Date: 18/11/2019	Client: Crawford Claims Management	
Diameter (mm): 75	Soil Description			Ground Level:		
Depth (m)	Soil Description	Thickness	Legend	Depth	Type	Result
0.00	Slab over CONCRETE	0.20				
0.20	MADEGROUND medium compact brown silty sandy clay with gravel and brick rubble	0.70				
0.90	Firm brown, grey veined silty CLAY with partings of orange silt and fine sand	0.50		1.00	DV	64
						58
1.40	Firm wet brown silty CLAY	1.80		1.50	DV	54
						52
				2.00	DV	50
						56
				2.50	DV	58
						64
				3.00	DV	68
						70
3.20	Stiff brown grey veined silty CLAY with partings of orange silt and fine sand	1.80		3.50	DV	114
						120
				4.00	DV	130+
						130+
				4.50	DV	130+
						130+
Remarks:				Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive		To Max Depth Dia (m) (mm)
Logged: KP	SA	Checked:	Approved:	Version V1.0 28/01/16	N.T.S.	

Borehole		2		Sheet:	1 of 1		Site:	151 Gloucester Avenue			
Boring Method:		Hand Auger		Job No:							
Diameter (mm):		75		Date:	18/11/2019						
Weather:		Dry		Ground Level:			Client:	Crawford Claims Management			
Depth	Soil Description						Thickness	Legend	Samples and Tests		
(m)									Depth	Type	Result
0.00	MADEGROUND medium compact brown gravelly silty clay with brick fragments						1.30				
									1.00	DM	12
											14
											13
											12
1.30	Stiff brown grey veined silty CLAY with partings of orange silt and fine sand						2.80				
									1.50	DV	76
											80
									2.00	DV	128
											130+
									2.50	DV	130+
											130+
									3.00	DV	130+
											130+
									3.50	DV	130+
											130+
									4.00	DV	130+
											130+
4.10	Stiff brown, grey veined silty CLAY with partings of orange silt and fine sand and claystone nodules						0.10				
4.20	Stiff brown, grey veined silty CLAY with partings of orange silt and fine sand						0.80				
									4.50	DV	130+
											130+
									5.00	DV	130+
											130+
5.00	End of BH										
Remarks: BH ends at 5.0m. BH dry and open on completion, no observed below 3.1m.						Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive			To Max Depth Dia (m) (mm) 3.10 1		
Logged:	KP	SA	Checked:	Approved:	Version	V1.0 28/01/16		N.T.S.			

Laboratory Summary Results

Our Ref : [REDACTED] Date Sampled: 18/11/2019
 Location : 151 Gloucester Avenue, London, NW1 Date Received : 19/11/2019
 Client : Crawford Claims Management Date Tested : 19/11/2019
 Address : [REDACTED] Date of Report : 27/11/2019

Sample Ref TP/BH No	Depth (m)	Type	Moisture Content (%) [11]	Soil Fraction > 0.425mm (%) [12]	Liquid Limit (%) [13]	Plastic Limit (%) [14]	Plasticity Index (%) [15]	Liquidity Index [16]	Modified Plasticity Index (%) [16]	Soil Class [17]	Filter Paper Contact Time (h.)	Soil Sample Suction (kPa) [18]	Oedometer Strain [19]	Estimated Heave Potential (mm) [20]	In situ Shear Vane Strength (kPa) [11]	Organic Content (%) [12]	pH Value [13]	Sulphate Content (g/l)		Class	
																		SO3	SO4		
BH1	1.0	D	38	<5	78	27	51	0.22	51	CV	168	69.8			61						
	1.5	D	50	<5							Unsuitable for suction testing - Too Soft			53							
	2.0	D	54	<5							Unsuitable for suction testing - Too Soft			53							
	2.5	D	57	<5	72	28	44	0.67	44	CV					61						
	3.0	D	42	<5							Unsuitable for suction testing - Too Soft			69							
	4.0	D	34	<5							168	336			> 130						
	5.0	D	32	<5	78	31	47	0.03	47	CV	168	323			> 130						

Test Methods / Notes

[1] In-house method S10 adopted from BS EN 12368-4:03
 [11] BS 1377: Part 2: 1990, Test No 3.2
 [12] Estimated if <5%, otherwise measured
 [13] BS 1377: Part 2: 1990, Test No 4.4
 [14] BS 1377: Part 2: 1990, Test No 5.3
 [15] BS 1377: Part 2: 1990, Test No 5.4
 [16] BRE Digest 240: 1993
 [17] BS 5910: 2018: Figure 8 - Plasticity Chart for the classification of fine soils

[18] In-house method S17 adopted from BS EN 12368-4:03
 [19] In-house Test Procedure S17: One Dimensional Swell/Shrink Test
 [20] Estimated Heave Potential (mm)
 [21] Values of shear strength were determined in-situ by CPT using a Pileon hand vane or Geosir vane (GV).
 [22] BS 1377: Part 3: 1990, Test No 4
 [23] BS 1377: Part 3: 1990, Test No 9
 [24] BS 1377: Part 3: 1990, Test No 5.6
 [25] SO₃ = 1.2 x SO₄

[11] BRE Special Digest One (Concrete on Aggressive Ground) August 2005
 Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.

* These tests are not UKAS accredited
 Full reports can be provided upon request.

KEY
 D Disturbed sample (small)
 B Disturbed sample (bulk)
 U Undisturbed sample
 W Groundwater sample
 ENP Essentially Non-Plastic by inspection
 US Underside of Foundation



Our Ref: [REDACTED]

Laboratory Testing Results

Date Sampled : 18/11/2019

Location : 151 Gloucester Avenue, London, NW1

Date Received : 19/11/2019

Client : Crawford Claims Management

Date Tested : 19/11/2019

Address: [REDACTED]

Date of Report : 27/11/2019

Sample Ref. TP/BH No.	Depth (m)	Type	Moisture Content (%) [11]	Soil Fracture >0.425mm (%) [12]	Liquid Limit (%) [13]	Plastic Limit (%) [14]	Plasticity Index (%) [15]	Liquidity Index [15]	Modified* Plasticity Index (%) [16]	Soil* Class [17]	Filter Paper Contact Time (h)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated Heave Potential (DM) (mm) [10]	In situ* Shear Vane Strength (kPa) [11]	Organic* Content (%) [12]	pH* Value [13]	Sulphate Content* (g/l)		* Class [16]	
																		SO3	SO4		
BH2	1.0	D	32	10	68	26	42	0.14	38	CH	Unsuitable for suction testing - Made Ground										
	1.5	D	31	<5							168	181			78						
	2.0	D	27	<5							168	403			129						
	2.5	D	28	<5	76	26	50	0.03	50	CV	168	574			> 130						
	3.0	D	30	<5							168	442			> 130						
	4.0	D	32	<5							168	325			> 130						
	5.0	D	34	<5	80	31	49	0.05	49	CV	168	370			> 130						

Test Methods / Notes

[11] BS 1377: Part 2: 1990, Test No 3.2
 [12] Estimated if <5%, otherwise measured
 [13] BS 1377: Part 2: 1990, Test No 4.4
 [14] BS 1377: Part 2: 1990, Test No 5.3
 [15] BS 1377: Part 2: 1990, Test No 5.4
 [16] BS 1377: Part 2: 1990, Test No 5.4
 [17] BS 5939: 1981: Figure 31 - Plasticity Chart for the classification of fine soils

[18] BS 1377: Part 2: 1990, Test No 4.4
 [19] In-house Test Procedure SI 7% One Dimensional Swell/Shrink Test
 [20] Estimated Heave Potential (DM)
 [21] Values of shear strength were determined in situ by CPT using a Pileon hand vane or Greater vane (GV).
 [22] BS 1377: Part 3: 1990, Test No 4
 [23] BS 1377: Part 3: 1990, Test No 9
 [24] BS 1377: Part 3: 1990, Test No 5.6
 [25] SO₃ = 12 x SO₄

[10] BRE: Special Phase One (Concrete in Aggressive Grounds) August 2006
 Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.
 * These tests are not UKAS accredited
 Full reports can be provided upon request

Key

D Disturbed sample (small)
 B Disturbed sample (b.R.)
 U Undisturbed sample
 W Groundwater sample
 ENP Essentially Non-Plastic by inspection
 US Underside of Foundation

Test results reported relate only to the items tested.
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Version: SBH V1.5 - 26.06.18

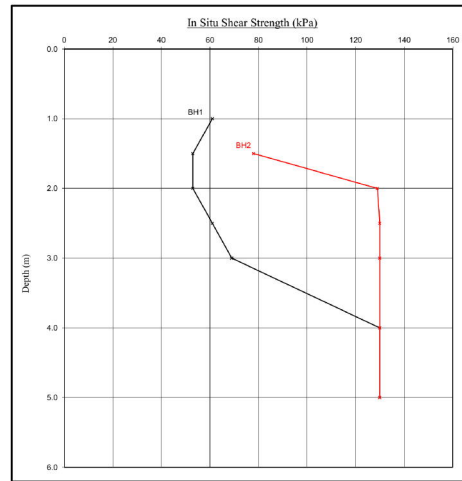
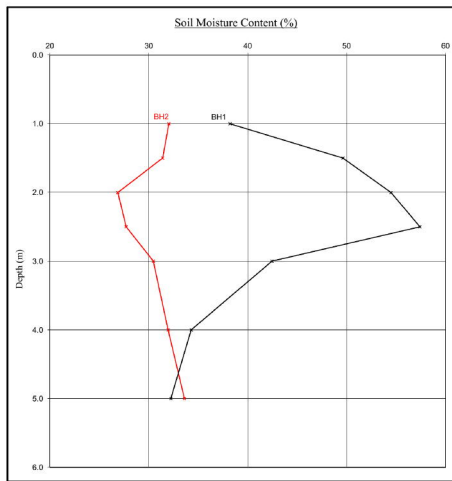
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Moisture Content Profiles

Our Ref: [REDACTED]
 Location: 151 Gloucester Avenue, London, NW1
 Work carried out for: Crawford Claims Management

Date Sampled: 18/11/2019
 Date Received: 19/11/2019
 Date Tested: 19/11/2019
 Date of Report: 27/11/2019

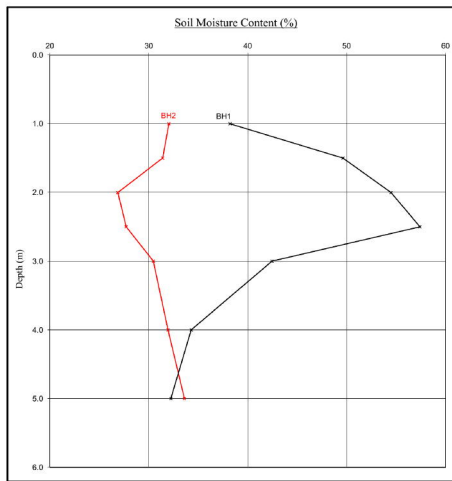


Notes
 1. If plotted, $0.4LL$ and $PI-2$ (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
 2. Unless specifically noted the profiles have not been related to a site datum.

Note
 1. Unless otherwise stated, values of Shear Strength were determined in situ by CET using a Picon Hand Vane the calibration of which is limited to a maximum reading of 130 kPa.
 2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

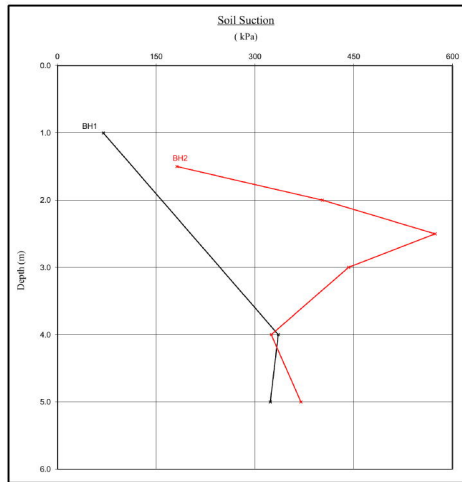
Our Ref: [REDACTED]
 Location: 151 Gloucester Avenue, London, NW1
 Work carried out for: Crawford Claims Management



- Notes**
1. If plotted, $0.4LL$ and $PI \times 2$ (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
 2. Unless specifically noted the profiles have not been related to a site datum.

Soil Suction Profiles

Date Sampled: 18/11/2019
 Date Received: 19/11/2019
 Date Tested: 19/11/2019
 Date of Report: 27/11/2019



- Note**
- When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

EPSL

European Plant Science Laboratory

Sheet: 1 of 1

Job No: [REDACTED]

Date: 21/11/2019

Order No: [REDACTED]

EPSL Ref: [REDACTED]

Site: 151 Gloucester Avenue,

Work carried out for: Crawford Claims MGMT SUS

Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.

The results were as follows -

<u>Trial pit/ Borehole number</u>	<u>Root diameter (mm)</u>	<u>Tree, shrub or climber from which root originates</u>	<u>Result of starch test</u>
BH1 (to 1.6m)	<1 mm	Platanus spp. 2 roots	Positive
BH2 (to 3.1m)	1 mm	Monocotyledon spp. 2 roots	Positive
BH2 (to 3.1m)	1.5 mm	Platanus spp. 2 roots	Positive

Platanus spp. include London plane and Oriental plane.

Monocotyledon spp. include palms, grasses, bamboos and lilies.

[REDACTED]
MDM

[REDACTED]
Head of Laboratory Services : M D Mitchell B.Sc. (Hons), M.Phil.

Plant Anatomist : Dr G S Turner B.Sc. (Hons), M.Sc., Ph.D

Plant Anatomist : Dr R J Shaw B.Sc. (Hons), Ph.D

Consultant: Dr M P Denne B.Sc. (Hons), M.Sc., Ph.D
[REDACTED]