

File Note

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Project title Haverstock Hill

Job number

268265-00

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File reference

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Date

15 October 2020

Subject Pre-planning - Condition 21

1 Introduction

Arup has been commissioned by OD Hotels Camden Ltd. to produce the documentation needed to meet Planning Condition 21 for the 5-17 Haverstock Hill site, NW3 2BL, London.

Condition 21 – SUDS

Condition 21 was defined by London Borough of Camden (LBC) and is worded as follows:

“Prior to commencement of the development, full details of the sustainable drainage system shall be submitted to and approved in writing by the local planning authority. Such a system should be designed to accommodate all storms up to and including a 1:100 year storm with a 30% provision for climate change, such that flooding does not occur in any part of a building or in any utility plant susceptible to water, and to achieve 50% reduction in run off (targeting a maximum of 14l/s runoff in all storm events up to and including the 1 in 100 year 6 hour storm). The system shall include blue/green roofs (providing 23m³ of storage) and an attenuation tank (providing 47m³ of storage). And shall thereafter be retained and maintained in accordance with the approved maintenance plan.”

2 Planning condition response

Please refer to Appendix A for the plan layout of the drainage design proposed for the development, and Appendix B for the modelling results of the drainage network, as modelled using MicroDrainage.

2.1 Existing Surface Water Runoff Rate

The existing hardstanding area to be drained has been determined to be 0.21ha.

The existing surface water runoff from the site are unknown. Using the Modified Rational Method, the surface water discharge was estimated for the existing site:

File Note

268265-00

15 October 2020

$$Q=2.78C_v C_r i A$$

Where:

Q=Peak discharge (/s)

C_v =Volumetric runoff coefficient

C_r =Dimensionless routing coefficient

i=Rainfall Intensity (mm/hr)

A=Catchment area (ha)

The Wallingford Procedure recommends that when used in urban catchments the following values can be used:

$C_v=0.75$ (range between 0.6 to 0.9)

$C_r=1.3$

The existing total site area is approximately 0.21ha and is assumed 100% impermeable. Therefore, the existing site runoff rate for the 100-year storm is calculated as:

A=0.21ha

i=88.14mm/hr (for 100-year return period 6 hour duration)

On this basis the pre-development runoff rate, Q, for the 1 in 100-year return period storm event of 30-minute duration is estimated to be 50.17l/s. Table 1 below presents the surface water run-off rates for the existing site for various rainfall events.

Table 1 Surface water run-off rates for different return periods

Return period	Rainfall intensity mm/hr	Existing Run-off Rate l/s
1 year	26.36	15.00
30 year	63.19	35.97
100-year	88.14	50.17

2.2 Proposed Drainage Strategy

The proposed development will have the same impermeable area as the existing site. In order to comply with the guidance outlined by LBC, it is proposed to attenuate surface water runoff in green/blue roofs and in below ground attenuation tank. The surface water runoff discharged from the site to the existing combined sewer owned by Thames Water will be through the use of a vortex flow control in the final surface water manhole.

The flow control device has been designed to limit the surface water runoff from the site to a maximum 14l/s for all rainfall events up to and including 100 year return period + 30% climate change allowance.

Condition 21 states that 23m³ of attenuation needs to be provided in blue or green roofs and 47m³ of attenuation will have to be provided below ground. Using MicroDrainage, the proposed surface

File Note

268265-00

15 October 2020

water drainage network was modelled based on the required attenuation volumes and the discharge flow rate stated in the Planning Condition 21.

The special requirements for attenuation have assumed, blue, green roof and the below ground attenuation as presented in Table 2:

Table 2 Attenuation breakdown for the proposed Site

Attenuation Type	Required area m2	Depth m	Storage coefficient	Volume m3
Blue Roof	220	0.1	90%	19.8
Green roof	54	0.08	84%	3.6
Below ground	75	0.66	96%	48
Total volume of attenuation provided				71.4

Table 3 provides a summary of the percentage reduction in surface water runoff between the existing and proposed site. The proposed run-off rates have calculated based on the FEH data available for the site.

Table 3 Existing and Proposed Surface Runoff Reduction

Return period	Existing Run-off Rate l/s	Proposed runoff rate l/s	Runoff reduction
1 year	15.00	6	60%
30 year	35.97	10	72.2%
100-year	50.17	14	72.1%
100-year+30%CC	65.22	14	78.5%

As it can be seen in Table 3, the proposed development will reduce the existing runoff rate to more than 50%. As it is presented in Appendix B, MicroDrainage modelling results show no flooding on site for any of the events, including 100year+30% CC event. Therefore, the requirements of Condition 21 have been met.

3 Maintenance Strategy

The ongoing maintenance and management of the proposed surface water drainage system will fall under the responsibility of the site owner. Best practice information is provided in the CIRIA SUDS Manual, excerpts of which have been included below for:

File Note

268265-00 15 October 2020

- Regular Maintenance
- Occasional Maintenance
- Remedial Actions

3.1 Green Roofs

The SUDS Manual 2015 Table 12.5 (below in Figure 10) gives a recommended maintenance schedule for green roofs. Following further design development of the green/blue roof solutions on the building roofs, the maintenance requirements and a detailed plan can be prepared.

TABLE 12.5 Operation and maintenance requirements for green roofs		
Maintenance schedule	Required action	Typical frequency
Regular inspections	Inspect all components including soil substrate, vegetation, drains, irrigation systems (if applicable), membranes and roof structure for proper operation, integrity of waterproofing and structural stability	Annually and after severe storms
	Inspect soil substrate for evidence of erosion channels and identify any sediment sources	Annually and after severe storms
	Inspect drain inlets to ensure unrestricted runoff from the drainage layer to the conveyance or roof drain system	Annually and after severe storms
	Inspect underside of roof for evidence of leakage	Annually and after severe storms
Regular maintenance	Remove debris and litter to prevent clogging of inlet drains and interference with plant growth	Six monthly and annually or as required
	During establishment (ie year one), replace dead plants as required	Monthly (but usually responsibility of manufacturer)
	Post establishment, replace dead plants as required (where > 5% of coverage)	Annually (in autumn)
	Remove fallen leaves and debris from deciduous plant foliage	Six monthly or as required
	Remove nuisance and invasive vegetation, including weeds	Six monthly or as required
	Mow grasses, prune shrubs and manage other planting (if appropriate) as required – clippings should be removed and not allowed to accumulate	Six monthly or as required
Remedial actions	If erosion channels are evident, these should be stabilised with extra soil substrate similar to the original material, and sources of erosion damage should be identified and controlled	As required
	If drain inlet has settled, cracked or moved, investigate and repair as appropriate	As required

Figure 1 Typical maintenance for green roofs (CIRIA SUDS Manual 2015)

3.2 Attenuation storage tank

Attenuation tanks are used to collect and store water prior discharging to the combined sewer on Adelaide Road. The SUDS Manual 2015 Table 21.3 (below in Figure 11) gives a recommended maintenance schedule for attenuation tanks:

File Note

268265-00

15 October 2020

TABLE 21.3 Operation and maintenance requirements for attenuation storage tanks			
Maintenance schedule	Required action	Typical frequency	
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually	
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly	
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary.	Annually	
	Remove sediment from pre-treatment structures and/or internal forebays	Annually, or as required	
Remedial actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required	
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually	
	Survey inside of tank for sediment build-up and remove if necessary	Every 5 years or as required	

Figure 2 Typical maintenance of attenuation tanks (CIRIA SUDS Manual 2015)

3.3 Flow control device

The flow control device is located at the end of the system to restrict surface water flows discharging from the site. The maintenance schedule was compiled from the Hydro-International website. Typical maintenance is presented in Table 7.

Table 4 Typical maintenance of vortex flow control

Maintenance schedule	Required action	Frequency
Regular Maintenance	Debris removal (leaves, rubbish, tree branches) from the drainage network	Monthly
Remedial Actions	When flooding occurs due to blockages in the drainage network, drain down manhole(s) and jet pipes to remove blockages.	As required
Monitoring	Inspect unit and hose down if required	Monthly for the first three months; Every six months after

3.4 Gullies

Inspection and removal of debris from silt trap once a year, preferable after the leaf fall in autumn.

File Note

268265-00

15 October 2020

3.5 Drainage pipes, manholes & silt traps

Inspect manholes & silt traps for build-up of silt and general debris (once a year, preferably after leaf fall in the autumn). If silt/debris is building up, a regular clean with jetting lorry / gully sucker is required. Pipe inspection after jetting should be undertaken and repeat cleaning if required. If the pipes to be jetted are plastic then a high flow, low pressure setting should be used so that the pipes are not damaged.

NOTE: Manhole covers can be heavy and suitable lifting equipment / procedures should be used. Untrained personnel should not enter manholes to carry out maintenance.

File Note

268265-00

15 October 2020

Appendix A – Drainage Strategy Layout

File Note

268265-00

15 October 2020

Appendix B - MicroDrainage Results

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Page 3

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XP Solutions

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Network 2020.1

Micro Drainage

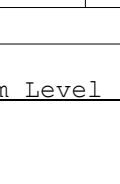
Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S8.001	4.176	8.550	0.5	0.000	0.00	0.0	0.600	o	100	Pipe/Conduit	
S8.002	10.359	0.207	50.0	0.017	0.00	0.0	0.600	o	150	Pipe/Conduit	
S7.003	9.259	0.579	16.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
S1.003	8.140	0.099	82.2	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.004	2.569	0.026	98.8	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.005	15.238	0.190	80.2	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S8.001	159.48	6.75	39.500	0.004	0.0	0.0	0.0	11.17	87.7	1.7
S8.002	158.29	6.87	30.900	0.021	0.0	0.0	0.0	1.43	25.2	9.1
S7.003	157.70	6.93	30.693	0.055	0.0	0.0	0.0	2.53	44.7	23.4
S1.003	155.02	7.22	30.039	0.201	0.0	0.0	0.0	1.44	57.4«	84.6
S1.004	154.72	7.25	29.940	0.201	0.0	0.0	0.0	1.32	52.3«	84.6
S1.005	153.15	7.42	29.914	0.201	0.0	0.0	0.0	1.46	58.1«	84.6

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1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coefficient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	7
Number of Online Controls	8	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.437
Region	England and Wales	Cv (Summer)	0.750
M5-60 (mm)	20.600	Cv (Winter)	0.840

Margin for Flood Risk Warning (mm)	300.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	OFF
DVD Status	ON
Inertia Status	OFF

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480
Return Period(s) (years)	1, 10, 30, 100
Climate Change (%)	0, 0, 0, 30

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surchage	First (Y) Flood	First (Z) Overflow
S1.000	SMH01	15 Winter	1	+0%	100/15 Summer		
S2.000	SRoofBlueN.1	360 Winter	1	+0%			
S2.001	SOrificeN.1	360 Winter	1	+0%			
S1.001	SMH002	15 Winter	1	+0%	100/15 Summer		
S3.000	SRoofBlueN.2	360 Winter	1	+0%			
S3.001	SOrificeN.2	360 Winter	1	+0%			
S1.002	SMH03	15 Winter	1	+0%	30/30 Winter		
S4.000	SMH04	15 Winter	1	+0%	100/15 Summer		
S4.001	SMH05	15 Winter	1	+0%	30/15 Summer		
S5.000	SRoodBlueS.1	180 Winter	1	+0%			
S5.001	SOrificeS.1	180 Winter	1	+0%			
S5.002	SMH06	15 Winter	1	+0%	100/15 Summer		
S4.002	SPump station	15 Winter	1	+0%	10/15 Summer		
S4.003	SMH08	15 Winter	1	+0%	10/15 Summer		
S6.000	SRoofBlueS.2	240 Winter	1	+0%			
S6.001	SOrificeS.2	240 Winter	1	+0%			
S4.004	SMH09	15 Winter	1	+0%	10/15 Summer		
S4.005	SMH10	15 Winter	1	+0%	10/15 Summer		

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30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

			Water	Surcharged	Flooded			Half Drain	Pipe
	US/MH	Overflow	Level	Depth	Volume	Flow /	Overflow	Time	Flow
PN	Name	Act.	(m)	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)
S1.000	SMH01		30.596	-0.054	0.000	0.72			10.1
S2.000	SRoofBlueN.1		39.876	-0.074	0.000	0.06		177	0.2
S2.001	SOrificeN.1		39.875	-0.075	0.000	0.00			0.2
S1.001	SMH002		30.516	-0.064	0.000	0.61			10.1
S3.000	SRoofBlueN.2		39.877	-0.073	0.000	0.06			0.2
S3.001	SOrificeN.2		39.875	-0.075	0.000	0.00			0.2
S1.002	SMH03		30.422	0.011	0.000	0.44			7.1
S4.000	SMH04		30.613	-0.012	0.000	0.10			4.9
S4.001	SMH05		30.607	0.105	0.000	0.25			13.0
S5.000	SRoodBlueS.1		39.522	-0.078	0.000	0.04		102	0.2
S5.001	SOrificeS.1		39.522	-0.078	0.000	0.00			0.2
S5.002	SMH06		30.598	-0.052	0.000	0.23			8.8
S4.002	SPump station		30.586	0.317	0.000	2.41			20.0
S4.003	SMH08		31.827	0.177	0.000	1.21			20.0
S6.000	SRoofBlueS.2		39.523	-0.077	0.000	0.04		105	0.2
S6.001	SOrificeS.2		39.522	-0.078	0.000	0.00			0.2
S4.004	SMH09		31.577	0.099	0.000	1.40			20.7
S4.005	SMH10		30.510	0.261	0.000	1.35			20.2

PN	US/MH	Status	Level
	Name		Exceeded
S1.000	SMH01	OK	
S2.000	SRoofBlueN.1	FLOOD RISK	
S2.001	SOrificeN.1	FLOOD RISK	
S1.001	SMH02	OK	
S3.000	SRoofBlueN.2	FLOOD RISK	
S3.001	SOrificeN.2	FLOOD RISK	
S1.002	SMH03	SURCHARGED	
S4.000	SMH04	OK	
S4.001	SMH05	SURCHARGED	
S5.000	SRoodBlueS.1	OK	
S5.001	SOrificeS.1	OK	
S5.002	SMH06	OK	
S4.002	SPump station	SURCHARGED	
S4.003	SMH08	FLOOD RISK	
S6.000	SRoofBlueS.2	OK	
S6.001	SOrificeS.2	OK	
S4.004	SMH09	SURCHARGED	
S4.005	SMH10	SURCHARGED	

100 year Return Period Summary of Critical Results by Maximum Level (Rank
1) for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	7
Number of Online Controls	8	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.437
Region	England and Wales	Cv (Summer)	0.750
M5-60 (mm)	20.600	Cv (Winter)	0.840

Margin for Flood Risk Warning (mm)	300.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	OFF
DVD Status	ON
Inertia Status	OFF

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480
Return Period(s) (years)	1, 10, 30, 100
Climate Change (%)	0, 0, 0, 30

US/MH			Return	Climate	First (X)	First (Y)	First (Z)
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow
S1.000	SMH01	60 Winter	100	+30%	100/15	Summer	
S2.000	SRoofBlueN.1	120 Winter	100	+30%			
S2.001	SOrificeN.1	120 Winter	100	+30%			
S1.001	SMH002	60 Winter	100	+30%	100/15	Summer	
S3.000	SRoofBlueN.2	120 Winter	100	+30%			
S3.001	SOrificeN.2	120 Winter	100	+30%			
S1.002	SMH03	60 Winter	100	+30%	30/30	Winter	
S4.000	SMH04	15 Winter	100	+30%	100/15	Summer	
S4.001	SMH05	15 Winter	100	+30%	30/15	Summer	
S5.000	SRoodBlueS.1	60 Winter	100	+30%			
S5.001	SOrificeS.1	60 Winter	100	+30%			
S5.002	SMH06	15 Winter	100	+30%	100/15	Summer	
S4.002	SPump station	15 Winter	100	+30%	10/15	Summer	
S4.003	SMH08	15 Winter	100	+30%	10/15	Summer	
S6.000	SRoofBlueS.2	60 Winter	100	+30%			
S6.001	SOrificeS.2	60 Winter	100	+30%			
S4.004	SMH09	15 Winter	100	+30%	10/15	Summer	
S4.005	SMH10	60 Winter	100	+30%	10/15	Summer	

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

			Water	Surcharged	Flooded			Half Drain	Pipe
	US/MH	Overflow	Level	Depth	Volume	Flow /	Overflow	Time	Flow
PN	Name	Act.	(m)	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)
S1.000	SMH01		30.986	0.336	0.000	0.62			8.7
S2.000	SRoofBlueN.1		39.890	-0.060	0.000	0.13		115	0.5
S2.001	SOrificeN.1		39.888	-0.062	0.000	0.01			0.5
S1.001	SMH002		30.980	0.400	0.000	0.52			8.6
S3.000	SRoofBlueN.2		39.891	-0.059	0.000	0.14		110	0.5
S3.001	SOrificeN.2		39.889	-0.061	0.000	0.01			0.5
S1.002	SMH03		30.970	0.559	0.000	0.67			10.8
S4.000	SMH04		31.749	1.124	0.000	0.10			4.8
S4.001	SMH05		31.743	1.241	0.000	0.28			14.7
S5.000	SRoodBlueS.1		39.534	-0.066	0.000	0.11		64	0.4
S5.001	SOrificeS.1		39.534	-0.066	0.000	0.01			0.4
S5.002	SMH06		31.734	1.084	0.000	0.29			10.9
S4.002	SPump station		31.721	1.452	0.000	2.41			20.0
S4.003	SMH08		31.848	0.198	0.000	1.21			20.1
S6.000	SRoofBlueS.2		39.534	-0.066	0.000	0.10		66	0.4
S6.001	SOrificeS.2		39.533	-0.067	0.000	0.01			0.4
S4.004	SMH09		31.599	0.121	0.000	1.46			21.6
S4.005	SMH10		30.996	0.747	0.000	1.38			20.6

PN	US/MH Name	Status	Level Exceeded
S1.000	SMH01	SURCHARGED	
S2.000	SRoofBlueN.1	FLOOD RISK	
S2.001	SRificeN.1	FLOOD RISK	
S1.001	SMH02	SURCHARGED	
S3.000	SRoofBlueN.2	FLOOD RISK	
S3.001	SRificeN.2	FLOOD RISK	
S1.002	SMH03	SURCHARGED	
S4.000	SMH04	FLOOD RISK	
S4.001	SMH05	FLOOD RISK	
S5.000	SRoodBlueS.1	OK	
S5.001	SRificeS.1	OK	
S5.002	SMH06	FLOOD RISK	
S4.002	SPump station	FLOOD RISK	
S4.003	SMH08	FLOOD RISK	
S6.000	SRoofBlueS.2	OK	
S6.001	SRificeS.2	OK	
S4.004	SMH09	SURCHARGED	
S4.005	SMH10	SURCHARGED	

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH		Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
	Name	Storm						
S7.000	SRoofGreen	N 60	Winter	100	+30%			
S7.001	SOrifice	GN 60	Winter	100	+30%			
S7.002		SMH11 15	Winter	100	+30%			
S8.000	SRoofGreen	.2 30	Winter	100	+30%			
S8.001	SOrifice	G2 30	Winter	100	+30%			
S8.002		SMH12 15	Winter	100	+30%			
S7.003		SMH13 60	Winter	100	+30%	100/60	Winter	
S1.003		SMH14 60	Winter	100	+30%	10/15	Winter	
S1.004		SMH15 30	Winter	100	+30%			
S1.005		SMH16 60	Summer	100	+30%			

		Water	Surcharged	Flooded		Half Drain	Pipe
	US/MH	Level	Depth	Volume	Flow / Overflow	Time	Flow
PN	Name	(m)	(m)	(m³)	Cap.	(l/s)	(l/s)
S7.000	SRoofGreenN	39.530	-0.070	0.000	0.07	46	0.3
S7.001	SOrificeGN	39.528	-0.072	0.000	0.00		0.3
S7.002	SMH11	31.580	-0.070	0.000	0.56		20.7
S8.000	SRoofGreen.2	39.552	-0.048	0.000	0.25	28	0.7
S8.001	SOrifice G2	39.547	-0.053	0.000	0.01		0.7
S8.002	SMH12	30.976	-0.074	0.000	0.51		11.4
S7.003	SMH13	30.969	0.126	0.000	0.40		15.8
S1.003	SMH14	30.955	0.691	0.000	0.32	57	14.0
S1.004	SMH15	30.048	-0.117	0.000	0.47		14.0
S1.005	SMH16	29.994	-0.145	0.000	0.27		14.0

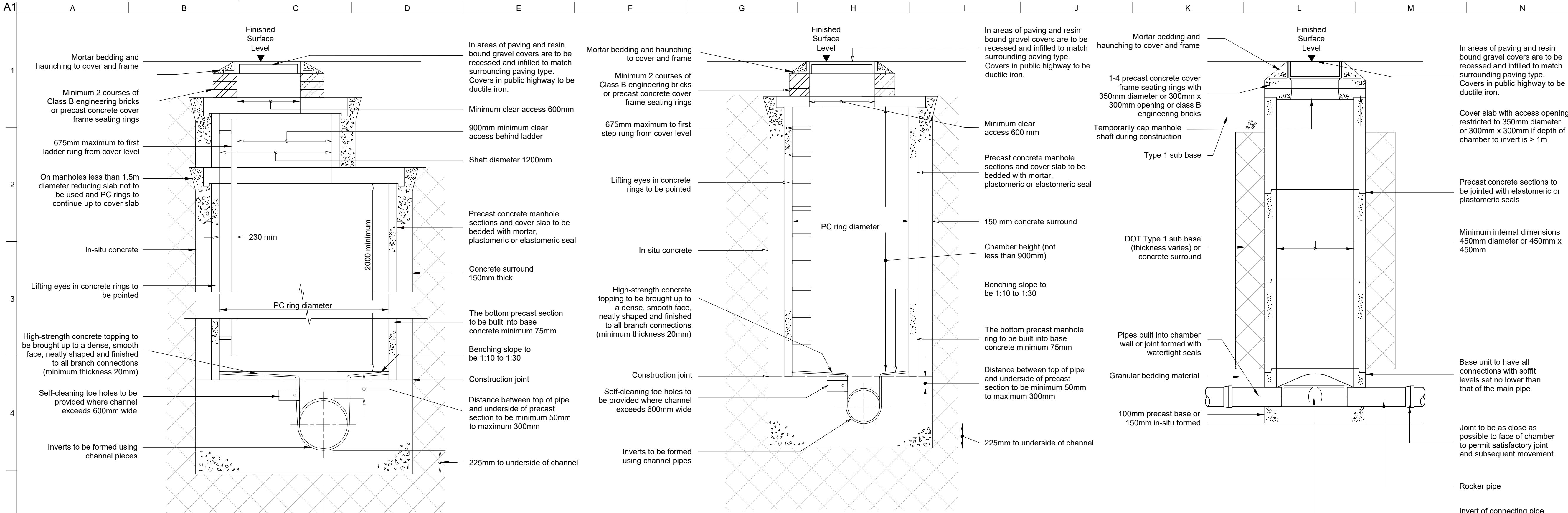
	US/MH	Level
PN	Name	Status Exceeded
S7.000	SRoofGreenN	OK
S7.001	SRorificeGN	OK
S7.002	SMH11	OK
S8.000	SRoofGreen.2	OK
S8.001	SRorifice G2	OK
S8.002	SMH12	OK
S7.003	SMH13	SURCHARGED
S1.003	SMH14	SURCHARGED
S1.004	SMH15	OK
S1.005	SMH16	OK

File Note

268265-00

15 October 2020

Appendix C - Drainage Details



- NOTES**
1. All dimensions are in millimetres unless otherwise noted.
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P01	23/10/20	TB	RO	TMcD
For Information				
Issue	Date	By	Chkd	Appd

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Client
Best Star Real Estate 2

Job Title
Haverstock Hill

Key Plan

Drawing Title
Proposed Drainage Details Sheet 1

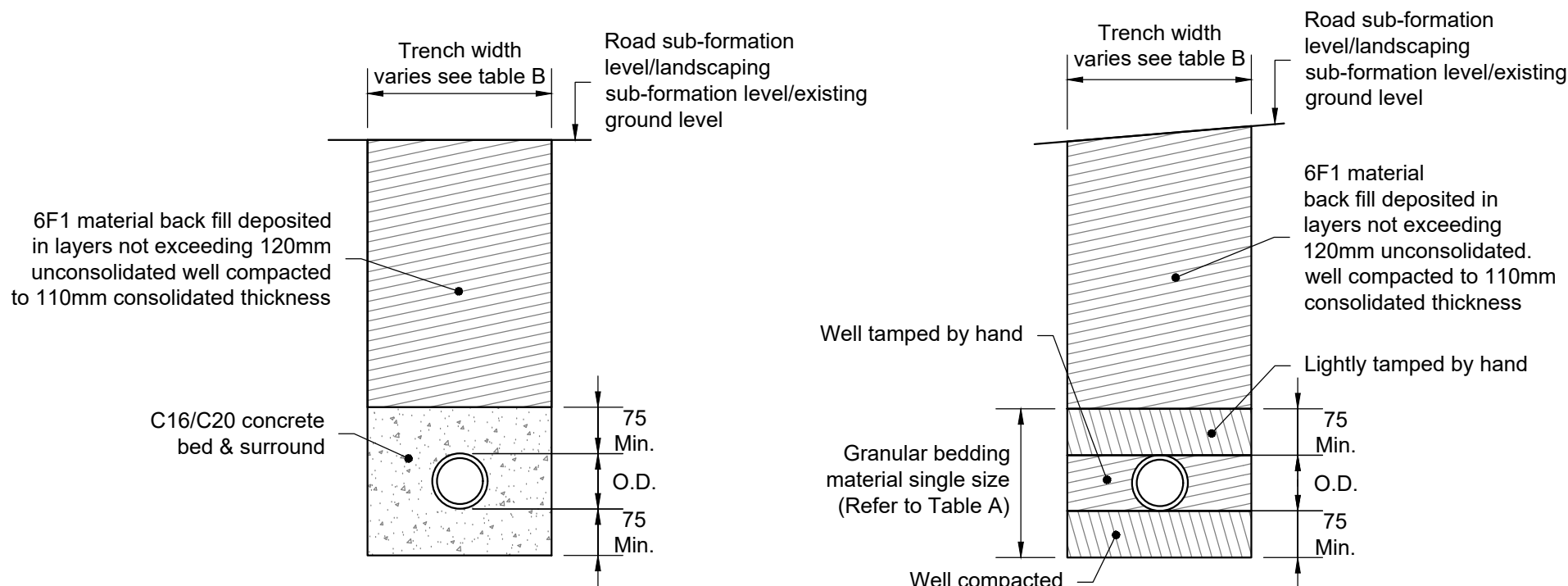
Scale at A1
1:200

Discipline
Civil

Job No
268265-00

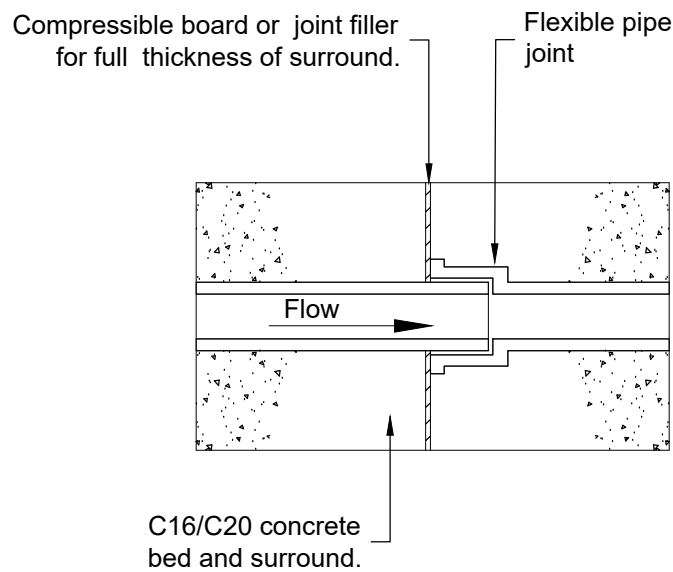
Drawing No
HH-ARP-DR-C-1010

Drawing Status
For Information

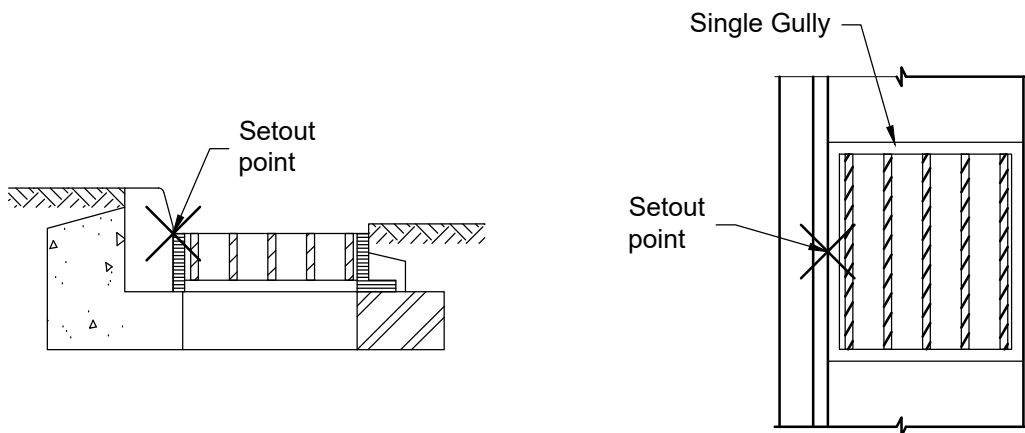


Class 'A' drainage pipe concrete bedding and surround detail

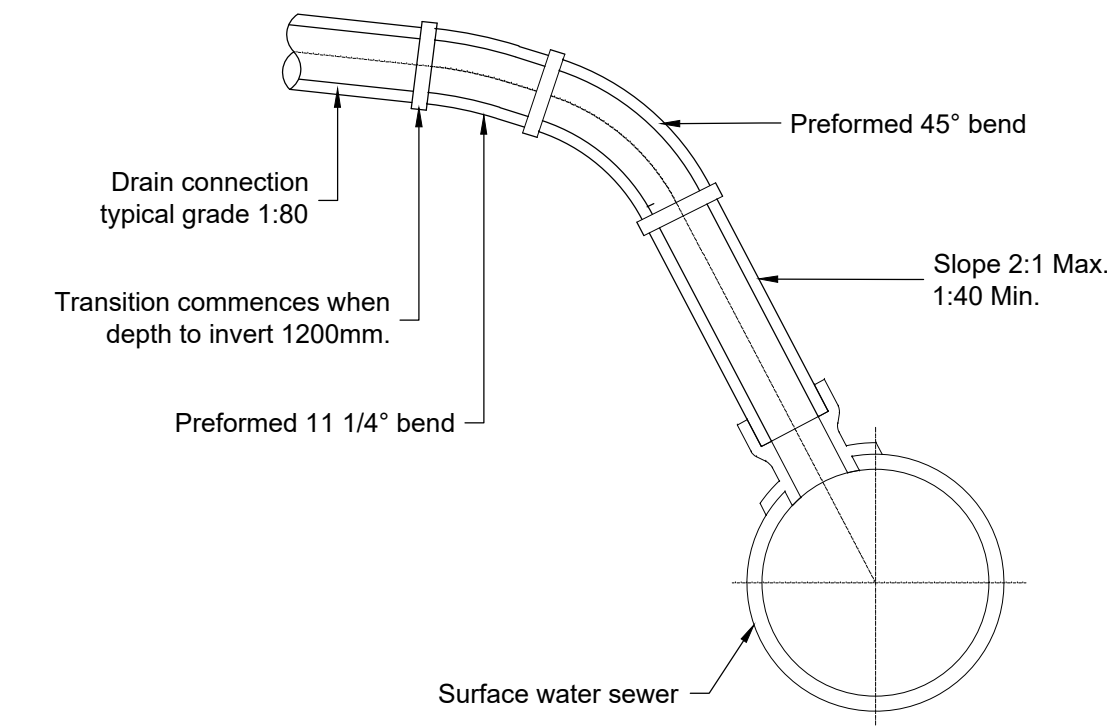
Class 'S' drainage pipe bedding detail



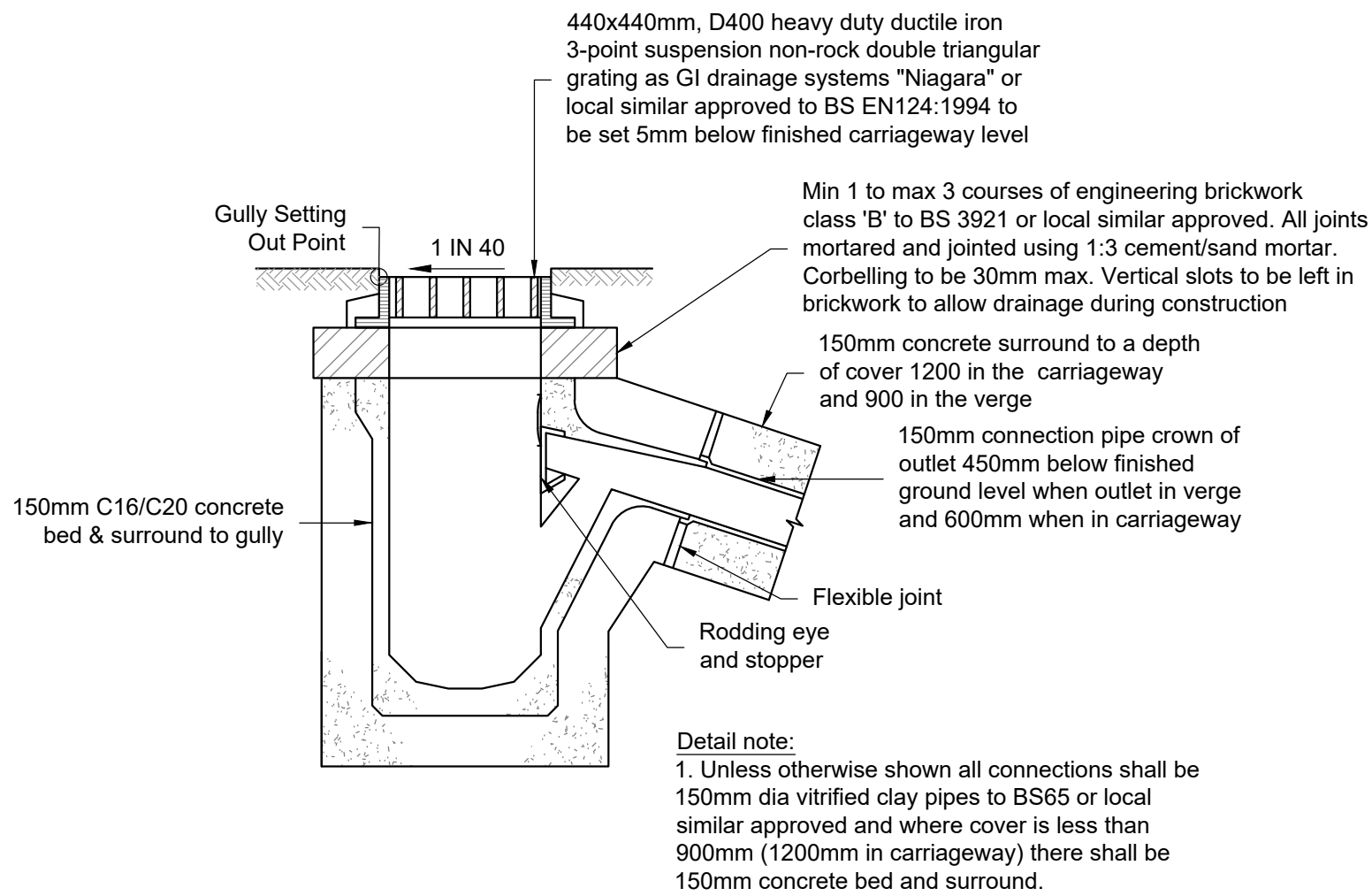
Flexible Joint for Concrete Surround Pipe Bedding and Surround



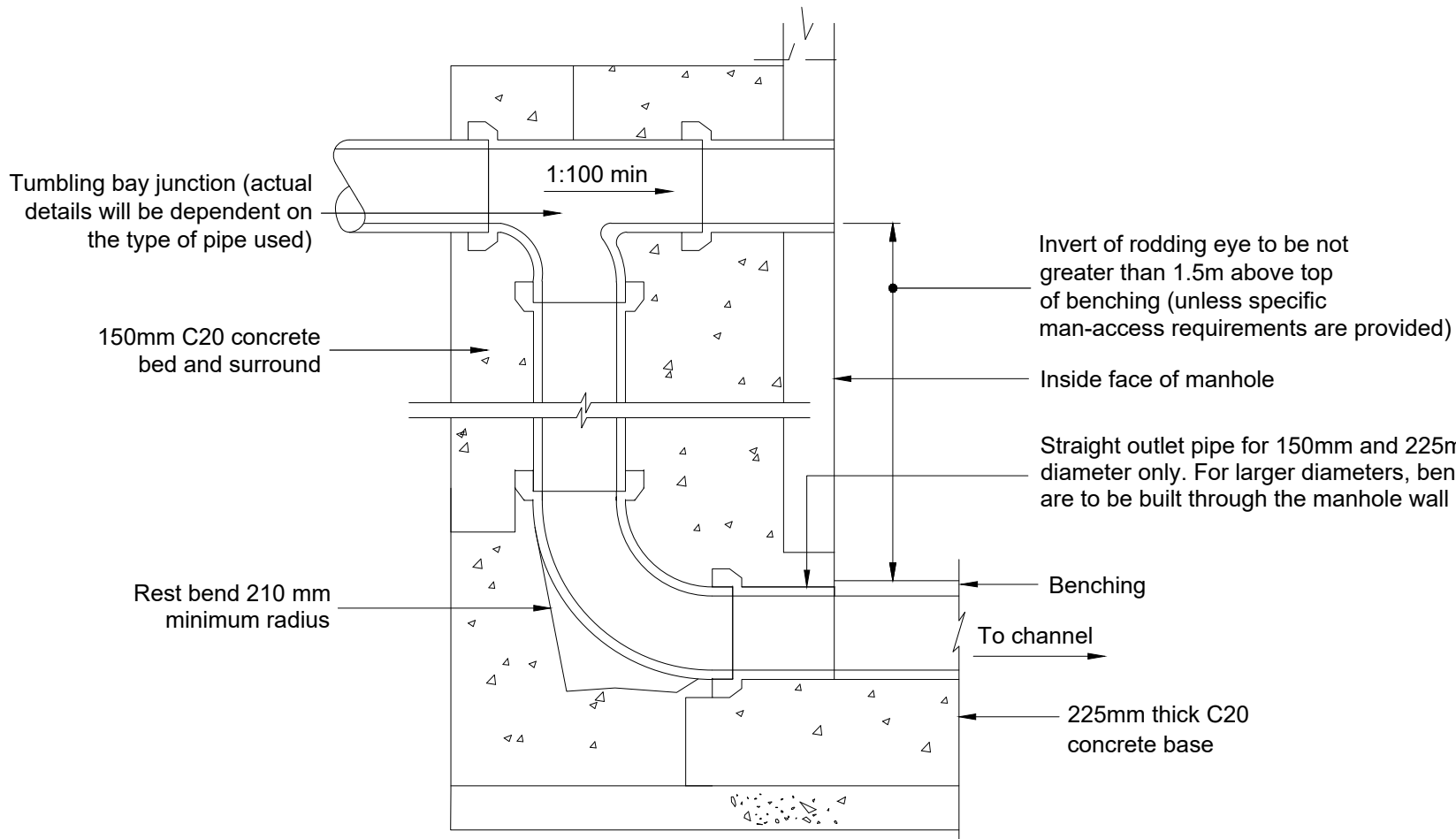
Typical Gully Setting out Point



Typical Gully Saddle Connection Detail



Surface Water Gully Detail



External Vertical Backdrop Detail

Pipe Dia.(mm)	Trench Width	
	Max	Min
150	600	490
225	700	580
300	750	680
375	1050	950
450	1150	1030
525	1200	1120
600	1350	1240
675	1450	1330
750	1500	1400
825	1600	1490
900	2100	1900
975	2150	1950
1050	2250	2050
1200	2500	2250
OVER 1200	O.D.+1000	O.D.+800

TABLE A
Note:
All excavation in excess of those stated shall be backfilled with pipe bedding material

Pipe Size	<300	300-800	>800
Sieve size	% by Mass passing	% by Mass passing	% by Mass passing
25mm	-	-	100
20mm	-	100	90-100
14mm	90-100	80-100	50-80
10mm	50-85	60-85	40-70
5mm	10-40	20-55	25-60
2.36mm	0-10	10-30	10-40
0.300mm	-	0-10	0-15

TABLE B

Use of Material	Sand Grading (100% < 2mm)				Organic content (%)
	<2µm	<20µm	<63µm	<250µm	
Sand for use in trenches ("sand in zandbed")		<=3 *	<=15		<=15

* Proportion of 63µm: 10% to 15%

TABLE C

Pipe Material	
<400mm	uPVC
>400mm	Concrete

TABLE D

NOTES

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Proposed Drainage
Details
Sheet 2

Scale at A1
1:200

Discipline
Civil

Job No
268265-00
Drawing No
HH-ARP-DR-C-1011

Drawing Status
For Information

P01

A1

A

B

C

D

E

F

G

H

I

J

K

L

M

N

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6

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8

9

10

11

Contractor to provide and install Flygt TOP Series package pumping station or similar approved. Pumping station to operate on a duty / standby basis pumping 20L/s over a static head of 2.4m and 6.5m total head. Pumping station to be installed in line with manufactures instructions.

Air vent in adjacent footpath

600 x 750mm clear opening with heavy duty access cover (B125). Exact location to be advised by pump manufacturer

Cable ducts for power supply and ultrasonic cables to be sealed on completion.

CL 32.00m

1200

Ø150mm MDPE SDR 17

Ductile iron/ MDPE coupling

Ø1500mm precast concrete manhole rings

150mm thick concrete designated mix RC 32/40 with A193 mesh reinforcement.

2 No. NP 3085 MT1 ~ Adaptive 462 Electric submersible drainage pumps or similar approved equivalent On Duty/Standby basis peak flow rate 20L/s.

Ø150mm dia MDPE SDR 17

IL 30.12m

1000

300

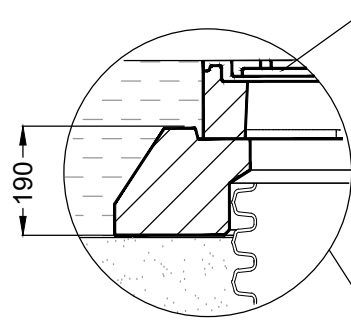
Concrete benching

50mm bedding mortar

300mm thick concrete base slab with A193 mesh reinforcement

75mm concrete blinding

Typical Pumping Station Detail
Not to Scale



Support frame and cover
Cast-in-place / precast concrete support collar. (If cast-in-place, ensure external shaft corrugations are not filled with concrete)

Access shaft
Stormbloc® optimum inspect

Road or footpath pavement build up to the Architects details

Finished Surface Level (31.90m)

1.2m

Stormbloc optimum system 1 bloc deep; 660mm height all blocks and inspect chambers =800mm long x 800mm wide

Geotextile fleece and 1mm polypropylene impermeable membrane to be installed around geocellular tank in accordance with C737 and manufacturers requirements

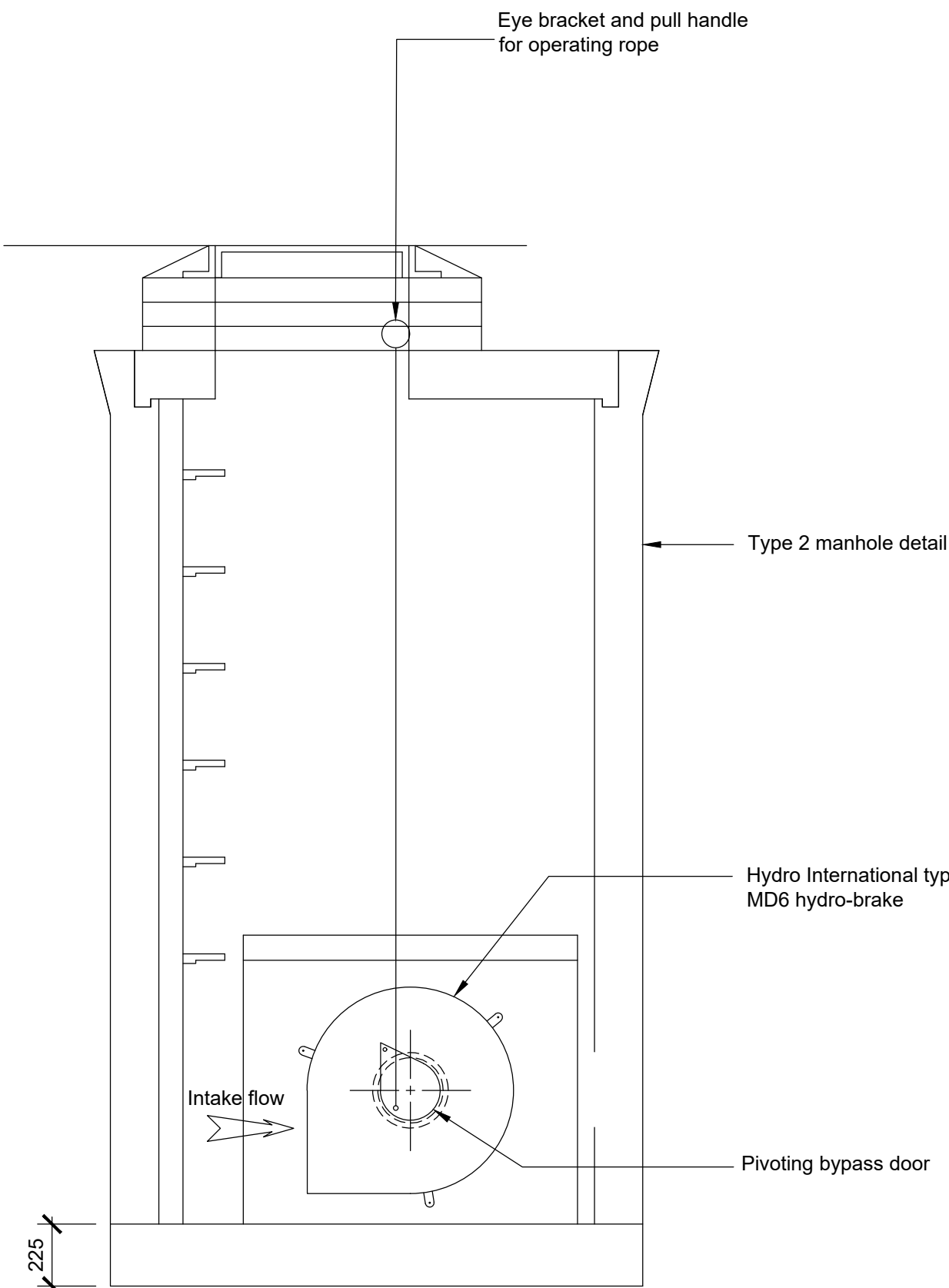
100mm thick minimum coarse sand/gravel surround prepared in accordance with C737

Existing Subgrade

IL 30.01m

Pipe connection + pipe adaptor

Typical Cellular Attenuation Tank Detail
Not to Scale



Section A-A

Eye bracket and pull handle for operating rope

Type 2 manhole detail

Hydro International type MD6 hydro-brake

Pivoting bypass door

225

Access to be positioned above pivoting by-pass door

Type 2 manhole detail

Fixing lugs (supplied with hydrobrake)

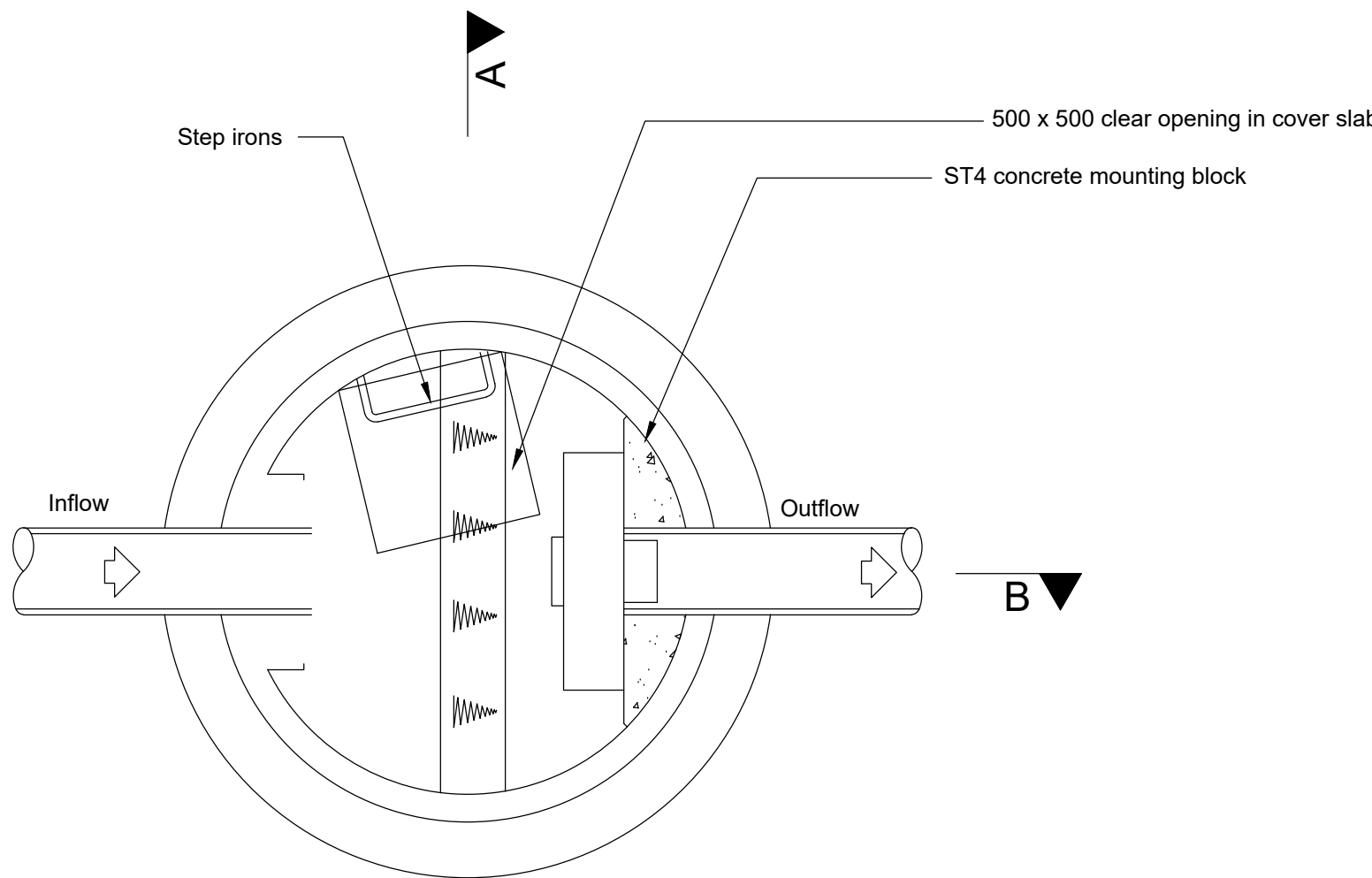
Neoprene rubber gasket (supplied with hydrobrake)

Inflow

Outflow

Sump

Section B-B



Section C-C

Hydro-Brake Detail for Outfall
Not to Scale

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Details
Sheet 3

Scale at A1

1:200

Discipline

Civil

Job No

268265-00

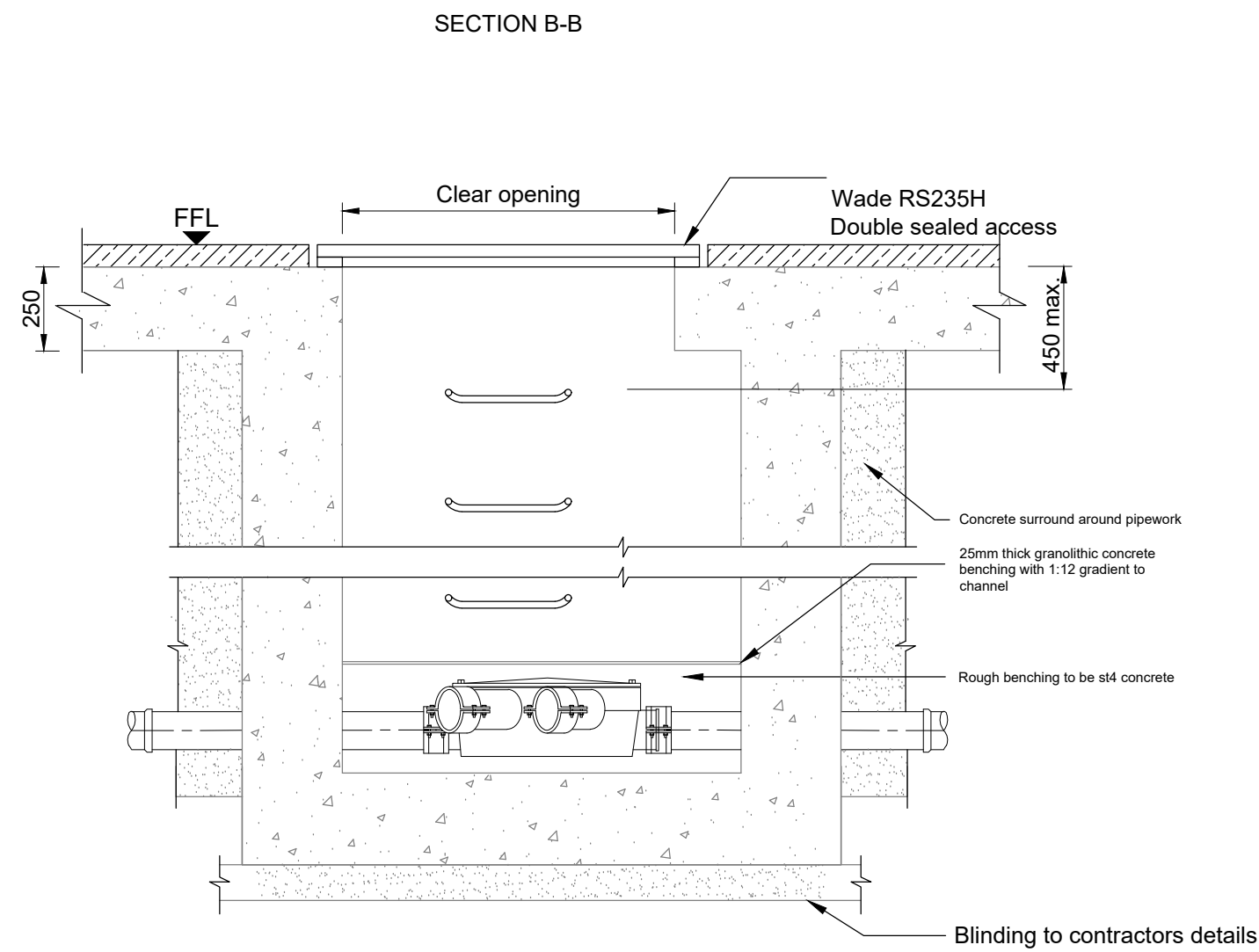
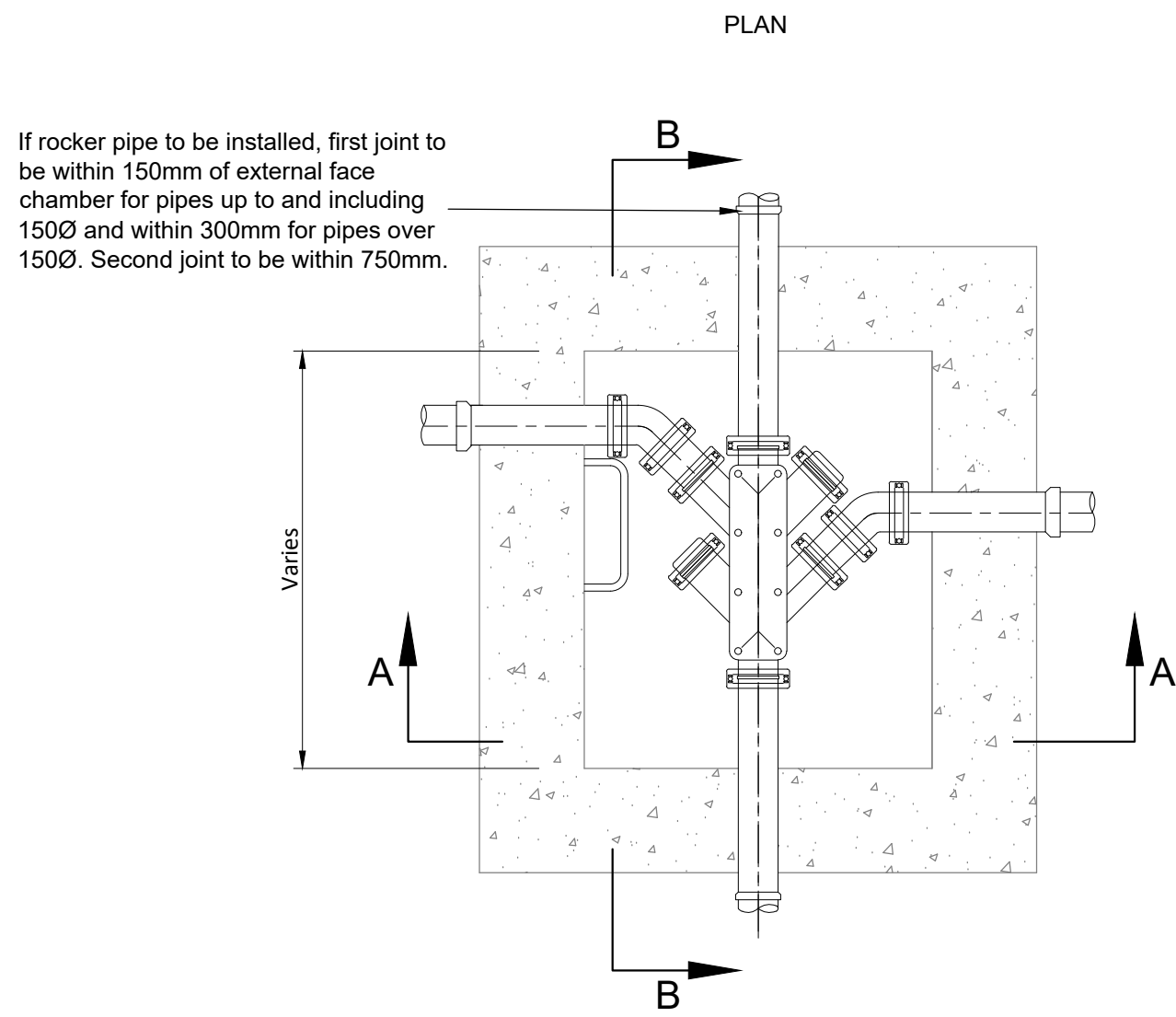
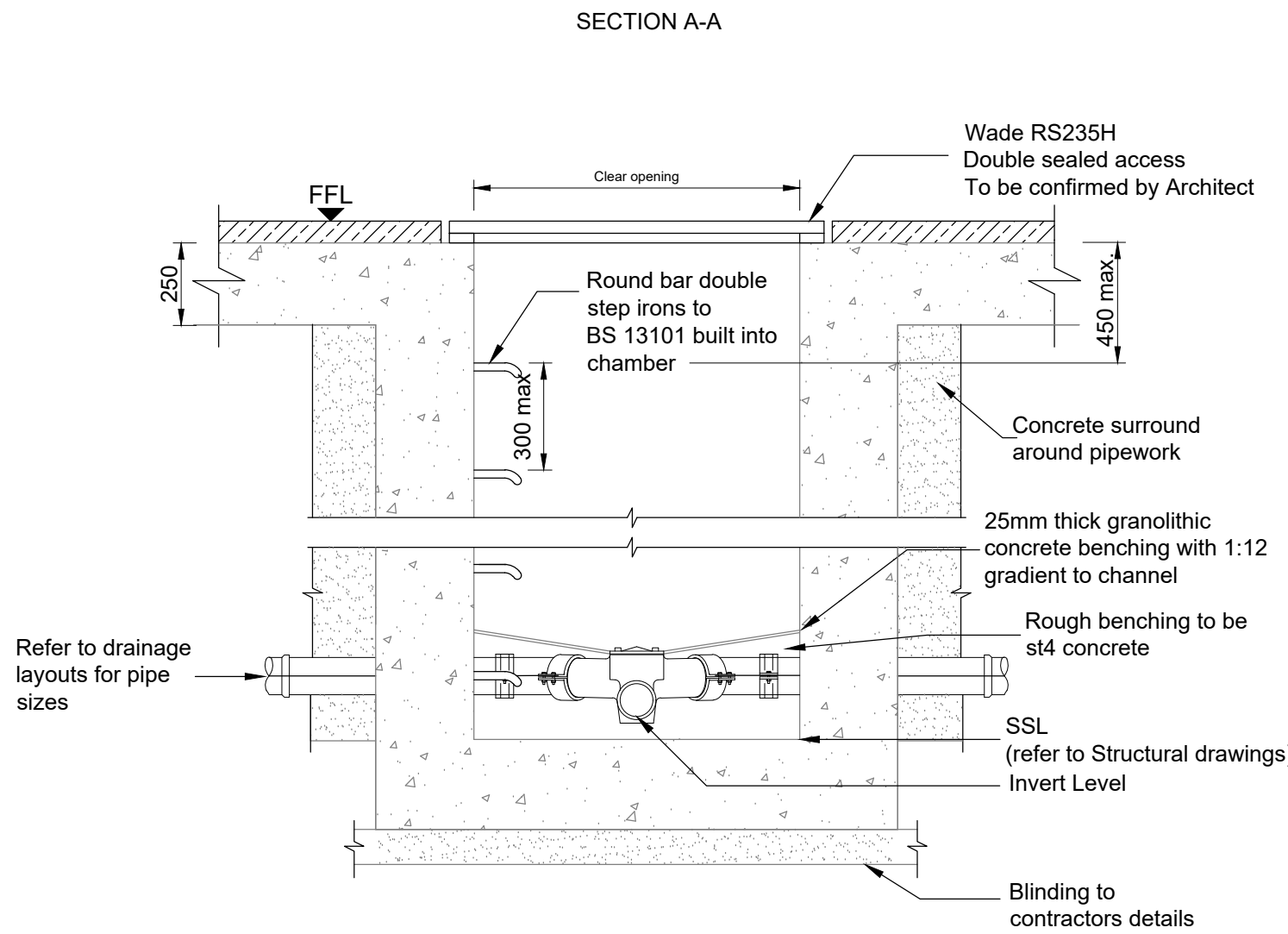
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For Information

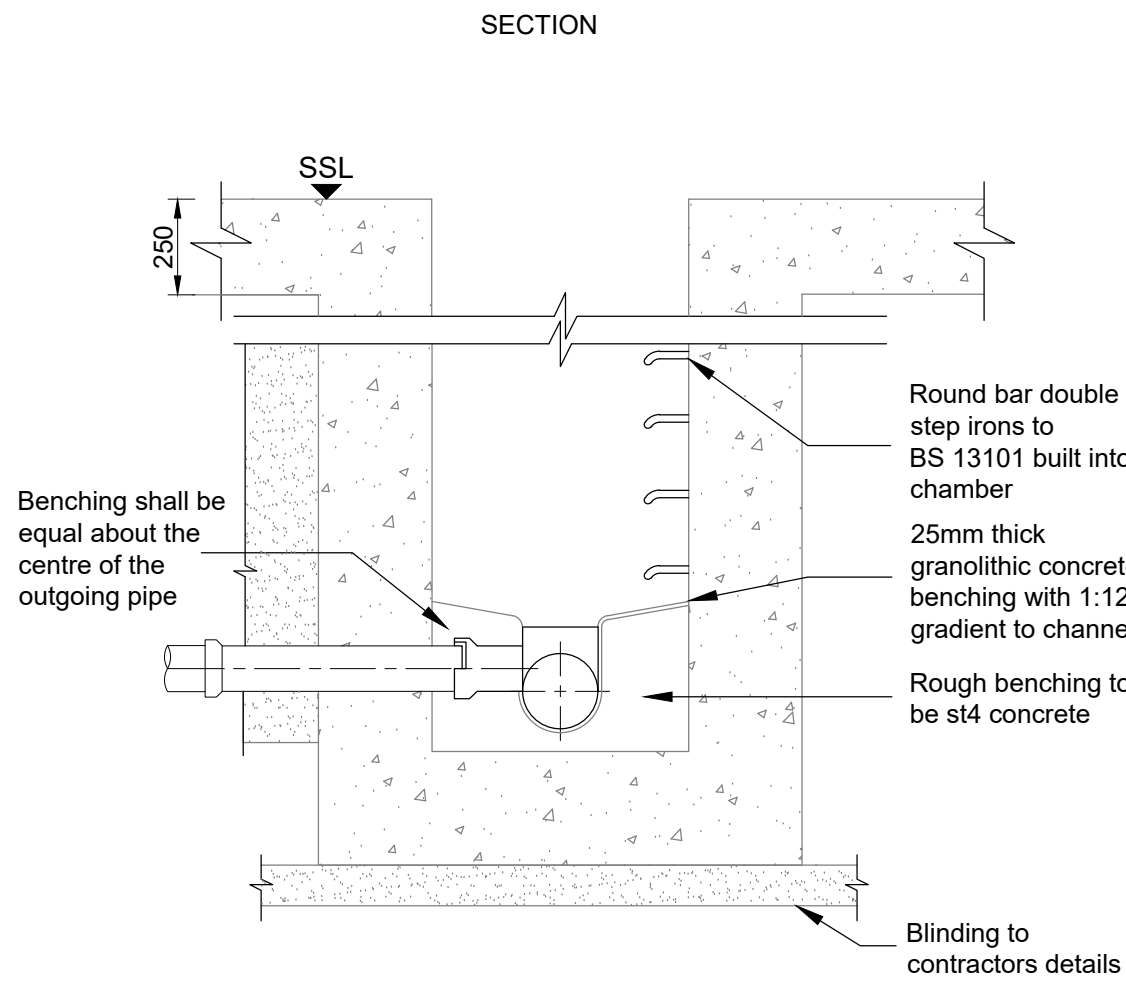
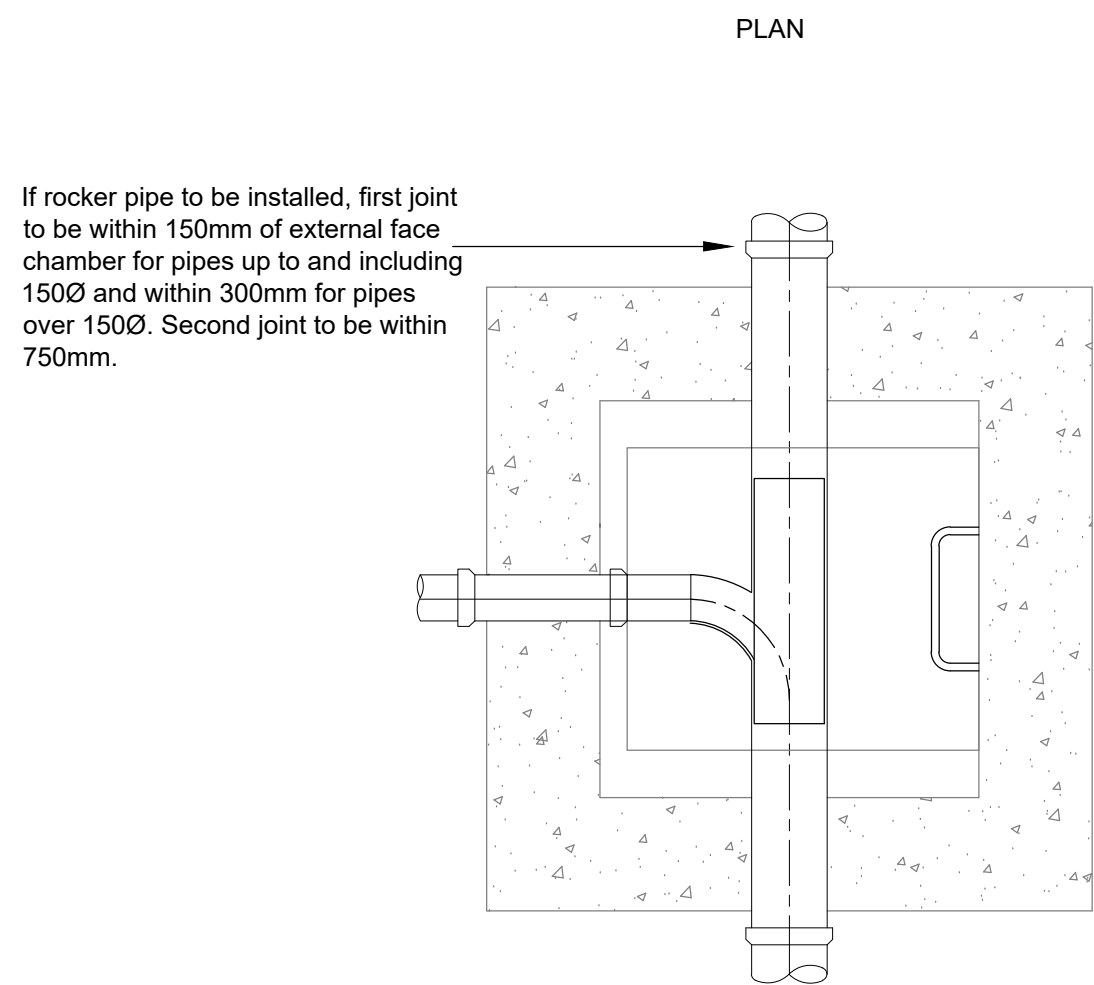
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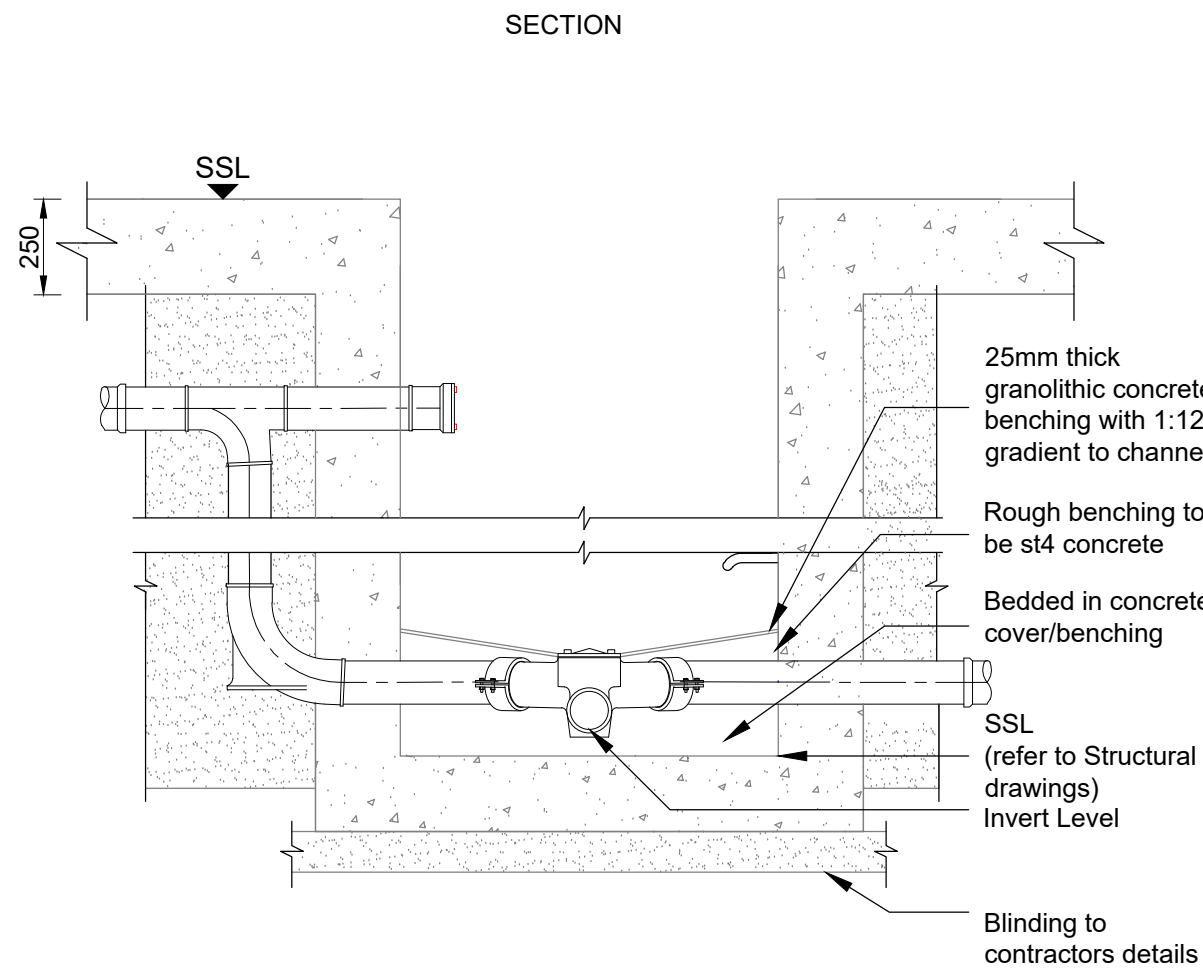
P01



TYPICAL INSITU MANHOLE WITH CAST IRON CHAMBER CONNECTIONS



INSITU MANHOLE WITH OPEN CHANNEL (LESS THAN 3.0M. TO TOP OF PIPE CROWN)



Max. Lower	Trench Width
Pipe Dia	Bd max
Ø100	525mm
Ø150	600mm

TYPICAL DETAIL VERTICAL BACKDROP INTO MANHOLE

- NOTES
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Drawing Title
Proposed Drainage Details Sheet 4

Scale at A1

1:200

Discipline

Civil

Job No

268265-00

Drawing Status

For Information

Drawing No

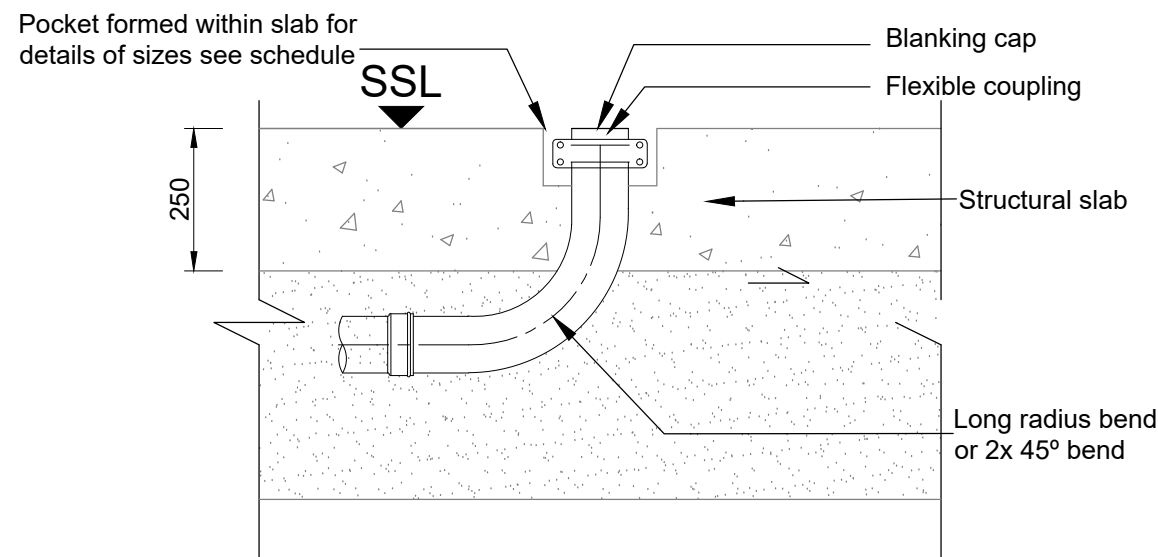
HH-ARP-DR-C-1013

P01

A1

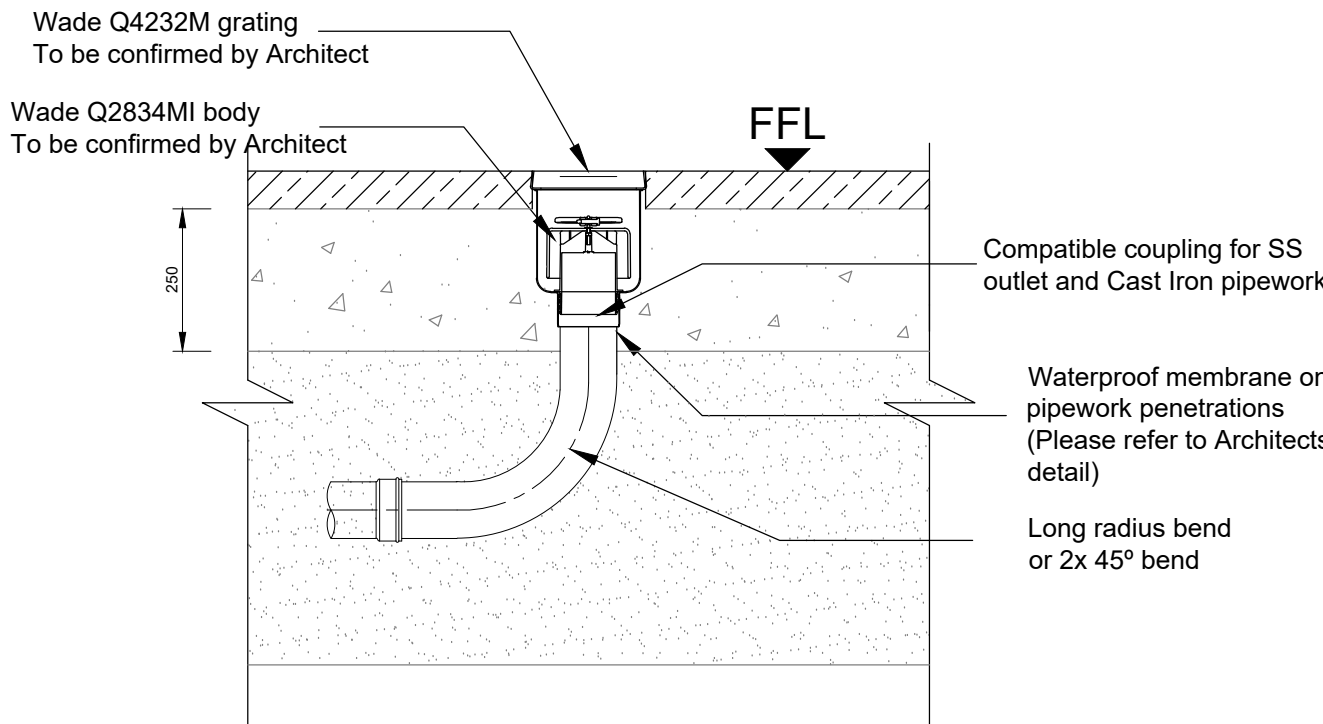
A B C D E F G H I J K L M N

1

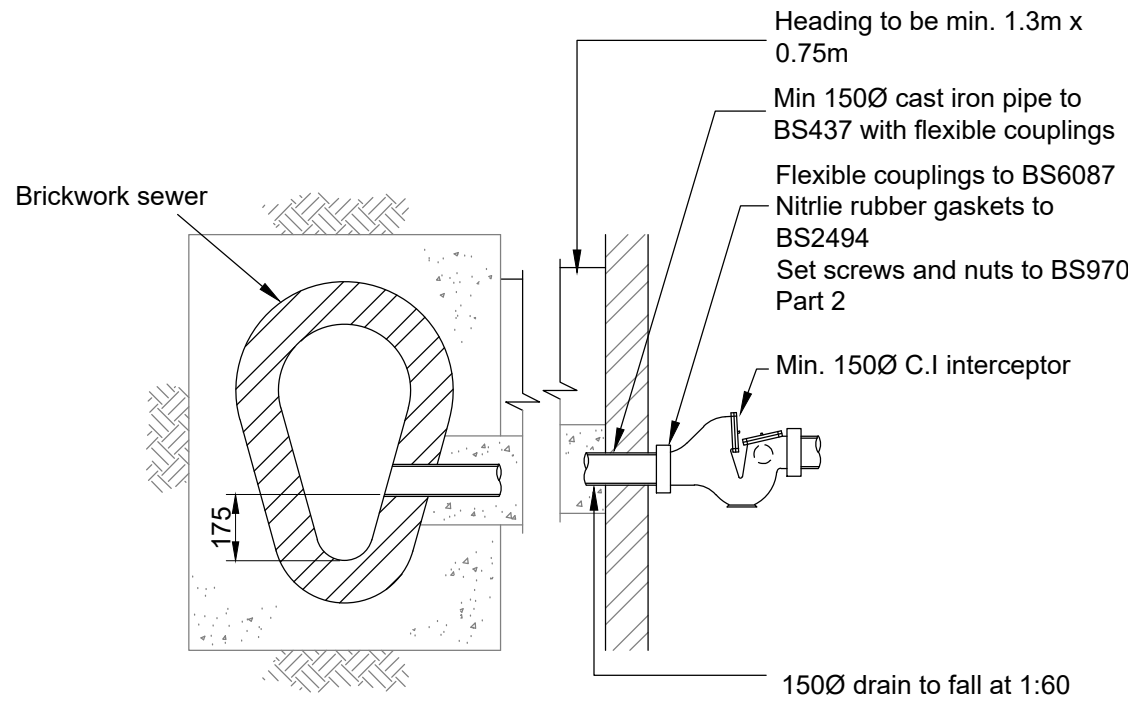


DRAIN SIZE DIA mm	POCKET SIZE		
	LENGTH mm	WIDTH mm	DEPTH mm
100	200	200	100
150	300	300	100

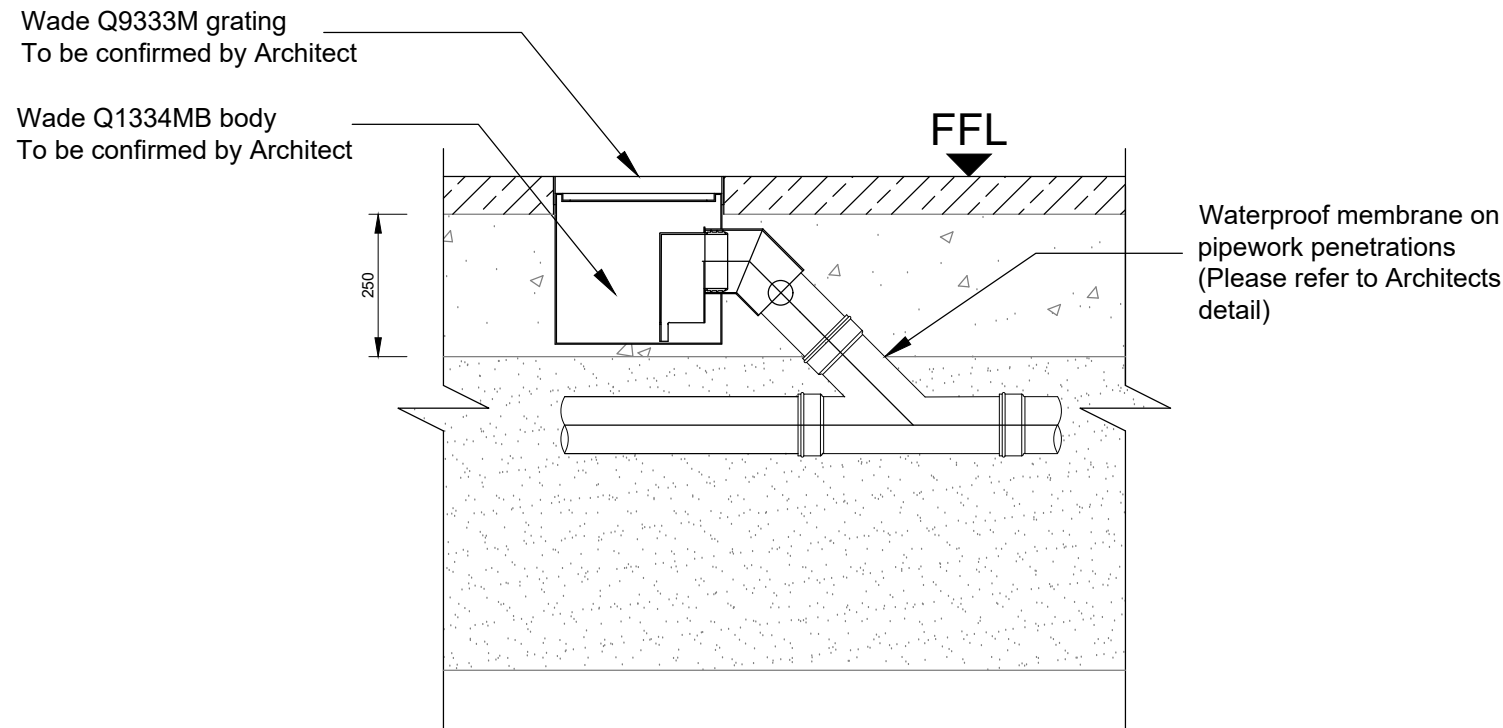
**TYPICAL CAST IN DRAIN PIPE PENETRATION
THROUGH SLAB FOR FUTURE
CONNECTIONS**



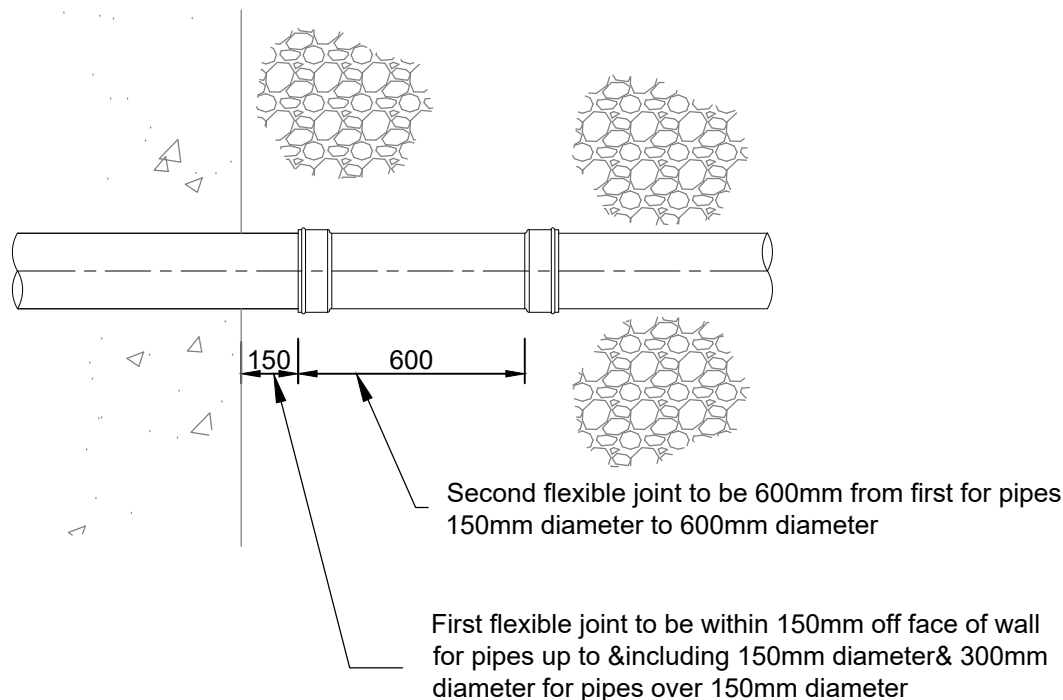
**TRAPPED FLOOR
GULLY**



**CONNECTION TO
CORPORATION OF
LONDON SEWER**

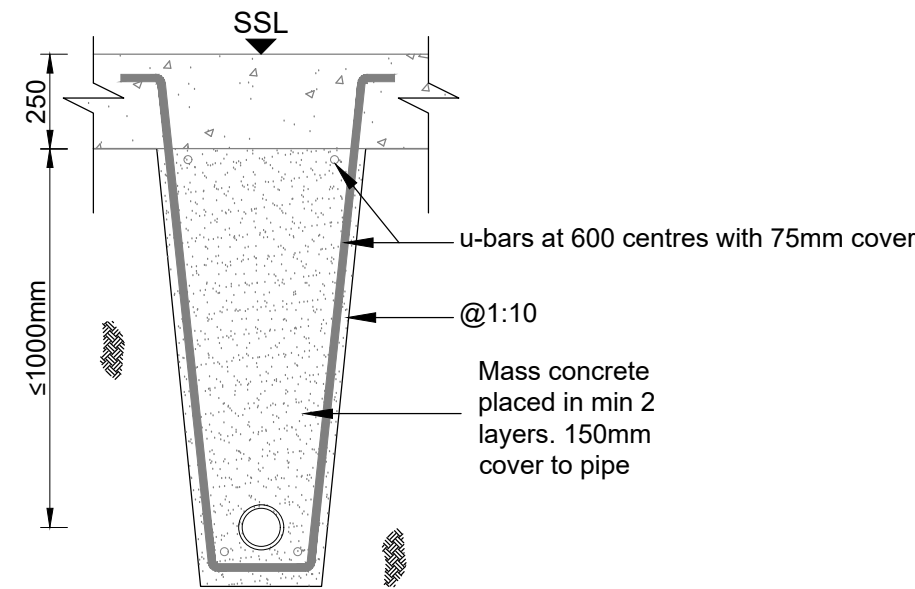


**TRAPPED FLOOR GULLY
FOR TOILET AREA**



Note:
Rocker pipes required to be installed at manholes where pipes pass through structures and where pipes leave concrete bed and surround.

**ROCKER PIPE
DETAIL**



Note:
Detail shows trenching for single pipe runs. Dimensions and reinforcing will vary for situations where more runs are combined.

**UNDERSLAB DRAINAGE TYPICAL SUPPORT
FOR SINGLE PIPE RUNS**

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**Proposed Drainage
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Sheet 5**

Scale at A1 1:200

Discipline Civil

Job No 268265-00 Drawing Status For Information

Drawing No HH-ARP-DR-C-1014 P01

File Note

268265-00

15 October 2020

DOCUMENT CHECKING (not mandatory for File Note)			
	Prepared by	Checked by	Approved by
Name	Raluca Olariu	Tristan McDonnell	Tristan McDonnell
Signature	