SITE INVESTIGATION
FACTUAL REPORT

Report No:	[redacted]
Client:	Sedgwick International UK - Maidstone
Site:	76 Agar Grove, London
Client Ref:	[redacted]
Date of Visit:	03/06/2020

[redacted]

Investigation Layout Plan

Sheet: 1 of 1

Job No: [redacted]

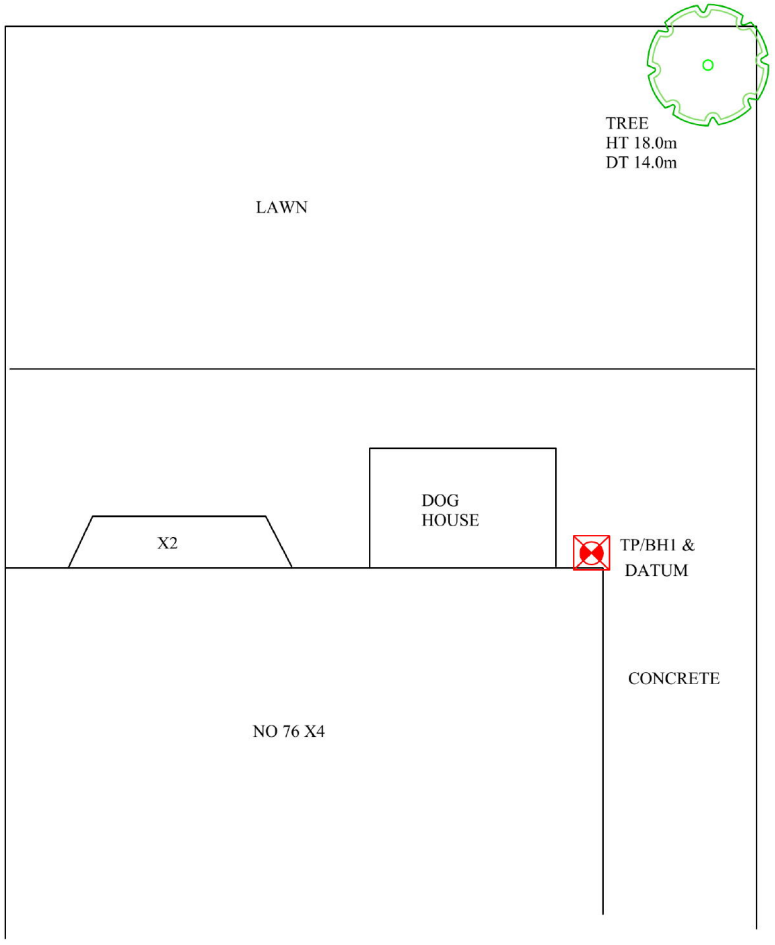
Date: 03/06/20

Site: 76, Agar Grove, NW1

Work carried out for: Sedgwick International UK

DB (SI) | SA (Checked) | Jo (Drawn)

Weather: Dry



DRAIN RECOMMENDATIONS

REMARKS: Scaffolding up around property - see photos

Scale: N.T.S.

Surface Water Drain

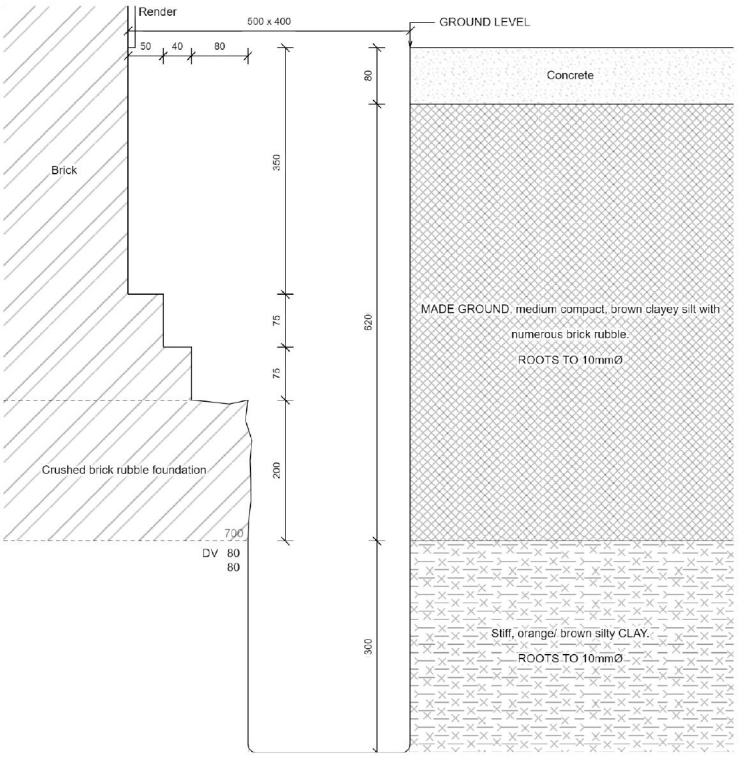
Foul Water Drain

PROPERTY ASSURANCE
Giving our all

TEST REPORT: **Trial Pit**

REPORT NUMBER: [redacted]

TRIAL PIT REF:	TP 1	DATE:	03/06/2020
CLIENT:	Sedgwick International UK	SITE:	76 Agar Grove
JOB NO:	[redacted]	WEATHER:	Dry
EXCAVATION METHOD:	Hand tools		



For Strata below 1000mm see Bore Hole log

Key:

D	Small disturbed sample	J	Jar sample
B	Bulk disturbed sample	V	Pilcon vane (kPa)
W	Water sample	M	Mackintosh probe
TDTD	Too dense to drive		

Remarks:
Test results reported relate only to the items tested.
This report shall not be reproduced except in full without approval of the Laboratory.

For and on behalf of CET
Scott Alger - Lab

Approved Signatory
04-Jun-20

Report Format: [redacted]

[redacted]

[redacted]

Borehole	1		Sheet:	1 of 1	Site:	76 Agar Grove
			Job No:			
			Date:	03/06/2020		
Boring Method:	Hand Auger		Ground Level:		Client:	Sedgwick International UK - Maidstone
Diameter (mm):	75	Weather:	Dry			

Depth	Soil Description	Thickness	Legend	Samples and Tests		
(m)				Depth	Type	Result
0.00	See Trial Pit	1.00				
1.00	Firm orange-brown silty CLAY	0.50		1.00	DV	70
						60
1.50	Stiff orange-brown silty CLAY	2.00		1.50	DV	90
						90
				2.00	DV	90
						100
				2.50	DV	114
						124
				3.00	DV	130
						140
3.50	Very stiff orange-brown silty CLAY	1.50		3.50	DV	140+
						140+
				4.00	DV	140+
						140+
				4.50	DV	140+
						140+
				5.00	DV	140+
5.00	End of BH					140+

Remarks:
BH ends at 5.0m.BH dry and open on completion,no roots observed below 2.0m.Datum installed at 5.0m.

Key:

D - Disturbed Sample
B - Bulk Sample
W - Water Sample Roots
J - Jar Sample Roots
V - Pilcon Shear Vane (kPa Roots
M - Mackintosh Probe Depth to Water (m)
TDTD - Too Dense To Drive

	To Depth (m)	Max Dia (mm)
Roots	1.50	3
Roots	2.00	1
Roots		
Depth to Water (m)		

Logged:	DB	SA	Checked:	Approved:	Version	V1.0 28/01/16	N.T.S.

Laboratory Summary Results

Our Ref : [redacted]

Location : 76 Agar Grove

Client: Sedgwick International UK - Maidstone

Address: [redacted]

Date Sampled: 03/06/2020

Date Received : 08/06/2020

Date Tested : 08/06/2020

Date of Report : 25/06/2020

Sample Ref TP/BH No	Depth (m)	Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity * Index [5]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (h)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated * Heave Potential (mm)[10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%)[12]	pH * Value [13]	Sulphate Content * (g / l) SO_3 [14]	Sulphate Content * (g / l) SO_4 [15]	* Class [16]
1	U/S 0.70	D	36	<5	73	30	43	0.14	43	CV	168	172			80					
	1.0	D	34	<5							168	494			65					
	1.5	D	30	<5	70	27	43	0.06	43	CV	168	658			90					
	2.0	D	29	<5							168	400			95					
	2.5	D	27	<5	68	25	43	0.04	43	CH					119					
	3.0	D	28	<5							168	678			135					
	3.5	D	28	<5	72	26	46	0.05	46	CV					140					
	4.0	D	29	<5							168	613			140					
	4.5	D													140					
	5.0	D	29	<5							168	575			140					

Test Methods / Notes

[1] BS 1377 : Part 2 : 1990, Test No 3.2

[2] Estimated if <5%, otherwise measured

[3] BS 1377 : Part 2 : 1990, Test No 4.4

[4] BS 1377 : Part 2 : 1990, Test No 5.3

[5] BS 1377 : Part 2 : 1990, Test No 5.4

[6] BRE Digest 240 : 1993

[7] BS 5930 : 2015 : Figure 8 - Plasticity Chart for the classification of fine soils

[8] In-house method S9a adapted from BRE IP 4/93

[9] In-house Test Procedure S17a: One Dimensional Swell/Strain Test

[10] Estimated Heave Potential

[11] Values of shear strength were determined in situ by CET using a Pilcon hand vane or Geonor vane (GV).

[12] BS 1377 : Part 3 : 1990, Test No 4

[13] BS 1377 : Part 2 : 1990, Test No 9

[14] BS 1377 : Part 3 : 1990, Test No 5.6

[15] $SO_3 = 1.2 \times SO_4$

[16] BRE Special Digest One (Concrete in Aggressive Ground) August 2005

Note that if the SO4 content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.

* These tests are not UKAS accredited

Full reports can be provided upon request.

Key

D	Disturbed sample (small)
B	Disturbed sample (bulk)
U	Undisturbed sample
W	Groundwater sample
ENP	Essentially Non-Plastic by inspection
U/S	Underside of foundation

[redacted]

Version: 5BH V1.1 - 13.01.2020

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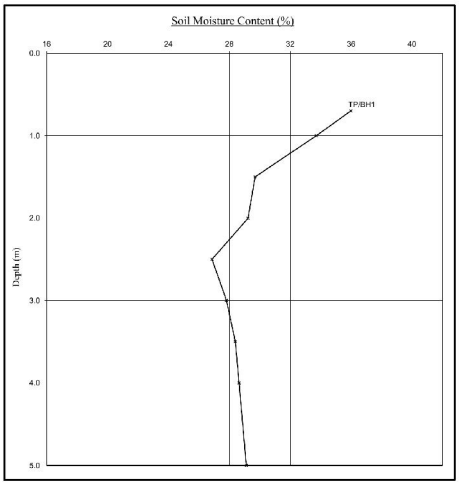
Moisture Content Profiles

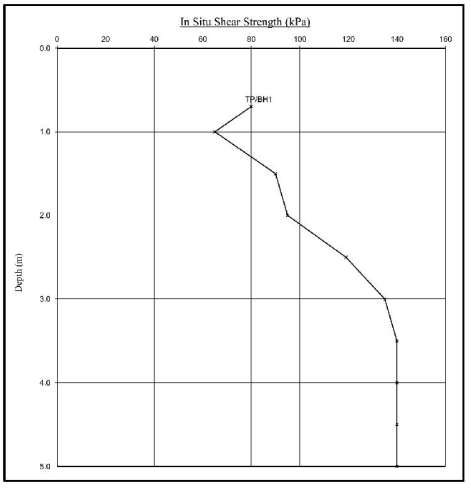
Our Ref : [redacted]
Location : 76 Agar Grove
Work carried out for: Sedgwick International UK - Maidstone



Notes
1. If plotted, 0.4 LL and PI/2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Shear Strength Profiles

Date Sampled : 03/06/2020
Date Received : 08/06/2020
Date Tested : 08/06/2020
Date of Report : 25/06/2020



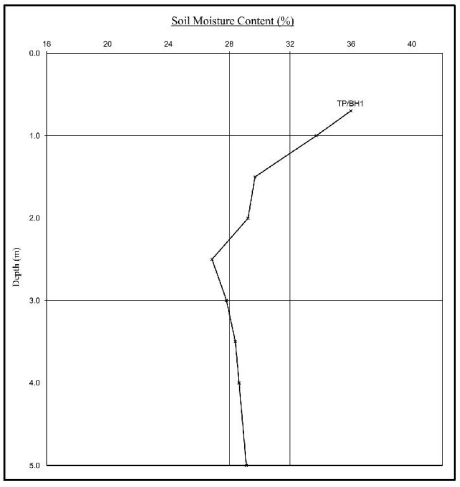
Note
1. Unless otherwise stated, values of Shear Strength were determined in situ by CPT using a Pilcon Hand Vane the calibration of which is limited to a maximum reading of 140 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

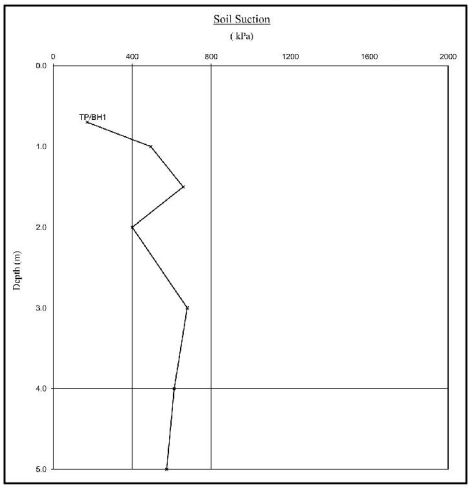
Moisture Content Profiles

Soil Suction Profiles

Our Ref : [redacted]
Location : 76 Agar Grove
Work carried out for: Sedgwick International UK - Maidstone

Date Sampled : 03/06/2020
Date Received : 08/06/2020
Date Tested : 08/06/2020
Date of Report : 25/06/2020





Notes
1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Note
When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependant on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

EPSL ***European Plant Science Laboratory***	Sheet: 1 of 1 Job No: [redacted] Date: **10/06/2020** Order No: [redacted] EPSL Ref [redacted]	Site: **76 Agar Grove,** Work carried out for: **Sedgwick International UK**

Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.
The results were as follows -

Trial pit/ Borehole number	**Root diameter (mm)**	**Tree, shrub or climber from which root originates**	**Result of starch test**
TP1 (USF)	5 mm	Tilia spp. 4 roots	Positive
BH1 (to 2.5m)	2 mm	Tilia spp. * 4 roots	Negative

* All in a state of decay.

Tilia spp. are limes.

[redacted]

RJS

[redacted]

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Plant Anatomist : Dr R J Shaw B.Sc. (Hons), Ph.D
Consultant: Dr M P Denne B.Sc. (Hons), M.Sc., Ph.D

[redacted]