

SITE INVESTIGATION FACTUAL REPORT

Report No: [REDACTED]
Client: Crawford Claims Management
Site: 9 Willoughby Road
Client Ref: [REDACTED]
Date of Visit: 07/05/2020



Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys



[illegible]

TEST REPORT: Trial Pit

REPORT NUMBER: [REDACTED]

TRIAL PIT REF: TP1

CLIENT: Crawford & Co

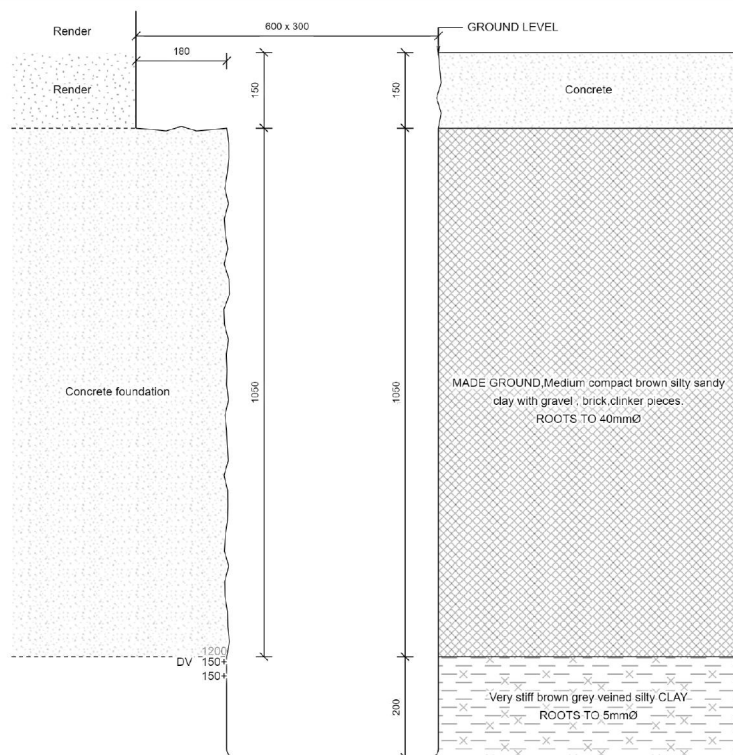
JOB NO: [REDACTED]

EXCAVATION METHOD: Hand tools

DATE: 07/05/2020

SITE: 9 Willoughby Road

WEATHER: Sunny



For Strata below 1400mm see Bore Hole log

Key:
D Small disturbed sample J Jar sample
B Bulk disturbed sample V Pilcon vane (kPa)
W Water sample M Mackintosh probe
TDTD Too dense to drive

Remarks:
Test results reported relate only to the items tested.
This report shall not be reproduced except in full without approval of the Laboratory.

For and on behalf of CET
Scott Alger - Lab

Report Format:

Approved Signatory
07-May-20

[REDACTED]

[REDACTED]

[REDACTED]

Report version 1

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[illegible]

Laboratory Summary Results

Our Ref : [REDACTED]

Location : 9 Willoughby Road
Client: Crawford Claims Management
Address: [REDACTED]

Date Sampled: 07/05/2020

Date Received : 12/05/2020

Date Tested : 12/05/2020

Date of Report : 19/05/2020

TP/BH No	Sample Ref Depth (m)	Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity Index [6]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (h)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated * Heave Potential (mm) [10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH * Value [13]	Sulphate Content * (g/l)		* Class [16]
																		SO ₃	SO ₄	
1	U/S 1.20	D	21	<5	58	19	39	0.05	39	CH	168	766			150					
	1.5	D	21	<5							168	615								
	2.0	D	22	<5	46	22	24	0.00	24	CI	168	280			130					
	2.5	D	24	<5							168	173								
	3.0	D	28	<5	55	27	28	0.03	28	CH	168	128			130					

Test Methods / Notes

[1] BS 1377: Part 2: 1990, Test No 3.2

[2] Test method 17-55, laboratory measured

[3] BS 1377: Part 2: 1990, Test No 4.4

[4] BS 1377: Part 2: 1990, Test No 5.1

[5] BS 1377: Part 2: 1990, Test No 5.4

[6] BS 1377: Part 2: 1990, Test No 5.4

[7] BS 5930: 2018, Figure 8 - Plasticity Chart for the classification

of fine soils

[8] In-house method S16 adapted from BS 1377: Part 2: 1990

[9] In-house Test Procedure S17: One Dimensional Swell/Shrink Test

[10] Estimated Heave Potential

[11] Values of shear strength were determined in situ by CPT using

a Platon hand vane or CPT vane (GV).

[12] BS 1377: Part 2: 1990, Test No 4

[13] BS 1377: Part 2: 1990, Test No 9

[14] BS 1377: Part 2: 1990, Test No 5.6

[15] SO₃ = 1.2 x SO₄

[16] BS 5930: 2018, Figure 8 - Plasticity Chart for the classification

Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be

prudent to consider the sample as falling into the DS-4 or DS-5

class respectively unless water soluble magnesium testing is undertaken

to prove otherwise.

* These tests are not UKAS accredited

Full reports can be provided upon request.

Key

D Disturbed sample (small)

B Disturbed sample (bulk)

U Undisturbed sample

W Groundwater sample

FNP Potentially Non-Plastic by inspection

U/S Underside of Foundation



Version: SBH V1.1 - 13.01.2020

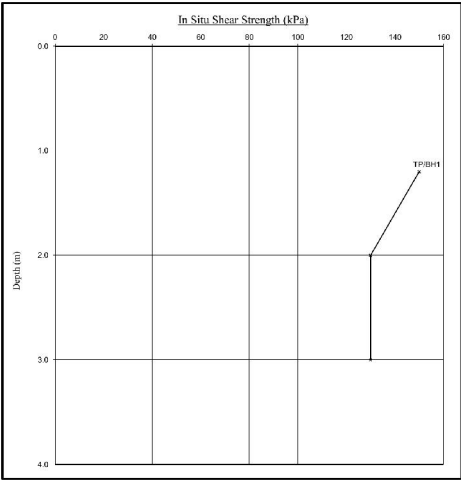
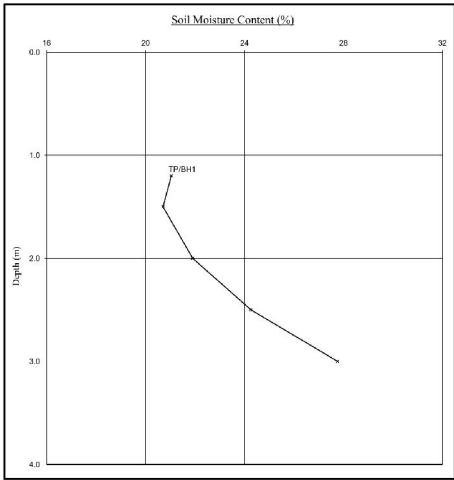
4161

Moisture Content Profiles

Our Ref: [redacted]
Location: 9 Willoughby Road
Work carried out for: Crawford Claims Management

Shear Strength Profiles

Date Sampled: 07/05/2020
Date Received: 12/05/2020
Date Tested: 12/05/2020
Date of Report: 19/05/2020

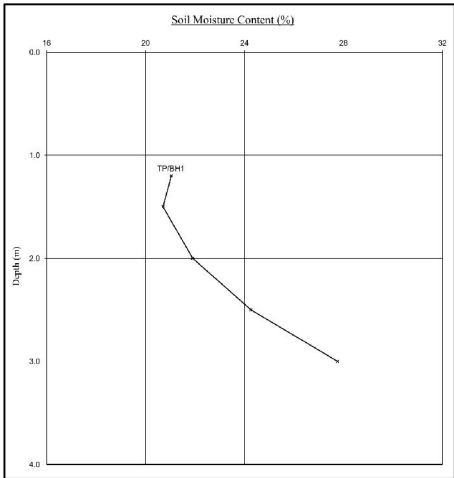


Notes:
1. If plotted, $0.4 I_L$ and $PI - 2$ (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Note:
1. Unless otherwise stated, values of Shear Strength were determined in situ by CPT using a Phoenix Hand Vane the calibration of which is limited to a maximum reading of 140 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

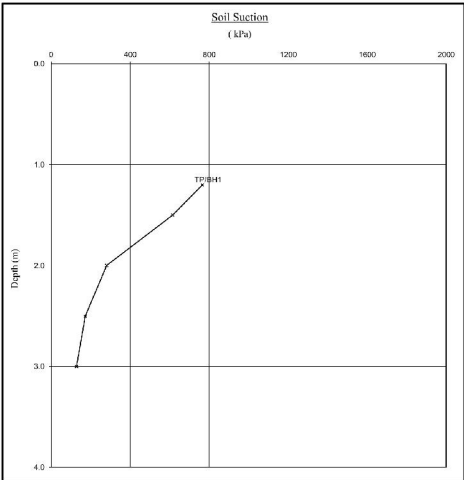
Our Ref: [redacted]
Location: 9 Willoughby Road
Work carried out for: Crawford Claims Management



Notes:
1. If plotted, 0.4 LI and PI-2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Soil Suction Profiles

Date Sampled: 07/05/2020
Date Received: 12/05/2020
Date Tested: 12/05/2020
Date of Report: 19/05/2020



Note:
When shown, the theoretical equilibrium suction profile are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BS7 on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.


EPSL European Plant Science Laboratory	Sheet: 1 of 1	Site: 9 Willoughby Road,
	Job No: [REDACTED]	Work carried out for: Crawford Claims MGMT SUS
	Date: 14/05/2020	
	Order No: [REDACTED]	
	EPSL Ref: [REDACTED]	

Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.
 The results were as follows -

<u>Trial pit/ Borehole number</u>	<u>Root diameter (mm)</u>	<u>Tree, shrub or climber from which root originates</u>	<u>Result of starch test</u>
TP1 (USF)	4 mm	Fraxinus spp. 3 roots	Positive
BH1 (to 2m)	1.5 mm	Fraxinus spp. 2 roots	Positive

Fraxinus spp. include common ash.


 MDM

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