

Basement Impact Assessment

15 Lyncroft Gardens, London NW6 1LB

Hydrogeology, Land Stability and Ground Movement Assessment

21 July 2020

MAUND GEO-CONSULTING

Produced for:

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Rear of 60 Saxon Road
London SE25 5EH

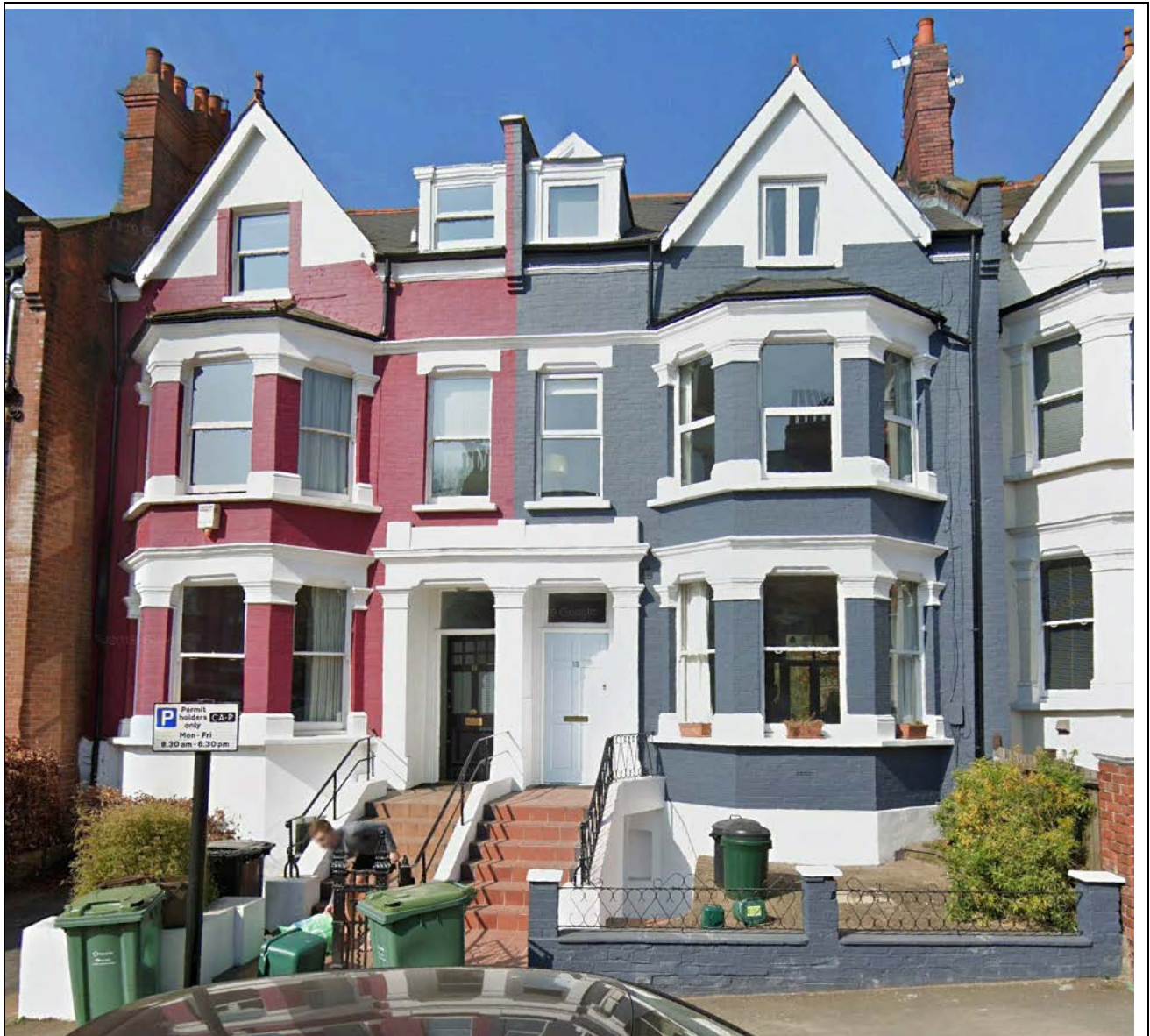
Prepared by:

Julian Maund BSc PhD CEng MIMMM CGeol FGS
UK Registered Ground Engineering Adviser

Maund Geo-Consulting Ltd
3 Coopers Square
Chipping Norton
OX7 5DG

T 07817018716
E julian.maund@gmail.com

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Report Title	Basement Impact Assessment	Site Address	15 Lyncroft Gardens, London NW6 1LB
Work Stage	Hydrogeology, Land Stability and Ground Movement Assessment	Report Date	July 2020
Brief Description of the Report Contents	Desk study and geotechnical interpretation of the ground and groundwater conditions, for a Basement Impact Assessment and Ground Movement Assessment		

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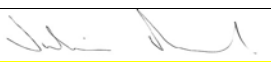

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Organisation	Contact	Copies	Date
Croft Structural Engineers	Philip Henry	1	21/07/2020
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Campbell Reith	Christos Botsialis	1	21/07/2020

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1 Non-Technical Summary

A basement impact assessment (BIA) has been undertaken for hydrogeology and land stability in general accordance with CPG4 (2018) for the site within the grounds of 15 Lyncroft Gardens, London NW6 1LB, in the London Borough of Camden.

The proposed basement is located at 15 Lyncroft Gardens, a mid-terrace house. The basement will extend an existing cellar to provide additional accommodation. The proposed basement will occupy an area of approximately 110 m².

The BIA report considered relevant information from existing sources included in the 'Guidance for subterranean development' produced for the London Borough of Camden' (November 2010) and a Groundsure Enviro/Geo insight Report with historical maps and BGS records.

A ground investigation at the site was undertaken by Maund Geo-Consulting Ltd in October 2019 which comprised one borehole and two hand dug trial pits to expose party wall footings. The borehole (BH01) was drilled to 7.95 m below ground level (bgl), while the trial pits (TP01 and TP02) were excavated to a depth of 0.4m bgl.

The ground investigation confirmed the ground conditions as a layer of Made Ground of sandy gravel composition to a depth of approximately 0.8m which overlies firm to stiff silty clay of the London Clay Formation to a depth of at least 7.95 m bgl. Groundwater was encountered as a seepage at 3.50m during the ground investigation. Subsequent monitoring indicated groundwater at 3.90 to 4.02 m bgl (approximately 65.98 to 66.10 mAOD).

An assessment of hydrogeology has shown that the strata underlying site is considered non-productive strata of very low permeability and is not designated as an aquifer within Environment Agency (EA) guidelines. Seepage of groundwater may be anticipated at a depth of 3.5 m, which would be anticipated to be below the base of the basement floor.

An assessment of land stability has been made from the excavation and construction of the basement. It has been calculated that heave in the centre of the basement is not expected to exceed 16 mm resulting from the excavation and construction. The foundation formation will be able to accommodate a maximum imposed load from the retaining wall of 296 kPa with net settlement of < 25 mm.

The maximum damage category for the adjacent properties has been calculated to be within Category 1 (slight damage). The assessment has indicated a potential movement of 2.8mm vertical and 4mm horizontal for the footway of Lyncroft Gardens. The maximum damage category for the host property has also been calculated to be within Category 1.

An appropriate monitoring regime should be adopted and maintained throughout construction to manage risk and potential damage to the neighbouring structures as construction progresses onsite.

2 Introduction

2.1 Terms of Reference

Maund Geo-Consulting Ltd was instructed on 16 September 2019 by Croft Structural Engineers Ltd (Croft) to undertake the hydrogeology and geology sections of a Basement Impact Assessment (BIA) including a Ground Movement Assessment (GMA) for a proposed development at 15 Lyncroft Gardens, London NW6 1LB. The hydrology section of the BIA is being undertaken separately by Croft.

2.2 Scope and Objective

This report has been written in general accordance with 'Camden geological, hydrogeological and hydrological study - Guidance for subterranean development' produced for the London Borough of Camden (LBC) by Arup (November 2010), hereafter referred to as the GSD. The guidance sets out the methodology for a risk-based impact assessment to be undertaken with regard to hydrology, hydrogeology and land stability in support of Local Plan Policy A5 (2017). The BIA comprises stages in which information is obtained to enable LBC to decide on the impact of the development for the planning application. The LBC Guidance CPG4 Basements (March 2018) requires a BIA to be undertaken for new basements in 5 stages:

1. Screening
2. Scoping
3. Site investigation
4. Impact assessment
5. Review and decision making (By LBC)

As a site investigation has already been undertaken as part of the BIA for 15 Lyncroft Gardens (Factual Report included in Appendix D) the screening part of the assessment has been assessed based on existing information including the site investigation, so the project has been completed in the following sequence:

1. Desk Study of background information
2. Site Investigation including interpretation
3. Screening
4. Scoping
5. Impact Assessment

This report considers the hydrogeological and land stability elements of the BIA only. Hydrology is considered in a separate report by Croft Structural Engineers Ltd.

2.3 Author

This report has been prepared by Dr Julian Maund, director of Maund Geo Consulting Ltd, who is a chartered engineer and chartered geologist with over 35 years' experience. Dr Maund is a UK and Ireland Registered Ground Engineering Adviser and a member of the Association of Geotechnical Specialists.

2.4 Sources of Information

Background information has been derived from Groundsure Geo Insight and Enviro Insight reports obtained on 19/10/19 for the site (Appendix B). Geological information has been derived from on-line BGS sources (Geology of Britain Viewer) and the GSD. Mapping and aerial photography have been obtained from Google Earth. The full list of information is shown below in Table 2.1. Information is also derived from the site investigation undertaken specifically for the proposed development by Maund Geo-Consulting Ltd on 18/10/19.

The following baseline data indicated in Table 2.1 have been referenced to complete the BIA in relation to the proposed development:

Table 2.1 Information type and sources

Information Type	Source
Site walkover and discussion with residents	During SI on 18/10/19
Current/historical mapping	Groundsure Reports, Google Earth
Geological mapping	GSD
Underground tunnels	Groundsure
Hydrogeological data	Groundsure/GSD/EA
Current/historical hydrological data	Groundsure/GSD/EA/ LBC
Flood risk mapping	Groundsure/GSD EA/ LBC
Unexploded Ordnance	(http://bombsight.org)
Ground and groundwater conditions	Site Investigation

3 Desk Study - Background Information on the Site

3.1 Location

The site is located at 15 Lyncroft Gardens, at approximate National Grid Reference TQ 25391, 85395 and Post Code NW6 1LB in the West Hampstead area of the London Borough of Camden.

3.2 Description

The existing building comprises a three storey terraced brick mid-terrace house occupying the western side of Lyncroft Gardens, as shown on the Street View image below in Figure 2.1. The proposed basement will occupy the area below the house, extending the existing cellar.

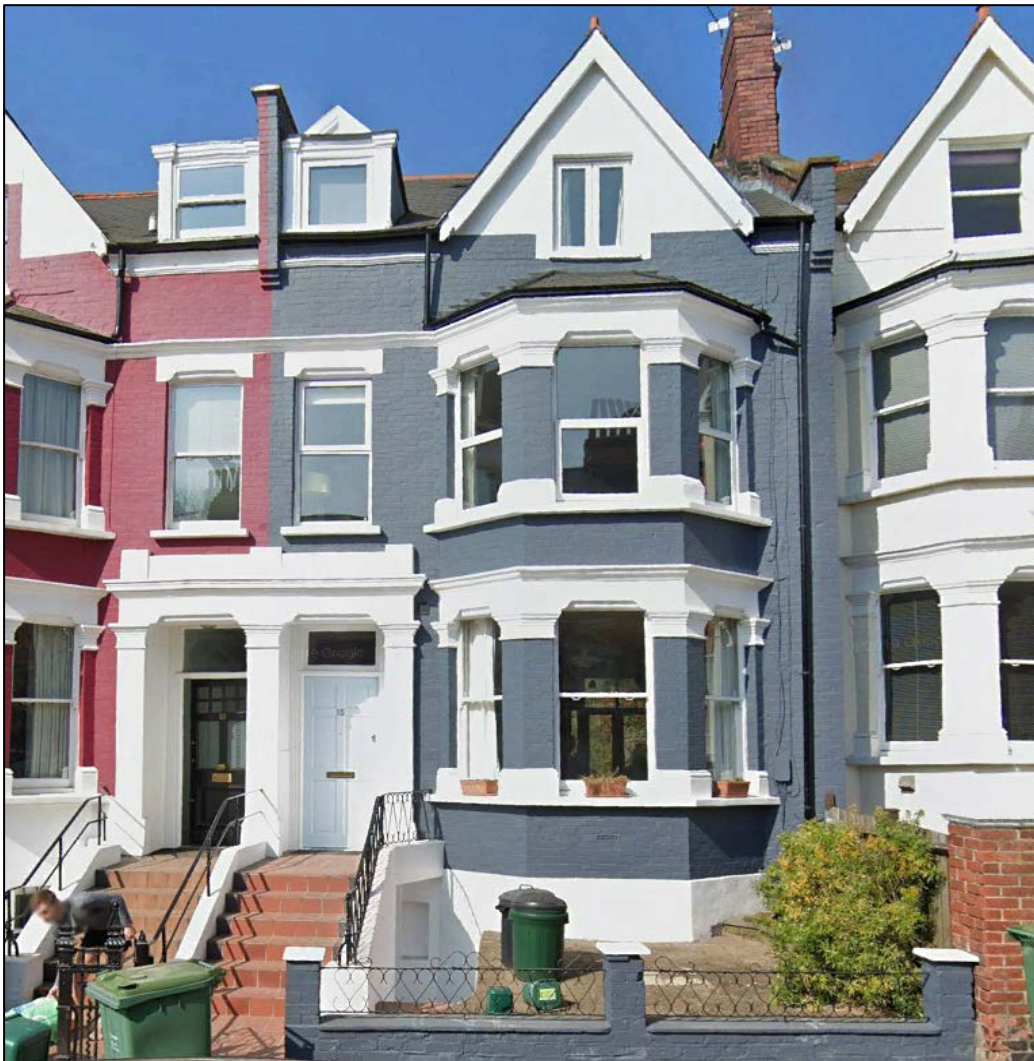


Figure 3.1 Street View Image of the site March 2018

3.3 Present use

15 Lyncroft Gardens is a residential dwelling and is currently occupied by the owner who is proposing the basement construction. The area of the proposed basement is partly occupied by a cellar, which will need to be increased in size beneath the property to make additional accommodation.

3.4 Proposed use

The proposed development relevant to this BIA is understood to comprise the extension of the existing cellar. The proposed basement measures approximately 21.5 m in an E-W direction and 6.5 m in an N – S direction including a lightwell as shown on Drawings 149206-08 dated 04/09/19 by Advantage Basements in Appendix A.

3.5 Topography, geomorphology and drainage

The ground level at the site is at approximately 70 m AOD. No detailed topographical survey is currently available. The land in the vicinity of the site slopes slightly along Lyncroft Garden from the junction with the Finchley Road (~75.00) in the north east to ~64 m AOD at Fortune Green Road in the south west.

There are no discernible geomorphological features in the vicinity of the site. There are no open watercourses within at least 500 m of the site.

The site itself is not within a Flood Zone, although the road has a low flood risk from surface water as indicated on the surface water flooding map in Figure 3.2 below.

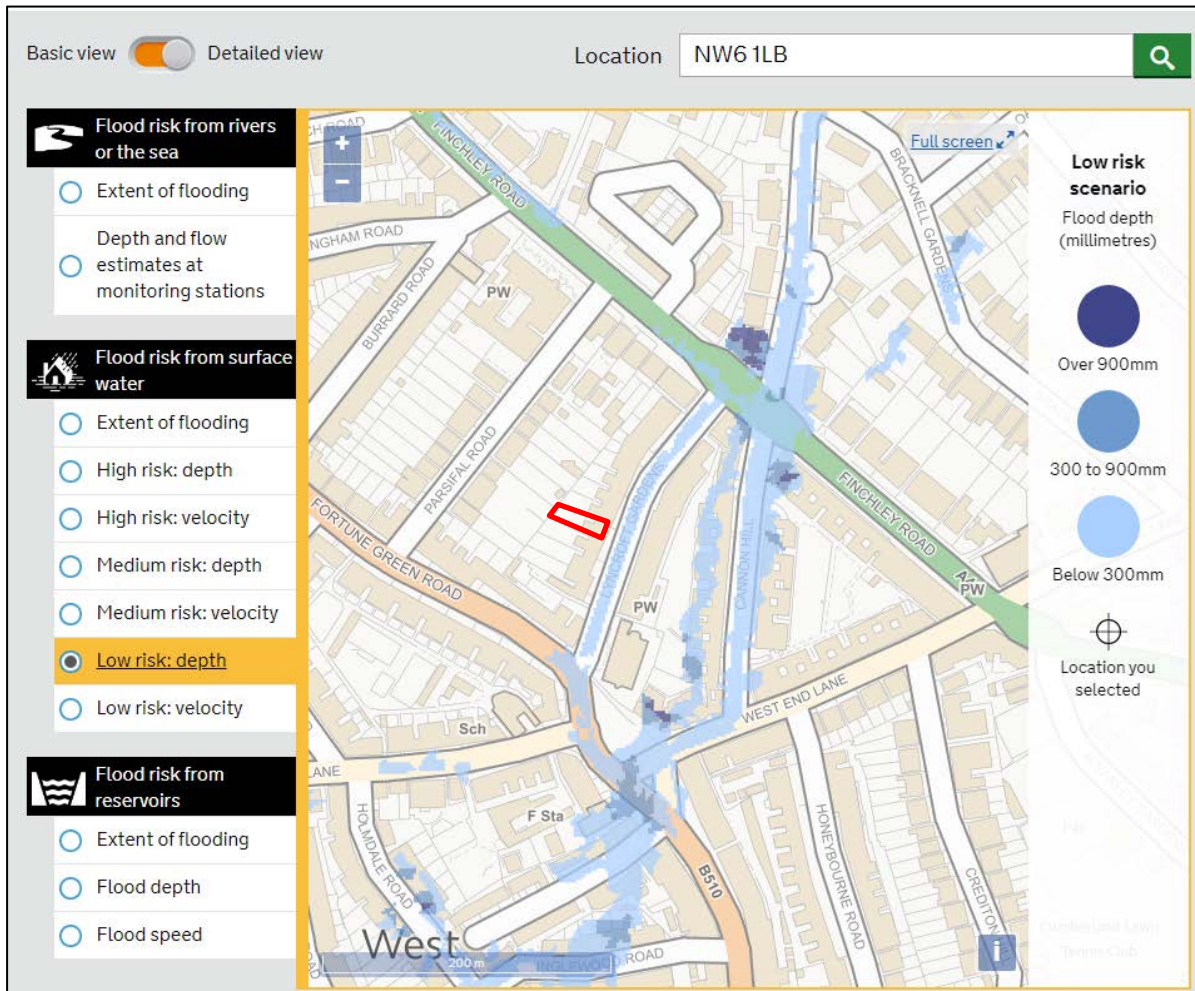


Figure 3.2 Surface water flooding

3.6 Geology

Geological information obtained from Figure 4 of the GSD at 1: 10 000 and the BGS website geological mapping at 1 50 000 scale shows the site to be directly underlain by the London Clay Formation. No superficial deposits are shown. A review of boreholes in the vicinity available from the BGS Geology of Britain Viewer indicates comparable geology.

3.7 Hydrogeology/groundwater

The property is located on the London Clay, which is classified as an unproductive stratum. Figure 8 of the GSD confirms this classification.

The site does not lie within a ground water protection zone. The closest protection zone is outside the search zone of a 500m radius.

The London Clay underlying the site is not classified as a groundwater vulnerability zone, as designated by the Environment Agency. The Groundsure Enviroinsight Report (Appendix B) indicates the nearest ground water vulnerability zone is outside the search zone of 250m.

3.8 Natural Hazards

The Groundsure report (Appendix B) findings on natural hazards are summarised in Table 3.1

Table 3.1 Natural Hazards

Natural Hazard	Risk (Stated by BGS in Groundsure report)	Comment
Natural ground subsidence	Moderate	Not applicable to the topography of the site.
Shrink-Swell	Moderate	The site is underlain by the London Clay Formation (LFC) which comprises potentially moderate to high plasticity clays. This material has potential shrink swell properties.
Landslide	Very Low	Not applicable to the site geology
Soluble Rock	Negligible	Not applicable to the site geology
Compressible Ground	Negligible	Clay soil of the LCF is subject to consolidation from additional imposed loads, which are limited by appropriate foundation design
Collapsible rock	Negligible	Not applicable to the site geology
Running Sand	Very Low	Not applicable to the site geology
Radon	Not in a Radon affected area	No Radon protection measures are necessary

3.9 History of site

The Groundsure Insights Maps in Appendix C includes historical mapping surveys from 1871 to 2014.

The earliest record of 1865-1870 indicates the site was undeveloped agricultural land. By 1915 current house at 15 Lyncroft Gardens was constructed, as was the rest of Lyncroft Gardens and is little changed to the present day.

3.9.1 WW2 bomb sites

A record of known bomb sites is presented in Figure 3.3 from the website <http://bombsight.org>. While this does not claim to be a definitive record, it shows nothing recorded in the environs of the site. The lack of change of building development in the area of the site suggests no bomb related destruction occurred at the site.

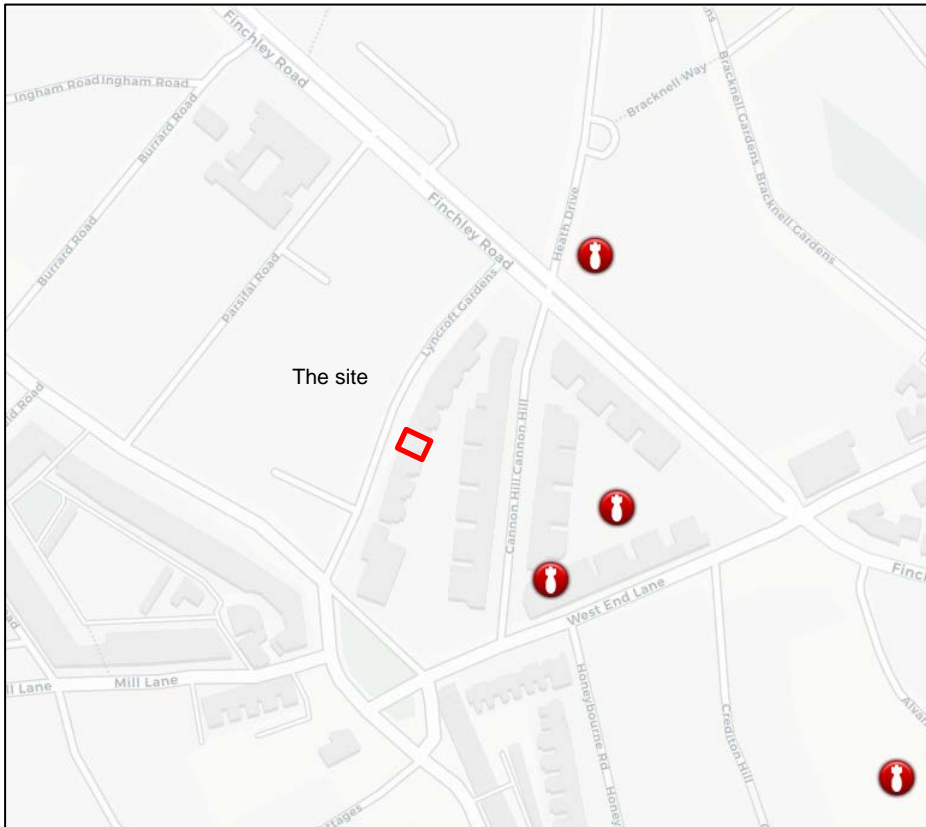


Figure 3.3 WWII Bomb record

3.10 Underground features

The Groundsure Geosight Report (Appendix B) has not identified any mining, underground workings or natural cavities within at least 500 m of the site.

The Groundsure Geosight Report (Appendix B) has not identified any tunnels or railways within 250m of the site.

3.11 Other factors e.g. contamination and archaeology

The Groundsure Enviroinsight Report (Appendix B) has not identified any ‘Environmental Permits, Incidents and Registers’ or ‘Landfill and Other Waste Sites’ within at least 500 m of the site boundary.

No specific archaeological investigation has been undertaken. The ‘Groundsure’ survey has not identified any known ‘Environmentally Designated Sensitive Sites’ within 250 m of the site (Appendix B).

4 Site Investigation

A site investigation was undertaken by Maund Geo-Consulting Ltd on 18/10/19. A report of the site investigation comprising exploratory hole records and laboratory testing is included in Appendix D.

The site investigation comprised:

- 1 No. borehole (BH01) carried out using cable percussive methods to a depth of 7.95 m bgl,
- 2 No. hand dug trial pits to expose footings,
- The in-situ strengths of the subsoil encountered were assessed by means of SPTs in BH01 at 1 m intervals,
- Disturbed soil samples were obtained from BH01 for laboratory geotechnical testing and further examination.
- A 50 mm diameter groundwater monitoring well was installed to a depth of 5.0 m in BH01

The locations of the above exploratory holes are shown in Figure 4.1 below. The exploratory hole records and laboratory test results are shown in Appendix D.

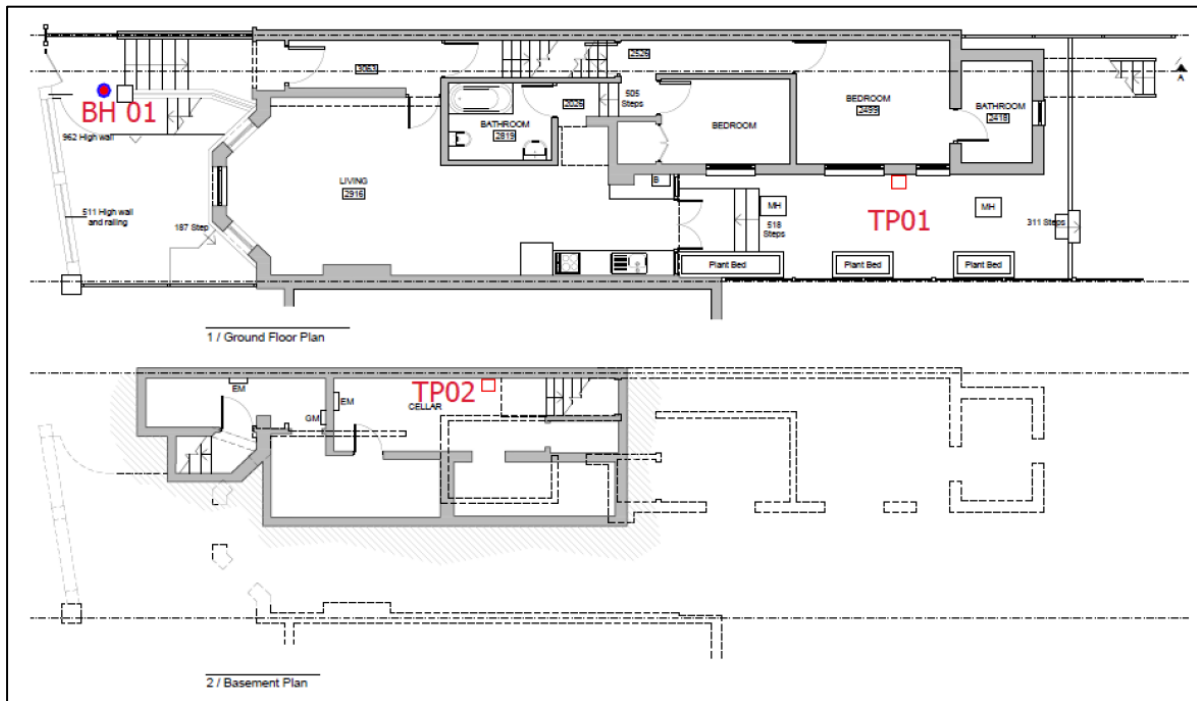


Figure 4.1 Exploratory hole locations

4.1 Details of laboratory tests

Laboratory tests to determine the geotechnical properties of the soil were scheduled by Maund Geo-Consulting Ltd and carried out by I2 Analytical Ltd generally in accordance with BS1377:1990 and BRE Special Digest 1 2005. The tests included:

- 5 moisture content and plasticity tests (BS1377:1990)
- 2 Water soluble sulphate and pH (BRE SD1)

5 Ground Conditions

5.1 Stratigraphy

The ground conditions encountered in BH01 are summarised in Table 5.1 below. For a full description refer to exploratory records in Appendix D.

Table 5.1 Summary of ground conditions

Stratum	Description	Depth at top of Strata (m)	Approx. level (m AOD)	Thickness of Strata (m bgl)
MADE GROUND	Ceramic tiles	0	70.0	0.02
MADE GROUND	Fine sandy clayey Gravel of brick and flint.	0.002	69.08	0.78
London Clay Formation	Firm to very stiff brown silty CLAY with occasional bands of fine sand and fine gravel sized selenite crystals	0.4	46.60	Proven for 7.15

5.2 Groundwater

Groundwater was encountered as a seepage during drilling at 3.50m (~66.50 m AOD).

Groundwater readings from post investigation monitoring on the site are shown in Table 5.2 indicating groundwater is present at a depth of up to 4.02m based on current readings

Table 5.2 Groundwater monitoring in BH01

Date of monitoring	Groundwater Depth (metres below ground level – Approximately 70.00 m AOD)	Approximate Groundwater level (m AOD)
28/10/19	3.90	66.10
08/11/19	4.02	65.98

5.3 Consideration of the individual strata, with reference to the basement.

The anticipated formation level of the basement floor slab will be approximately 2.5 m bgl at 67.5 m AOD, within the London Clay. An overall excavation depth of 3.00 m is assumed for a ground movement assessment.

The overall ground model is illustrated in the conceptual model in Section 6.2 below.

5.3.1 *Made Ground*

Below the existing concrete surface, the made ground has been described as a sandy, clayey red brown fine to coarse Gravel of brick and chert. Made ground encountered was approximately 0.8m thick. No SPT results were obtained in this material. This material is considered to represent build-up of site levels at the front of the house. Trial pits TP01 to the rear of the property had 0.4m of made ground and TP02 in the cellar showed 0.075m of made ground.

The made ground is described as an inert material with no visual or olfactory indications of contamination.

The risk of the onset of contamination leaching from the site is considered to be negligible considering the thickness of the made ground, the lack of indication of contaminants, and the impermeability of the underlying strata.

5.3.2 *London Clay*

The London Clay Formation (LCF) was encountered during the site investigation at a depth of 0.8 m bgl to termination of BH01 at 7.95 m bgl. A plot of SPT N values against depth is shown in Figure 5.1 which shows N values ranging between 9 and 26, with a clear pattern of N values increasing with depth.

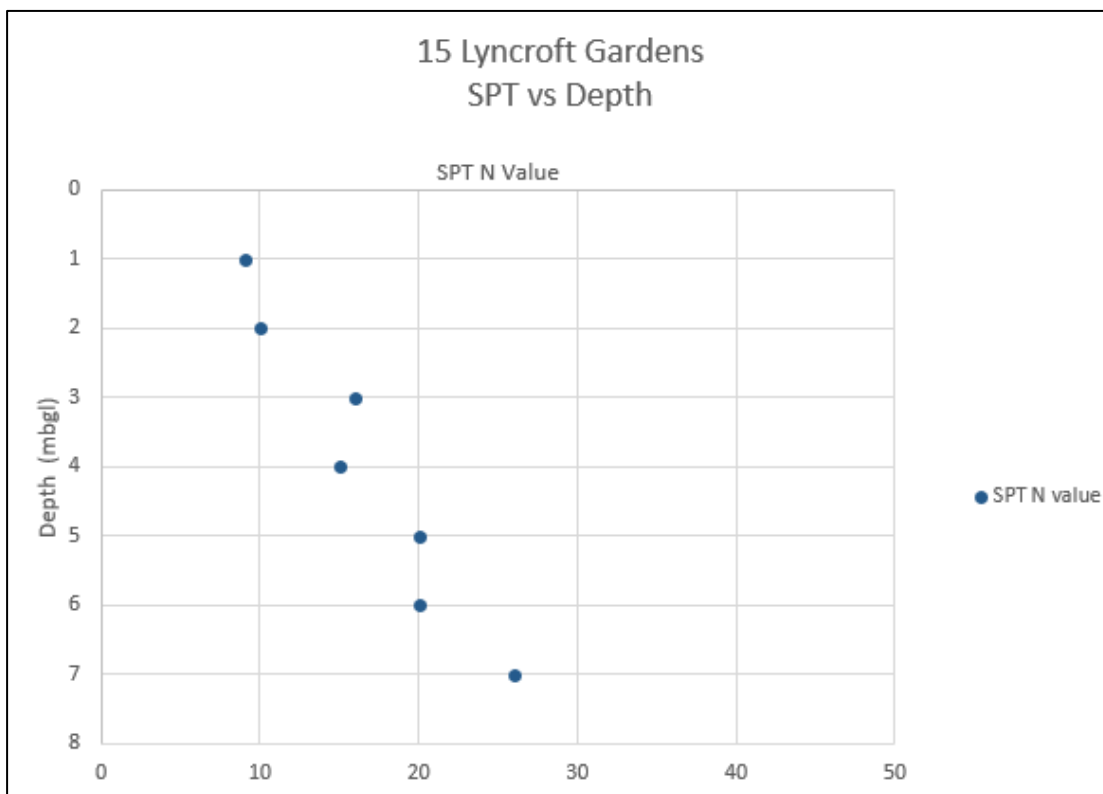


Figure 5.1 SPT N values for London Clay Formation

5 No. Atterberg Limit tests carried out on samples of London Clay showed high to very high plasticity (PI average of 38%). Given a F1 of 4.5 (Stroud and Butler 1975) the recorded SPTN values correlate to undrained strengths of 40 kPa to 117 kPa.

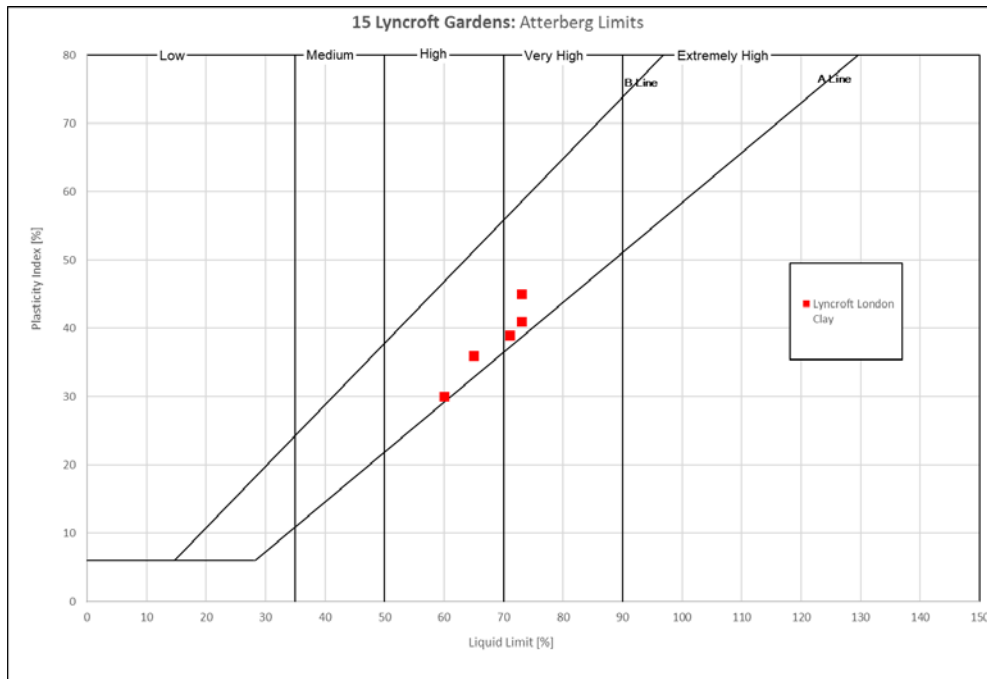


Figure 5.2 Atterberg Limits London Clay Formation

Figure 5.3 shows the undrained stiffness profile based on correlation with SPT N values.

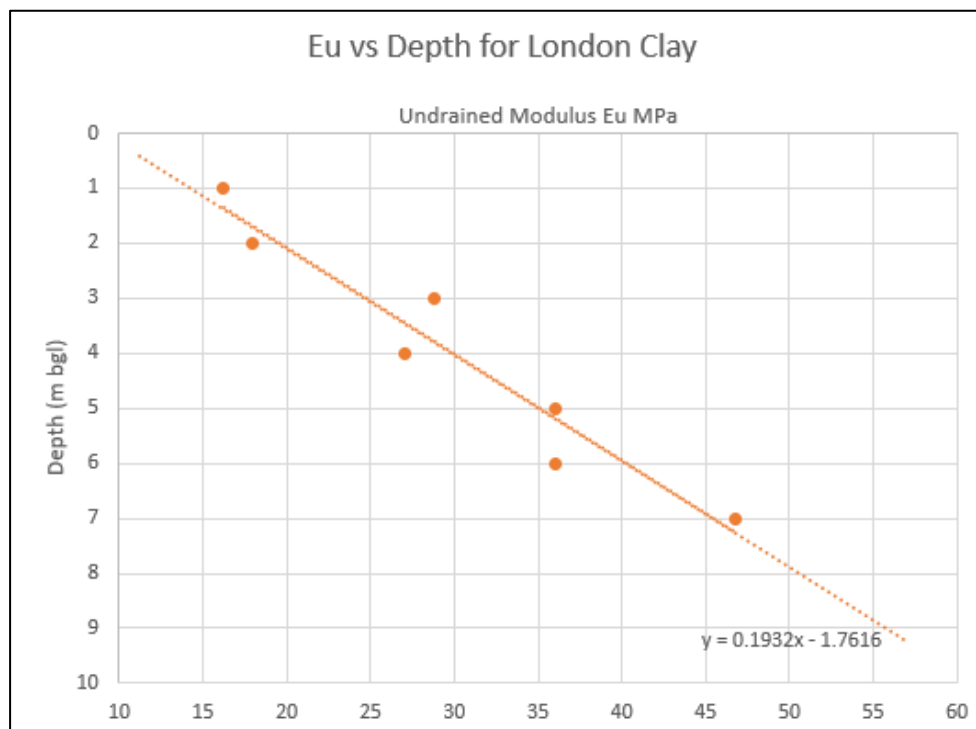


Figure 5.3 Relationship of stiffness with depth in London Clay Formation

The deformation moduli (E_u and E') of the LCF has been cautiously estimated from the relationship between undrained cohesion for an axial strain of 0.1% and plasticity of the LCF where E_u is based on a PI of 38% and an OCR >4 giving a $E_u/C_u \sim 400$ (after Jamiolkowski et al. 1979). and E' is $0.75 E_u$ after Burland, Standing J.R., and Jardine F.M. (eds.) (2001). Poisson Ratio is taken as $\nu_u = 0.5$ undrained and $\nu' = 0.2$ drained.

As there is a clear linear relationship of SPT and C_u with depth, the corresponding E_u / E' is assessed to increase linearly with depth from 15/11.25 MPa at the interface with made ground to 50.0/37.5 MPa at 8.00 m AOD as indicated in Figure 5.3, for purposes of settlement / heave modelling in Section 6.

The characteristic values of geotechnical parameters are a cautious estimate in accordance with BS EN 1997, based on the data obtained from the ground investigation (Appendix D) have been summarised in Table 5.3 as follows:

Table 5.3 Geotechnical Design Parameters

Strata	Design Level m bgl / (mAOD)	Class	Undrained Cohesion C_u (kPa)	Effective angle of shearing resistance MPa	Bulk unit weight kN/m ³	Deformation Modulus E_u (MPa)	K_a	K_p
London Clay Formation	0.8 (69.20)	CH/ CV	27 – 90 (32 +11.5z)	24*	20**	15 + 5z*** (11.25 +7z)	0.35	2.5

Notes:

* Burland, Standing J.R., and Jardine F.M. (eds.) (2001)

**BS8004 2015

*** E_u is based on $400 C_u$. (Jamiolkowski et al. 1979). E' based on $0.75E_u$. (Burland, Standing J.R., and Jardine F.M. (eds.) 2001).

Active and Passive pressure coefficients k_a and k_p from BS EN 1997-1 Annex C

The parameters in Table 5.3 are unfactored (Serviceability Limit State) and considered to be 'a cautious estimate'.

Groundwater is assumed to be below formation level, at circa 3.5 m bgl.

6 Geotechnical Assessment of Ground Conditions

6.1 Introduction

The information obtained from the ground investigation on the soil conditions in relation to the proposed basement construction has been assessed for impacts on existing building structures. The principle impacts are ground movements from the excavation for the basement. These movements are vertical and horizontal movements of the foundation formation level from isostatic readjustment from the excavation and possible vertical and horizontal impacts of existing structures from the basement wall construction.

6.2 Presumed Bearing resistance

The foundation formation level of the basement will be at approximately 67.00 m AOD or 3.0 m below ground level. At the formation level an undrained shear strength of approximately 59 kPa has been evaluated from the SPT profile. Wall loads provided by Croft Structural Engineers (Drawing SK01 in Appendix A) comprise the following shown in Table 6.1, based on a wall thickness of 0.35m:

Table 6.1 Wall loading

Wall No.	Combined SLS kN/m	Total kPa	Net Loading kPa	Adequacy Factor (DA1- 2)
1	100	296	225	1.09
2	60	176	116	1.76
3	30	86	26	3.29
4	60	176	116	1.76
5	100	296	225	1.09
6	30	176	116	3.29
A	60	176	116	1.76
B	60	176	116	1.76

The net loading allows for the removal of 3m depth of soil (~60 kPa) over part of the house where full excavation is required to basement level. Where there is an existing cellar the excavation will be extended approximately 1.7m resulting in removal of a load of ~34 kPa. The wall loads will be taken by the basement floor slab which will be initially 1m wide (Drawing SK01-10 in Appendix A). Preliminary calculations show that there will be an adequacy factor (overdesign factor) of between 1.09 and 3.29 (EC7 DA1 Combination 2). This indicates the ground will accommodate the imposed load without significant (<25 mm) settlement. The actual settlement will however be determined from the net effect from the removal of soil during the basement excavation where heave is a more significant factor.

6.3 Effect of Heave from soil excavation

The proposed basement will require the excavation from the exiting ground level of approximately 70.0 m AOD to approximately 67.00 m AOD (3.0 m depth). For purposes of this assessment it is assumed the unit weight of the soil (γ_k) to be removed is 20 kN/m³ giving an overall negative load of 60 kPa in areas of full excavation and 34kPa where 1.7m of soil will be removed.

Dimensions of the excavation is based on Drawing SK-01 dated September 19, included Appendix A.

The ground model is based on the ground conditions assessment in Section 5. The effects of short term un-drained and long term drained conditions have been considered cumulatively, which is a conservative assessment as a worst case. The long and cross sections in Figures 6.1 and 6.2 have been drawing to intersect the greatest movement contours from the Pdisp plot.

The heave has been evaluated using Pdisp version 20.12, which shows a maximum heave of up to - 9.6 mm¹ under short term undrained conditions as shown in Figure 6.1 below in which location of adjacent properties 13 and 17 Lyncroft Gardens are diagrammatically indicated. Long term drained conditions are shown in Figure 6.2 where up to -6.3 mm was determined. As can be seen from Figures 6.3 and 6.4 the short term displacement becomes less than -4.0 mm and -5.2 mm at the boundary with the nearest properties of 13 and 17 Lyncroft Gardens respectively, reducing to less than 1 mm within those properties for each case. Similarly, long term movements reduce to less than +0.5 mm and -0.75 mm at the boundary (Figure 6.5 and 6.5). The combined movements are discussed further in Section 9 and 10.

¹ Note that heave is stated as a negative number in PDisp, but is a positive number in the Ground Movement Assessment in Section 9

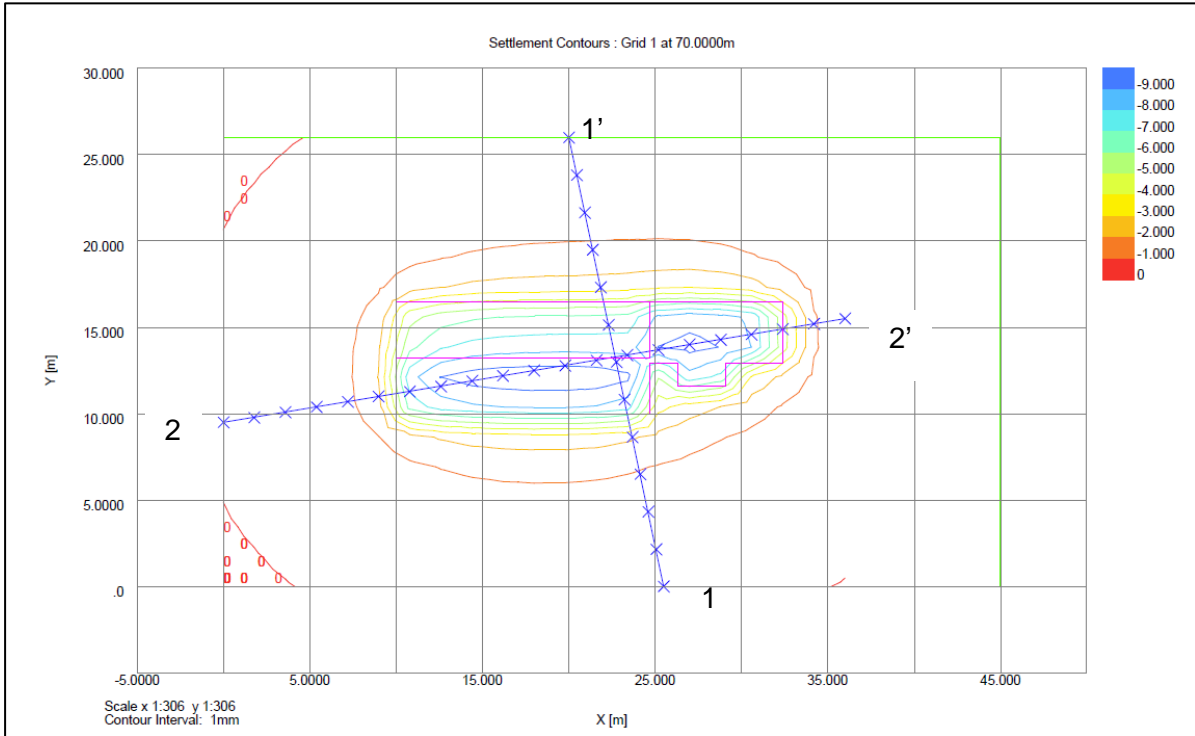


Figure 6.1 Heave- short term undrained condition

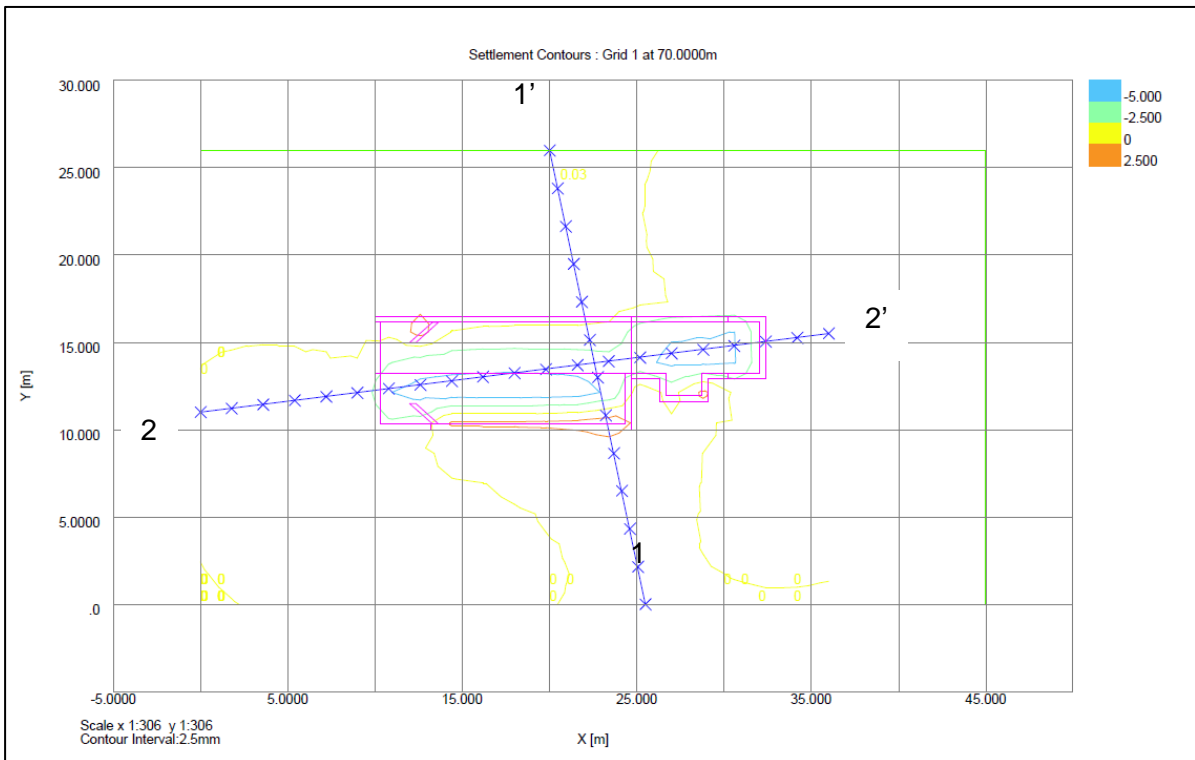


Figure 6.2 Heave- long term drained condition

Cross sections of the effects of the basement excavation and construction are shown in Figures 6.3 and 6.4, in which the boundaries with 13 and 17 Lyncroft are diagrammatically indicated. These models have been used as a basis for the ground movement assessment and damage assessment in Section 9 and 10 respectively.

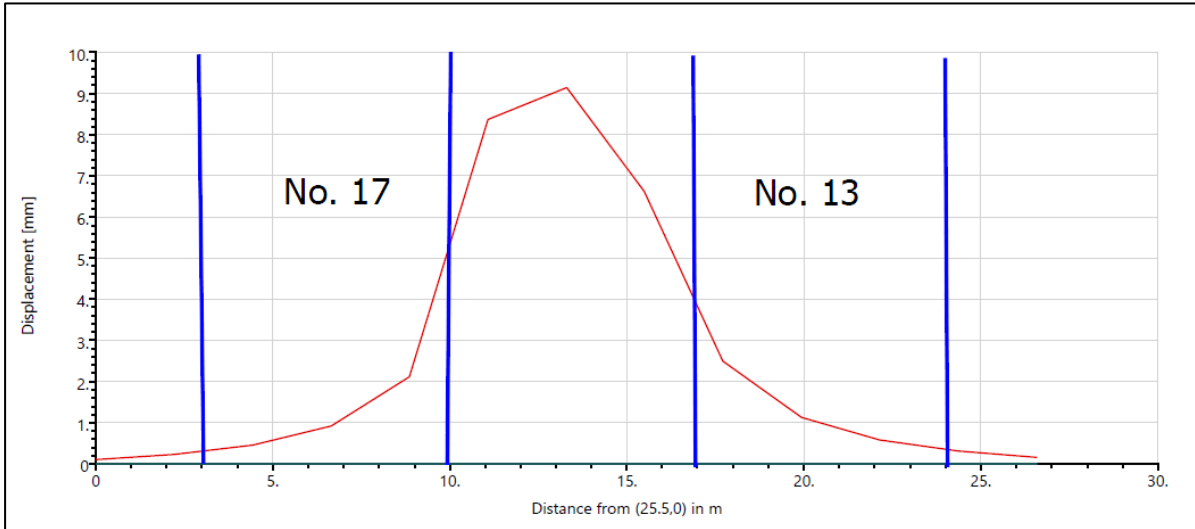


Figure 6.3 Heave- short term undrained condition in excavation- Section 1-1'

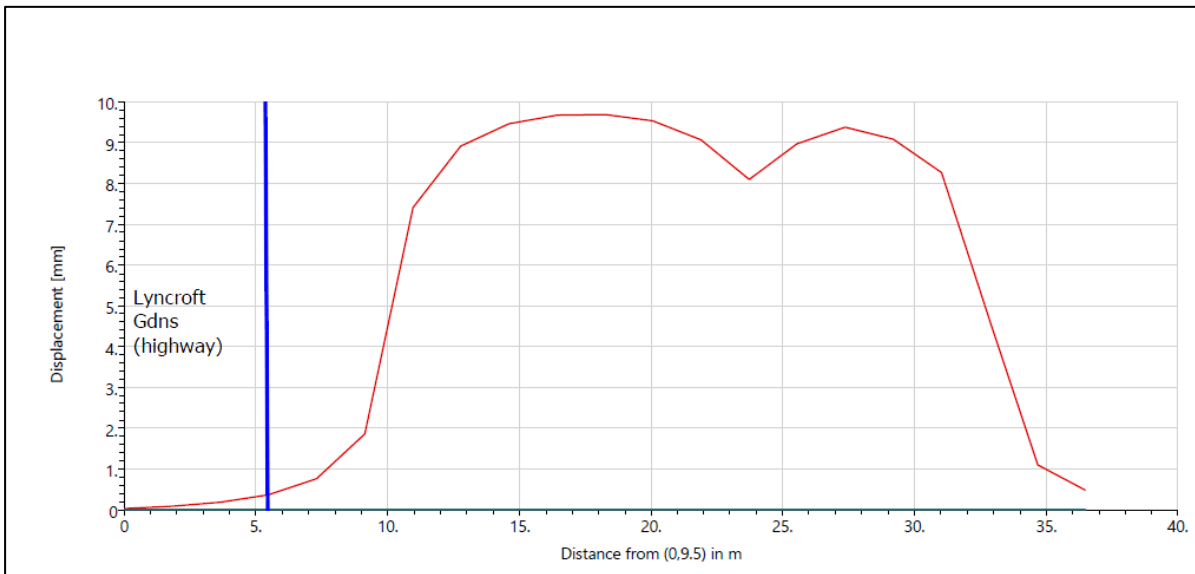


Figure 6.4 Heave- short term undrained condition in excavation- Section 2-2'

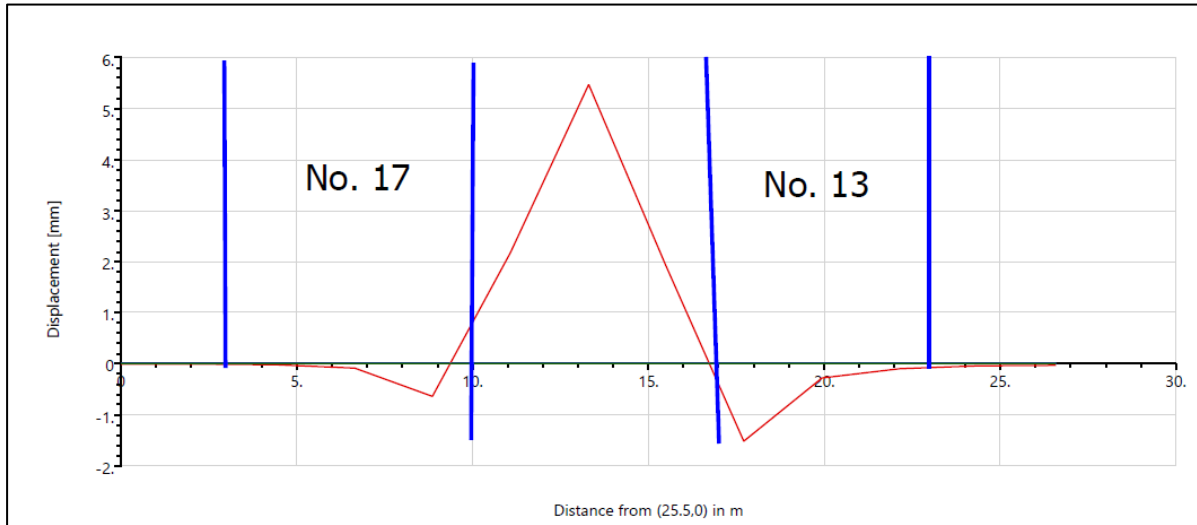


Figure 6.5 Heave- long term drained condition- Section 1-1'

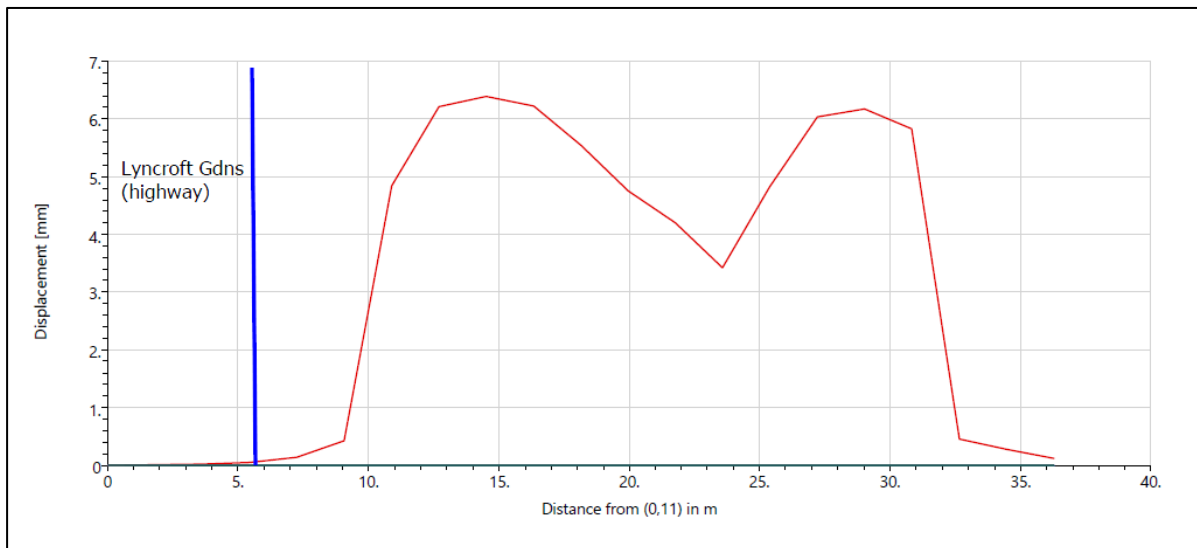


Figure 6.6 Heave- long term drained condition- Section 2-2'

Full output of the PDisp model is included in Appendix D.

6.4 Sub –surface Concrete

The results of lab testing for sulphate and pH are summarised below in Table 5.1. The full analysis is included in Appendix D.

Table 6.1 Sulphate and pH categories

Sample depth	Soil Type	Sulphate S04 2:1 extract	pH	Sulphate Class (DS)	ACEC Class
0.4	Made Ground	0.044 g/l	7.8	DS-1	AC1s
1.8	London Clay Formation	0.096 g/l	8.0	DS-1	AC1s

It is recommended that an overall design sulphate class of DS-1 and an Aggressive Chemical Environment for Concrete (ACEC) class of AC1s is adopted for the basement slab and underpinning. If a concrete piled solution is to be adopted, then DS-3 AC2s should be considered to take into account the greater presence of gypsum related minerals which occur more frequently at greater depth.

7 Screening

7.1 Introduction

Screening is undertaken as outlined in Section 6.2 of the GSD recommendations. It identifies if there are hydrogeological and land stability issues associated with the proposed development that requires detailed analysis and investigation. If there are no significant issues identified in the screening stage, then further stages are not required. The report follows the flow charts set out in CPG4 (2018) and makes reference to the GSD.

7.2 Subterranean (Groundwater) flow

This section answers questions in Figure 1 of CPG4:

The source of information for the assessment of subterranean flow is from the GSD and a site-specific Groundsure Environmental Insight Report obtained in October 2019 for 15 Lyncroft Gardens (Appendices B and C) along with the ground investigation undertaken at 15 Lyncroft Gardens on 18 October 2019 (Appendix D).

Table 7.1: Responses to Figure 1, CPG4

Question	Response	Action required
1a. Is the site located directly above an aquifer?	No. The site is underlain by the London Clay Formation. This is considered an unproductive stratum.	None
1b. Will the proposed basement extend beneath the water table surface?	Groundwater was not struck during investigation to 7.95m bgl. Post investigation monitoring shows groundwater ranges between 3.9 and 4.02 m bgl (65.98 to 66.10m AOD) which is below the basement formation level of at 3m	It is recommended that groundwater seepage is allowed for in the Basement Method Statement.

Question	Response	Action required
2. Is the site within 100m of a watercourse, well, or potential spring line?	No. There are no known wells or spring-lines within 100 m of the site ^{b,c} .	None
3. Is the site within the catchment of the pond chains on Hampstead Heath?	No. The site is not within the catchment of the ponds ^b	None
4. Will the proposed basement development result in a change in the proportion of hard surfaced/paved areas?	No The basement is entirely below the existing building and concrete forecourt	None
5. As part of site drainage, will more surface water than at present be discharged to ground (e.g. via soakaways and/or SUDS)?	No, the basement is entirely below the existing building and concrete forecourt.	None. Due to the geology of the London Clay Formation close to ground level, soakaway drainage will not be suitable
6. Is the lowest point of the proposed excavation (allowing for any drainage and foundation space under the basement floor) close to, or lower than, the mean water level in any local pond or spring lines?	No. There are no recorded local ponds or spring lines within 250 m of the site	None

- a. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 8).
- b. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 11).
- c. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 14).

In summary, the site is located on the London Clay Formation. Post investigation monitoring of 1 No. boreholes drilled at the site to a depth of 7.95 m bgl indicated that groundwater was encountered between 3.9 and 4.02 bgl, or at least 0.9 m below the basement excavation.

7.3 Slope / Land Stability

This section answers questions posed by Figure 2 in CPG4.

Table 7.2: Responses to Figure 2, CPG4

Question	Response	Action required
1. Does the site include slopes, natural or man-made, greater than about 1 in 8?	No. The site is on very slightly sloping ground of 1 in 20 or less along Lyncroft Gardens	None
2. Will the proposed re-profiling of the landscaping at site change slopes at the property boundary to greater than about 1 in 8?	No.	None
3. Does the development neighbour's land including railway cuttings and the like with a slope greater than about 1 in 8?	No No railway is present with 500m of the site	None.
4. Is the site within a wider hillside setting in which the general slope is greater than about 1 in 8?	No.	None
5. Is the London Clay the shallowest stratum on site?	Yes. Below a layer ($\leq 0.8\text{m}$) of made ground, the site bears directly onto London Clay.	Determine heave and ground movement from the excavation of the clay and construction of basement walls.

Question	Response	Action required
<p>6. Will any trees be felled as part of the proposed development and/or are any works proposed within any tree protection zones where trees are to be retained?</p>	<p>No trees will be felled.</p>	<p>None</p>
<p>7. Is there a history of shrink/swell subsidence in the local area and/or evidence of such at the site.</p>	<p>No records.</p>	<p>None</p>
<p>8. Is the site within 100 m of a watercourse or a potential spring line?</p>	<p>No ^{a,b}.</p>	<p>None</p>
<p>9. Is the site within an area of previously worked ground?</p>	<p>No.</p> <p>Natural soil occurs less than 0.8 m below the surface of the site.</p> <p>Historical mapping shows no change in land use from at least 1882 to the present day.</p>	<p>None</p>

Question	Response	Action required
10. Is the site within an aquifer?	No. The site is underlain by the London Clay. This is considered an unproductive stratum in EA classifications.	None
11. Is the site within 50m of the Hampstead Heath Ponds?	No.	None
12. Is the site within 5 m of a highway or pedestrian right of way?	Yes The basement lightwell will be 2.5 m from the pedestrian walkway and 4.25 m from the highway.	Assess the ground movement from the basement construction on the pedestrian walkway.
13. Will the proposed basement significantly increase the differential depth of foundations relative to neighbouring properties?	No Both No. 13 and No. 17 have cellars and or basements	A ground movement assessment will be undertaken to assess impact (Burland Damage Assessment) as a precaution
14. Is the site over (or within the exclusion zone of) any tunnels?	No.	None.

- a. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 8).
- b. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 11).
- c. Camden Geological, Hydrogeological, and Hydrological Study, Arup, 2010. (Fig. 14).
- d. Groundsure Report (Appendix C) September 2016

In summary, the proposed basement is located on level ground and will be founded within the London Clay Formation, which is present from 0.8 m depth below the site surface.

8 Scoping

8.1 Introduction

This section considers the output from the screening survey where further actions are required. It considers the scope of information required in addressing these actions and what the potential impacts are of the basement construction. The existing ground conditions and the location of the basement can be summarised in a conceptual site model as indicated in Figure 8.1.

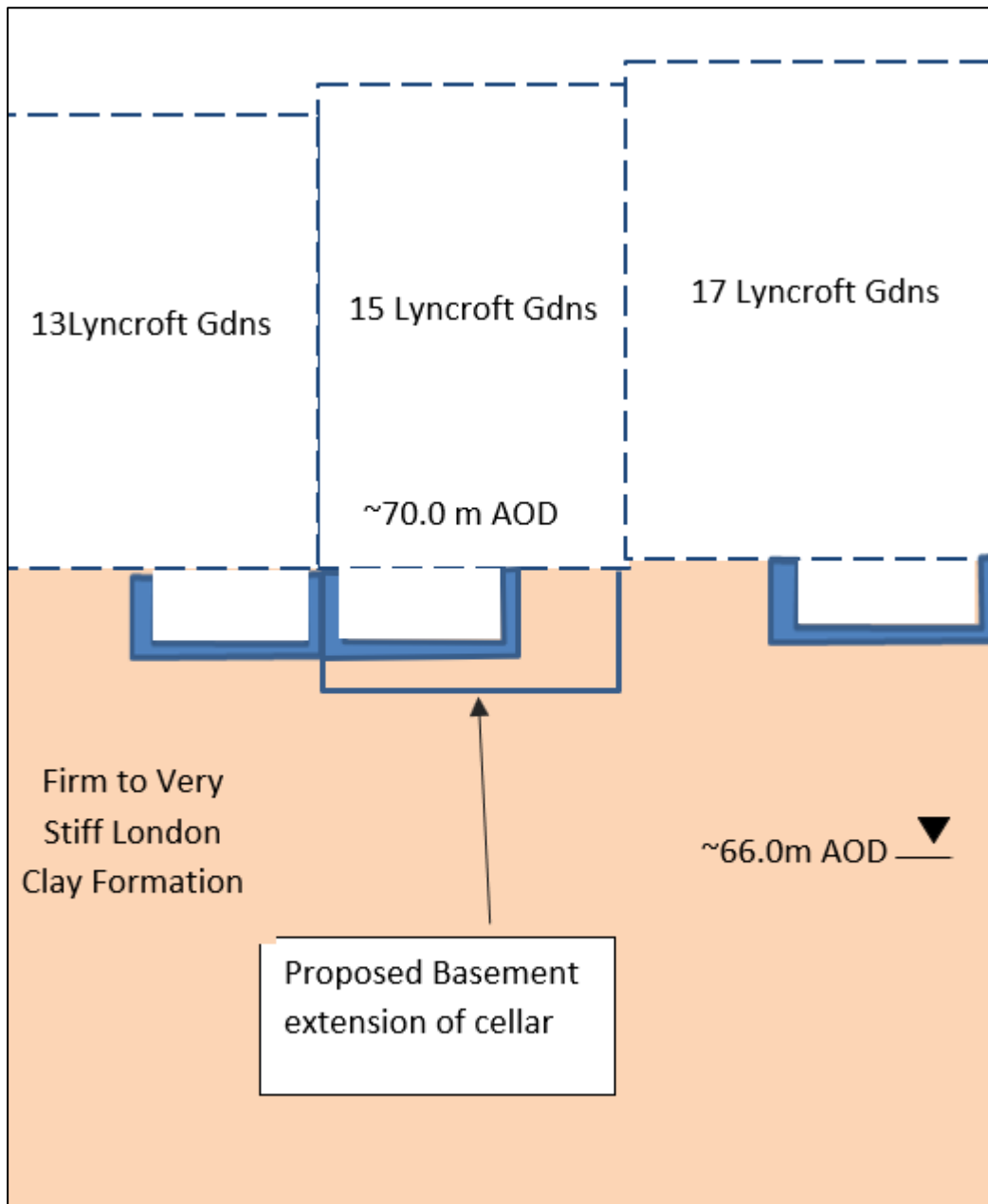


Figure 8.1 Conceptual Site Model (Not to scale, approx. m AOD)

There does not appear to be any requirement for groundwater mitigation measures for groundwater due to the depth of groundwater, as summarised in Table 8.1

Table 8.1 Summary of Scoping Requirements - Hydrogeology

Screening questions of concern - Hydrogeology	Potential Impact	Mitigation
1b. Will the proposed basement extend beneath the water table surface?	Monitoring indicates the basement level is 1.4 m above the recorded water level.	It is recommended that groundwater seepage is allowed for in the Basement Method Statement

The land stability issue relates to the ground movements resulting from the excavation within the London Clay Formation which will be addressed by a ground movement analysis as summarised in Table 8.2

Table 8.2 Summary of Scoping Requirements – Land Stability

Screening questions of concern – Land Stability	Potential Impact	Mitigation
5. Is the London Clay the shallowest stratum on site?	Yes. Below a layer ($\leq 0.8\text{m}$) of made ground, the site bears directly onto London Clay.	Determine heave and ground movement from the excavation of the clay and construction of basement walls.
12. Is the site within 5 m of a highway or pedestrian right of way?	Yes The basement lightwell will be 2.5 m from the pedestrian walkway and 4.25 m from the highway.	Assess the ground movement from the basement construction on the pedestrian walkway.

9 Impact Assessment

9.1 Groundwater

9.1.1 Groundwater level

The screening process has shown from borehole information that groundwater occurs at a depth of between 3.9 and 4.02m bgl, or approximately 65.98 to 66.10m AOD². At this level groundwater will be 0.9 to 1.02 m below the excavation depth of 3.0 m or 67.00 m AOD (this assumes total excavation allowing for blinding and basement floor slab).

It is proposed that pumping to deal with seepage in the low permeability London Clay Formation is included in the Basement Method Statement.

9.1.2 Impact on groundwater by any contamination from the made ground

The made ground encountered on the site is described as an inert material with no visual or olfactory indications of contamination.

The natural strata underlying the site is of very low permeability and classified as non-water bearing. The risk of leaching into ground water is therefore considered negligible.

9.2 Land Stability

The screening process has identified three issues which require an impact assessment listed below from Tables 7.2 and 8.2.

- Presence of London Clay as the shallowest stratum (excepting a thin layer of granular made ground of < 0.4m);
- Proximity to the highway and
- Proximity of an adjacent structure with differential depth of foundations.

9.2.1 Presence of the London Clay Formation at the surface

The ground investigation indicates that the soil can be readily excavated using conventional plant appropriate for the access constraints imposed by the location of the property. Groundwater is not anticipated to be encountered, based on monitoring records from the site investigation for the full depth of the excavation, although allowance for seepage is recommended.

The impact of the excavation on ground heave has been assessed in Section 6 of this report, which concludes that total heave will be less than 16 mm, which is considered within normal construction tolerance. For evaluation of all ground movements both short term during excavation and long term after construction it was considered necessary to undertake a Ground Movement Assessment, which is included in Section 10 of this report.

² Levels are approximate and subject to a site topographical survey.

The ground movement assessment evaluates ground movement in relation to neighbouring properties No. 13 and 17 Lyndhurst Gardens, the footway/highway and the host property.

9.2.2 Stability of Temporary Excavations

It is proposed that the basement retaining walls will be constructed using a hit and miss underpinning technique, with temporary propping supporting the excavation, which is set out in the Basement Method Statement issued by Croft and indicated in Drawing No. SK01 included in Appendix A.

9.2.3 Groundwater Control

As discussed in Section 8.1.1 groundwater is not anticipated to affect the construction works. If localised seepages are encountered of groundwater that is likely to impact the works, groundwater could be controlled by pumping to a tank prior to disposal by tanker to an approved facility. Alternatively discharge of the groundwater could be made to the sewer subject to an agreement at detailed design stage from the local water company in terms of water quality, flow rate and quantity.

9.2.4 Monitoring of groundwater and ground movements

Groundwater levels should be monitored before the works as a precaution. Monitoring of adjacent structures and the highway should be carried out before, during and after construction.

10 Ground Movement Assessment

10.1 Introduction

This section provides an assessment of ground movement that may result from the construction of the basement and to determine how these may affect the adjacent building structures, the highway and host property.

The proposed construction sequence for the basement is summarised as:

Phase 1: Excavate in 1m width max. and install props and trench sheets as excavation progresses

Phase 2: Cast retaining wall and base. Remove props after concrete has gained sufficient strength

Phase 3: Excavate and cast remainder of base slab. Install full length props while central soil mass is being removed. Proceed with above ground construction.

The full details of the construction are included in the Basement Method Statement by Croft. The sequence of casting the retaining walls sections is shown in Drawing SK-01 in Appendix A.

Ground movements resulting from underpinning are not well documented and there is no specific method for assessing their magnitude. It should be noted that CIRIA C760 (2017), which is often used as a reference for ground movement assessments, is for embedded retaining walls and not concrete underpins.

When underpinning is carried out in a well-controlled manner, movements are typically small. A widely accepted movement from the installation of underpins is for 5mm of horizontal and vertical movement for a single stage underpinning, in addition to the global movements from excavation and subsequent settlement from the imposed loads acting on the underpins.

The ground conditions at 15 Lyncroft Gardens are predominantly London Clay, which will display heave from excavation and long term settlement from the imposed loads.

The following ground movements have been assessed:

- **Short term vertical heave / settlement movements:** London Clay and is susceptible to short term heave and time dependent swelling on unloading, which will occur because of basement excavation, generating upward ground movements. Short term heave has been analysed by Pdisp in the undrained condition.
- **Long term vertical ground movement in the drained condition:** The net loading / unloading on formation soils will generate ground movement, which could affect adjacent foundations which will happen over a period after construction. This has

been modelled with Pdisp. This takes into account existing stress conditions, and the weight of soil removed and the load from the new basement.

- **Vertical and horizontal movement from underpin installation:** Underpins act as stiff concrete retaining walls, which limits the potential for wall deflection. However, deflections that do occur may generate surface settlements, which could impact adjacent properties.

From experience within in the industry, at least 5mm of additional ground movement (both vertical and horizontal) is typically anticipated for the proposed single stage underpinning.

10.2 Modelling of movements due to vertical and horizontal stress changes

The predicted ground response due to vertical unloading of the ground through excavation for the proposed basement has been modelled using the OASYS program PDisp version 20.12.

PDisp assumes a linear elastic behaviour of the soil and a flexible structure. The finite stiffness of the structures will tend to redistribute or smooth out the movements, when compared to those predicted by PDisp. The settlement calculations therefore represent free field movements unaffected by the stiffness of the structures and are likely to be conservative (i.e. the distortions of the structure would be less than those obtained from the predicted movements).

The analysis was undertaken for the combination of short-term undrained movements and long-term drained movements. The 'hard layer' base to the analysis was taken as 25 m below ground level. In addition, it has been assumed for ground modelling that the soil mass is removed in its entirety before the underpins and are placed, when in reality this is an incremental process. When the overall mass of the soil removed relative to the load of the re-imposed structure is considered onto a cohesive soil this presents a reasonable scenario.

10.2.1 Vertical Movements due to excavation (Undrained/short term)

The excavation level was assumed at 3.0 m below ground level where full excavation is required or 1.7 where the existing cellar is to be extended. Demolition and excavation of up to 3.0 m of soil will therefore produce an unload at new formation level of - 60 kPa or 34Kpa below the cellar. Poisson's Ratio for London Clay as $\nu_u = 0.5$.

A short term (undrained) analysis was undertaken using parameters in Table 5.3 above to determine the heave movements likely to arise as a result of the excavation (i.e., the movements likely to occur prior to the construction of the new structural elements and the consequential vertical loading of the soil). The analysis indicated a maximum heave of 9.2 mm occurring centrally within the excavation (Figure 6.1, 6.3 & 6.4), with 4.0 mm at the boundary with 13 Lyncroft Gardens, and 5.2 mm at 17 Lyncroft Gardens.

10.2.2 Vertical movements following construction of the new basement (drained/long-term)

The movements of the ground following construction are assessed for the long term (drained) case using parameters in Table 5.3 above.

The PDisp assessment indicates that peak heave movements in the long term again occur under the centre of the basement, with a magnitude of 6.3 mm occurring centrally within the excavation (Figure 6.2, 6.5 & 6.6), with -0.5 mm at the boundary with 13 Lyncroft Gardens, and 0.75 mm at 13 Lyncroft Gardens.

10.2.3 Vertical deflection from underpin installation

As indicated above in Section 10.1, 5mm of vertical movement is assumed for installation. The distance behind the wall to which negligible movement occurs has been assumed at 3.5 times the wall height.

10.2.4 Horizontal deflection from underpin installation

As indicated above in Section 10.1, 5mm of horizontal movement (δ_{max}) at the basement wall is assumed for installation. The distance behind the wall to which negligible movement occurs is assumed to be 4 times the height of the underpin of 3m. δ_h is the difference between δ_{max} and the movement of the far wall of the neighbouring property.

It should be reiterated that the movements due to vertical and horizontal stress changes do not occur in isolation to the other movements resulting from the basement construction process and the actual ground movements, particularly around and beyond the perimeter of the proposed basement, will be from the quality of workmanship during excavation and installation.

11 Damage Category Assessment

11.1 Introduction

The calculated ground movements have been used to assess potential ‘damage categories’ that may apply to neighbouring properties, the highway footway and host property due to the proposed basement construction. The methodology proposed by Burland and Wroth and later supplemented by the work of Boscardin and Cording has been used, as described in *CIRIA Special Publication 200* and *CIRIA C760*. General damage categories are summarised in Table 11.1 below:

Table 11.1: Classification of damage visible to walls (reproduction of Table 6.4, CIRIA C760)

Category	Description	Approx. Crack Width (mm)	Limiting tensile Strain (ϵ_{lim})
0 (Negligible)	Negligible – hairline cracks	<0.1	0.0 – 0.05
1 (Very slight)	Fine cracks that can easily be treated during normal decoration	<1	0.05 – 0.075
2 (Slight)	Cracks easily filled; redecoration probably required. Some repointing may be required externally.	<5	0.075 – 0.15
3 (Moderate)	The cracks require some opening up and can be patched by a mason. Recurrent cracks can be masked by suitable linings. Repointing of external brickwork and possibly a small amount of brickwork to be replaced.	5 -15 or a number of cracks > 3	0.15 – 0.3
4 (Severe)	Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows.	15-25 but also depends on number of cracks	> 0.3
5 (Very Severe)	This requires a major repair involving partial or complete re-building.	> 25 but also depends on number of cracks	

11.2 Damage Assessment Categories for neighbouring properties 13 and 17

Lyncroft Gardens

Vertical ground movement for a section through 13 and 17 Lyncroft Gardens are shown in Figure 11.1 and 11.2 respectively. For these wall sections, the combined impact of short-term

heave and long-term settlement/ heave and installation has been shown. The location of the sections is shown diagrammatically in Figures 6.1/6.2. The difference in actual values between 13 and 17 Lyncroft Gardens reflects the deeper excavation on the party wall with No.17, due to the presence of an existing cellar against the party wall with 13 Lyncroft Gardens.

Table 11.2 incorporates superimposed horizontal and vertical movements derived from the wall deflection and heave/settlement due to excavation as outlined in Section 10. The assessment is based on the assumption of stiff clays. While the base of the excavation has an undrained cohesion of 67.5 kPa or 'firm' the clay increases steadily in strength below the excavation therefore an overall category of 'stiff' (75 kPa and greater) is considered appropriate. The assessment has been based on the limiting tensile strain for Category 1 of a strain of 0.075 %.

Table 11.2: Summary of ground movements and corresponding damage category 13 & 17 Lyncroft Gardens

Adjacent Property	13 Lyncroft	17 Lyncroft
Building width - L (m)	6.5	6.5
Building height - H (m)	11	11
L/H = 0.6 (approximated for plotting)	0.5	0.5
max deflection (Δ) in metres (from Fig 11.1/11.2)	0.0021	0.0028
Δ/L (%)	0.032	0.043
ϵ_{lim}	0.075	0.075
$\Delta/L/\epsilon_{lim}$	0.43	0.57
length to negligible horizontal movement - 4x wall height (m)	12	12
δh_{max} (m)	0.005	0.005
δh (m)	0.0026	0.026
$\delta h/L$ (%) = ϵ_h	0.040	0.040
Damage Category	1	1

11.3 Impact on Highway/Footway

The ground movement assessment has been undertaken for the footway pavement of Lyncroft Gardens, which is approximately 2.5m from the lightwell. Figure 11.3 indicates that a combined vertical movement of 2.8mm and a horizontal movement of 5mm (a maximum movement assuming no attenuation in the distance from the lightwell). It should be noted that from a utilities search (Croft BIA ref: 190906 Appendix C) only a power cable was identified within the footway. Therefore, it is not considered this ground movement will have a significant impact on the footway or the highway.

11.4 Damage Assessment Categories for 15 Lyncroft Gardens

The Damage Assessment Category for the host property No. 15 Lyndhurst Gardens has been considered. The key factor in considering the damage assessment is the differential movements which might be generated. It is recognised that within the centre of the basement there will be heave of up to 11mm. However, this will act on the basement floor slab only and not on the structure above. It is common practice to install void former below the basement slab to account for heave and the value of 11mm is well within normal construction tolerance. Due to the different depth of excavation on either side of the property differential movements could arise, in the order of 2.5mm, creating a vertical movement strain as indicated in Figure 11.4. Additionally, horizontal strains may be generated. The maximum horizontal strain has been modelled as 5mm for the wall being installed. The overall damage assessment category for 15 Lyncroft Gardens is 1, as indicated in Figure 11.5 and 11.6, as indicated in Table 11.3

Table 11.3: Summary of ground movements and corresponding damage category 15 Lyncroft Gardens

Property	15 Lyncroft
Building width - L (m)	6.5
Building height - H (m)	11
L/H = 0.6 (approximated for plotting)	0.5
max deflection (Δ) in metres (from Fig 11.4)	0.0025
Δ/L (%)	0.038
ϵ_{lim}	0.075
$\Delta/L/\epsilon_{lim}$	0.51
length to negligible horizontal movement - 4x wall height (m)	12

Property	15 Lyncroft
δh_{\max} (m)	0.005
δh (m)	0.0026
$\delta h/L$ (%) = ϵh	0.04
Damage Category	1

12 Monitoring Strategy

The results of the ground movement analysis show that with good construction control, damage to adjacent structures generated by the assumed construction methods and sequence can be controlled to be within Category 1 'slight' damage. A formal monitoring strategy should be implemented on site in order to observe and control ground movements during construction.

The system should operate broadly in accordance with the 'Observational Method' as defined in CIRIA Report 185. Monitoring can be undertaken by installing survey targets to the top of the wall and face of the adjacent building. Baseline values should be established prior to commencement of works. Monitoring of these targets should be carried out at regular time intervals and the results should be analysed to determine if any horizontal translation of the wall or tilt/settlement of the neighbouring structure is occurring. Regular monitoring of these targets will allow ground movement trends to be detected early and a mitigation strategy can be implemented to control further movement. Monitoring data should be checked against predefined trigger limits and can also be further analysed to assess and manage the damage category of the adjacent buildings as construction progresses.

It is recommended that a condition survey is undertaken on all adjacent property facades prior to the works commencing and ideally when monitoring baseline values are established. Existing cracks or structural defects should be carefully recorded, documented and regularly inspected as construction progresses.

13 Conclusions

The results of this Basement Impact Assessment are supported by site investigation data and outline construction methods and sequence provided by the structural engineer.

The maximum damage category for the adjacent properties has been calculated to be within Category 1 (slight damage). The assessment has indicated a potential movement of 2.8 vertical and 4mm horizontal for the footway of Lyncroft Gardens. The maximum damage category for the host property has also been calculated to be within Category 1.

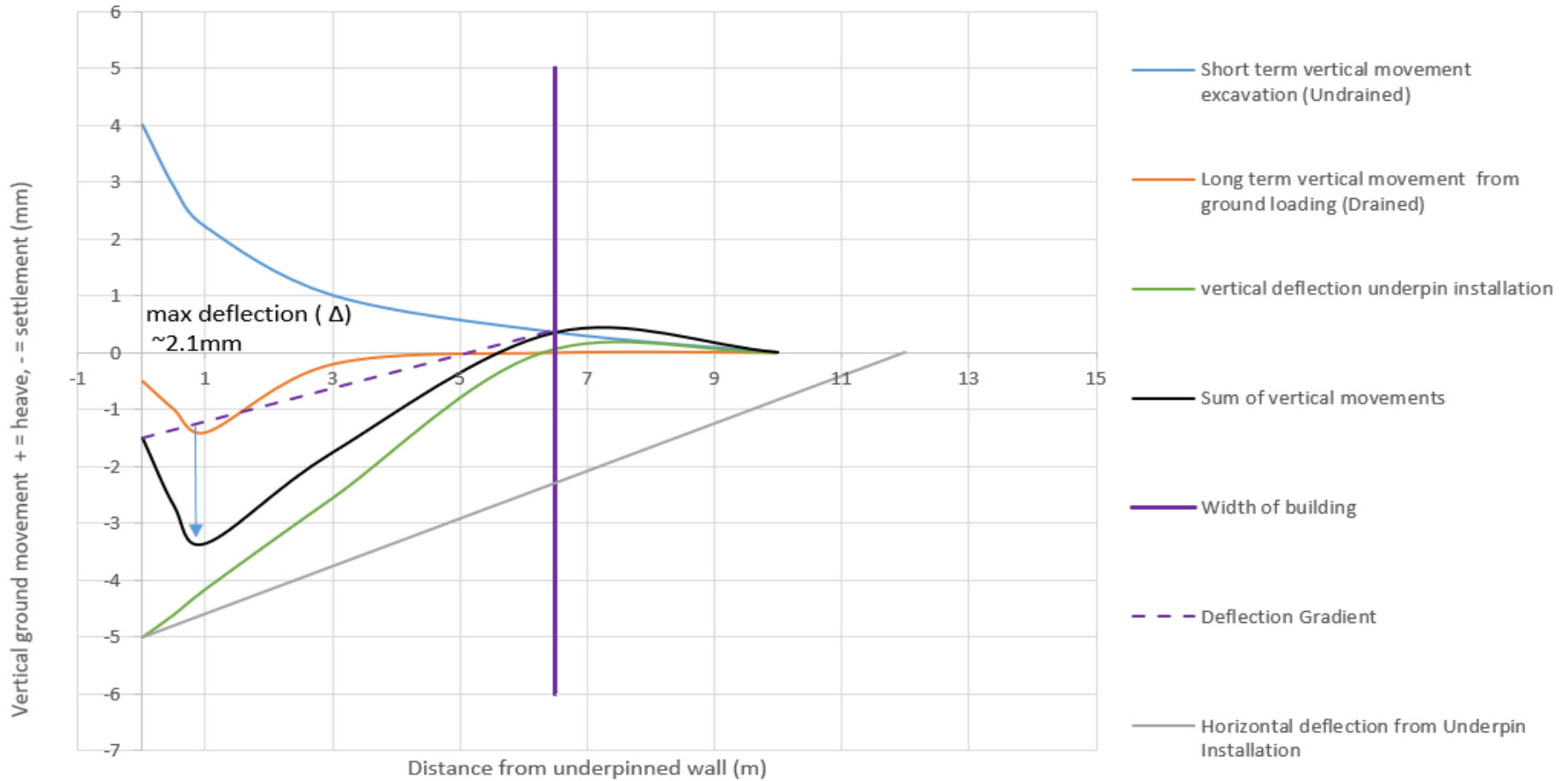
An appropriate monitoring regime should be adopted and maintained throughout construction to manage risk and potential damage to the neighbouring structures as construction progresses onsite.

14 References

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Figures

No. 13 Lyncroft Gardens



Client

Croft Structural Engineers Ltd.



Project

15 Lyncroft Gardens NW6 1LB

Job No.

MGC/19/29

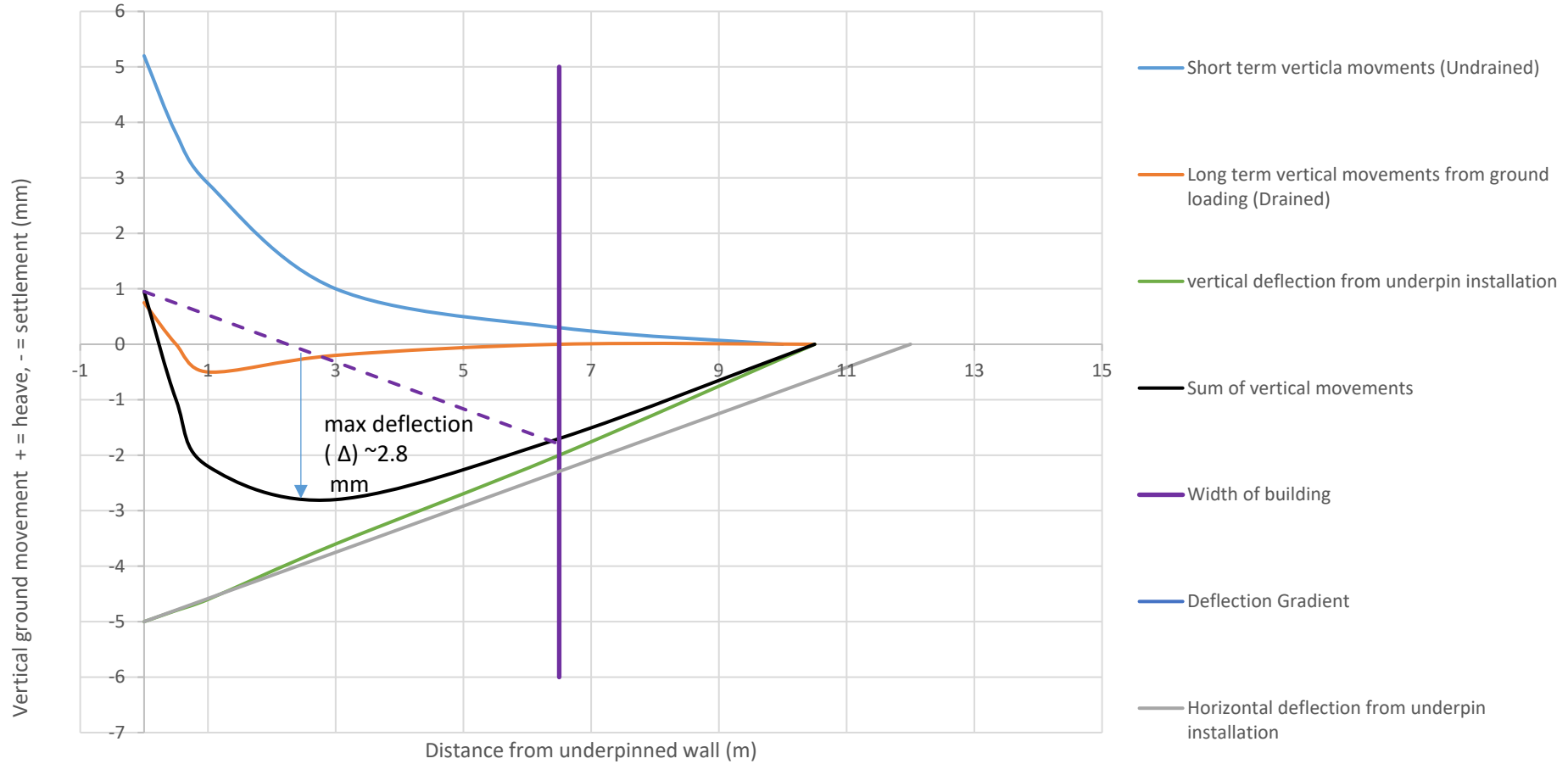
Title

Combined Vertical Movements for 13 Lyncroft Gardens

Figure

11.1

No. 17 Lyncroft Gardens



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Job No.

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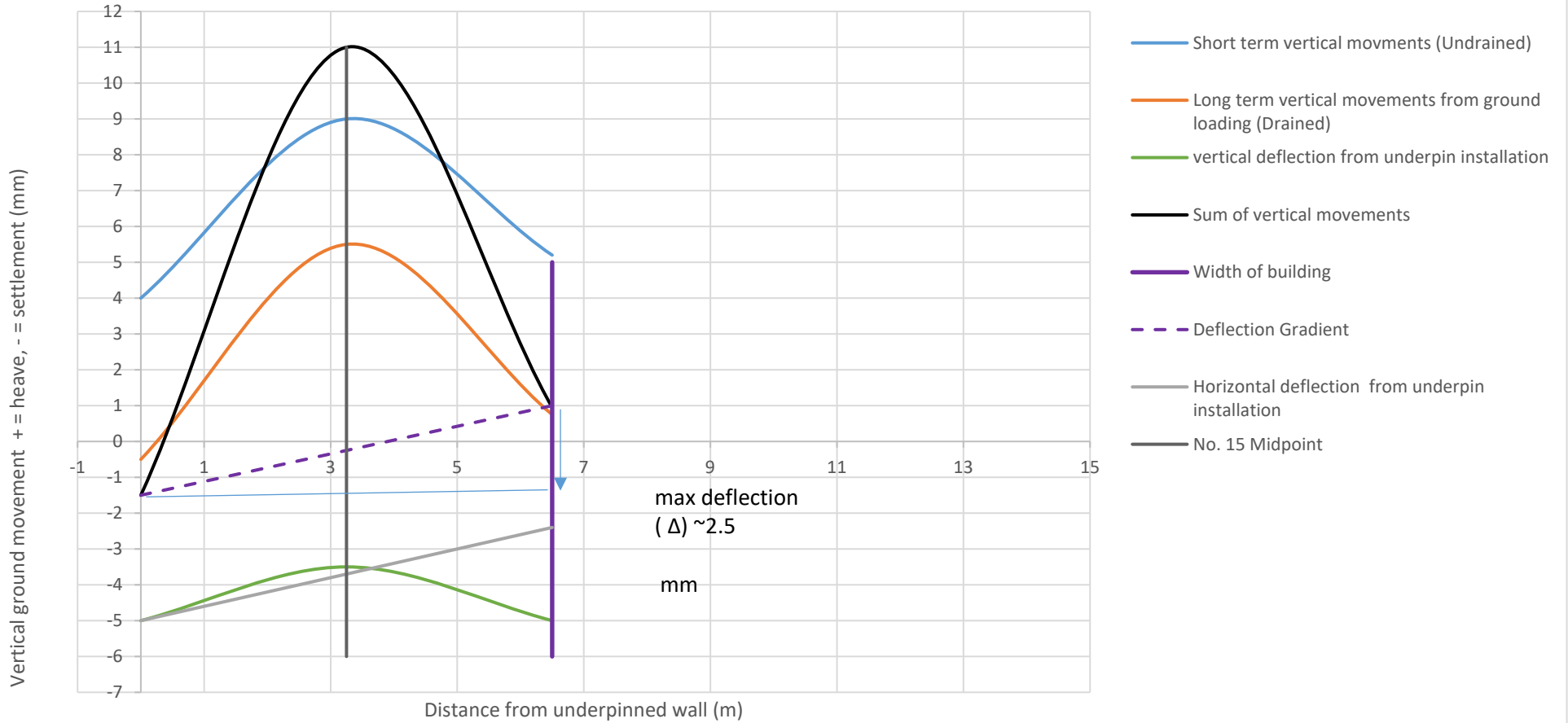
Title

Combined Movements Section 1-1', 17 Lyncroft Gardens

Figure

11.2

No. 15 Lyncroft Gardens



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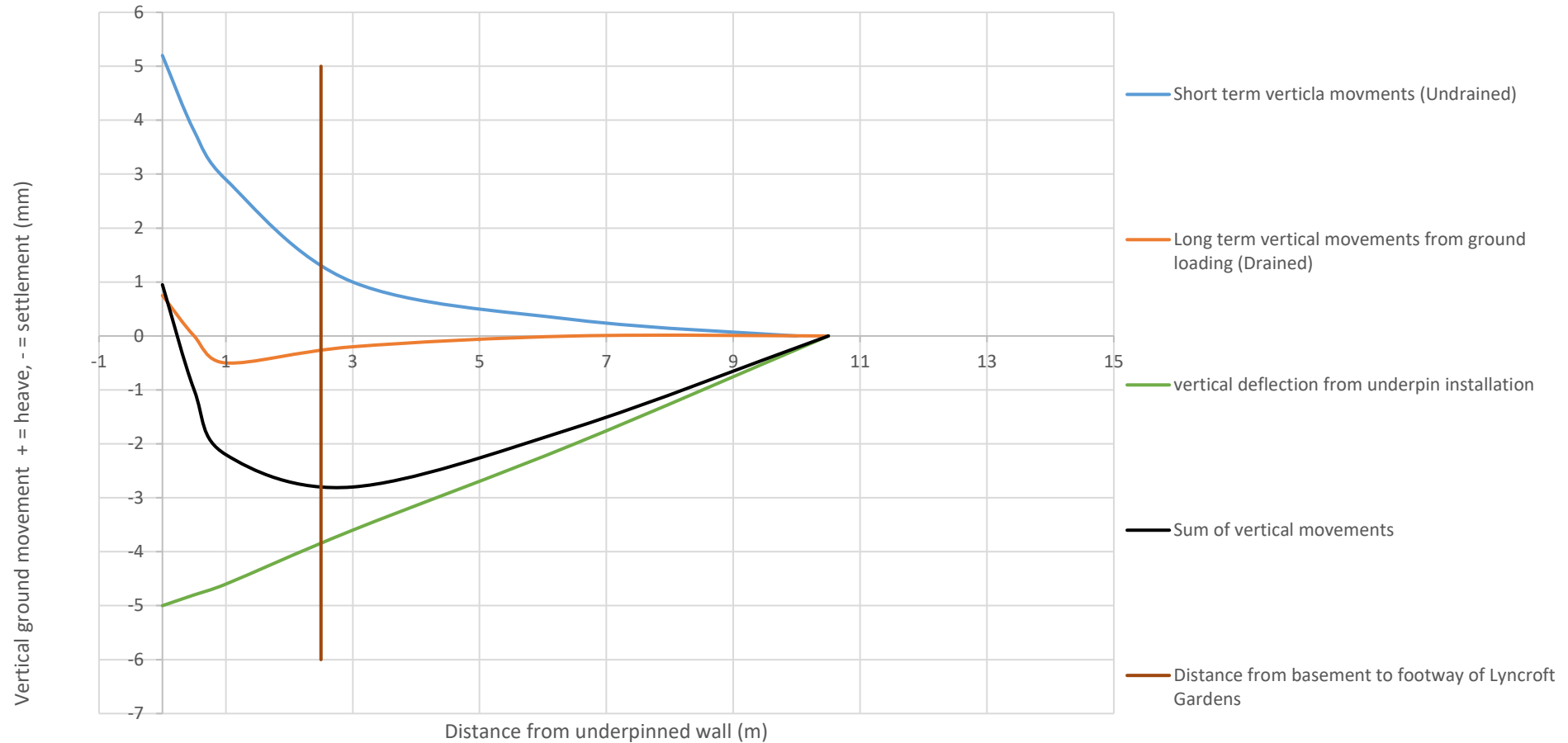
Title

Combined Vertical Movements for Host Property

Figure

11.4

Lyncroft Gardens (Footway)



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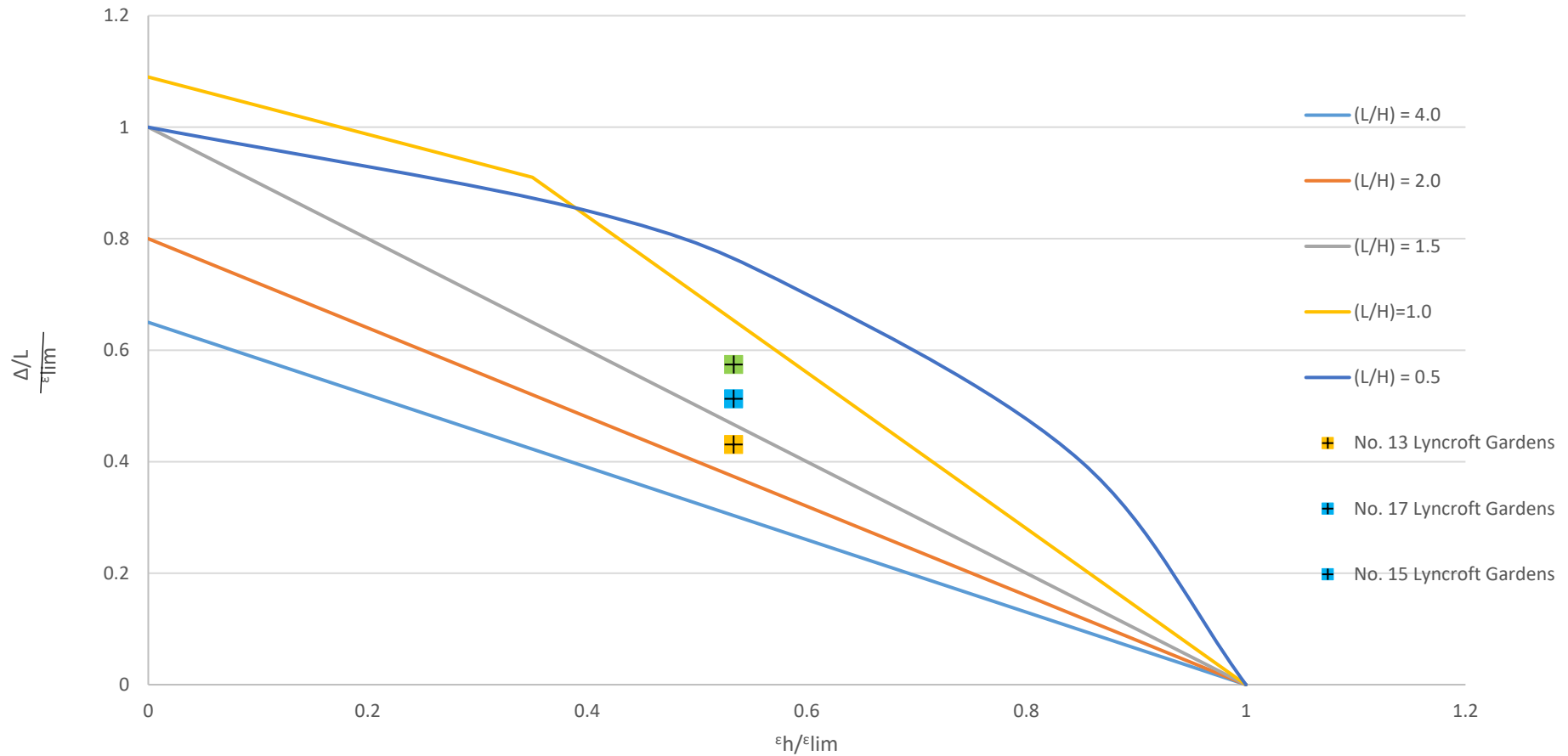
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Combined Vertical Movements for Lyncroft Gardens Footway Pavement

Figure

11.3

Influence of Horizontal Strain on $(\Delta/L)/\epsilon_{lim}$



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Job No.

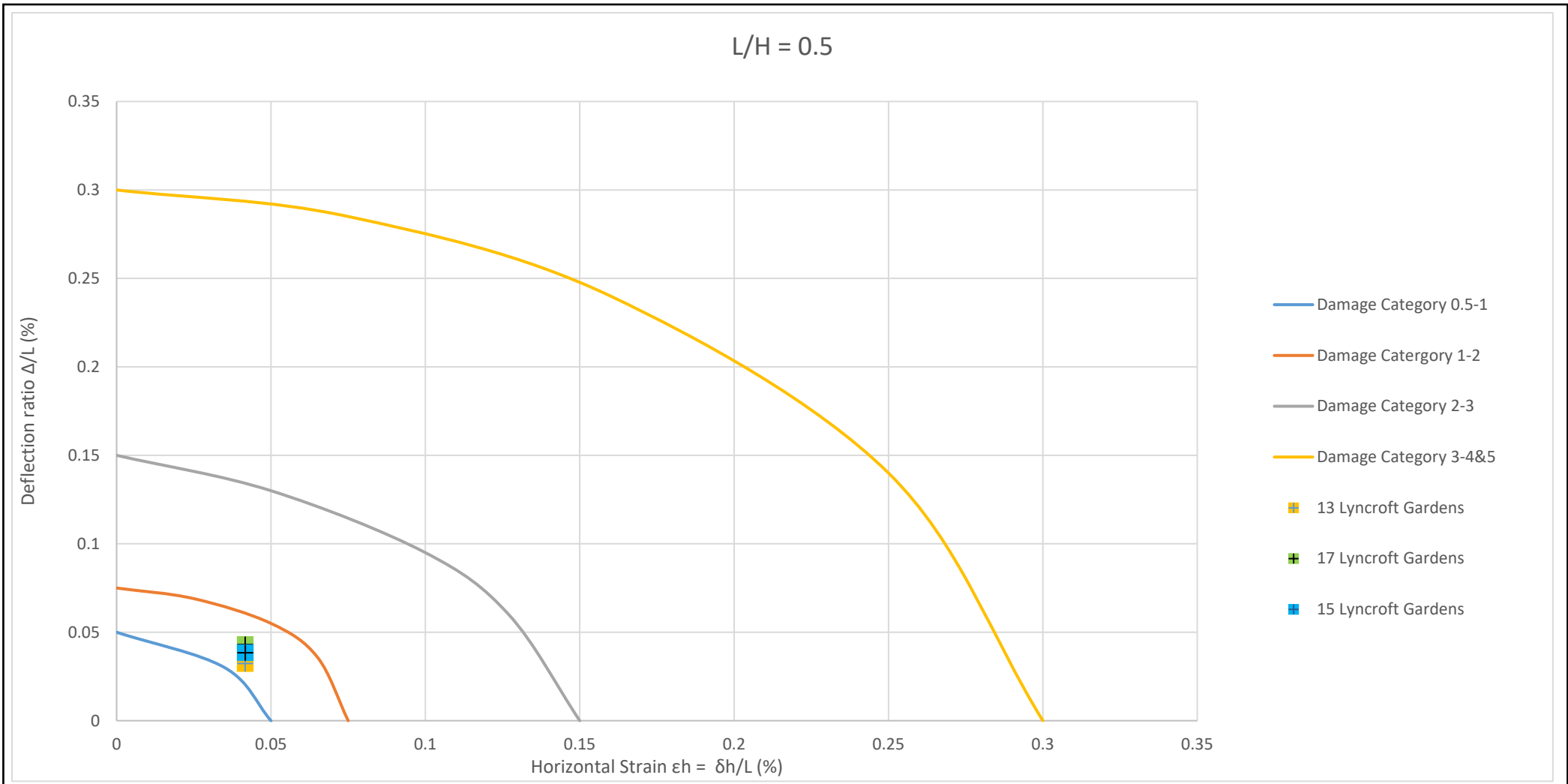
MGC/19/29

Title

Influence of Horizontal Strain on Deflection Ratio

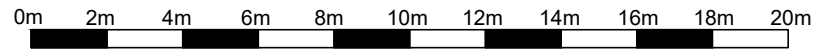
Figure

11.5



Client Croft Structural Engineers Ltd.	<div style="background-color: #555; color: white; padding: 10px; border-radius: 15px; display: inline-block;"> MAUND GEO-CONSULTING </div>	Project 15 Lyncroft Gardens NW6 1LB
Job No. MGC/19/29	Title Damage Assessment Category	Figure 11.6

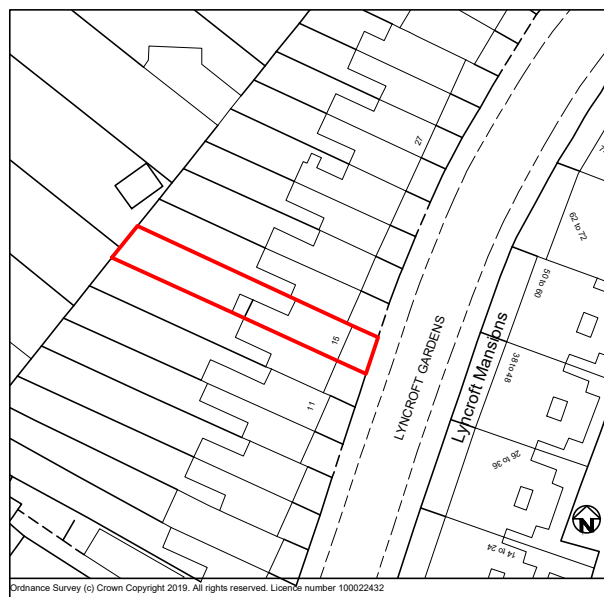
Appendix A Proposed Basement Drawings



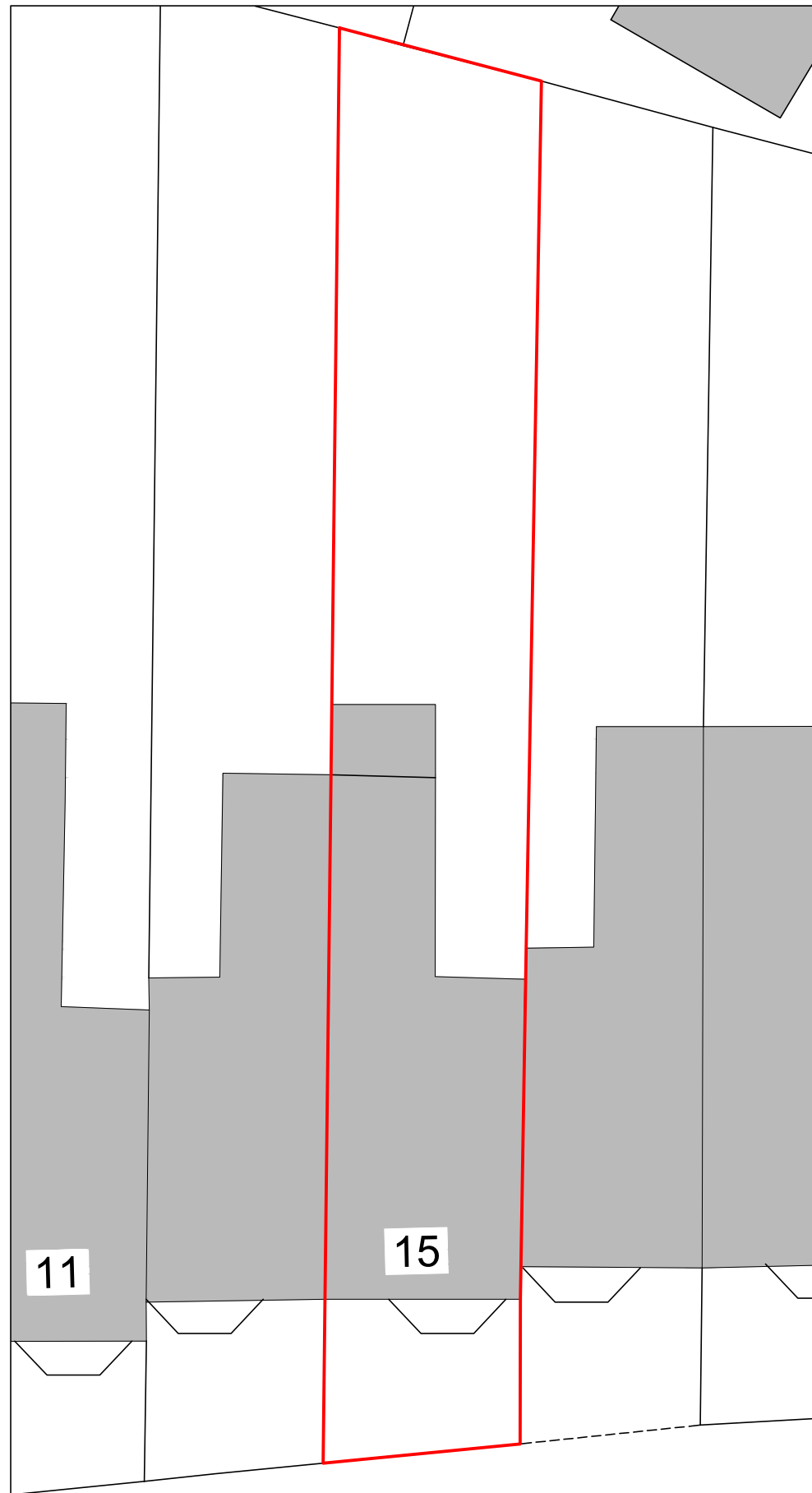
Scale 1:200



Scale 1:1250



1 / Location Plan 1:1250



2 / Existing Site Plan 1:200



3 / Proposed Site Plan 1:200

Job Title:
Basement Extension

Drawing Title:
Site and Location Plans

Revision Notes:

CLIENT : Mr Richard Banks
SITE : 15 Lyncroft Gardens, London NW6 1LB

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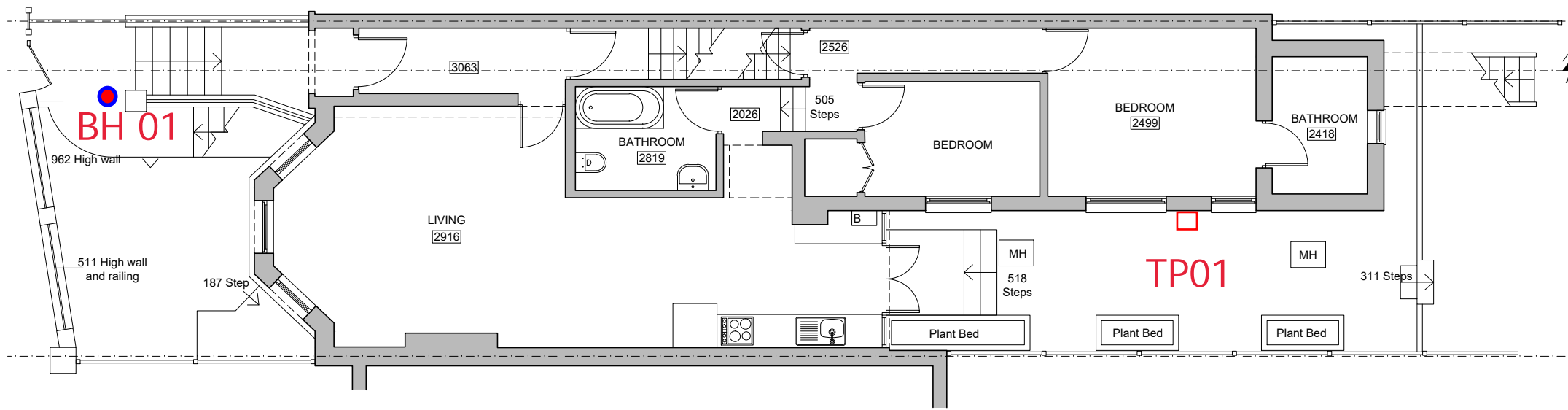


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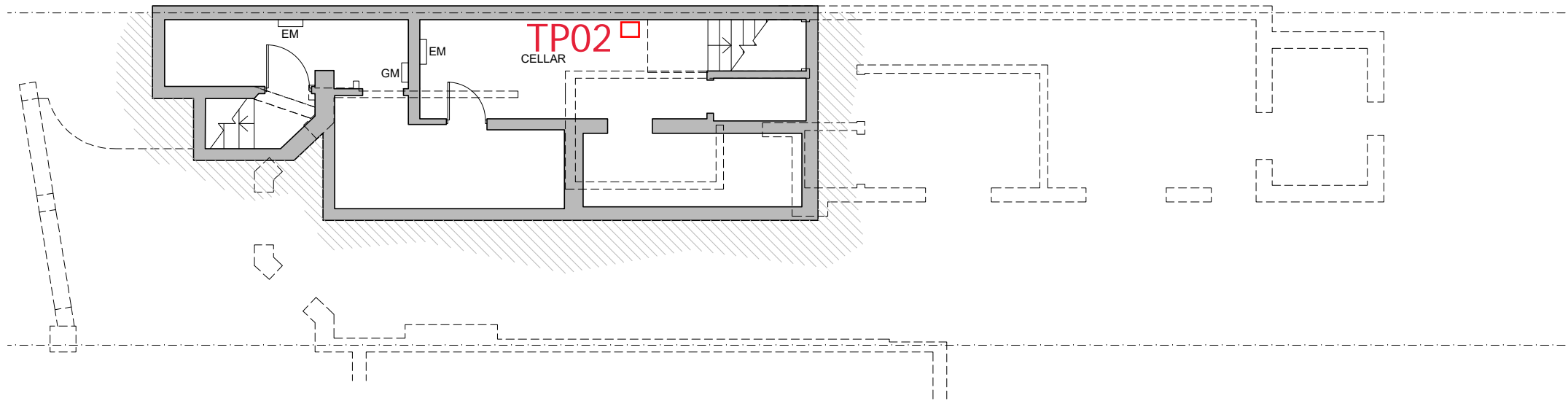
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Drawn By: SS Checked By: CG

PROJECT	Drawing No	Rev
1492	01	-



1 / Ground Floor Plan



2 / Basement Plan

Job Title:
Basement Extension

Drawing Title:
Existing Floor Plans

Revision Notes:

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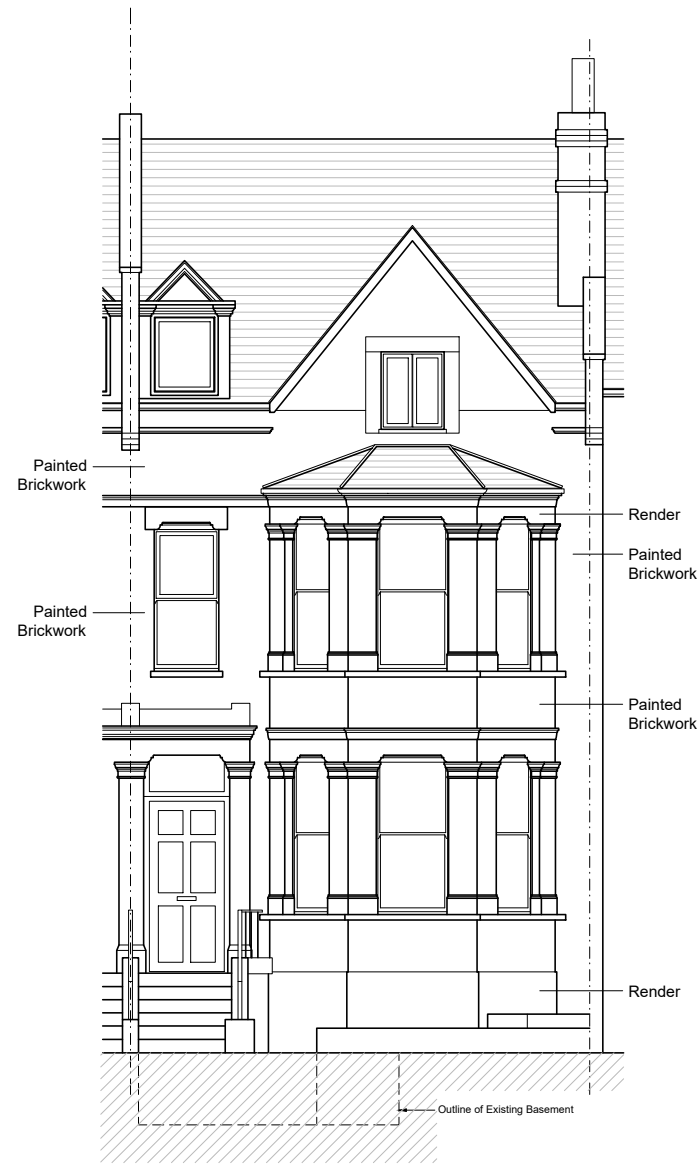
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1492	02	-

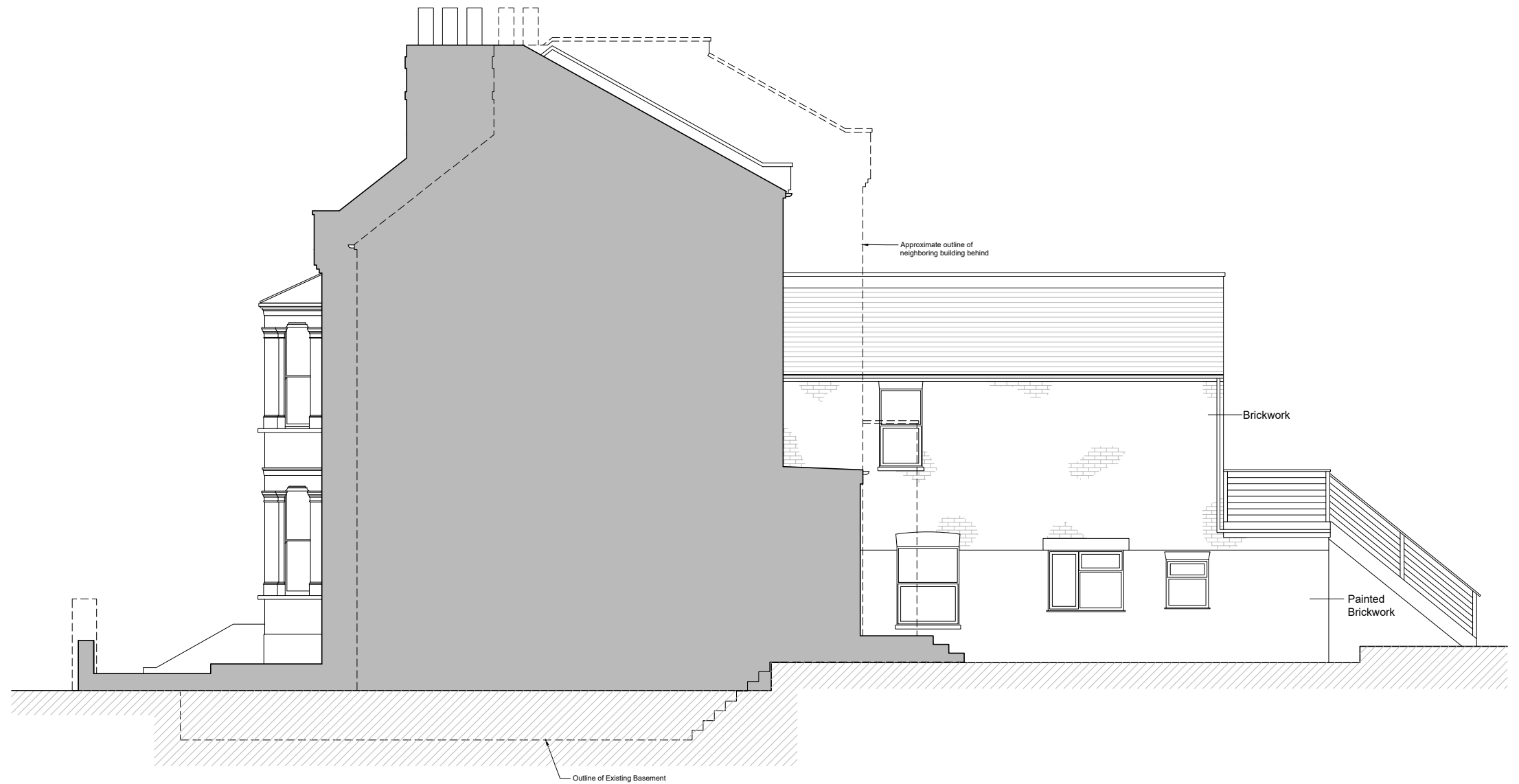
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0m 1m 2m 3m 4m 5m 6m 7m 8m 9m 10m

Scale 1:100



1 / Front Elevation



2 / Side Elevation

Job Title:
Basement Extension

Drawing Title:
Existing Elevations

Revision Notes:

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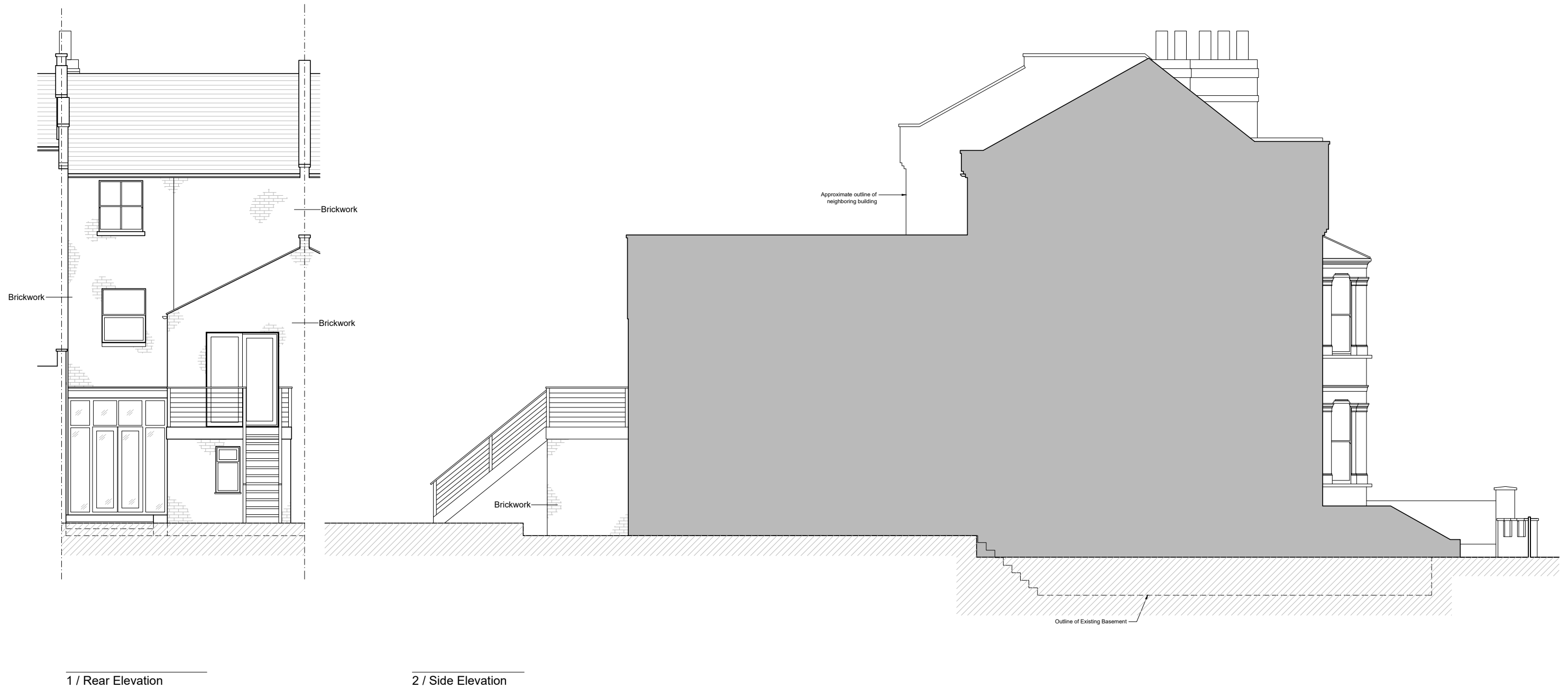
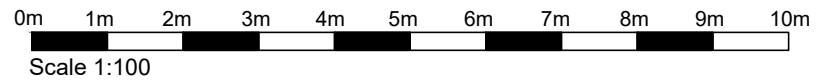
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PROJECT
1492

Drawing No
03

Rev
-



Job Title:
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Drawing Title:
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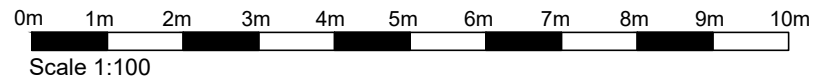
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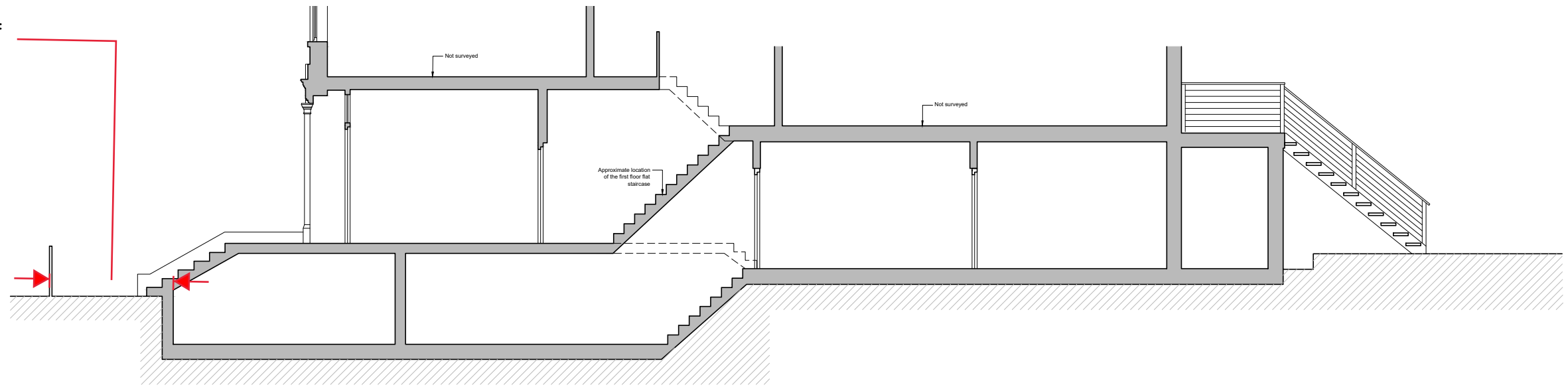
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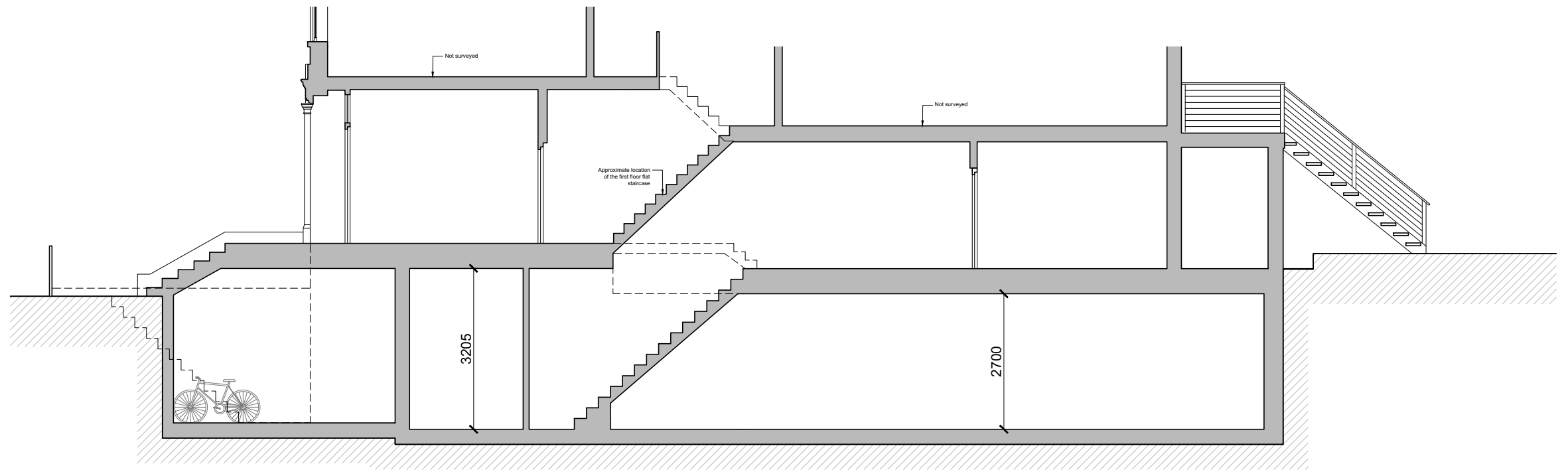
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Distance:
2,466.77



1 / Existing Section A-A



2 / Proposed Section A-A

Job Title:
Basement Extension

Drawing Title:
Existing and Proposed Sections

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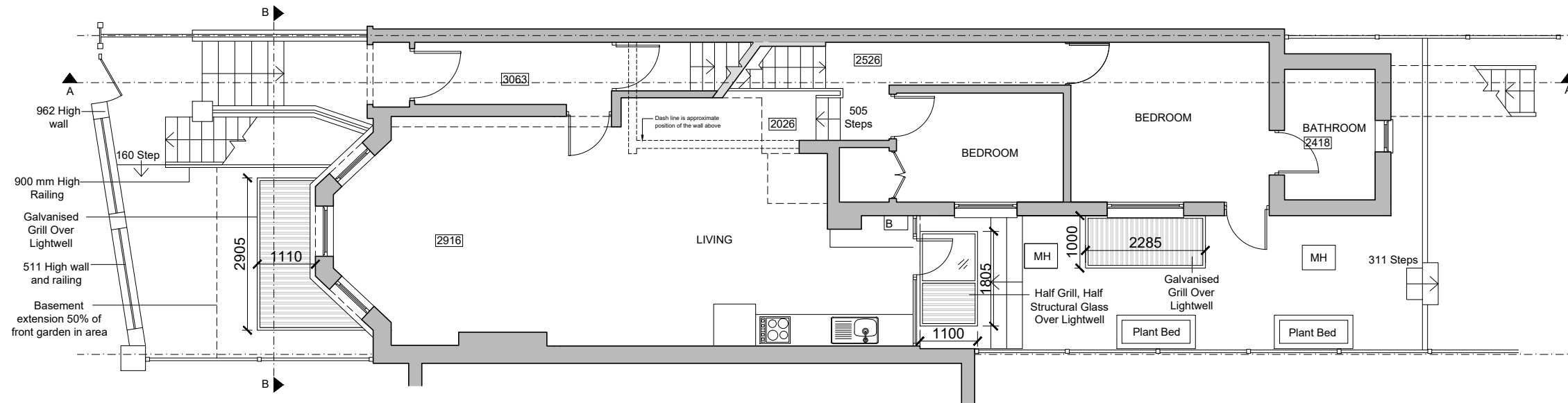
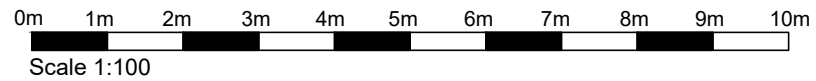
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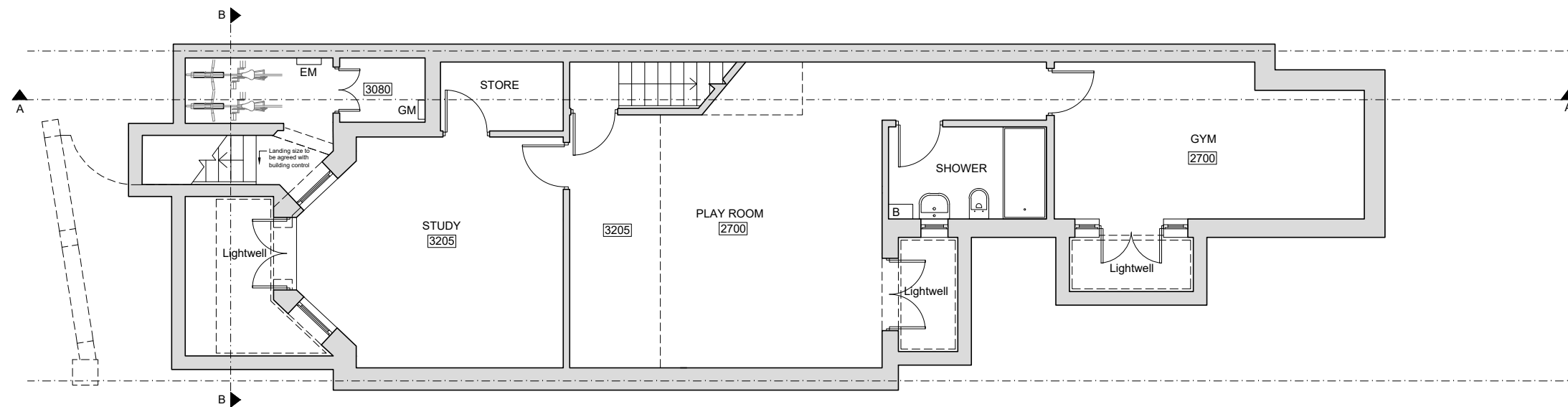
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Drawing No
05

Rev
-



1 / Ground Floor Plan



2 / Basement Plan

Job Title:
Basement Extension

Drawing Title:
Proposed Floor Plans

Revision Notes:

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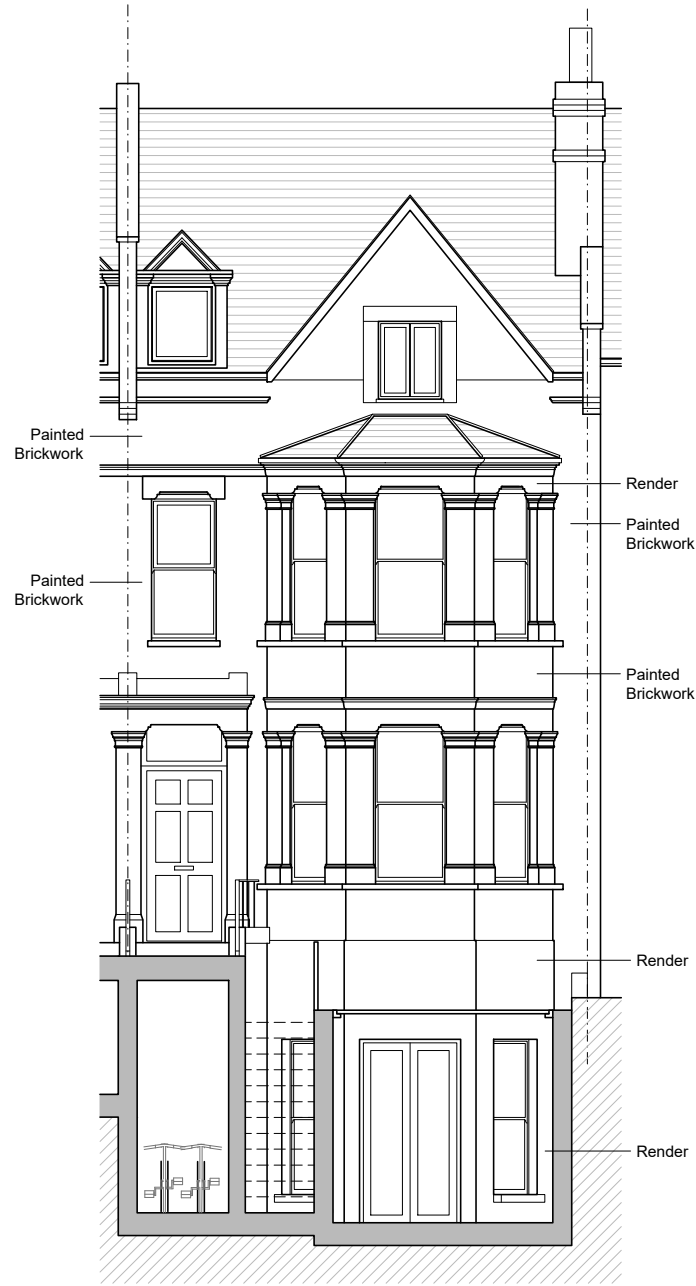
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06

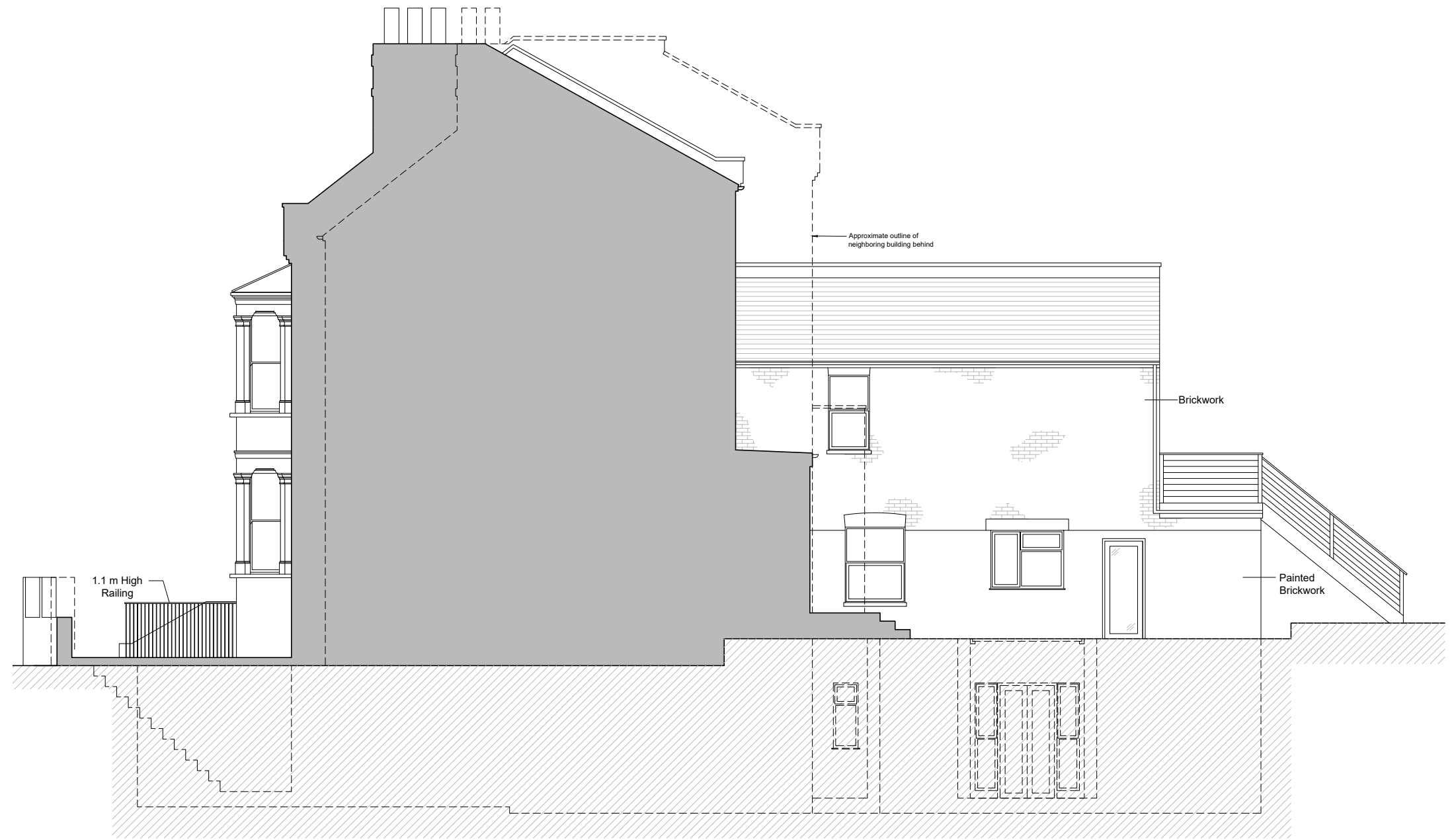
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0m 1m 2m 3m 4m 5m 6m 7m 8m 9m 10m

Scale 1:100



1 / Front Elevation



2 / Side Elevation

Job Title:

Basement Extension

Drawing Title:

Proposed Elevations

Revision Notes:

-

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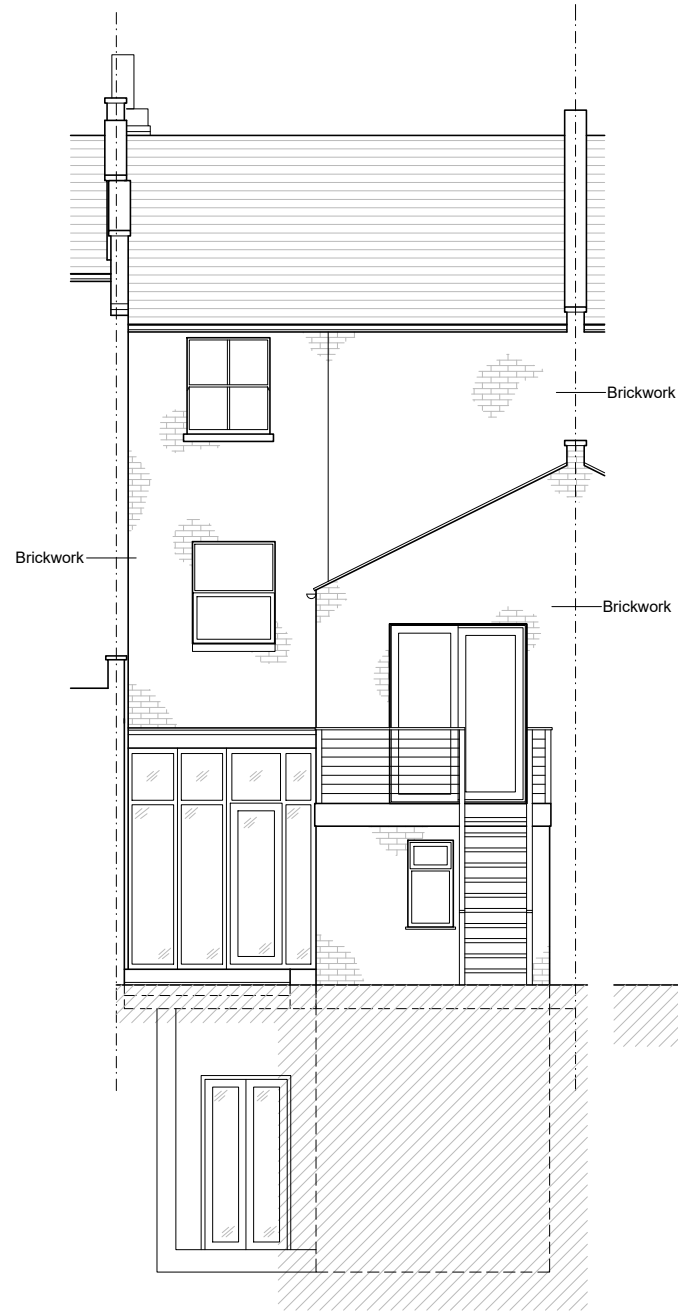
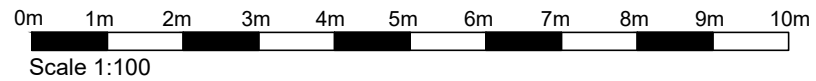
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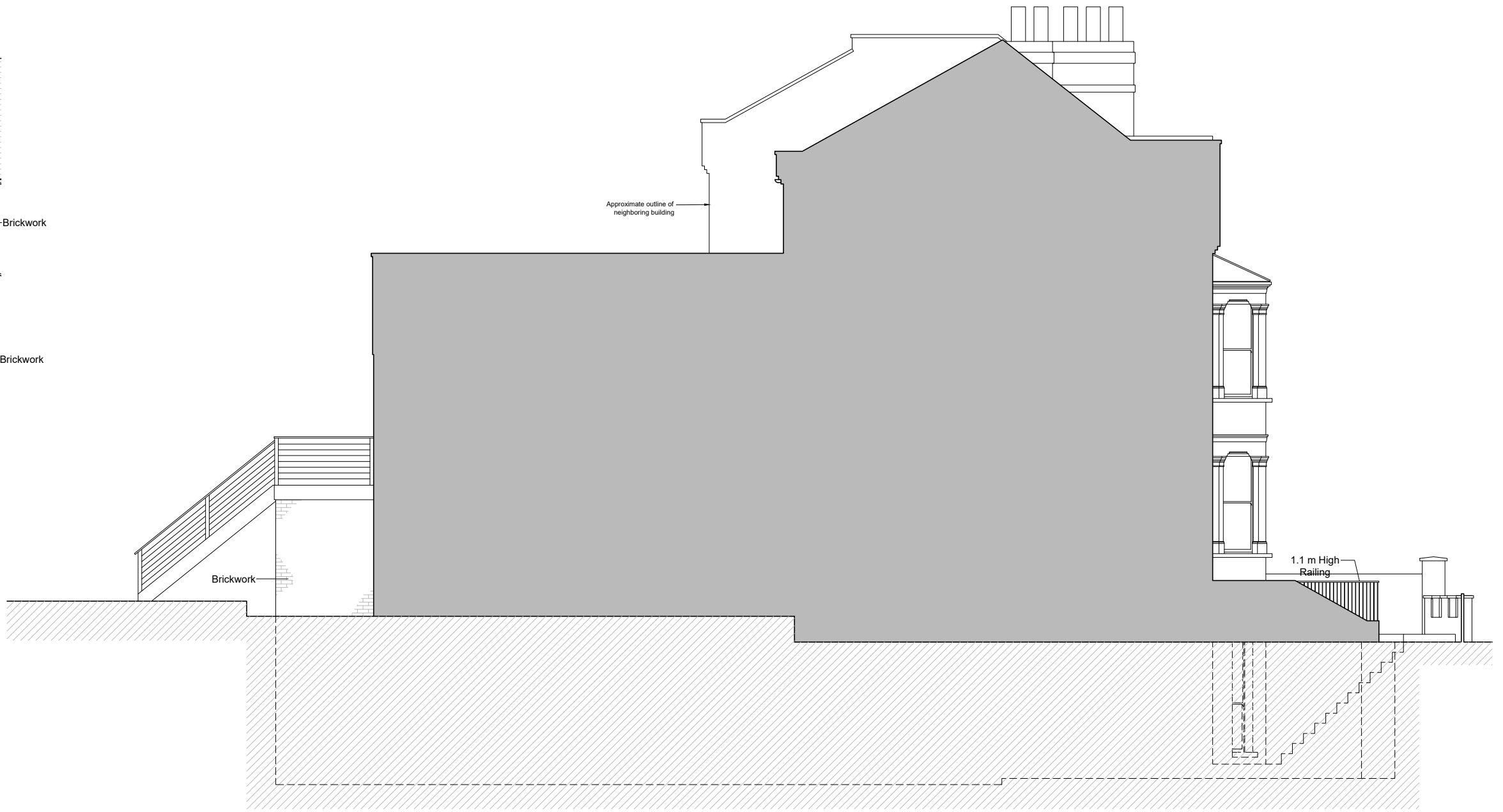
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Drawing No
07

Rev
-



1 / Rear Elevation



2 / Side Elevation

Job Title:
Basement Extension

Drawing Title:
Proposed Elevations

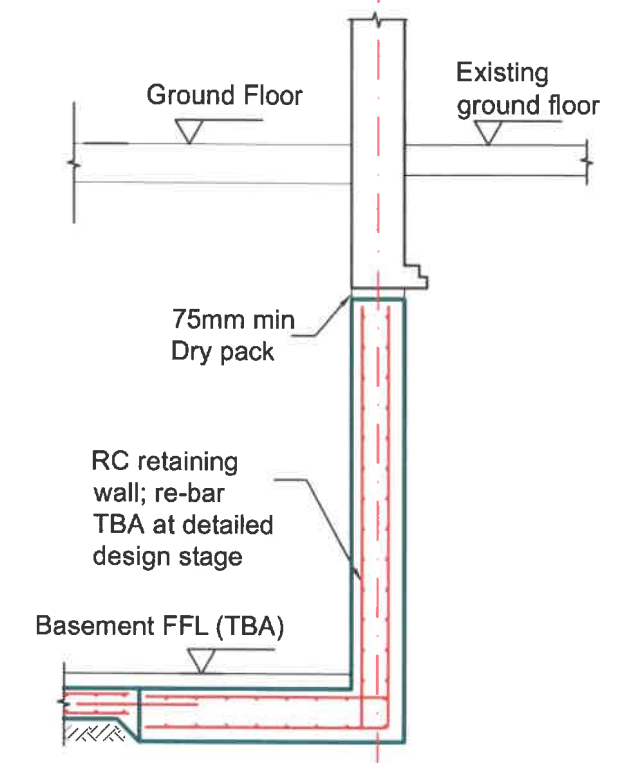
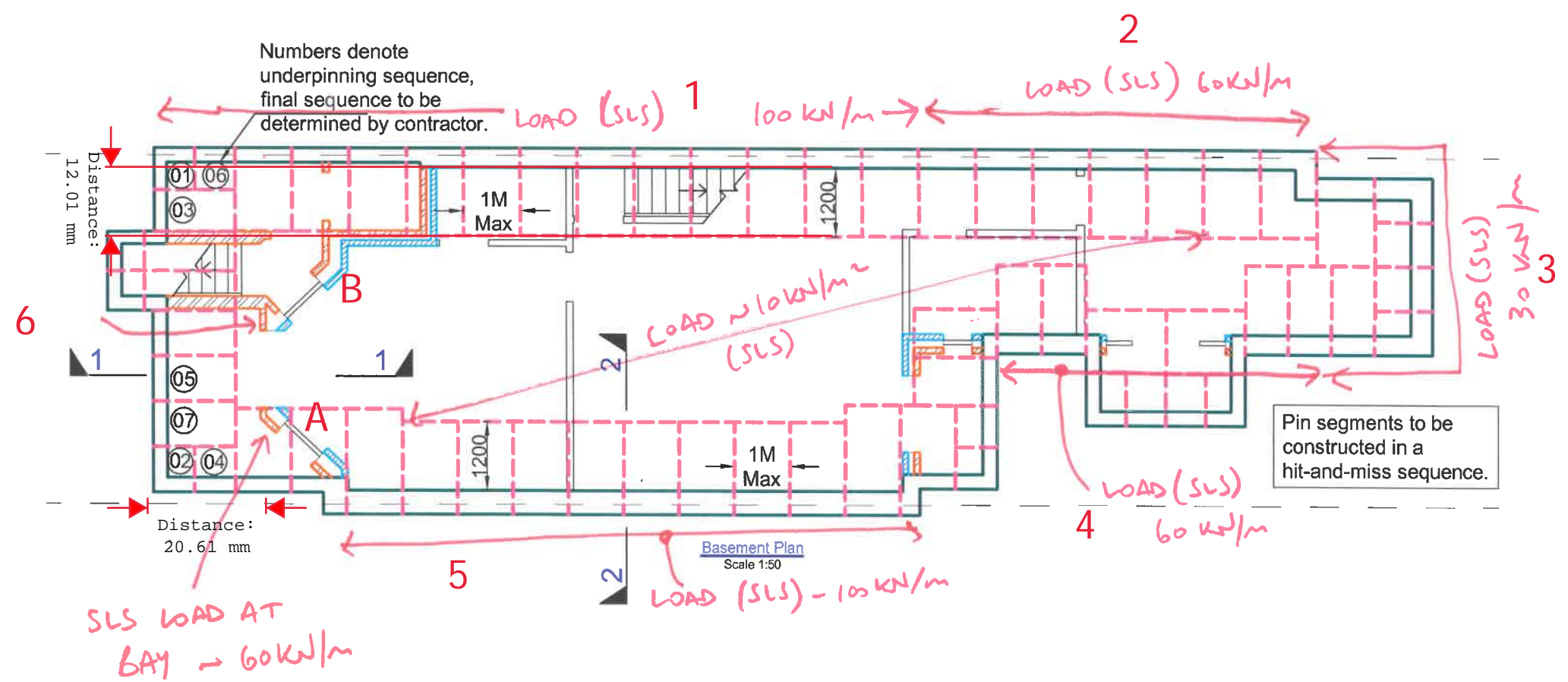
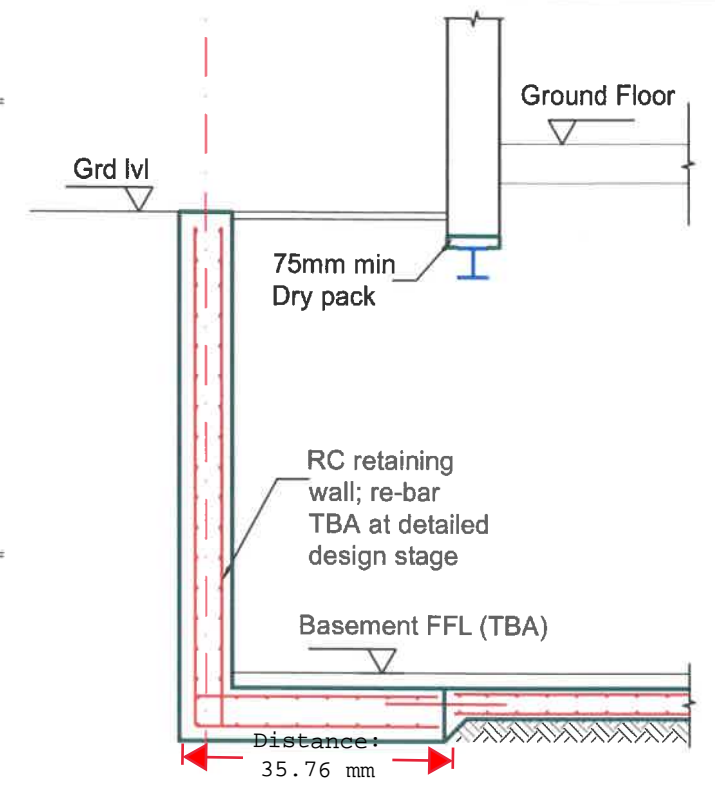
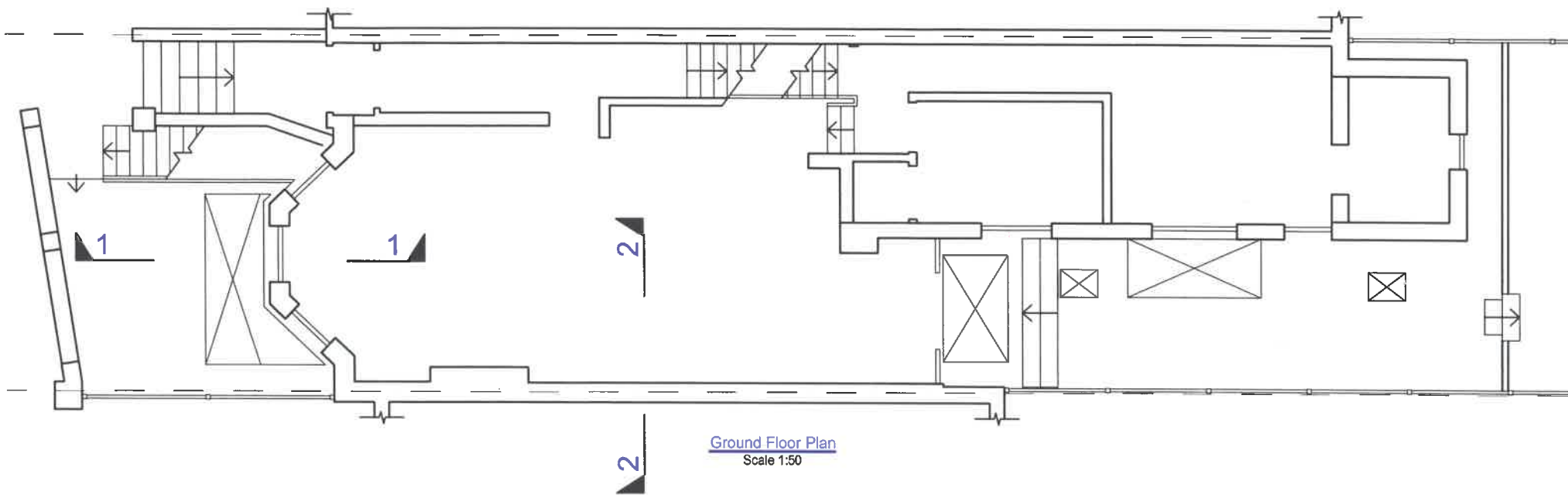
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PROJECT 1492	Drawing No 08	Rev -



Job Number 190906	Date Sept '19
Dwg Number SL-10 SK-01	Rev -
Drawn CT pdh	Chkd CT
Scale As shown @ A3	

Client: Advantage Basement & Cellar Company Ltd

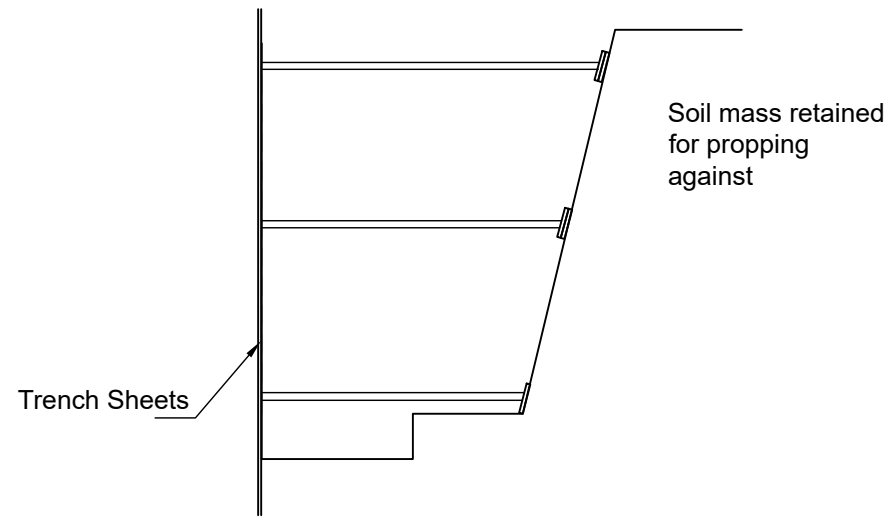
Project: 15 Lyncroft Gardens

Title: Plans and Sections BIA

Croft Structural Engineers

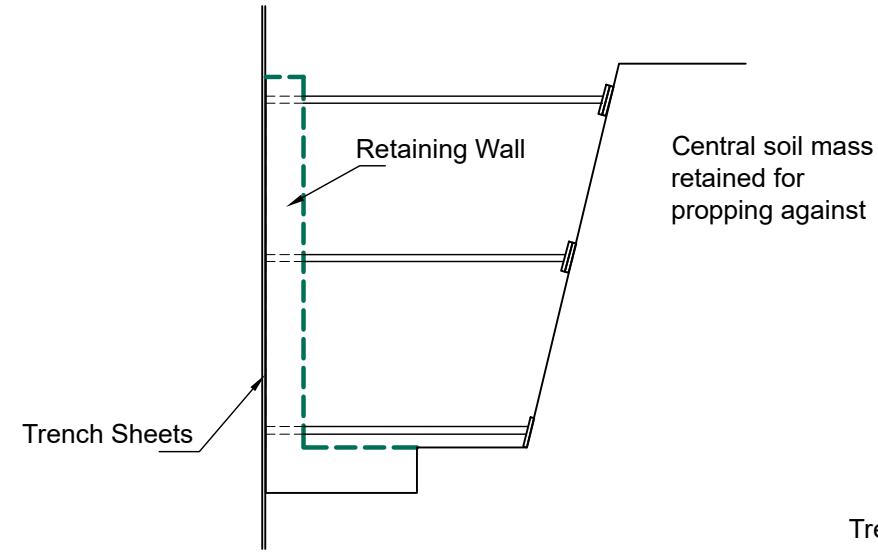
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r/o 60 Saxon Rd,
London, SE25 5EH.
020 8684 4744
www.croftse.co.uk

Rev	Date	Amendments
-	11/11/2019 00/00/2019	First issue for comment



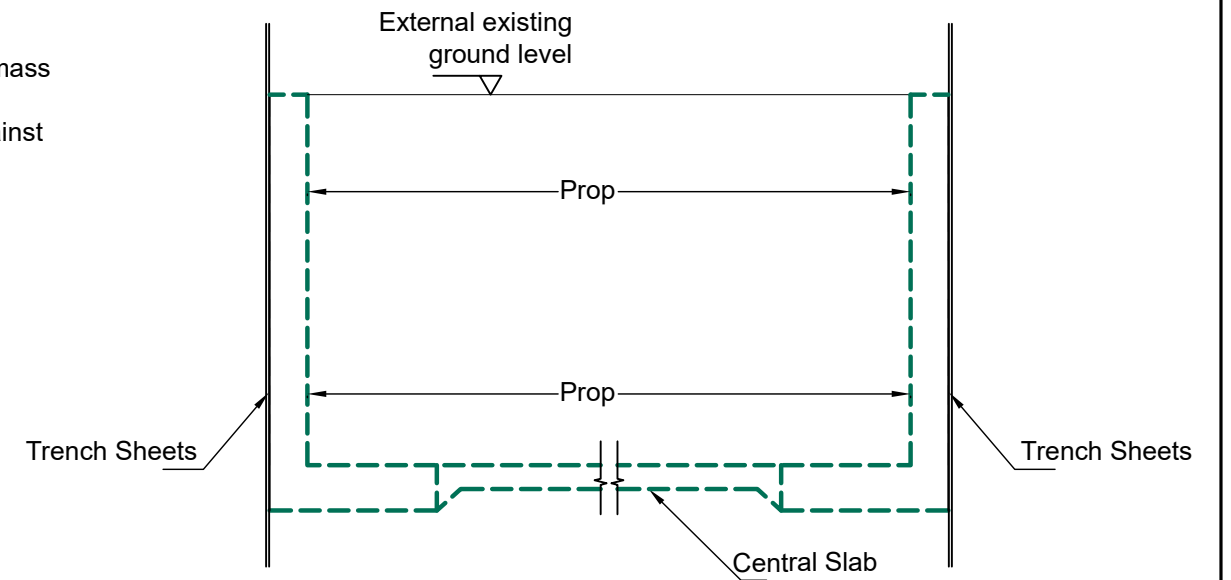
Phase 1

1. Excavate in 1m width maximum and install props and trench sheets and excavation progresses



Phase 2

1. Cast Retaining wall and Base.
2. Remove Props after concrete gain sufficient strength



Phase 3

1. Excavate and cast remainder of base slab. Install full length props while central soil mass is getting removed
2. Proceed with above ground construction

Job Number 190906	Date Nov '19
Dwg Number ST-30	Rev -
Drawn sb	Ch'kd pdh
Scale As shown @ A3	

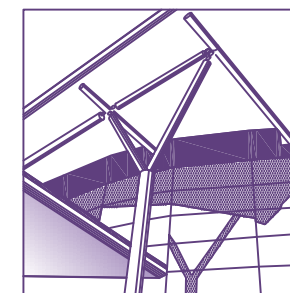
Client: **Advantage Basement & Cellar Company Ltd**

Project: **15 Lyncroft Gardens**

Title : **Temporary works propping**

Croft Structural Engineers

Clockshop Mews,
r/o 60 Saxon Rd,
London, SE25 5EH.
020 8684 4744
www.croftse.co.uk



Rev	Date	Amendments
-	27/11/2019	First issue for comment

Appendix B Groundsure Geo and Enviro Insight Reports



Maund Geo- Consulting Ltd
20, Mortlake Avenue,
Worcester, WR5 IQD

Groundsure Reference: HMD-6410190

Your Reference: MGC_19-29

Report Date 19 Oct 2019

Report Delivery Method: Email - pdf

Enviro Insight

Address: 15, LYNCROFT GARDENS, LONDON, NW6 1LB

Dear Sir/ Madam,

Thank you for placing your order with Groundsure. Please find enclosed the **Groundsure Enviro Insight** as requested.

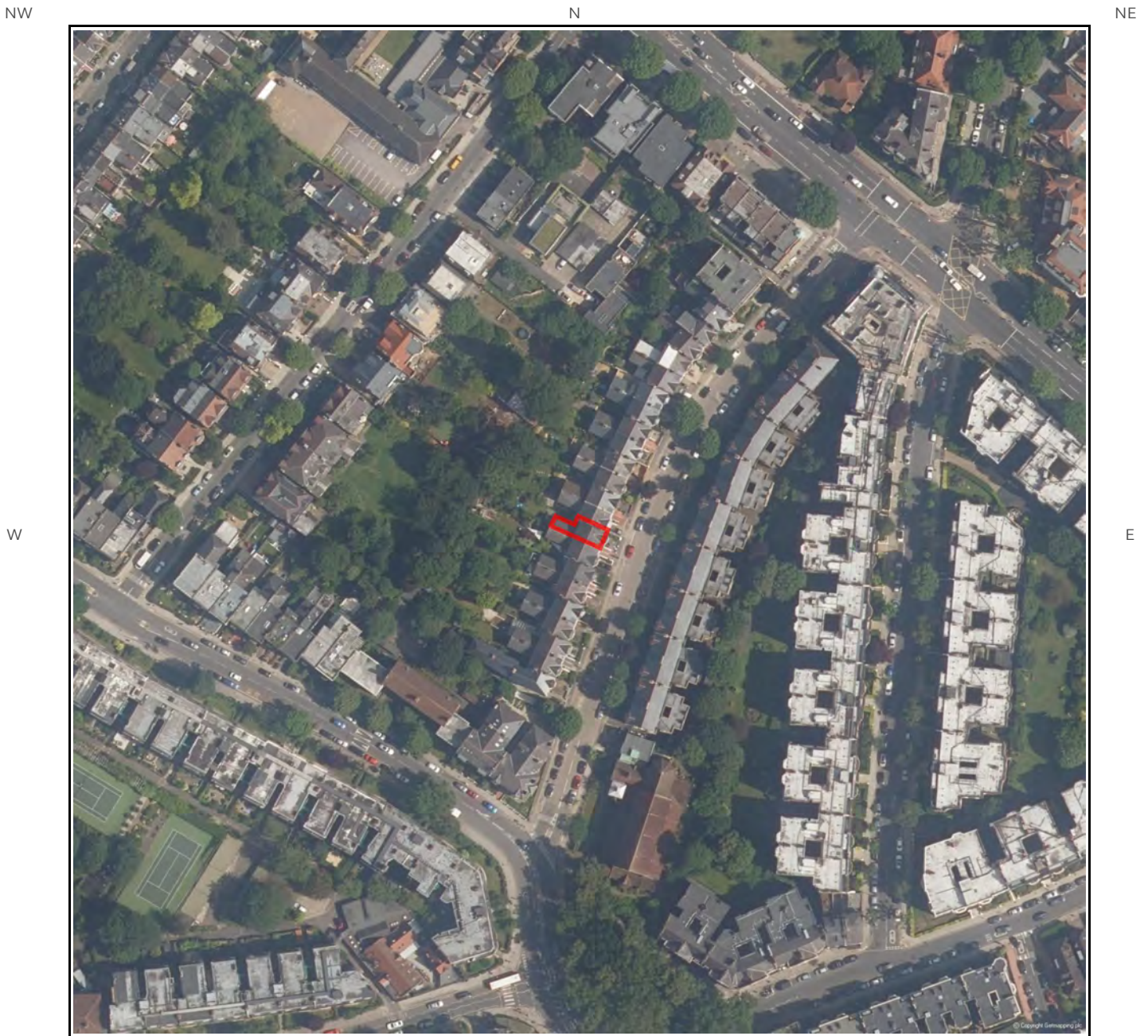
If you need any further assistance, please do not hesitate to contact our helpline on 08444 159000 quoting the above Groundsure reference number.

Yours faithfully,

Managing Director
Groundsure Limited

Enc.
Groundsure Enviroinsight

Address: 15, LYNCROFT GARDENS, LONDON, NW6 1LB
Date: 19 Oct 2019
Reference: HMD-6410190
Client: Maund Geo- Consulting Ltd



Aerial Photograph Capture date: 12-Aug-2016
Grid Reference: 525391,185395
Site Size: 0.0087ha

Report Reference: HMD-6410190
Client Reference: MGC_19-29

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Overview of Findings

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Section 1: Historical Industrial Sites	On-site	0-50	51-250	251-500
1.1 Potentially Contaminative Uses identified from 1:10,000 scale mapping	0	0	8	13
1.2 Additional Information – Historical Tank Database	0	0	1	4
1.3 Additional Information – Historical Energy Features Database	0	4	18	27
1.4 Additional Information – Historical Petrol and Fuel Site Database	0	0	0	0
1.5 Additional Information – Historical Garage and Motor Vehicle Repair Database	0	0	27	20
1.6 Historical military sites	0	0	0	0
1.7 Potentially Infilled Land	0	0	3	30
Section 2: Environmental Permits, Incidents and Registers	On-site	0-50m	51-250	251-500
2.1 Industrial Sites Holding Environmental Permits and/or Authorisations				
2.1.1 Records of historic IPC Authorisations	0	0	0	0
2.1.2 Records of Part A(1) and IPPC Authorised Activities	0	0	0	0
2.1.3 Records of Red List Discharge Consents	0	0	0	0
2.1.4 Records of List 1 Dangerous Substances Inventory sites	0	0	0	0
2.1.5 Records of List 2 Dangerous Substances Inventory sites	0	0	0	0
2.1.6 Records of Part A(2) and Part B Activities and Enforcements	0	0	8	5
2.1.7 Records of Category 3 or 4 Radioactive Substances Authorisations	0	0	0	0
2.1.8 Records of Licensed Discharge Consents	0	0	0	0
2.1.9 Records of Water Industry Referrals	0	0	0	0
2.1.10 Records of Planning Hazardous Substance Consents and Enforcements within 500m of the study site	0	0	0	0
2.2 Records of COMAH and NIHHS sites	0	0	0	0
2.3 Environment Agency/Natural Resources Wales Recorded Pollution Incidents				
2.3.1 National Incidents Recording System, List 2	0	0	0	0
2.3.2 National Incidents Recording System, List 1	0	0	0	0
2.4 Sites Determined as Contaminated Land under Part 2A EPA 1990	0	0	0	0

Section 3: Landfill and Other Waste Sites	On-site	0-50m	51-250	251-500	501-1000	1000-1500
3.1 Landfill Sites						
3.1.1 Environment Agency/Natural Resources Wales Registered Landfill Sites	0	0	0	0	0	Not searched
3.1.2 Environment Agency/Natural Resources Wales Historic Landfill Sites	0	0	0	0	1	0
3.1.3 BGS/DoE Landfill Site Survey	0	0	0	0	0	0
3.1.4 Records of Landfills in Local Authority and Historical Mapping Records	0	0	0	0	0	0
3.2 Landfill and Other Waste Sites Findings						
3.2.1 Operational and Non-Operational Waste Treatment, Transfer and Disposal Sites	0	0	0	0	Not searched	Not searched
3.2.2 Environment Agency/Natural Resources Wales Licensed Waste Sites	0	0	0	0	0	0

Section 4: Current Land Use	On-site	0-50m	51-250	251-500
4.1 Current Industrial Sites Data	0	1	14	Not searched
4.2 Records of Petrol and Fuel Sites	0	0	0	2
4.3 National Grid Underground Electricity Cables	0	0	0	0
4.4 National Grid Gas Transmission Pipelines	0	0	0	0

Section 5: Geology	
5.1 Records of Artificial Ground and Made Ground present beneath the study site	None identified
5.2 Records of Superficial Ground and Drift Geology present beneath the study site	None identified
5.3 For records of Bedrock and Solid Geology beneath the study site see the detailed findings section.	

Section 6: Hydrogeology and Hydrology	0-500m					
6.1 Records of Strata Classification in the Superficial Geology within 500m of the study site	None identified					
6.2 Records of Strata Classification in the Bedrock Geology within 500m of the study site	Identified					
	On-site	0-50m	51-250	251-500	501-1000	1000-2000
6.3 Groundwater Abstraction Licences (within 2000m of the study site)	0	0	0	0	0	4
6.4 Surface Water Abstraction Licences (within 2000m of the study site)	0	0	0	0	0	0
6.5 Potable Water Abstraction Licences (within 2000m of the study site)	0	0	0	0	0	0
6.6 Source Protection Zones (within 500m of the study site)	0	0	0	0	Not searched	Not searched
6.7 Source Protection Zones within Confined Aquifer	0	0	0	0	Not searched	Not searched
6.8 Groundwater Vulnerability and Soil Leaching Potential (within 500m of the study site)	0	0	0	1	Not searched	Not searched

Section 6: Hydrogeology and Hydrology	0-500m					
	On-site	0-50m	51-250	251-500	501-1000	1000-1500
6.9 Environment Agency/Natural Resources Wales information on river quality within 1500m of the study site	No	No	No	No	No	No
6.10 Ordnance Survey MasterMap Water Network entries within 500m of the site	0	0	0	0	Not searched	Not searched
6.11 Surface water features within 250m of the study site	No	No	No	Not searched	Not searched	Not searched

Section 7: Flooding	
7.1 Environment Agency Zone 2 floodplains within 250m of the study site	None identified
7.2 Environment Agency/Natural Resources Wales Zone 3 floodplains within 250m of the study site	None identified
7.3 Risk of flooding from Rivers and the Sea (RoFRaS) rating for the study site	Very Low
7.4 Flood Defences within 250m of the study site	None identified
7.5 Areas benefiting from Flood Defences within 250m of the study site	None identified
7.6 Areas used for Flood Storage within 250m of the study site	None identified
7.7 Maximum BGS Groundwater Flooding susceptibility within 50m of the study site	Not Prone
7.8 BGS confidence rating for the Groundwater Flooding susceptibility areas	Not Applicable

Section 8: Designated Environmentally Sensitive Sites	On-site	0-50m	51-250	251-500	501-1000	1000-2000
8.1 Records of Sites of Special Scientific Interest (SSSI)	0	0	0	0	0	0
8.2 Records of National Nature Reserves (NNR)	0	0	0	0	0	0
8.3 Records of Special Areas of Conservation (SAC)	0	0	0	0	0	0
8.4 Records of Special Protection Areas (SPA)	0	0	0	0	0	0
8.5 Records of Ramsar sites	0	0	0	0	0	0
8.6 Records of Ancient Woodlands	0	0	0	0	0	1
8.7 Records of Local Nature Reserves (LNR)	0	0	0	0	2	0
8.8 Records of World Heritage Sites	0	0	0	0	0	0
8.9 Records of Environmentally Sensitive Areas	0	0	0	0	0	0

Section 8: Designated Environmentally Sensitive Sites	On-site	0-50m	51-250	251-500	501-1000	1000-2000
8.10 Records of Areas of Outstanding Natural Beauty (AONB)	0	0	0	0	0	0
8.11 Records of National Parks	0	0	0	0	0	0
8.12 Records of Nitrate Sensitive Areas	0	0	0	0	0	0
8.13 Records of Nitrate Vulnerable Zones	0	0	0	0	0	0
8.14 Records of Green Belt land	0	0	0	0	0	0

Section 9: Natural Hazards

9.1 Maximum risk of natural ground subsidence	Moderate
9.1.1 Maximum Shrink-Swell hazard rating identified on the study site	Moderate
9.1.2 Maximum Landslides hazard rating identified on the study site	Very Low
9.1.3 Maximum Soluble Rocks hazard rating identified on the study site	Negligible
9.1.4 Maximum Compressible Ground hazard rating identified on the study site	Negligible
9.1.5 Maximum Collapsible Rocks hazard rating identified on the study site	Very Low
9.1.6 Maximum Running Sand hazard rating identified on the study site	Very Low
9.2 Radon	
9.2.1 Is the property in a Radon Affected Area as defined by the Health Protection Agency (HPA) and if so what percentage of homes are above the Action Level?	The site is not in a Radon Affected Area, as less than 1% of properties are above the Action Level.
9.2.2 Is the property in an area where Radon Protection are required for new properties or extensions to existing ones as described in publication BR211 by the Building Research Establishment?	No radon protective measures are necessary.

Section 10: Mining

10.1 Coal mining areas within 75m of the study site	None identified
10.2 Non-Coal Mining areas within 50m of the study site boundary	None identified
10.3 Brine affected areas within 75m of the study site	None identified

Using this report

The following report is designed by Environmental Consultants for Environmental Professionals bringing together the most up-to-date market leading environmental data. This report is provided under and subject to the Terms & Conditions agreed between Groundsure and the Client. The document contains the following sections:

1. Historical Industrial Sites

Provides information on past land uses that may pose a risk to the study site in terms of potential contamination from activities or processes. Potentially Infilled Land features are also included. This search is conducted using radii of up to 500m.

2. Environmental Permits, Incidents and Registers

Provides information on Regulated Industrial Activities and Pollution Incidents as recorded by Regulatory Authorities, and sites determined as Contaminated Land. This search is conducted using radii up to 500m.

3. Landfills and Other Waste Sites

Provides information on landfills and other waste sites that may pose a risk to the study site. This search is conducted using radii up to 1500m.

4. Current Land Uses

Provides information on current land uses that may pose a risk to the study site in terms of potential contamination from activities or processes. These searches are conducted using radii of up to 500m. This includes information on potentially contaminative industrial sites, petrol stations and fuel sites as well as high pressure gas pipelines and underground electricity transmission lines.

5. Geology

Provides information on artificial and superficial deposits and bedrock beneath the study site.

6. Hydrogeology and Hydrology

Provides information on productive strata within the bedrock and superficial geological layers, abstraction licences, Source Protection Zones (SPZs) and river quality. These searches are conducted using radii of up to 2000m.

7. Flooding

Provides information on river and coastal flooding, flood defences, flood storage areas and groundwater flood areas. This search is conducted using radii of up to 250m.

8. Designated Environmentally Sensitive Sites

Provides information on the Sites of Special Scientific Interest (SSSI), National Nature Reserves (NNR), Special Areas of Conservation (SAC), Special Protection Areas (SPA), Ramsar sites, Local Nature Reserves (LNR), Areas of Outstanding Natural Beauty (AONB), National Parks (NP), Environmentally Sensitive Areas, Nitrate Sensitive Areas, Nitrate Vulnerable Zones and World Heritage Sites and Scheduled Ancient Woodland. These searches are conducted using radii of up to 2000m.

9. Natural Hazards

Provides information on a range of natural hazards that may pose a risk to the study site. These factors include natural ground subsidence and radon..

10. Mining

Provides information on areas of coal and non-coal mining and brine affected areas.

11. Contacts

This section of the report provides contact points for statutory bodies and data providers that may be able to provide further information on issues raised within this report. Alternatively, Groundsure provide a free Technical Helpline (08444 159000) for further information and guidance.

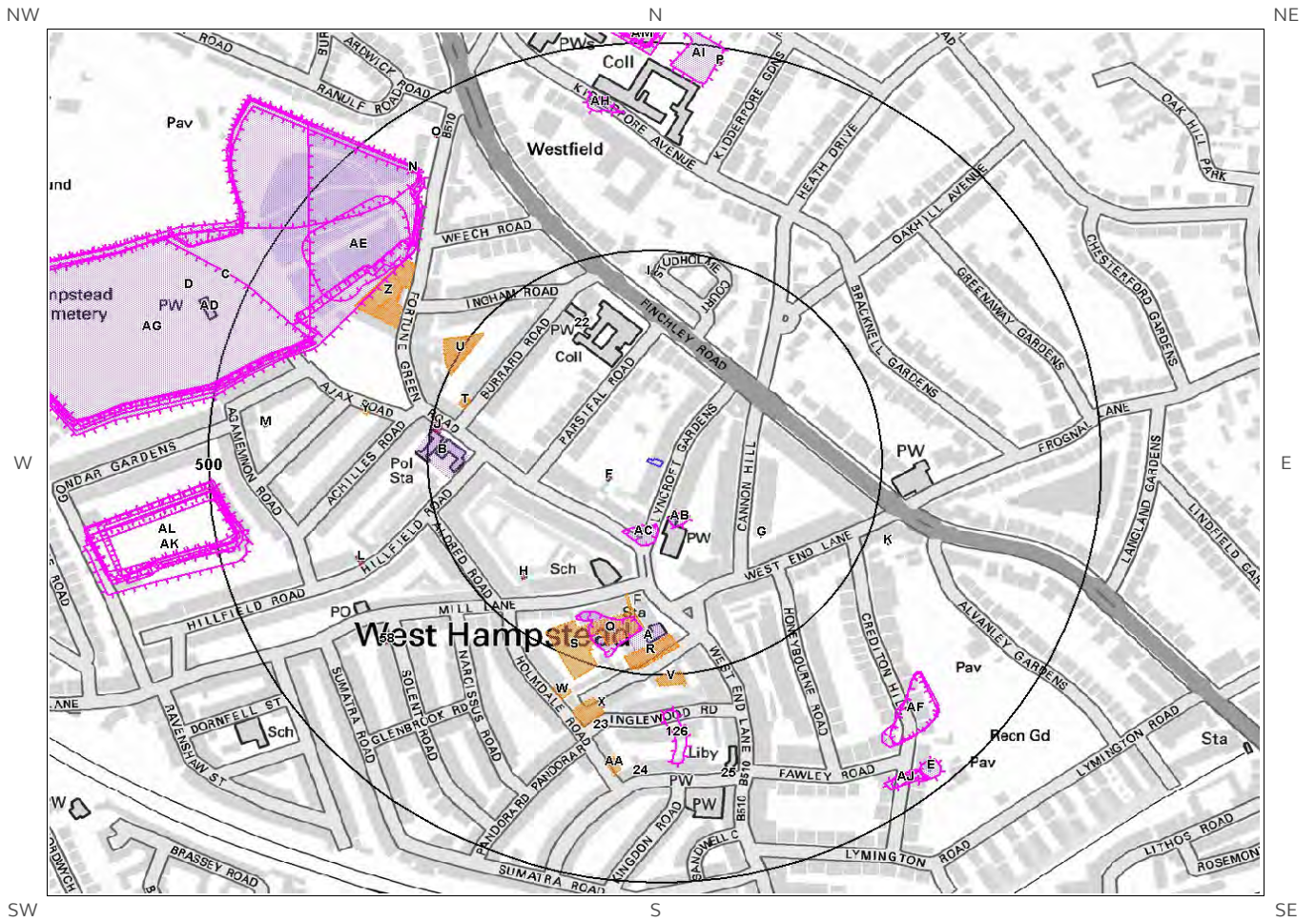
Note: Maps

Only certain features are placed on the maps within the report. All features represented on maps found within this search are given an identification number. This number identifies the feature on the mapping and correlates it to the additional information provided below. This identification number precedes all other information and takes the following format -Id: 1, Id: 2, etc. Where numerous features on the same map are in such close proximity that the numbers would obscure each other a letter identifier is used instead to represent the features. (e.g. Three features which overlap may be given the identifier "A" on the map and would be identified separately as features 1A, 3A, 10A on the data tables provided).

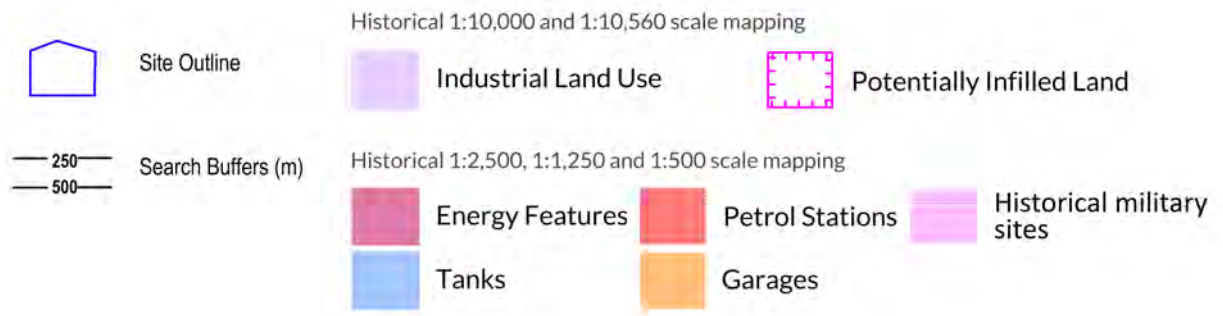
Where a feature is reported in the data tables to a distance greater than the map area, it is noted in the data table as "Not Shown".

All distances given in this report are in Metres (m). Directions are given as compass headings such as N: North, E: East, NE: North East from the nearest point of the study site boundary.

1. Historical Land Use



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1. Historical Industrial Sites

1.1 Potentially Contaminative Uses identified from 1:10,000 scale Mapping

The systematic analysis of data extracted from standard 1:10,560 and 1:10,000 scale historical maps provides the following information:

Records of sites with a potentially contaminative past land use within 500m of the search boundary: 21

ID	Distance [m]	Direction	Use	Date
1AB	58	S	Gravel Pit	1873
2AC	69	S	Gravel Pits	1873
3Q	183	S	Gravel Pit	1873
4A	187	S	Fire Station	1974
5A	187	S	Fire Station	1996
6A	187	S	Fire Station	1965
7B	203	W	Police Station	1974
8B	203	W	Police Station	1996
9C	365	NW	Cemetery	1920
10AD	365	NW	Cemetery	1911
11AE	366	NW	Cemetery	1949
12C	371	NW	Cemetery	1938
13C	372	NW	Cemetery	1938
14AG	404	W	Cemetery	1894
15D	411	W	Cemetery	1949
16D	411	W	Cemetery	1966
17D	411	W	Cemetery	1976
18D	411	W	Cemetery	1993
19AH	418	N	Gravel Pit	1873
20AI	455	N	Unspecified Ground Workings	1938
21E	466	SE	Unspecified Heap	1894

1.2 Additional Information – Historical Tank Database

The systematic analysis of data extracted from High Detailed 1:1,250 and 1:2,500 scale historical maps provides the following information.

Records of historical tanks within 500m of the search boundary: 5

ID	Distance (m)	Direction	Use	Date
22	173	NW	Unspecified Tank	1971
23	319	S	Unspecified Tank	1871

24	368	S	Unspecified Tank	1871
25	379	S	Unspecified Tank	1871
26E	474	SE	Unspecified Tank	1871

1.3 Additional Information – Historical Energy Features Database

The systematic analysis of data extracted from High Detailed 1:1,250 and 1:2,500 scale historical maps provides the following information.

Records of historical energy features within 500m of the search boundary:

49

ID	Distance (m)	Direction	Use	Date
27F	45	SW	Electricity Substation	1973
28F	45	SW	Electricity Substation	1991
29F	46	SW	Electricity Substation	1992
30F	46	SW	Electricity Substation	1994
31G	140	SE	Electricity Substation	1995
32G	140	SE	Electricity Substation	1991
33G	140	SE	Electricity Substation	1979
34G	140	SE	Electricity Substation	1971
35H	191	SW	Electricity Substation	1991
36H	192	SW	Electricity Substation	1953
37H	192	SW	Electricity Substation	1953
38H	192	SW	Electricity Substation	1953
39H	192	SW	Electricity Substation	1973
40H	192	SW	Electricity Substation	1994
41H	192	SW	Electricity Substation	1992
42I	220	N	Electricity Substation	1994
43I	221	N	Electricity Substation	1991
44I	224	N	Electricity Substation	1979
45J	237	W	Electricity Substation	1991
46J	241	W	Electricity Substation	1992
47J	241	W	Electricity Substation	1994
48J	241	W	Electricity Substation	1973
49K	271	E	Electricity Substation	1995
50K	271	E	Electricity Substation	1971
51K	271	E	Electricity Substation	1979
52K	271	E	Electricity Substation	1991
53X	292	S	Electricity Substation	1973
54L	345	W	Electricity Substation	1973
55L	345	W	Electricity Substation	1992
56L	345	W	Electricity Substation	1994
57L	345	W	Electricity Substation	1991
58	365	SW	Electricity Substation	1973

59M	434	W	Electricity Substation	1953
60M	434	W	Electricity Substation	1991
61M	434	W	Electricity Substation	1991
62M	435	W	Electricity Substation	1974
63M	435	W	Electricity Substation	1953
64N	436	NW	Electricity Substation	1991
65N	437	NW	Electricity Substation	1994
66N	437	NW	Electricity Substation	1979
67N	437	NW	Electricity Substation	1971
68O	455	NW	Electricity Substation	1991
69O	455	NW	Electricity Substation	1994
70O	456	NW	Electricity Substation	1971
71O	456	NW	Electricity Substation	1979
72P	478	N	Electricity Substation	1991
73P	478	N	Electricity Substation	1979
74P	478	N	Electricity Substation	1971
75P	479	N	Electricity Substation	1994

1.4 Additional Information – Historical Petrol and Fuel Site Database

The systematic analysis of data extracted from High Detailed 1:1,250 and 1:2,500 scale historical maps provides the following information.

Records of historical petrol stations and fuel sites within 500m of the search boundary: 0

Database searched and no data found.

1.5 Additional Information – Historical Garage and Motor Vehicle Repair Database

The systematic analysis of data extracted from High Detailed 1:1,250 and 1:2,500 scale historical maps provides the following information.

Records of historical garage and motor vehicle repair sites within 500m of the search boundary: 47

ID	Distance (m)	Direction	Use	Date
76Q	157	S	Garage	1953
77Q	158	S	Garage	1953
78Q	158	S	Garage	1953
79R	201	S	Garage	1991
80R	201	S	Garage	1953
81R	201	S	Garage	1953
82R	201	S	Garage	1953
83R	201	S	Garage	1973
84R	202	S	Garage	1992

85R	202	S	Garage	1994
86S	209	SW	Post Office Garage	1953
87S	209	SW	Post Office Garage	1953
88S	209	SW	Post Office Garage	1953
89T	213	W	Garages	1953
90T	214	W	Garages	1953
91T	214	W	Garages	1953
92U	240	NW	Garage	1953
93U	240	NW	Garage	1953
94U	240	NW	Garage	1971
95U	240	NW	Garage	1953
96U	240	NW	Garage	1979
97U	240	NW	Garage	1962
98U	240	NW	Garage	1994
99U	242	NW	Garage	1991
100V	245	S	Garages	1953
101V	245	S	Garages	1953
102V	246	S	Garages	1953
103W	284	S	Garages	1953
104W	284	S	Garages	1953
105W	285	S	Garages	1953
106X	287	S	Garages	1953
107X	287	S	Garages	1953
108X	288	S	Garages	1953
109Y	322	W	Garages	1953
110Y	322	W	Garages	1953
111Y	322	W	Garages	1953
112Z	327	NW	Garage	1971
113Z	327	NW	Garage	1953
114Z	327	NW	Garage	1979
115Z	327	NW	Garage	1962
116Z	327	NW	Garage	1994
117Z	342	NW	Garage	1953
118Z	342	NW	Garage	1953
119Z	343	NW	Garage	1991
120AA	351	S	Garages	1953
121AA	351	S	Garages	1953
122AA	353	S	Garages	1953

1.6 Historical military sites

Certain military installations were not noted on historic mapping for security reasons. Whilst not all military land is necessarily of concern, Groundsure has researched and digitised a number of Ordnance Factories and other military industrial features (e.g. Ordnance Depots, Munitions Testing Grounds) which may be of contaminative concern. This research was drawn from a number of different sources, and should not be regarded as a definitive or exhaustive database of potentially contaminative military installations. The boundaries of sites within this database have been estimated from the best evidence available to Groundsure at the time of compilation.

Records of historical military sites within 500m of the search boundary: 0
Database searched and no data found.

1.7 Potentially Infilled Land

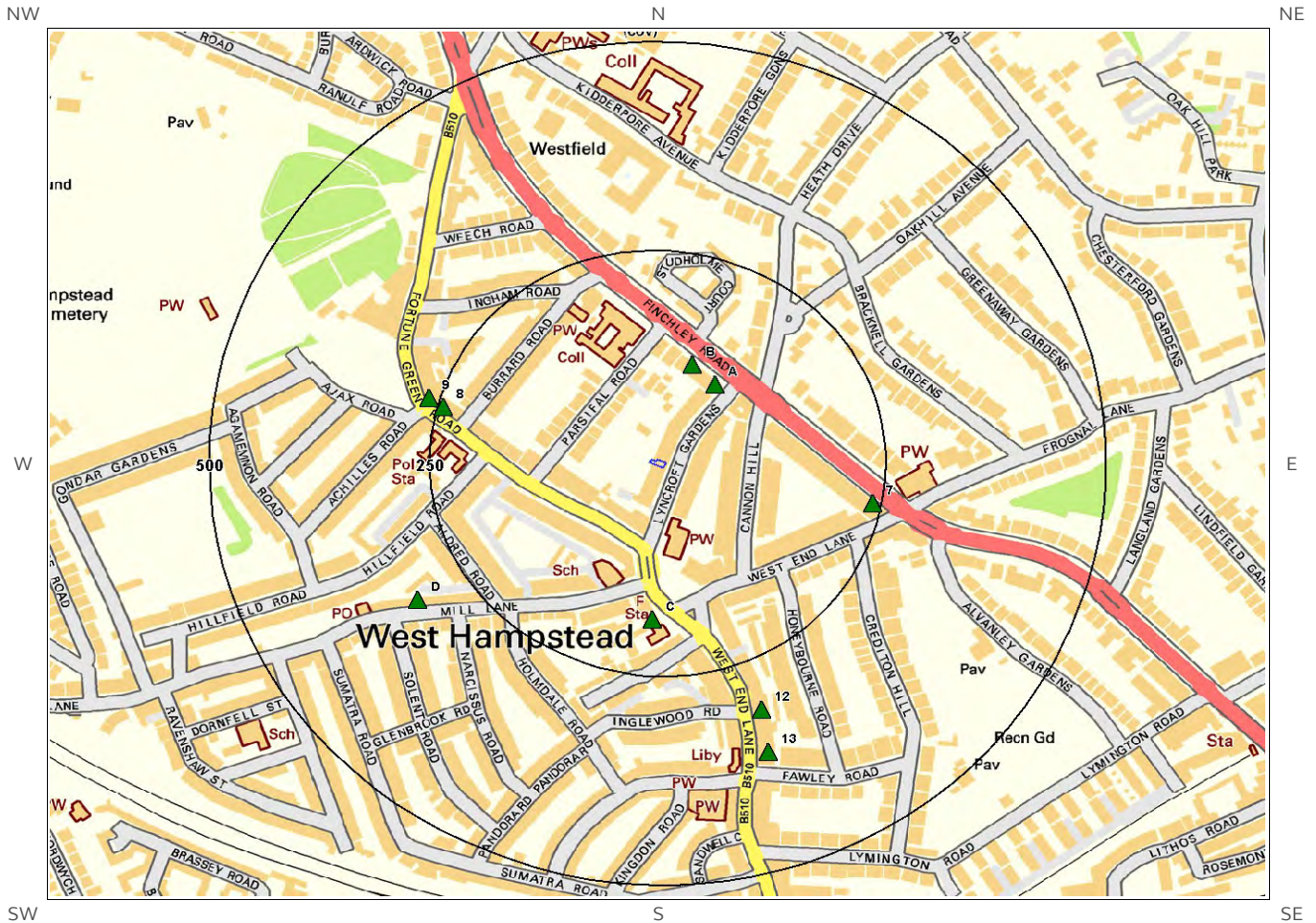
Records of Potentially Infilled Features from 1:10,000 scale mapping within 500m of the study site: 33

The following Historical Potentially Infilled Features derived from the Historical Mapping information is provided by Groundsure:


ID	Distance(m)	Direction	Use	Date
123AB	58	S	Gravel Pit	1873
124AC	69	S	Gravel Pits	1873
125Q	183	S	Gravel Pit	1873
126	293	S	Pond	1866
127C	365	NW	Cemetery	1920
128AD	365	NW	Cemetery	1911
129AE	366	NW	Cemetery	1949
130C	371	NW	Cemetery	1938
131C	372	NW	Cemetery	1938
132AF	381	SE	Pond	1866
133AF	384	SE	Pond	1894
134AG	404	W	Cemetery	1894
135D	411	W	Cemetery	1949
136D	411	W	Cemetery	1993
137D	411	W	Cemetery	1976
138D	411	W	Cemetery	1966
139AH	418	N	Gravel Pit	1873
140AI	455	N	Unspecified Ground Workings	1938
141AJ	458	SE	Pond	1894
142AJ	459	SE	Pond	1866
143AK	463	W	Reservoir	1911
144AJ	466	SE	Unspecified Heap	1894
145AL	466	W	Reservoir	1938
146AK	472	W	Covered Reservoir	1894
147AL	475	W	Reservoir	1938
148AL	476	W	Covered Reservoir	1993

149AL	476	W	Covered Reservoir	1976
150AK	476	W	Reservoir	1920
151AL	478	W	Reservoir	1949
152AM	485	N	Reservoir	1920
153AM	493	N	Reservoir	1938
154AM	494	N	Reservoir	1938
155AM	495	N	Reservoir	1949

2. Environmental Permits, Incidents and Registers Map



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- | | | | | | |
|---|-------------------------------|---|--|---|---|
|  | Site Outline |  | Recorded Pollution Incident |  | RAS 3 & 4 Authorisations |
|  | Dangerous Substances (List 1) |  | Part A(1) Authorised Processes and Historic IPC Authorisations |  | Part A(2) and Part B Authorised Processes |
|  | Dangerous Substances (List 2) |  | Water Industry Referrals |  | COMAH / NIHHS Sites |
|  | Search Buffers (m) |  | Licensed Discharge Consents |  | Sites Determined as Contaminated Land |
|  | 250 |  | Red List Discharge Consents |  | Hazardous Substance Consents and Enforcements |
|  | 500 | | | | |

2. Environmental Permits, Incidents and Registers

2.1 Industrial Sites Holding Licences and/or Authorisations

Searches of information provided by the Environment Agency/Natural Resources Wales and Local Authorities reveal the following information:

2.1.1 Records of historic IPC Authorisations within 500m of the study site:

0

Database searched and no data found.

2.1.2 Records of Part A(1) and IPPC Authorised Activities within 500m of the study site:

0

Database searched and no data found.

2.1.3 Records of Red List Discharge Consents (potentially harmful discharges to controlled waters) within 500m of the study site:

0

Database searched and no data found.

2.1.4 Records of List 1 Dangerous Substances Inventory Sites within 500m of the study site:

0

Database searched and no data found.

2.1.5 Records of List 2 Dangerous Substance Inventory Sites within 500m of the study site:

0

Database searched and no data found.

2.1.6 Records of Part A(2) and Part B Activities and Enforcements within 500m of the study site:

13

The following Part A(2) and Part B Activities are represented as points on the Environmental Permits, Incidents and Registers Map:

ID	Distance (m)	Direction	NGR	Details
1A	108	NE	525457 185487	Address: Cottontail Cleaners, 509 Finchley Road, Hamstead, NW3 7BB Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
2A	108	NE	525457 185487	Address: Cottontail Cleaners, 509 Finchley Road, NW3 7BB Process: Dry Cleaning Status: Historical Permit Permit Type: Part B Enforcement: Enforcement Notified Date of Enforcement: 19/01/2009 Comment: Non - payment of the annual subsistence fee.
3B	118	N	525431 185510	Address: The London Dry Cleaning Company, 519A Finchley Road, NW3 7BB Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
4B	118	N	525431 185510	Address: The London Dry Cleaning Company, 519A Finchley Road, NW3 7BB Process: Dry Cleaning Status: Historical Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
5C	184	S	525386 185205	Address: Sparkle Dry Cleaning , 329 West End Lane, NW6 1RS Process: Dry Cleaning Status: Historical Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
6C	184	S	525386 185205	Address: Sparkle Dry Cleaning, 329 West End Lane, NW6 1RS Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
7	240	E	525636 185344	Address: J D Dry Cleaners, 469 Finchley Road, NW3 6HP Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
8	243	W	525149 185460	Address: Texaco 63 Fortune Green, London, NW6 1DR Process: Unloading of Petrol into Storage at Service Stations Status: Historical Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
9	262	W	525133 185471	Address: D&D Dry Cleaners, 68 Fortune Green Rd, NW6 1DS Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
10D	312	SW	525120 185229	Address: Cotton Club Dry Cleaners, 57 Mill Lane, NW6 1NB Process: Dry Cleaning Status: Current Permit Permit Type: Part B Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified

ID	Distance (m)	Direction	NGR	Details	
11D	312	SW	525120 185229	Address: Cotton Club Dry Cleaners, 57 Mill Lane, NW6 1NB Process: Dry Cleaning Status: Historical Permit Permit Type: Part B	Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
12	312	S	525510 185097	Address: Shamrock Express Cleaners, 210 West End Lane, NW6 1UU Process: Dry Cleaning Status: Current Permit Permit Type: Part B	Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified
13	362	S	525518 185047	Address: Shamrock, 210 West End Lane, NW6 1UU Process: Dry Cleaning Status: Revoked Permit Type: Part B	Enforcement: No Enforcement Notified Date of Enforcement: No Enforcement Notified Comment: No Enforcement Notified

2.1.7 Records of Category 3 or 4 Radioactive Substances Authorisations:

0

Database searched and no data found.

2.1.8 Records of Licensed Discharge Consents within 500m of the study site:

0

Database searched and no data found.

2.1.9 Records of Water Industry Referrals (potentially harmful discharges to the public sewer) within 500m of the study site:

0

Database searched and no data found.

2.1.10 Records of Planning Hazardous Substance Consents and Enforcements within 500m of the study site:

0

Database searched and no data found.

2.2 Dangerous or Hazardous Sites

Records of COMAH & NIHHS sites within 500m of the study site: 0

Database searched and no data found.

2.3 Environment Agency/Natural Resources Wales Recorded Pollution Incidents

2.3.1 Records of National Incidents Recording System, List 2 within 500m of the study site:

0

Database searched and no data found.

2.3.2 Records of National Incidents Recording System, List 1 within 500m of the study site:

0

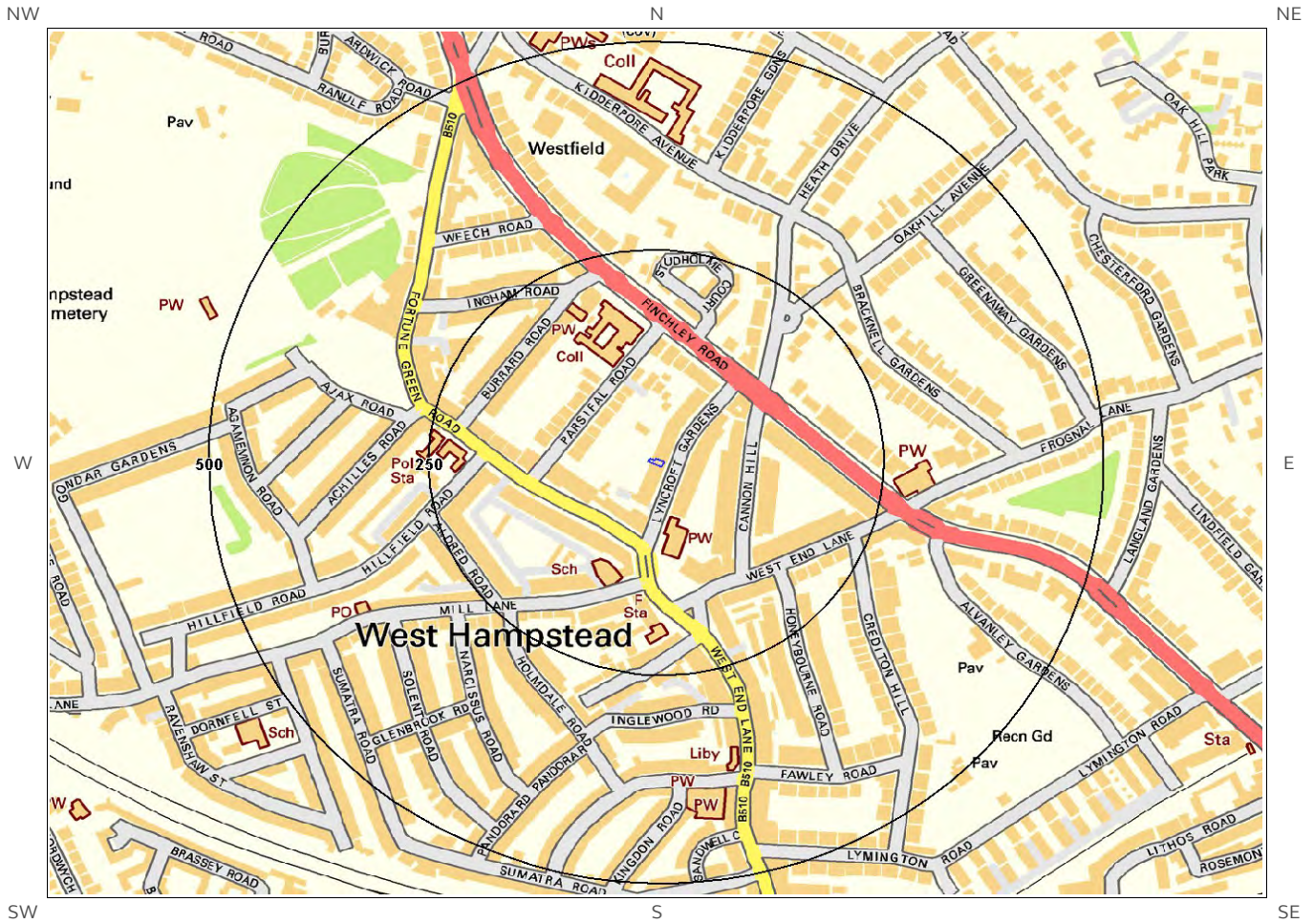
Database searched and no data found.

2.4 Sites Determined as Contaminated Land under Part 2A EPA 1990










Records of sites determined as contaminated land under Section 78R of the Environmental Protection Act 1990 are there within 500m of the study site 0

Database searched and no data found.

3. Landfill and Other Waste Sites Map



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- | | | | | | |
|---|------------------------|---|---------------------------|---|---|
|  | Site Outline |  | EA/NRW Active Landfill |  | Historic and Planned Waste Sites |
|  | 250 Search Buffers (m) |  | EA/NRW Historic Landfill |  | EA/NRW Licensed Waste Site |
|  | 500 Search Buffers (m) |  | BGS / DoE Survey Landfill |  | Local Authority/Historical Mapping Landfill Records |

3. Landfill and Other Waste Sites

3.1 Landfill Sites

3.1.1 Records from Environment Agency/Natural Resources Wales landfill data within 1000m of the study site:

0

Database searched and no data found.

3.1.2 Records of Environment Agency/Natural Resources Wales historic landfill sites within 1500m of the study site:

1

The following landfill records are represented as either points or polygons on the Landfill and Other Waste Sites map:

ID	Distance (m)	Direction	NGR	Details
Not shown	857	SE		Site Address: Canfield Place, London NW6 Waste Licence: - Site Reference: DON009 Waste Type: - Environmental Permitting Regulations (Waste) Reference: - Licence Issue: Licence Surrendered: Licence Holder Address: - Operator: - Licence Holder: - First Recorded: - Last Recorded: -

3.1.3 Records of BGS/DoE non-operational landfill sites within 1500m of the study site:

0

Database searched and no data found.

3.1.4 Records of Landfills from Local Authority and Historical Mapping Records within 1500m of the study site:

0

Database searched and no data found.

3.2 Other Waste Sites

3.2.1 Records of waste treatment, transfer or disposal sites within 500m of the study site:

0

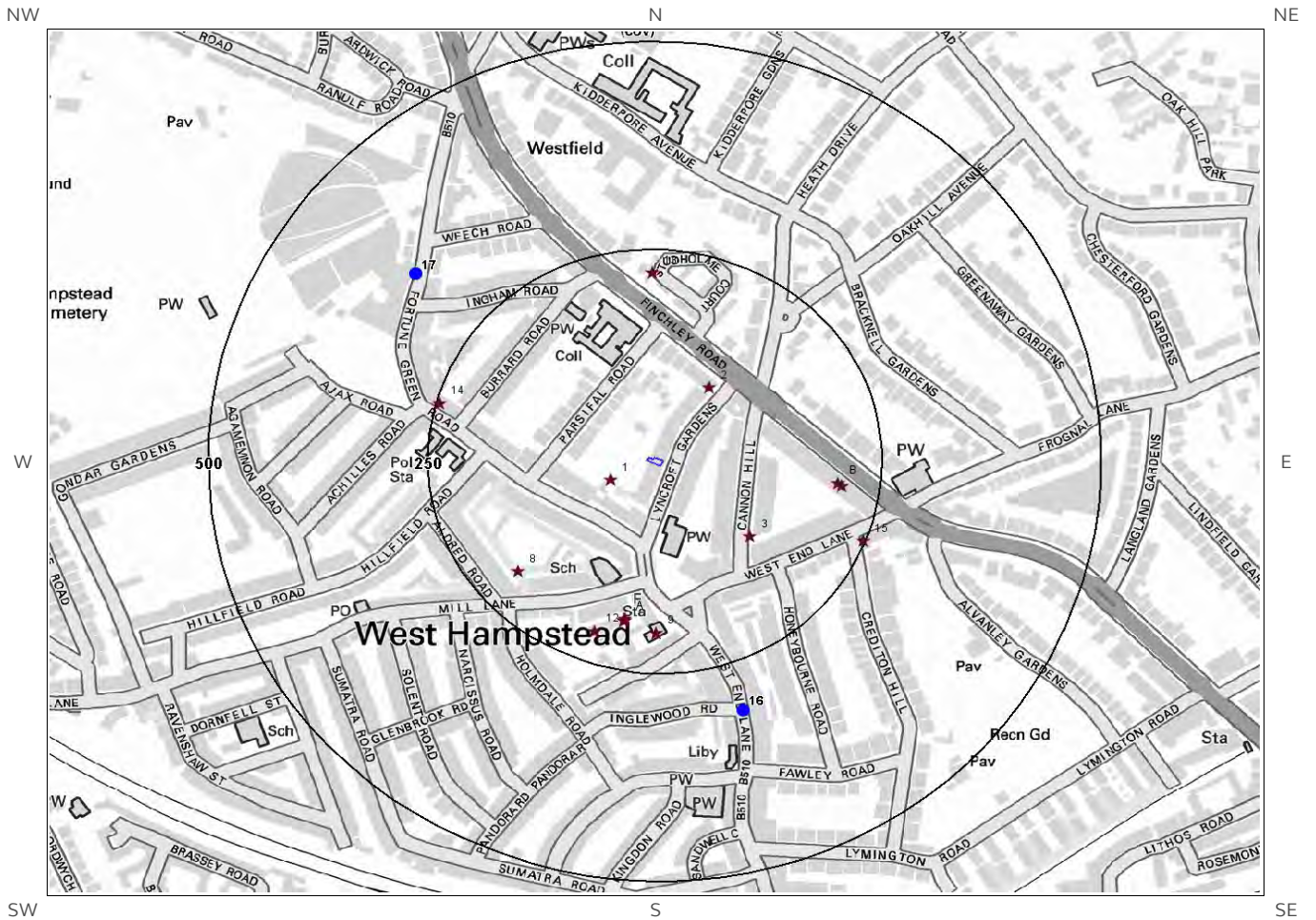
Database searched and no data found.

3.2.2 Records of Environment Agency/Natural Resources Wales licensed waste sites within 1500m of the study site:

0

Database searched and no data found.

4. Current Land Use Map



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-  Site Outline
-  Current Industrial Sites
-  Electricity Transmission Cables
-  Petrol & Fuel Sites
-  Gas Transmission Pipelines
-  Search Buffers (m)
- 

4. Current Land Uses

4.1 Current Industrial Data

Records of potentially contaminative industrial sites within 250m of the study site:

15

The following records are represented as points on the Current Land Uses map.

ID	Distance (m)	Direction	Company	NGR	Address	Activity	Category
1	48	SW	Electricity Sub Station	525342 185373	Greater London, NW6	Electrical Features	Infrastructure and Facilities
2	103	NE	Curtains & Blinds	525454 185483	509, Finchley Road, Hampstead, London, Greater London, NW3 7BB	Curtains and Blinds	Consumer Products
3	132	SE	Electricity Sub Station	525501 185305	Greater London, NW6	Electrical Features	Infrastructure and Facilities
4A	188	S	First Aid Wheels	525358 185206	174, Mill Lane, West Hampstead, London, Greater London, NW6 1TB	Vehicle Repair, Testing and Servicing	Repair and Servicing
5A	190	S	Mac Daddy IT Ltd	525357 185204	West Heath Yard 174, Mill Lane, London, Greater London, NW6 1TB	Electrical Equipment Repair and Servicing	Repair and Servicing
6A	190	S	Woodstock Motors	525357 185204	West Heath Yard 174, Mill Lane, London, Greater London, NW6 1TB	Vehicle Repair, Testing and Servicing	Repair and Servicing
7A	190	S	First Aid Wheels	525358 185204	West Heath Yard, 174 Mill Lane, London, Greater London, NW6 1TB	Special Purpose Machinery and Equipment	Industrial Products
8	199	SW	Electricity Sub Station	525236 185263	Greater London, NW6	Electrical Features	Infrastructure and Facilities
9	200	S	West Hampstead Fire Station	525394 185189	West Hampstead Fire Station 325, West End Lane, London, Greater London, NW6 1RR	Fire Brigade Stations	Central and Local Government
10B	201	E	Stone of London	525601 185368	485, Finchley Road, London, Greater London, NW3 6HS	Stone Quarrying and Preparation	Extractive Industries
11B	206	E	Chessams Upholsterers	525605 185365	483a, Finchley Road, London, Greater London, NW3 6HS	Furniture	Consumer Products
12	212	S	Works	525324 185191	Greater London, NW6	Unspecified Works Or Factories	Industrial Features
13	223	N	Electricity Sub Station	525389 185622	Greater London, NW3	Electrical Features	Infrastructure and Facilities
14	246	W	Cromwell Security & Fire Services Ltd	525147 185465	64, Fortune Green Road, London, Greater London, NW6 1DT	Electronic Equipment	Industrial Products
15	248	SE	Carmel Garage	525630 185299	322, West End Lane, London, Greater London, NW6 1LN	Vehicle Repair, Testing and Servicing	Repair and Servicing

4.2 Petrol and Fuel Sites

Records of petrol or fuel sites within 500m of the study site:

2

The following petrol or fuel site records provided by Catalist are represented as points on the Current Land Use map:

ID	Distance (m)	Direction	NGR	Company	Address	LPG	Status
16	308	S	525493 185095	OBSOLETE	West End Lane, London, Inner London, NW6 1XF	Not Applicable	Obsolete
17	345	NW	525120 185620	TEXACO	63-65, Fortune Green Road, Fortune Green, London, Inner London, NW6 1DR	Not Applicable	Obsolete

4.3 National Grid High Voltage Underground Electricity Transmission Cables

This dataset identifies the high voltage electricity transmission lines running between generating power plants and electricity substations. The dataset does not include the electricity distribution network (smaller, lower voltage cables distributing power from substations to the local user network). This information has been extracted from databases held by National Grid and is provided for information only with no guarantee as to its completeness or accuracy. National Grid do not offer any warranty as to the accuracy of the available data and are excluded from any liability for any such inaccuracies or errors.

Records of National Grid high voltage underground electricity transmission cables within 500m of the study site:

0

Database searched and no data found.

4.4 National Grid High Pressure Gas Transmission Pipelines

This dataset identifies high-pressure, large diameter pipelines which carry gas between gas terminals, power stations, compressors and storage facilities. The dataset does not include the Local Transmission System (LTS) which supplies gas directly into homes and businesses. This information has been extracted from databases held by National Grid and is provided for information only with no guarantee as to its completeness or accuracy. National Grid do not offer any warranty as to the accuracy of the available data and are excluded from any liability for any such inaccuracies or errors.

Records of National Grid high pressure gas transmission pipelines within 500m of the study site:

0

Database searched and no data found.

5. Geology

5.1 Artificial Ground and Made Ground

Database searched and no data found.

The database has been searched on site, including a 50m buffer.

5.2 Superficial Ground and Drift Geology

Database searched and no data found.

The database has been searched on site, including a 50m buffer.

5.3 Bedrock and Solid Geology

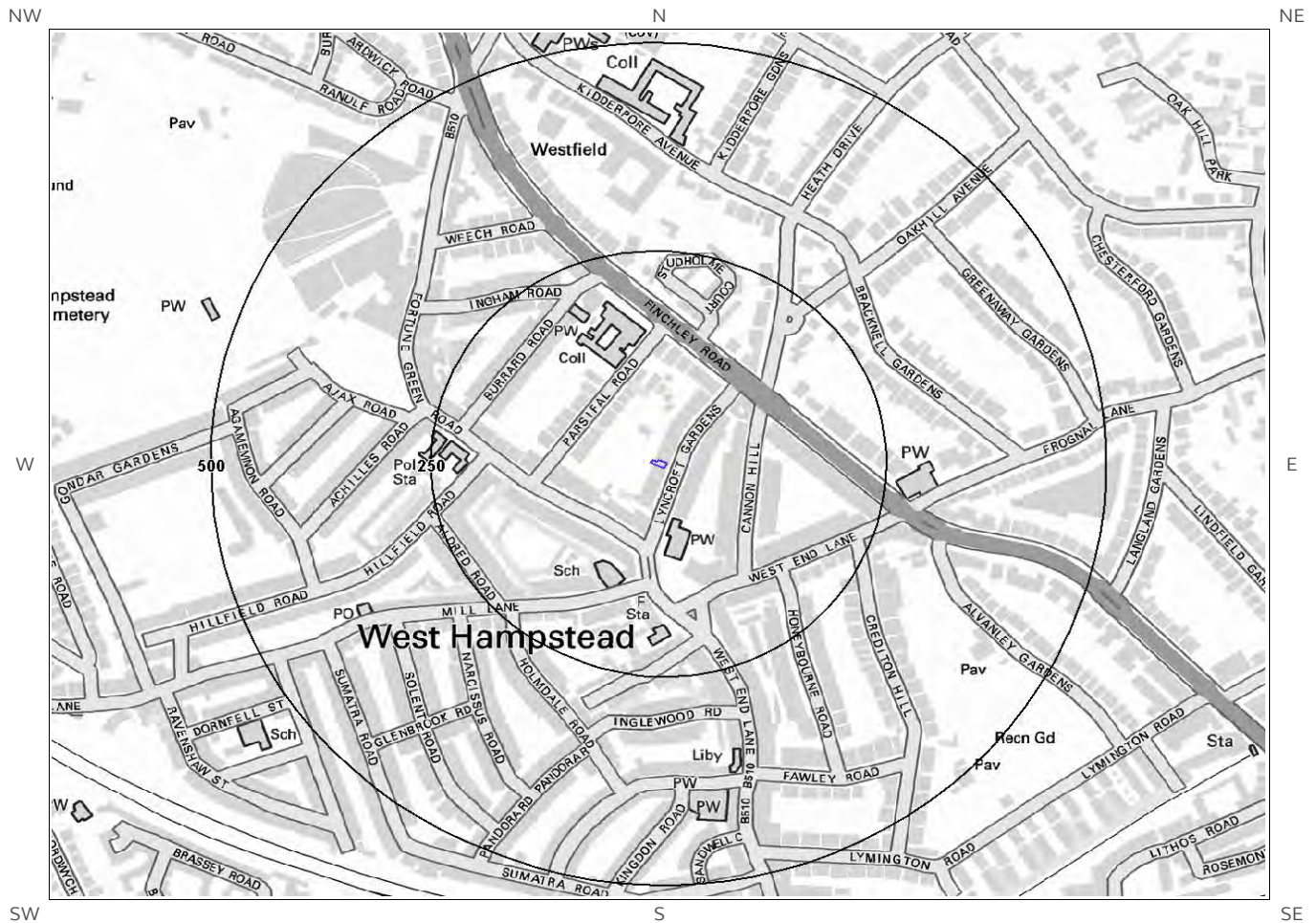
The database has been searched on site, including a 50m buffer.

Lex Code	Description	Rock Type
LC-XCZS	LONDON CLAY FORMATION	CLAY, SILT AND SAND

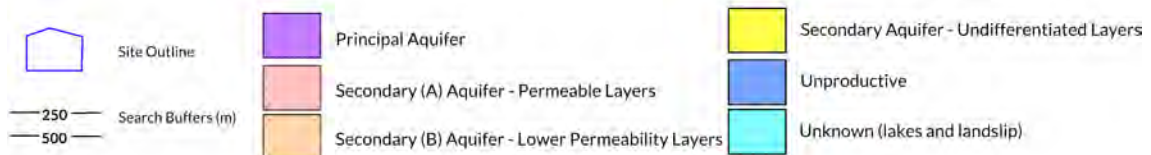
(Derived from the BGS 1:50,000 Digital Geological Map of Great Britain)

6 Hydrogeology and Hydrology

6a. Aquifer Within Superficial Geology



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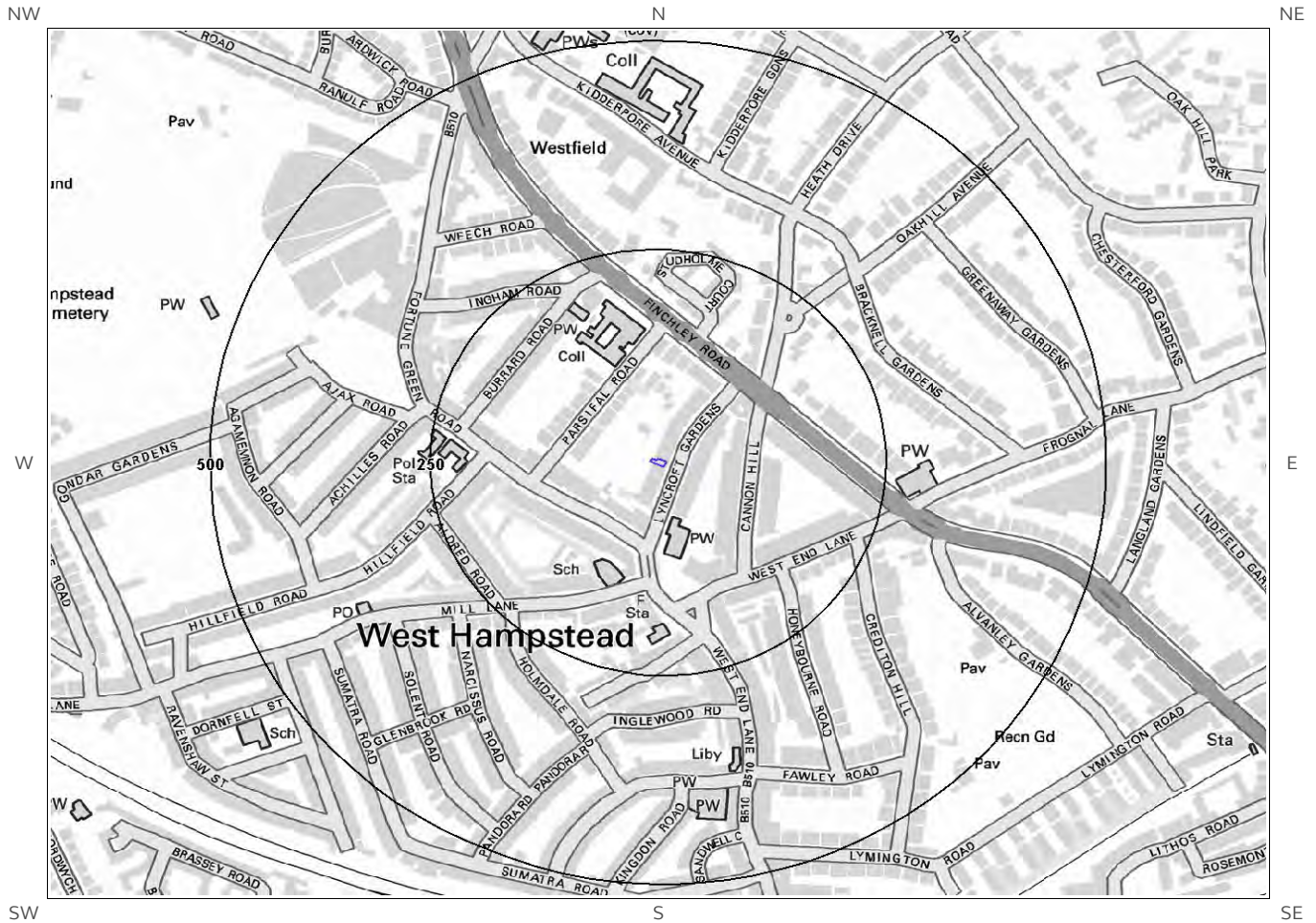
6b. Aquifer Within Bedrock Geology and Abstraction Licences



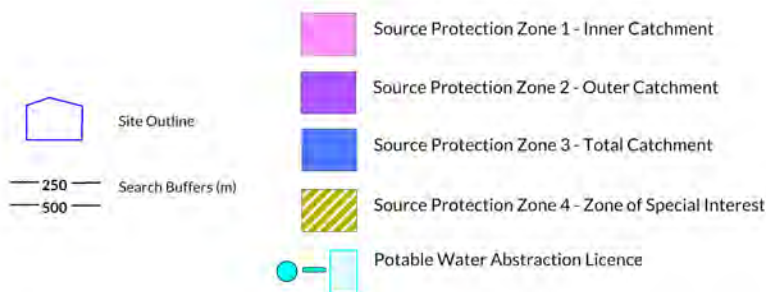
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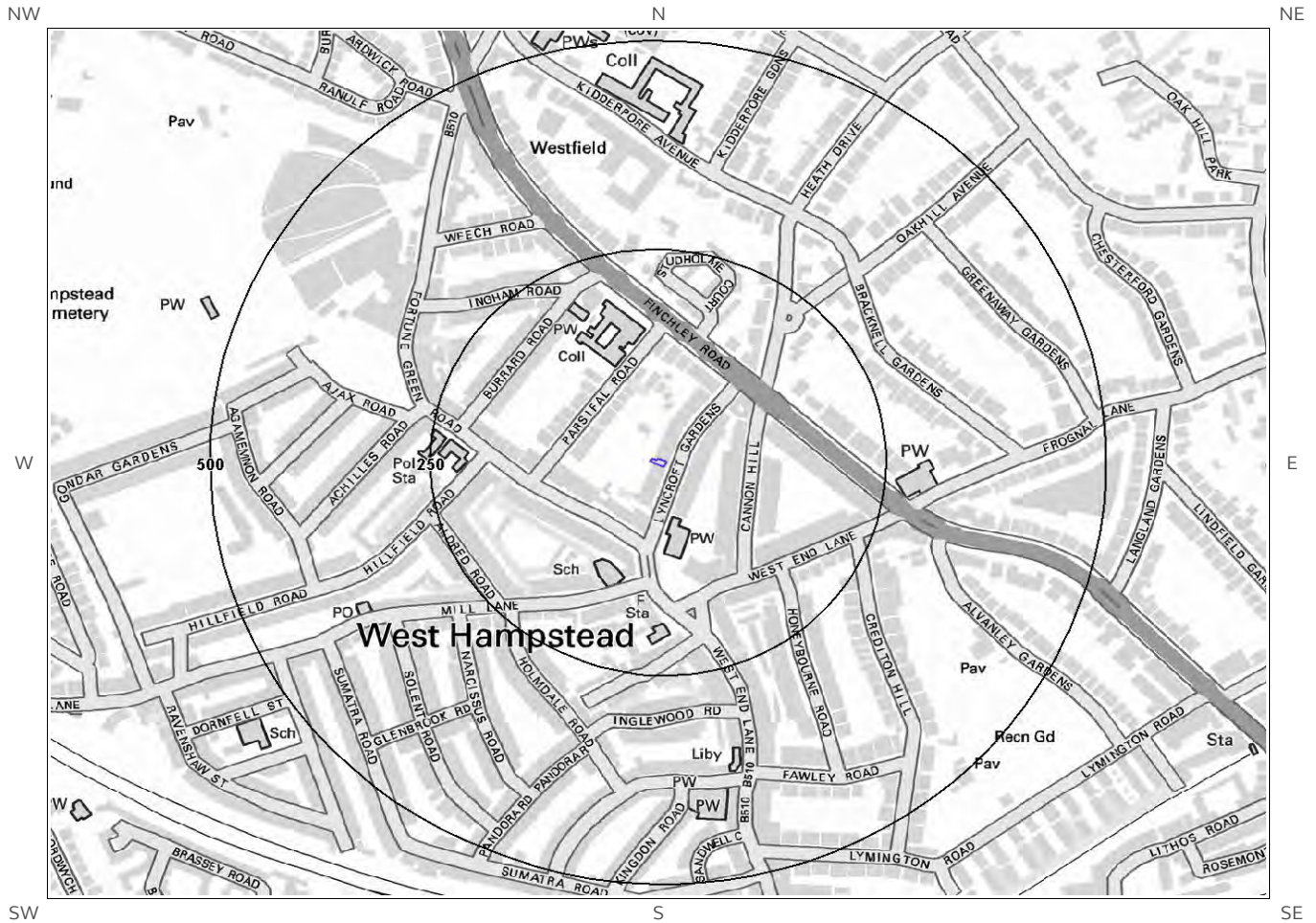
6c. Hydrogeology – Source Protection Zones and Potable Water Abstraction Licences



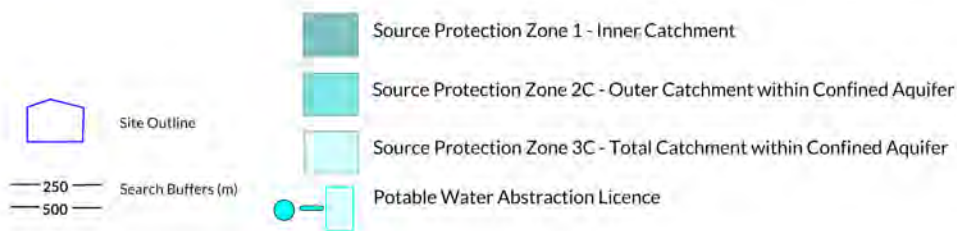
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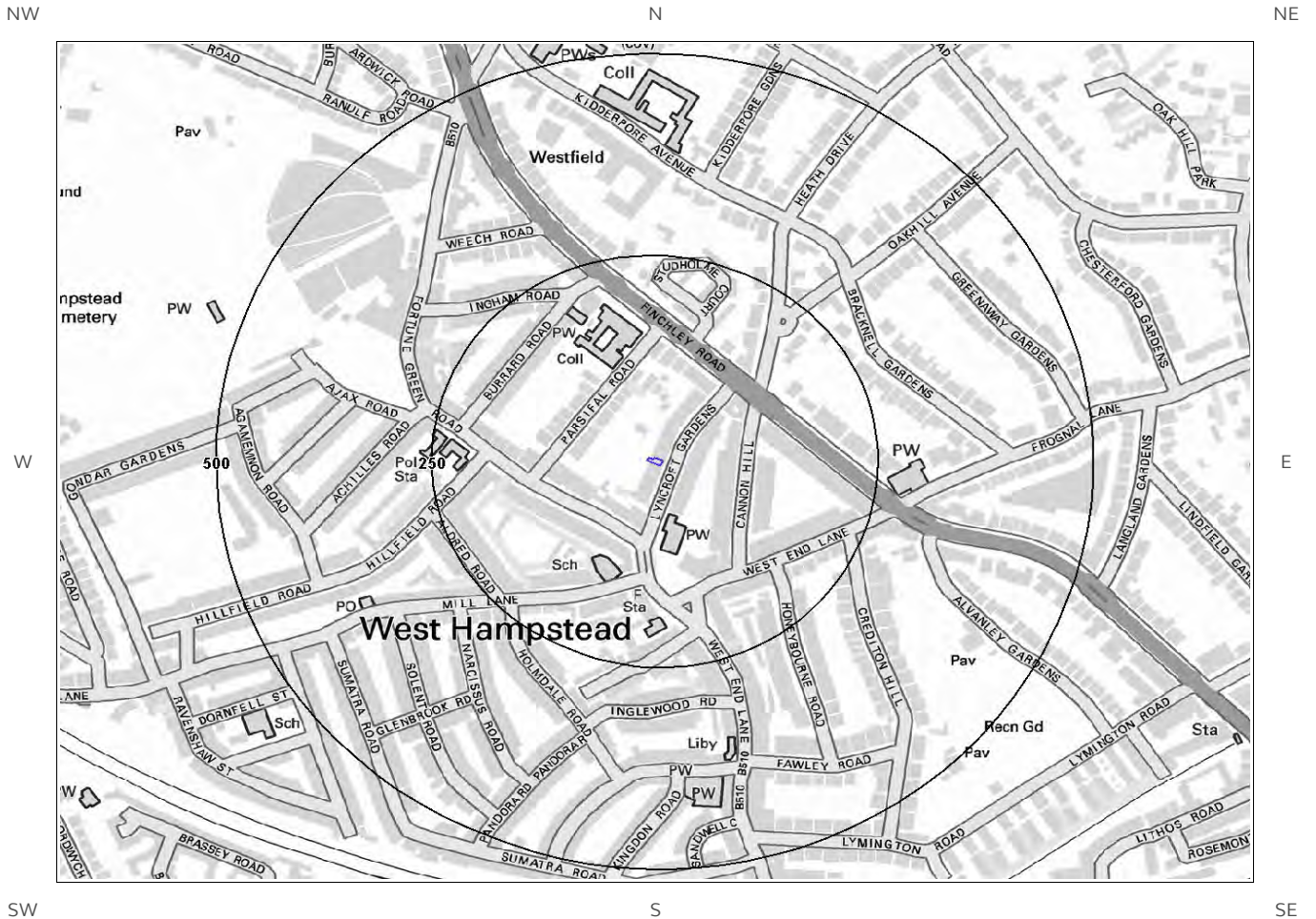
6d. Hydrogeology – Source Protection Zones within confined aquifer



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6e. Hydrology – Watercourse Network and River Quality



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6. Hydrogeology and Hydrology

6.1 Aquifer within Superficial Deposits

Records of strata classification within the superficial geology at or in proximity to the property No

Database searched and no data found.

From 1 April 2010, the Environment Agency/Natural Resources Wales's Groundwater Protection Policy has been using aquifer designations consistent with the Water Framework Directive. For further details on the designation and interpretation of this information, please refer to the Groundsure Enviro Insight User Guide.

6.2 Aquifer within Bedrock Deposits

Records of strata classification within the bedrock geology at or in proximity to the property Yes

From 1 April 2010, the Environment Agency/Natural Resources Wales's Groundwater Protection Policy has been using aquifer designations consistent with the Water Framework Directive. For further details on the designation and interpretation of this information, please refer to the Groundsure Enviro Insight User Guide.

The following aquifer records are shown on the Aquifer within Bedrock Geology Map (6b):

ID	Distance (m)	Direction	Designation	Description
2	0	On Site	Unproductive	These are rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow
1	298	N	Secondary A	Permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as minor aquifers
3	384	W	Unproductive	These are rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow
4	389	S	Unproductive	These are rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow

6.3 Groundwater Abstraction Licences

Groundwater Abstraction Licences within 2000m of the study site

Identified

The following Abstraction Licences records are represented as points, lines and regions on the Aquifer within Bedrock Geology Map (6b):

ID	Distance (m)	Direction	NGR	Details	
Not shown	1760	SE	526750 184261	Status: Active Licence No: TH/039/0039/087 Details: Spray Irrigation - Direct Direct Source: THAMES GROUNDWATER Point: SWISS COTTAGE OPEN SPACE-BOREHOLE Data Type: Point Name: LONDON BOROUGH OF CAMDEN	Annual Volume (m ³): 10,512 Max Daily Volume (m ³): 29 Original Application No: - Original Start Date: Expiry Date: Issue No: Version Start Date: 05/12/2013 Version End Date:
Not shown	1760	SE	526750 184261	Status: Active Licence No: TH/039/0039/087 Details: Lake & Pond Throughflow Direct Source: THAMES GROUNDWATER Point: SWISS COTTAGE OPEN SPACE-BOREHOLE Data Type: Point Name: LONDON BOROUGH OF CAMDEN	Annual Volume (m ³): 10,512 Max Daily Volume (m ³): 29 Original Application No: - Original Start Date: Expiry Date: Issue No: Version Start Date: 05/12/2013 Version End Date:
Not shown	1760	SE	526750 184261	Status: Active Licence No: TH/039/0039/087 Details: General Washing/Process Washing Direct Source: THAMES GROUNDWATER Point: SWISS COTTAGE OPEN SPACE-BOREHOLE Data Type: Point Name: LONDON BOROUGH OF CAMDEN	Annual Volume (m ³): 10,512 Max Daily Volume (m ³): 29 Original Application No: - Original Start Date: Expiry Date: Issue No: Version Start Date: 05/12/2013 Version End Date:
Not shown	1787	SE	526800 184280	Status: Historical Licence No: 28/39/39/0219 Details: Spray Irrigation - Direct Direct Source: THAMES GROUNDWATER Point: SWISS COTTAGE OPEN SPACE-BOREHOLE Data Type: Point Name: LONDON BOROUGH OF CAMDEN	Annual Volume (m ³): 10,512 Max Daily Volume (m ³): 29 Original Application No: - Original Start Date: Expiry Date: Issue No: Version Start Date: 01/04/2008 Version End Date:

6.4 Surface Water Abstraction Licences

Surface Water Abstraction Licences within 2000m of the study site

None identified

Database searched and no data found.

6.5 Potable Water Abstraction Licences

Potable Water Abstraction Licences within 2000m of the study site

None identified

Database searched and no data found.

6.6 Source Protection Zones

Source Protection Zones within 500m of the study site

None identified

Database searched and no data found.

6.7 Source Protection Zones within Confined Aquifer

Source Protection Zones within the Confined Aquifer within 500m of the study site

None identified

Historically, Source Protection Zone maps have been focused on regulation of activities which occur at or near the ground surface, such as prevention of point source pollution and bacterial contamination of water supplies. Sources in confined aquifers were often considered to be protected from these surface pressures due to the presence of a low permeability confining layer (e.g. glacial till, clay). The increased interest in subsurface activities such as onshore oil and gas exploration, ground source heating and cooling requires protection zones for confined sources to be marked on SPZ maps where this has not already been done.

Database searched and no data found.

6.8 Groundwater Vulnerability and Soil Leaching Potential

Environment Agency/Natural Resources Wales information on groundwater vulnerability and soil leaching potential within 500m of the study site

Identified

Distance (m)	Direction	Classification	Soil Vulnerability Category	Description
284	NE	Minor Aquifer/High Leaching Potential	HU	Soil information for urban areas and restored mineral workings. These soils are therefore assumed to be highly permeable in the absence of site-specific information.

6.9 River Quality

Environment Agency/Natural Resources Wales information on river quality within 1500m of the study site

None identified

6.9.1 Biological Quality:

Database searched and no data found.

6.9.2 Chemical Quality:

Database searched and no data found.

6.10 Ordnance Survey MasterMap Water Network

Ordnance Survey MasterMap Water Network entries within 500m of the study site

Database searched and no data found.

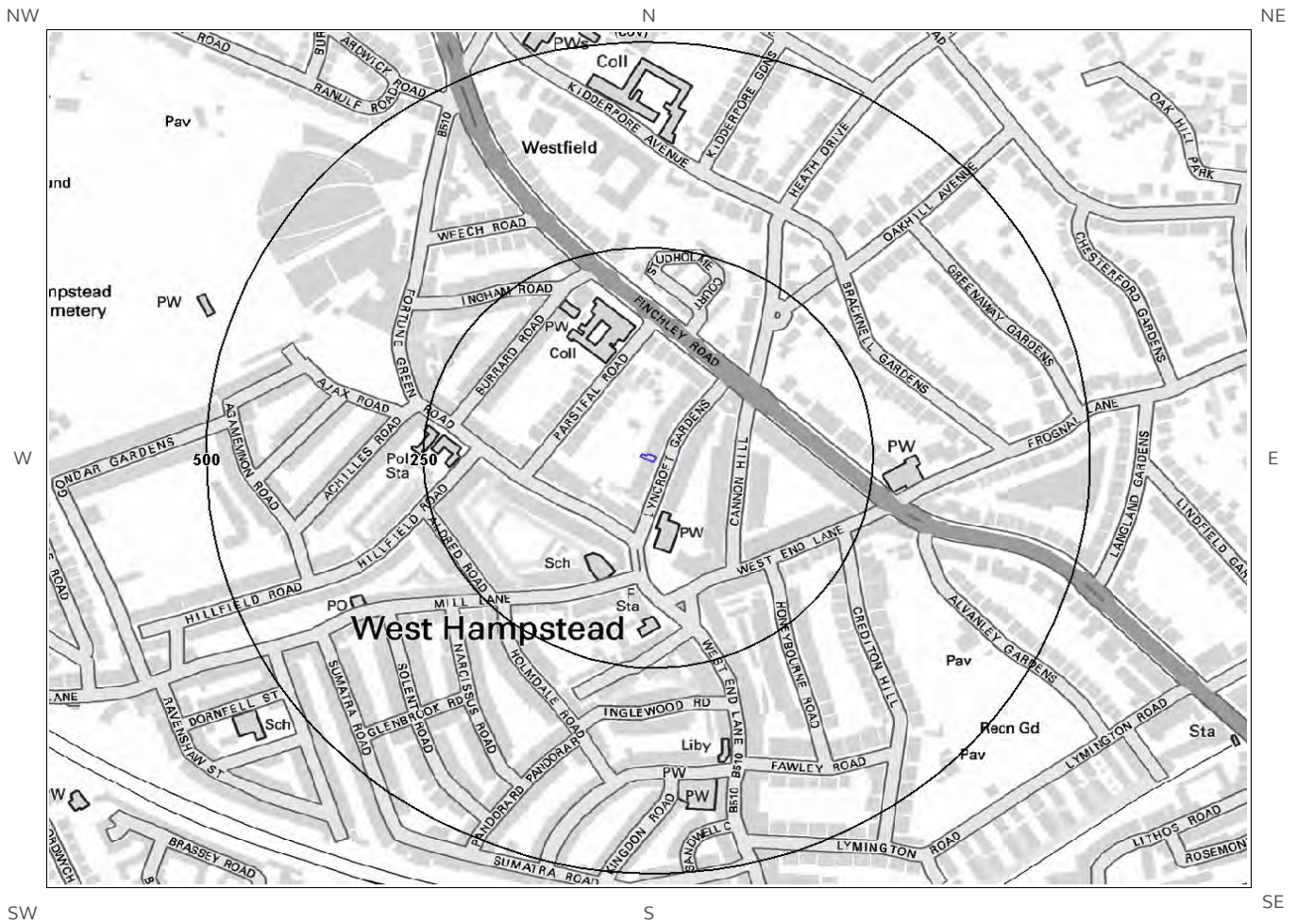
6.11 Surface Water Features

Surface water features within 250m of the study site

None identified

Database searched and no data found.

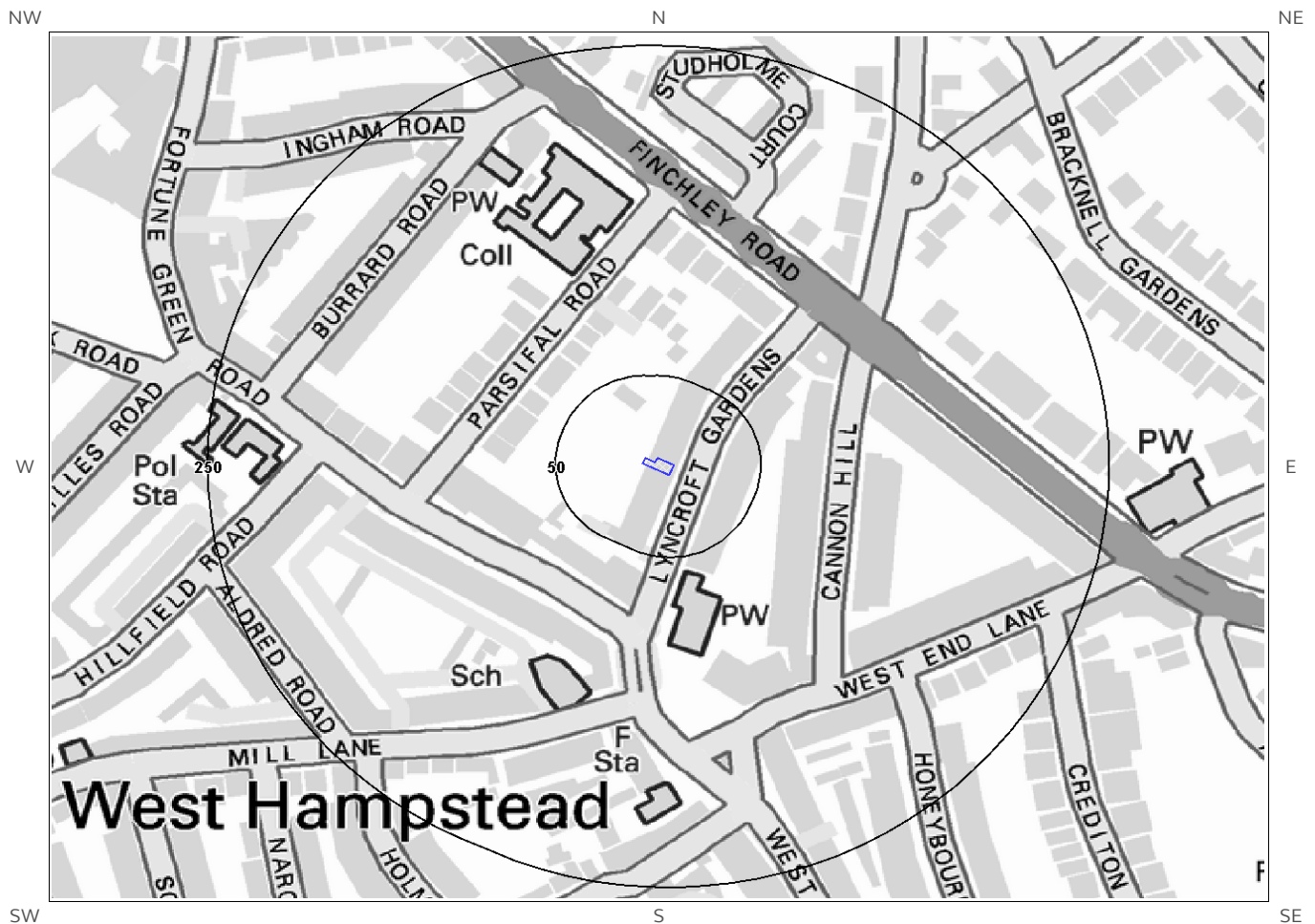
7a. Environment Agency/Natural Resources Wales Flood Map for Planning (from rivers and the sea)



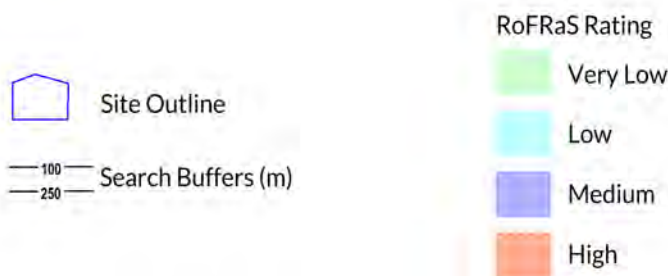
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7b. Environment Agency/Natural Resources Wales Risk of Flooding from Rivers and the Sea (RoFRaS) Map



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7 Flooding

7.1 River and Coastal Zone 2 Flooding

Environment Agency/Natural Resources Wales Zone 2 floodplain within 250m None identified

Environment Agency/Natural Resources Wales Zone 2 floodplains estimate the annual probability of flooding as between 1 in 1000 (0.1%) and 1 in 100 (1%) from rivers and between 1 in 1000 (0.1%) and 1 in 200 (0.5%) from the sea. Any relevant data is represented on Map 7a – Flood Map for Planning:

Database searched and no data found.

7.2 River and Coastal Zone 3 Flooding

Environment Agency/Natural Resources Wales Zone 3 floodplain within 250m None identified

Zone 3 shows the extent of a river flood with a 1 in 100 (1%) or greater chance of occurring in any year or a sea flood with a 1 in 200 (0.5%) or greater chance of occurring in any year. Any relevant data is represented on Map 7a – Flood Map for Planning.

Database searched and no data found.

7.3 Risk of Flooding from Rivers and the Sea (RoFRaS) Flood Rating

Highest risk of flooding onsite Very Low

The Environment Agency/Natural Resources Wales RoFRaS database provides an indication of river and coastal flood risk at a national level on a 50m grid with the flood rating at the centre of the grid calculated and given above. The data considers the probability that the flood defences will overtop or breach by considering their location, type, condition and standard of protection.

RoFRaS data for the study site indicates the property is in an area with a Very Low (less than 1 in 1000) chance of flooding in any given year.

7.4 Flood Defences

Flood Defences within 250m of the study site None identified
Database searched and no data found.

7.5 Areas benefiting from Flood Defences

Areas benefiting from Flood Defences within 250m of the study site None identified

7.6 Areas benefiting from Flood Storage

Areas used for Flood Storage within 250m of the study site

None identified

7.7 Groundwater Flooding Susceptibility Areas

7.7.1 British Geological Survey groundwater flooding susceptibility areas within 50m of the boundary of the study site

None identified

Notes: Groundwater flooding may either be associated with shallow unconsolidated sedimentary aquifers which overlie unproductive aquifers (Superficial Deposits Flooding), or with unconfined aquifers (Clearwater Flooding).

7.7.2 Highest susceptibility to groundwater flooding in the search area based on the underlying geological conditions

Not Prone

The area is not considered to be prone to groundwater flooding based on rock type.

7.8 Groundwater Flooding Confidence Areas

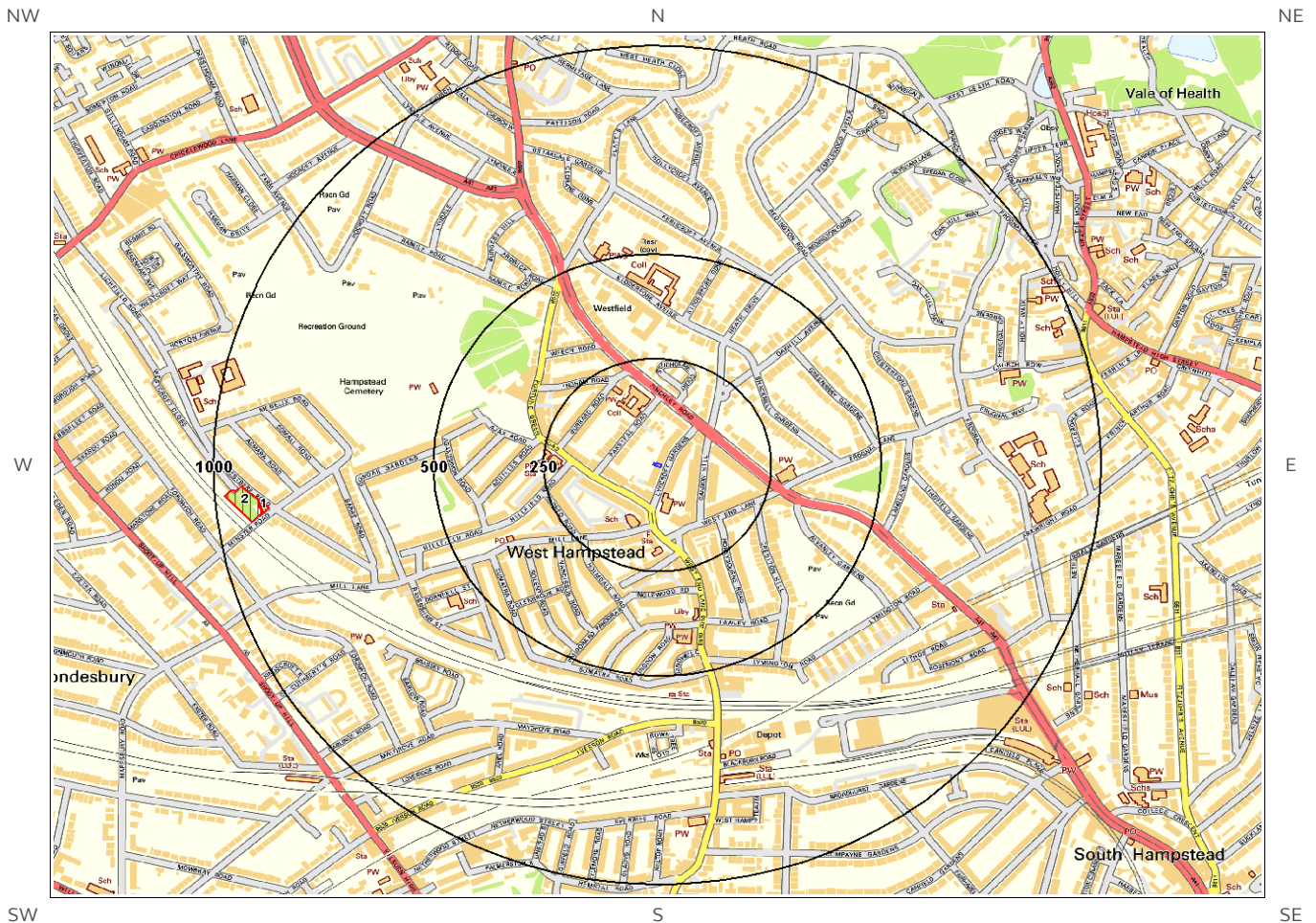
British Geological Survey confidence rating in this result

Not Applicable

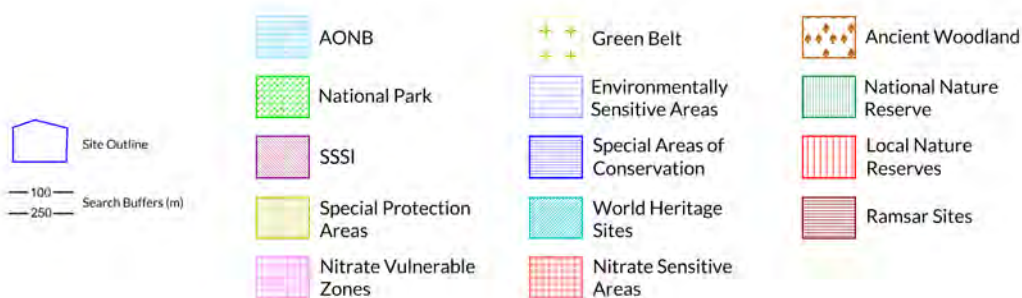
Notes: Groundwater flooding is defined as the emergence of groundwater at the ground surface or the rising of groundwater into man-made ground under conditions where the normal range of groundwater levels is exceeded.

The confidence rating is on a threefold scale - Low, Moderate and High. This provides a relative indication of the BGS confidence in the accuracy of the susceptibility result for groundwater flooding. This is based on the amount and precision of the information used in the assessment. In areas with a relatively lower level of confidence the susceptibility result should be treated with more caution. In other areas with higher levels of confidence the susceptibility result can be used with more confidence.

8. Designated Environmentally Sensitive Sites Map



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8. Designated Environmentally Sensitive Sites

Designated Environmentally Sensitive Sites within 2000m of the study site

Identified

8.1 Records of Sites of Special Scientific Interest (SSSI) within 2000m of the study site:

0

Database searched and no data found.

8.2 Records of National Nature Reserves (NNR) within 2000m of the study site:

0

Database searched and no data found.

8.3 Records of Special Areas of Conservation (SAC) within 2000m of the study site:

0

Database searched and no data found.

8.4 Records of Special Protection Areas (SPA) within 2000m of the study site:

0

Database searched and no data found.

8.5 Records of Ramsar sites within 2000m of the study site:

0

Database searched and no data found.

8.6 Records of Ancient Woodland within 2000m of the study site:

1

The following records of Designated Ancient Woodland provided by Natural England/Natural Resources Wales are represented as polygons on the Designated Environmentally Sensitive Sites Map:

ID	Distance (m)	Direction	Ancient Woodland Name	Data Source
Not shown	1481	NE	BISHOPS WOOD	Ancient & Semi-Natural Woodland

8.7 Records of Local Nature Reserves (LNR) within 2000m of the study site:

2

The following Local Nature Reserve (LNR) records provided by Natural England/Natural Resources Wales are represented as polygons on the Designated Environmentally Sensitive Sites Map:

ID	Distance (m)	Direction	LNR Name	Data Source
1	880	W	Westbere Copse	Natural England
2	899	W	Westbere Copse	Natural England

8.8 Records of World Heritage Sites within 2000m of the study site:

0

Database searched and no data found.

8.9 Records of Environmentally Sensitive Areas within 2000m of the study site:

0

Database searched and no data found.

8.10 Records of Areas of Outstanding Natural Beauty (AONB) within 2000m of the study site:

0

Database searched and no data found.

8.11 Records of National Parks (NP) within 2000m of the study site:

0

Database searched and no data found.

8.12 Records of Nitrate Sensitive Areas within 2000m of the study site:

0

Database searched and no data found.

8.13 Records of Nitrate Vulnerable Zones within 2000m of the study site:

0

Database searched and no data found.

8.14 Records of Green Belt land within 2000m of the study site:

0

Database searched and no data found.

9. Natural Hazards Findings

9.1 Detailed BGS GeoSure Data

BGS GeoSure Data has been searched to 50m. The data is included in tabular format. If you require further information on geology and ground stability, please obtain a **Groundsure Geo Insight**, available from our **website**. The following information has been found:

9.1.1 Shrink Swell

Maximum Shrink-Swell** hazard rating identified on the study site Moderate

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard
Ground conditions predominantly high plasticity. Do not plant or remove trees or shrubs near to buildings without expert advice about their effect and management. For new build, consideration should be given to advice published by the National House Building Council (NHBC) and the Building Research Establishment (BRE). There is a probable increase in construction cost to reduce potential shrink-swell problems. For existing property, there is a probable increase in insurance risk during droughts or where vegetation with high moisture demands is present.

9.1.2 Landslides

Maximum Landslide* hazard rating identified on the study site Very Low

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard
Slope instability problems are unlikely to be present. No special actions required to avoid problems due to landslides. No special ground investigation required, and increased construction costs or increased financial risks are unlikely due to potential problems with landslides.

9.1.3 Soluble Rocks

Maximum Soluble Rocks* hazard rating identified on the study site Negligible

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard
Soluble rocks are present, but unlikely to cause problems except under exceptional conditions. No special actions required to avoid problems due to soluble rocks. No special ground investigation required, and increased construction costs or increased financial risks are unlikely due to potential problems with soluble rocks.

* This indicates an automatically generated 50m buffer and site.

9.1.4 Compressible Ground

Maximum Compressible Ground* hazard rating identified on the study site

Negligible

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard

No indicators for compressible deposits identified. No special actions required to avoid problems due to compressible deposits. No special ground investigation required, and increased construction costs or increased financial risks are unlikely due to potential problems with compressible deposits.

9.1.5 Collapsible Rocks

Maximum Collapsible Rocks* hazard rating identified on the study site

Very Low

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard

Deposits with potential to collapse when loaded and saturated are unlikely to be present. No special ground investigation required or increased construction costs or increased financial risk due to potential problems with collapsible deposits.

9.1.6 Running Sand

Maximum Running Sand** hazard rating identified on the study site

Very Low

The following natural subsidence information provided by the British Geological Survey is not represented on mapping:

Hazard

Very low potential for running sand problems if water table rises or if sandy strata are exposed to water. No special actions required, to avoid problems due to running sand. No special ground investigation required, and increased construction costs or increased financial risks are unlikely due to potential problems with running sand.

* This indicates an automatically generated 50m buffer and site.

9.2 Radon

9.2.1 Radon Affected Areas

Is the property in a Radon Affected Area as defined by the Health Protection Agency (HPA) and if so what percentage of homes are above the Action Level? The site is not in a Radon Affected Area, as less than 1% of properties are above the Action Level.

The radon data in this report is supplied by the BGS/Public Health England and is the definitive map of Radon Affected Areas in Great Britain and Northern Ireland. The dataset was created using long-term radon measurements in over 479,000 homes across Great Britain and 23,000 homes across Northern Ireland, combined with geological data. The dataset is considered accurate to 50m to allow for the margin of error in geological lines, and the findings of this report supercede any answer given in the less accurate Indicative Atlas of Radon in Great Britain, which simplifies the data to give the highest risk within any given 1km grid square. As such, the radon atlas is considered indicative, whereas the data given in this report is considered definitive.

9.2.2 Radon Protection

Is the property in an area where Radon Protection are required for new properties or extensions to existing ones as described in publication BR211 by the Building Research Establishment? No radon protective measures are necessary.

10. Mining

10.1 Coal Mining

Coal mining areas within 75m of the study site

None identified

Database searched and no data found.

10.2 Non-Coal Mining

Non-Coal Mining areas within 50m of the study site boundary

None identified

Database searched and no data found.

10.3 Brine Affected Areas

Brine affected areas within 75m of the study site

None identified

Guidance: No Guidance Required.

Contact Details

Groundsure Helpline
Telephone: 08444 159 000
info@groundsure.com

British Geological Survey Enquiries

Kingsley Dunham Centre
Keyworth, Nottingham NG12 5GG
Tel: 0115 936 3143.
Fax: 0115 936 3276.
Email:

Web: www.bgs.ac.uk

BGS Geological Hazards Reports and general geological enquiries:
enquiries@bgs.ac.uk

Environment Agency

National Customer Contact Centre, PO Box 544
Rotherham, S60 1BY
Tel: 03708 506 506

Web: www.environment-agency.gov.uk

Email: enquiries@environment-agency.gov.uk

Public Health England

Public information access office
Public Health England, Wellington House
133-155 Waterloo Road, London, SE1 8UG
www.gov.uk/phe

Email: enquiries@phe.gov.uk
Main switchboard: 020 7654 8000

The Coal Authority

200 Lichfield Lane
Mansfield
Notts NG18 4RG
Tel: 0345 7626 848
DX 716176 Mansfield 5
www.coal.gov.uk

Ordnance Survey

Adanac Drive, Southampton
SO16 0AS
Tel: 08456 050505

Local Authority

Authority: London Borough of Camden
Phone: 020 7974 4444

Web: <http://www.camden.gov.uk/>

Address: Camden Town Hall, Judd Street, London, WC1H 9JE

Gemapping PLC

Virginia Villas, High Street, Hartley Witney,
Hampshire RG27 8NW
Tel: 01252 845444



Public Health
England



The Coal
Authority



Acknowledgements: Site of Special Scientific Interest, National Nature Reserve, Ramsar Site, Special Protection Area, Special Area of Conservation data is provided by, and used with the permission of, Natural England/Natural Resources Wales who retain the Copyright and Intellectual Property Rights for the data.

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<https://www.groundsure.com/terms-and-conditions-feb11-2019>

Appendix C Historical Maps (Groundsure)

Site Details:

15, LINCROFT GARDENS,
LONDON, NW6 1LB

Client Ref: MGC_19-29
Report Ref: HMD-6410192
Grid Ref: 525393, 185394

Map Name: County Series

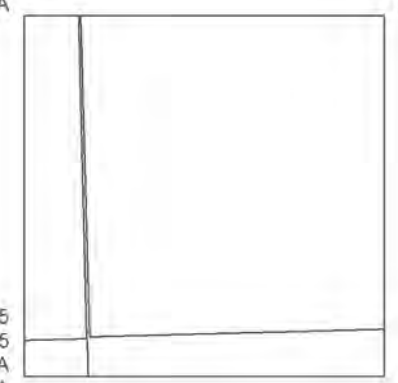
Map date: 1865-1870

Scale: 1:2,500

Printed at: 1:2,500



Surveyed 1866
Revised 1866
Edition N/A
Copyright N/A
Levelled N/A



Surveyed 1865
Revised 1865
Edition N/A
Copyright N/A
Levelled N/A

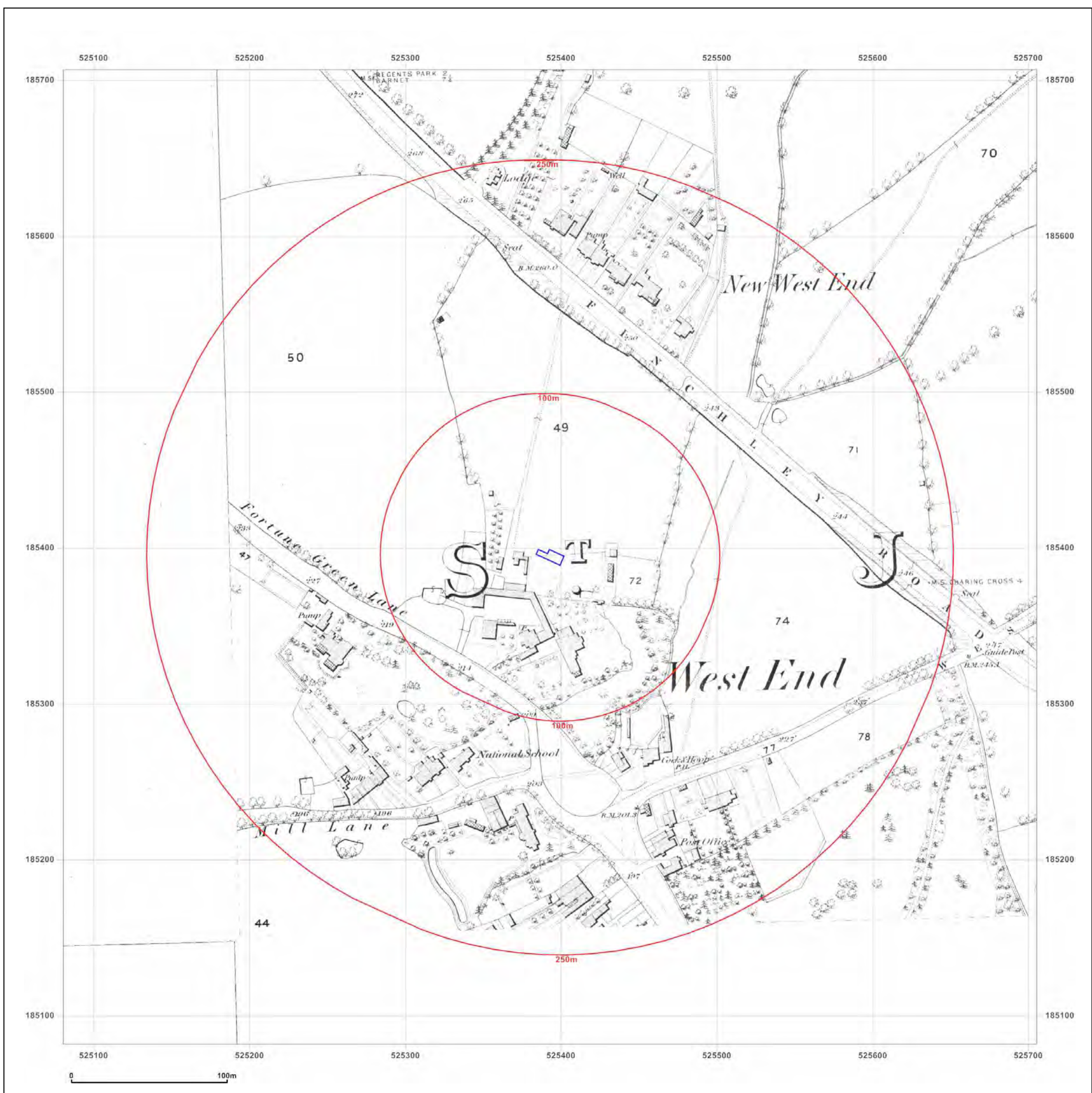


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E: info@groundsure.com
W: www.groundsure.com

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Production date: 19 October 2019

Map legend available at:
www.groundsure.com/sites/default/files/groundsure_legend.pdf



Site Details:

15, LINCROFT GARDENS,
LONDON, NW6 1LB

Client Ref: MGC_19-29
Report Ref: HMD-6410192
Grid Ref: 525393, 185394

Map Name: County Series

Map date: 1896

Scale: 1:2,500

Printed at: 1:2,500



Surveyed 1896
Revised 1896
Edition N/A
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Levelled N/A

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Revised 1896
Edition N/A
Copyright N/A
Levelled N/A

Surveyed 1896
Revised 1896
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Surveyed 1896
Revised 1896
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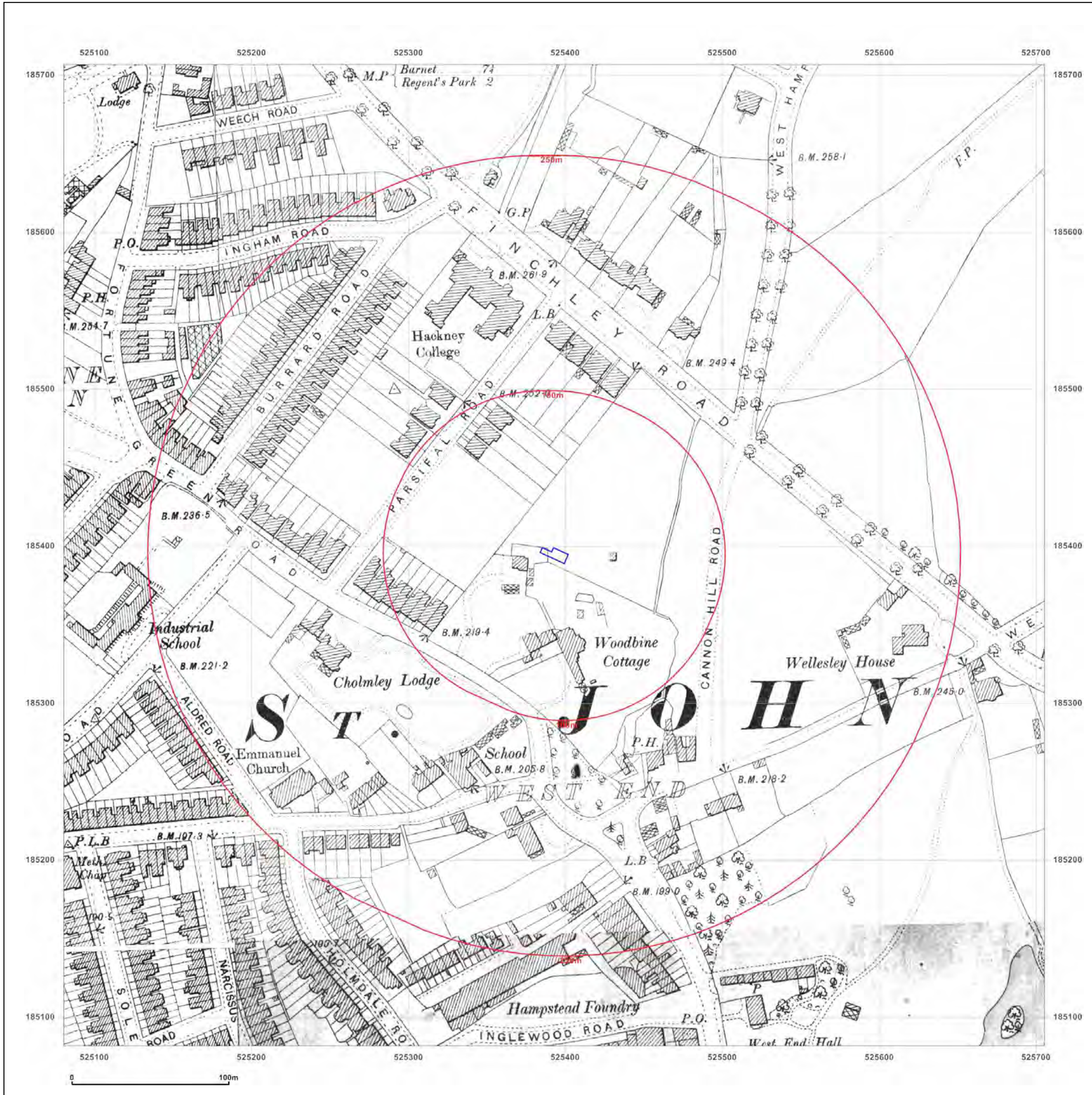


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Production date: 19 October 2019

Map legend available at:
www.groundsure.com/sites/default/files/groundsure_legend.pdf



Site Details:

15, LINCROFT GARDENS,
LONDON, NW6 1LB

Client Ref: MGC_19-29
Report Ref: HMD-6410192
Grid Ref: 525393, 185394

Map Name: County Series

Map date: 1915

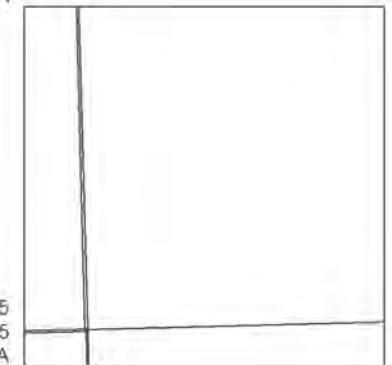
Scale: 1:2,500

Printed at: 1:2,500



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Revised 1915
Edition N/A
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Levelled N/A

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Revised 1915
Edition N/A
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Surveyed 1915
Revised 1915
Edition N/A
Copyright N/A
Levelled N/A

Surveyed 1915
Revised 1915
Edition N/A
Copyright N/A
Levelled N/A

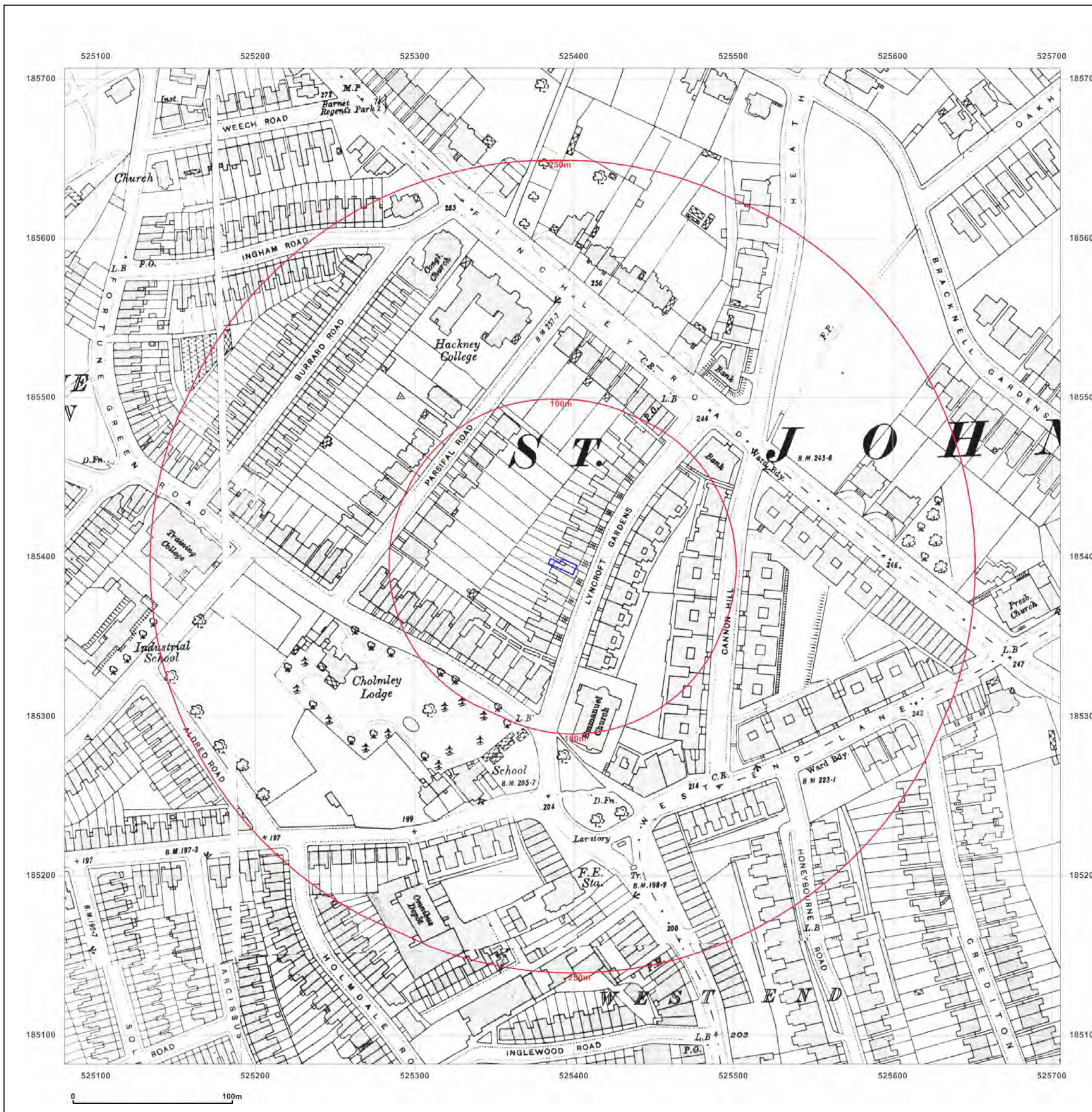


Produced by
Groundsure Insights
T: 08444 159000
E: info@groundsure.com
W: www.groundsure.com

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Production date: 19 October 2019

Map legend available at:
www.groundsure.com/sites/default/files/groundsure_legend.pdf



Site Details:

15, LYNCROFT GARDENS,
LONDON, NW6 1LB

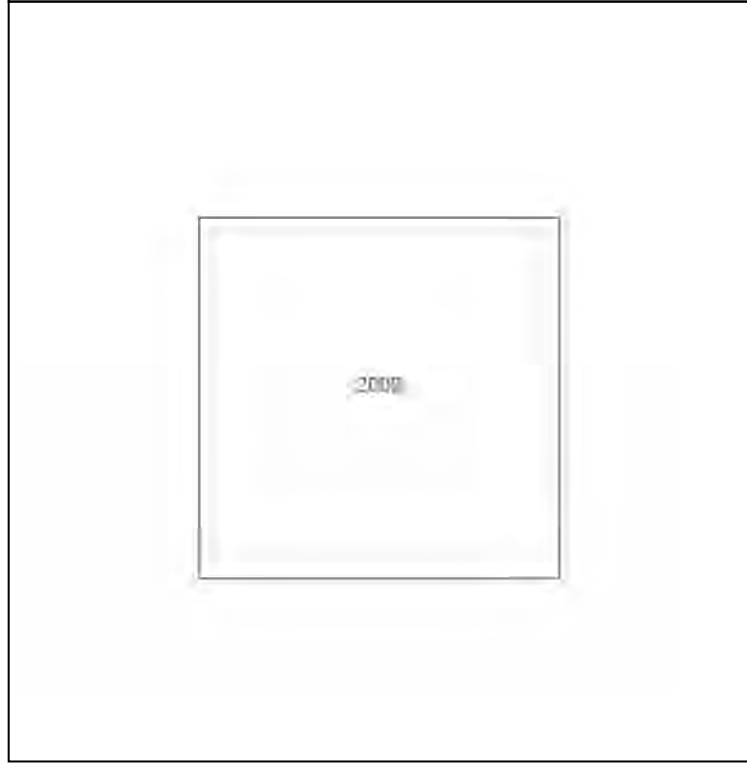
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Report Ref: HMD-6410192
Grid Ref: 525393, 185394

Map Name: LandLine

Map date: 2003

Scale: 1:1,250

Printed at: 1:1,250

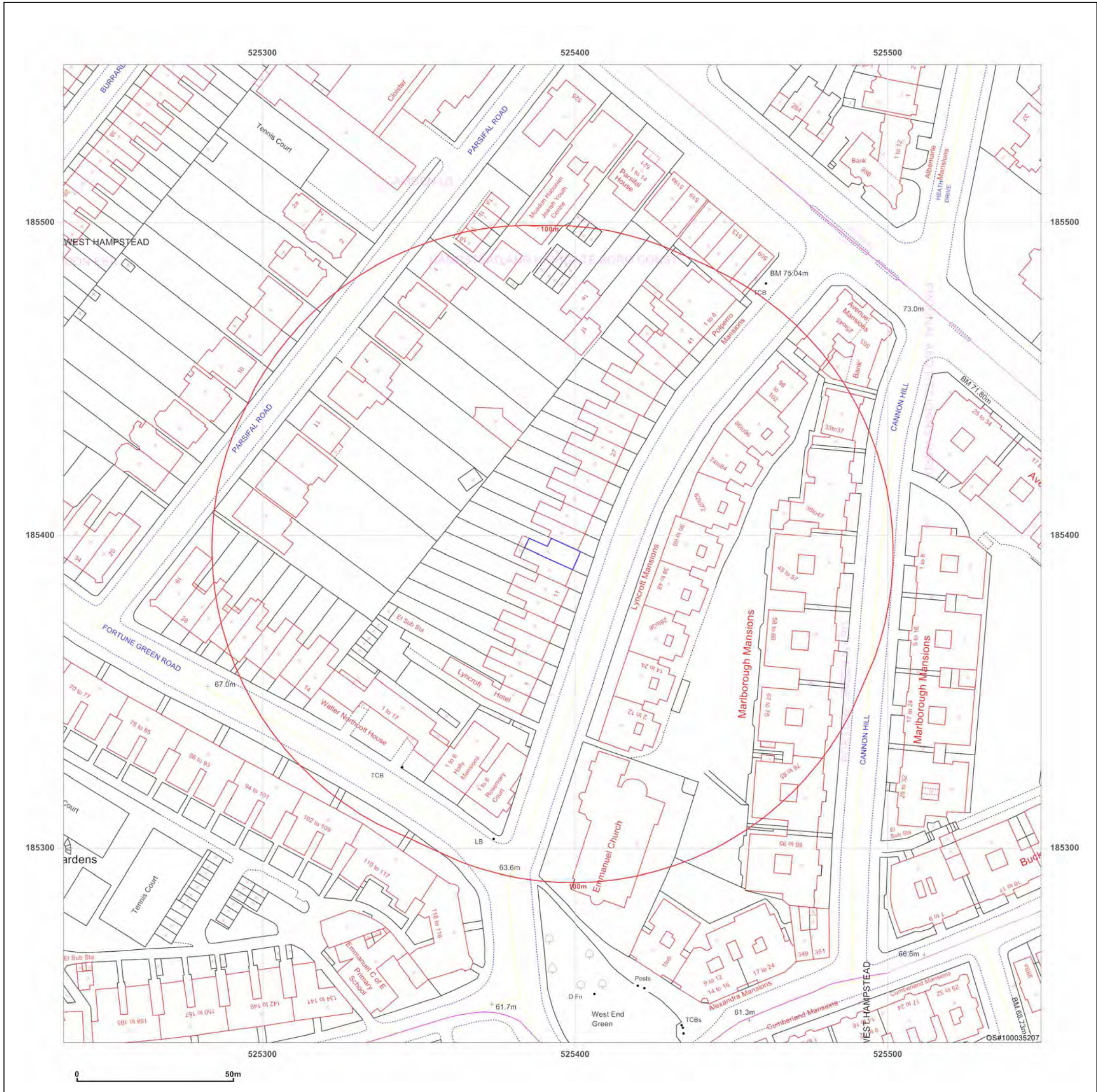


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Production date: 19 October 2019

Map legend available at:
www.groundsure.com/sites/default/files/groundsure_legend.pdf



Appendix D Ground Investigation Report

Site Investigation

15 Lyncroft Gdns, London NW6 1LB

Geotechnical Factual Report

8 November 2019

MAUND GEO-CONSULTING

Produced for :

Richard Banks
15 Lyncroft Gardens
London NW6 1LB

Prepared by:

Julian Maund BSc PhD CEng MIMMM CGeol FGS
UK Registered Ground Engineering Adviser

Maund Geo-Consulting Ltd
3 Coopers Square
Chipping Norton
OX7 5DG

T 07817018716
E julian.maund@gmail.com

T 07817 018716
E julian.maund@gmail.com

MGC-FR-19-29-V1



Report Title	Factual Report	Site Address	15 Lyncroft Gardens, London NW6 1LB
Work Stage	Site Investigation	Report Date	November 2019
Brief Description of the Report Contents	Factual report on a geotechnical site investigation undertaken in October 2019 as part of a Basement Impact Assessment for the London Borough of Camden		

Contents

1	Introduction	1
1.1	Terms of Reference	1
1.2	Limitations.....	1
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2.2	Geology	2
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3.1	General	3
3.1	Service Clearance and survey	3
3.2	Drilling depths	3
3.3	Insitu Testing	3
3.4	Sampling	3
3.5	Installation.....	3
4	Laboratory Testing	4
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	Appendix A Exploratory Location Plan and Hole Records	
	Appendix B Geotechnical Laboratory Test Report	

1 Introduction

1.1 Terms of Reference

Maund Geo-Consulting Ltd (MGC) was instructed on 16 September 2019 by Croft Structural Engineers Ltd on behalf of Richard Banks to undertake a ground investigation at 15 Lyncroft Gardens to provide information on the ground conditions to support a Basement Impact Assessment to satisfy planning requirements for a basement extension.

1.2 Limitations

Notwithstanding anything to the contrary contained in the report, Maund Geo Consulting Limited (MGC) has exercised reasonable skill, care and diligence in the performance of the services required by Richard Banks and MGC shall not be liable except to the extent that it has failed to exercise reasonable skill, care and diligence and this report shall be read and construed accordingly. Information provided by third parties has been used in good faith and is taken at face value; however, MGC cannot guarantee its accuracy or completeness. The inherent variation of ground conditions allows only definition of the actual conditions at the locations and depths at the time of the investigation. At intermediate locations, conditions can only be inferred.

2 The Site

2.1 Location

The property is located at 15 Lyncroft Gardens NW6 1LB, National Grid Reference TQ253853, which is within the London Borough of Camden, shown in Figure 2.1



2.2 Geology

Information obtained from the British Geological Survey website viewer <http://mapapps.bgs.ac.uk/geologyofbritain/home.html> indicates that the site is located on The London Clay Formation.

3 Ground Investigation

3.1 General

A ground investigation was undertaken on 18 October 2019. The investigation was carried out by PM Sampling Ltd which comprised one borehole and two hand dug trial pits to expose party wall footings. The borehole was undertaken using a specialist Archway Trent rig suitable for areas of limited space. The sample was recovered in plastic tubular liners with 95+% recovery. The location of the exploratory holes is shown in Drawing MGC-19-29-001 in Appendix A. The exploratory hole records are also included in Appendix A

3.1 Service Clearance and survey

The borehole and trial pit locations were checked for service clearance by available drawings and an inspection pit for the borehole. The location of the borehole and trial pits was referenced to existing features.

3.2 Drilling depths

Borehole BH01 was drilled to 7.95 m. The trial pits were excavated to approximately 0.5 to 0.75 m depth.

3.3 Insitu Testing

Insitu Standard Penetration Tests were undertaken at regular intervals as shown on borehole records included in Appendix B.

3.4 Sampling

Disturbed samples were taken in the borehole from the split sampler at all SPT depths from the borehole. A bulk sample was taken from made ground in the borehole inspection pit.

3.5 Installation

A groundwater monitoring standpipe was installed in BH01 to a depth of 6 m. Details of the installation are included in the borehole records in Appendix B.

4 Laboratory Testing

4.1 General

Selected samples were sent to i2 Laboratories Ltd for geotechnical testing.

4.2 Geotechnical Testing on soil

The following tests were undertaken in accordance with BS1377:1990. The test results with methodologies are included in Appendix B.

Test type	No. of tests	Test Method
Moisture Content	5	BS1377:1990
Plasticity Index - 1 point Liquid Limit	5	BS1377:1990
pH, and water-soluble sulphate,	2	BRE SD1

5 Groundwater Monitoring

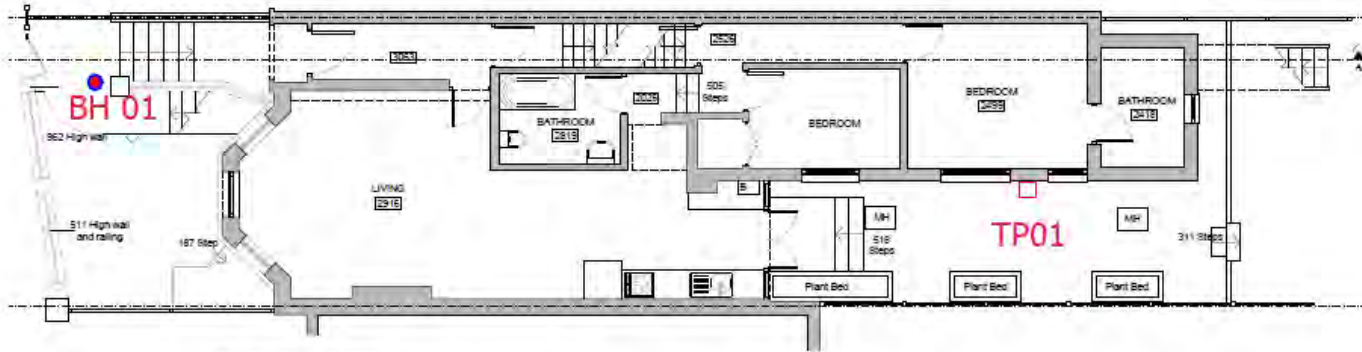
Groundwater levels in the borehole installation in BH01 was monitored while on site on 18/10/19 and then on 28/10/19 and 08/11/19. The results are provided in Table 5.1.

Table 5.1

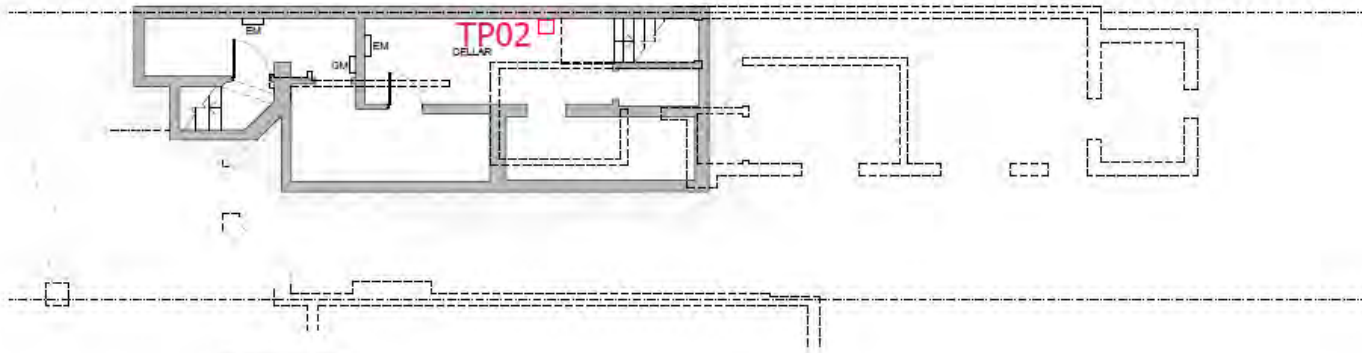
Date of Monitoring	Groundwater (depth metres below ground level)
18/10/19	Dry
28/10/19	4.02
08/11/19	3.90

Appendix A

Exploratory Location Plan and Exploratory Hole Records



1 / Ground Floor Plan



2 / Basement Plan

Client:
Richard Banks

Date:
November 2019

Project:
15 Lyncroft Gardens, NW6 1BL

Title:
Exploratory Hole Location Plan

MAUND GEO-CONSULTING
Julian Maund BSc PhD MIMMM CEng FGS
Registered Ground Engineering Adviser

Drawing Number:
MGC-19-29-001

PDXQG#JHRØRQVXOWIQJ

Mxdg# dxgg#V#kG#P IP P #Hgj#J V#J hro
Uhj lwhung# urxqg#qj lghulgj #gylhu#

Maund Geo-Consulting Ltd
3 Coopers Square
Chipping Norton
OX7 5DG
julian.maund@gmail.com

Borehole No.

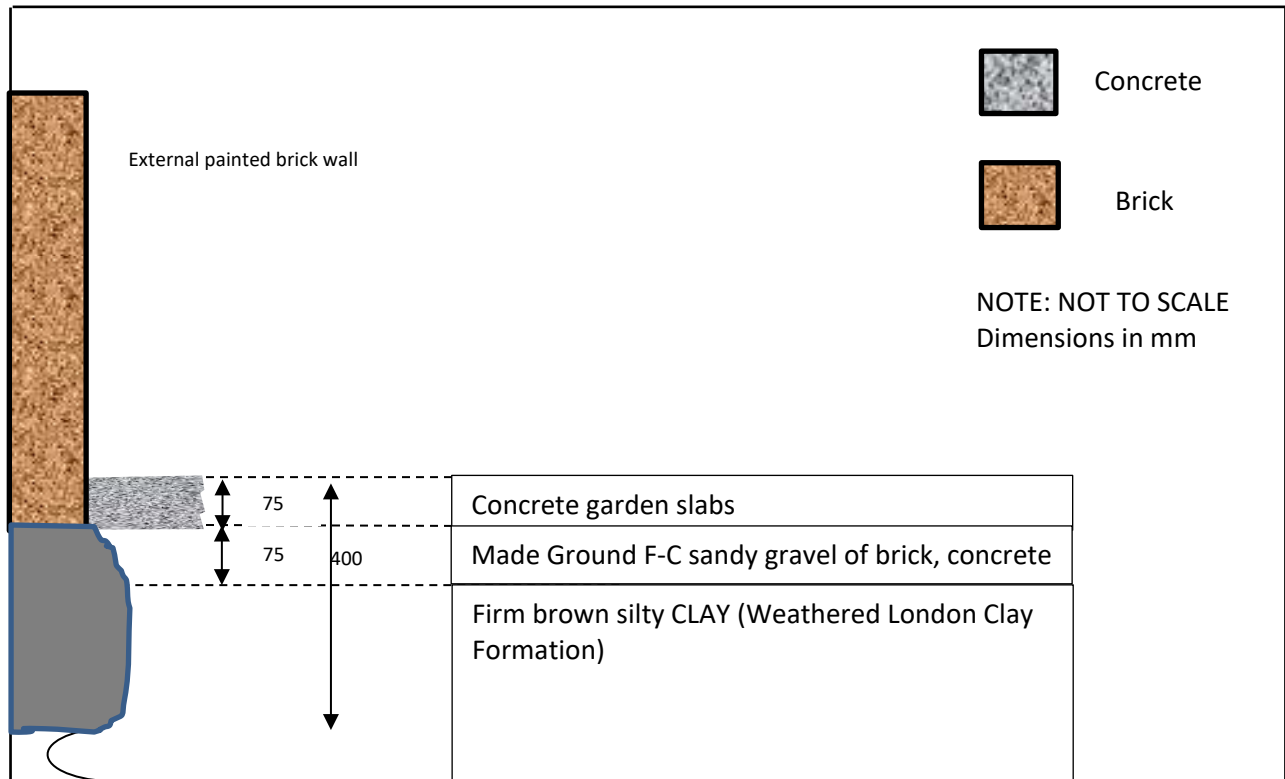
BH01

Sheet 1 of 1

Project Name:	15 Lyncroft Gardens NW6 1LB	Project No:	19-29	Co -ords	525404, 185388	Hole Type:	PM Trent lined
Location:	Bottom of front steps, 15 Lyncroft Gardens NW6 1LB			Level: (m AOD)	70.00 (Approx.)	Hole Diameter:	100 mm
Client:	Richard Banks			Date Drilled:	18/10/19	Logged By:	JGM

Well	Water Strikes	Sampling and Insitu Testing			Depth (m)	Level (m AOD)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.07	69.93		Made Ground: Terracotta tile over concrete
					0.17	69.83		Made Ground: Sandy, clayey, red brown fine to coarse GRAVEL of brick, and chert
	0.4-0.6	D1			0.80	69.2		Made Ground: Soft to firm brown grey CLAY with fine to medium gravel sized fragments of red brick
	1.00		N=9 (1/1,2,2,2,3)					Firm yellow-brown silty CLAY with a trace of fine brown sand (London Clay Formation) ... rootlets noted at 1.80
	1.80	D2						... becoming firm to stiff mottled brown and grey with fine sandy partings and coarse sand sized selenite crystals
	2.00		N=10 (2/1,2,3,2,3)					... cobble sized claystone at 3.50 with groundwater
	3.00	D3		N= 16 (2/2,4,3,5,4)				... becoming firm to stiff mottled brown and grey with fine sandy partings and coarse sand sized selenite crystals moist at 4.0 m
	3.50					66.50		... yellow sandy, possibly fine gravelly partings at 5.75
	3.90	D4		N= 15 (2/2,3,4,3,5)				
	4.00		N= 20 (3/4,4,5,5,6)					
5.00								
5.75	D5		N= 20 (4/4,5,4,5,6)					
6.00								
6.95	D6		N=26 (4,4,5,6,7,8)					
7.50								
					7.95	62.05		Borehole complete at 7.95 m

Remarks
Terracotta tile cored. Inspection pit to 0.8 m.
Borehole dry on completion.
Standpipe Piezometer installed to depth of 5 m. Bentonite seal from 1.0 to 0.3 m. Gas bung and flat cover



The foundation comprises a rubble of brick and clinker to a depth of 400mm below top of slab.

Client:
Richard Banks

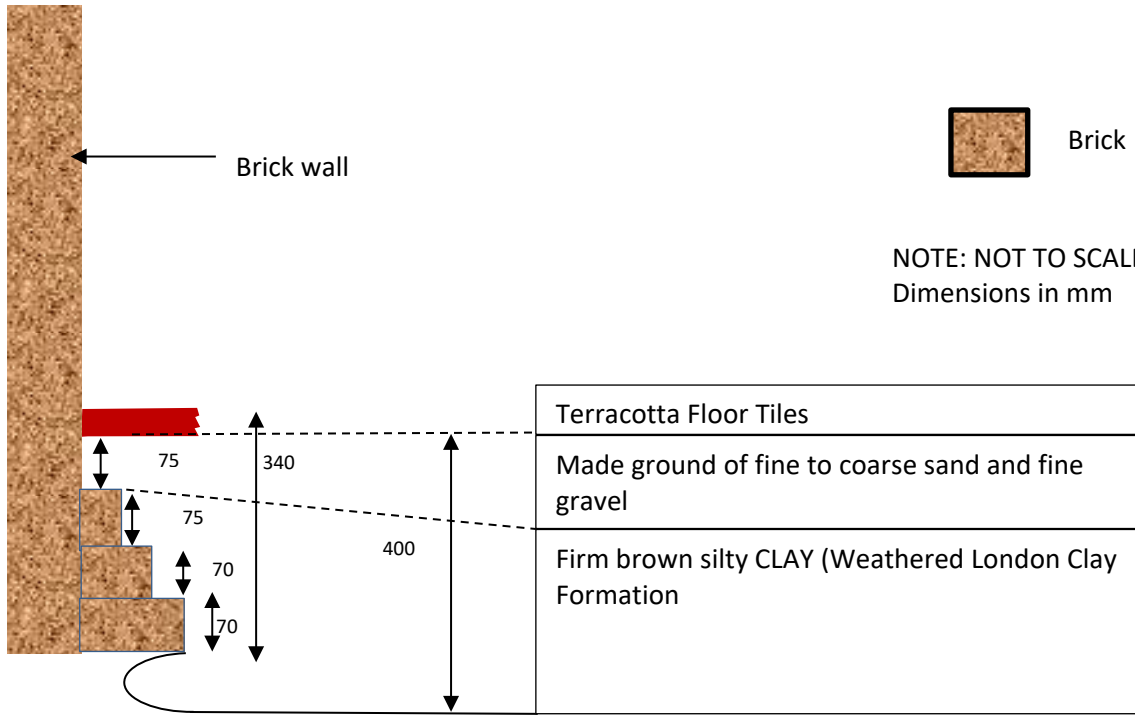
Date:
November 2019

Project:
**15 Lyncroft Gardens,
NW6 1BL**

Title:
Foundation Exposure TP01

MAUND GEO-CONSULTING
Julian Maund BSc PhD MIMMM CEng FGS
Registered Ground Engineering Adviser

Drawing Number:
MGC-19-29-002



The foundation comprises brick corbels extending 150mm from the wall to a depth of 340mm below top of tiles.

Client: Richard Banks	Date: November 2019	Project: 15 Lyncroft Gardens NW6 1LB	
Title: Foundation Exposure TP02	MAUND GEO-CONSULTING Julian Maund BSc PhD MIMMM CEng FGS Registered Ground Engineering Adviser		Drawing Number: MGC 19-29-003

Appendix C Geotechnical Laboratory Test Report



TEST CERTIFICATE

Liquid and Plastic Limits

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

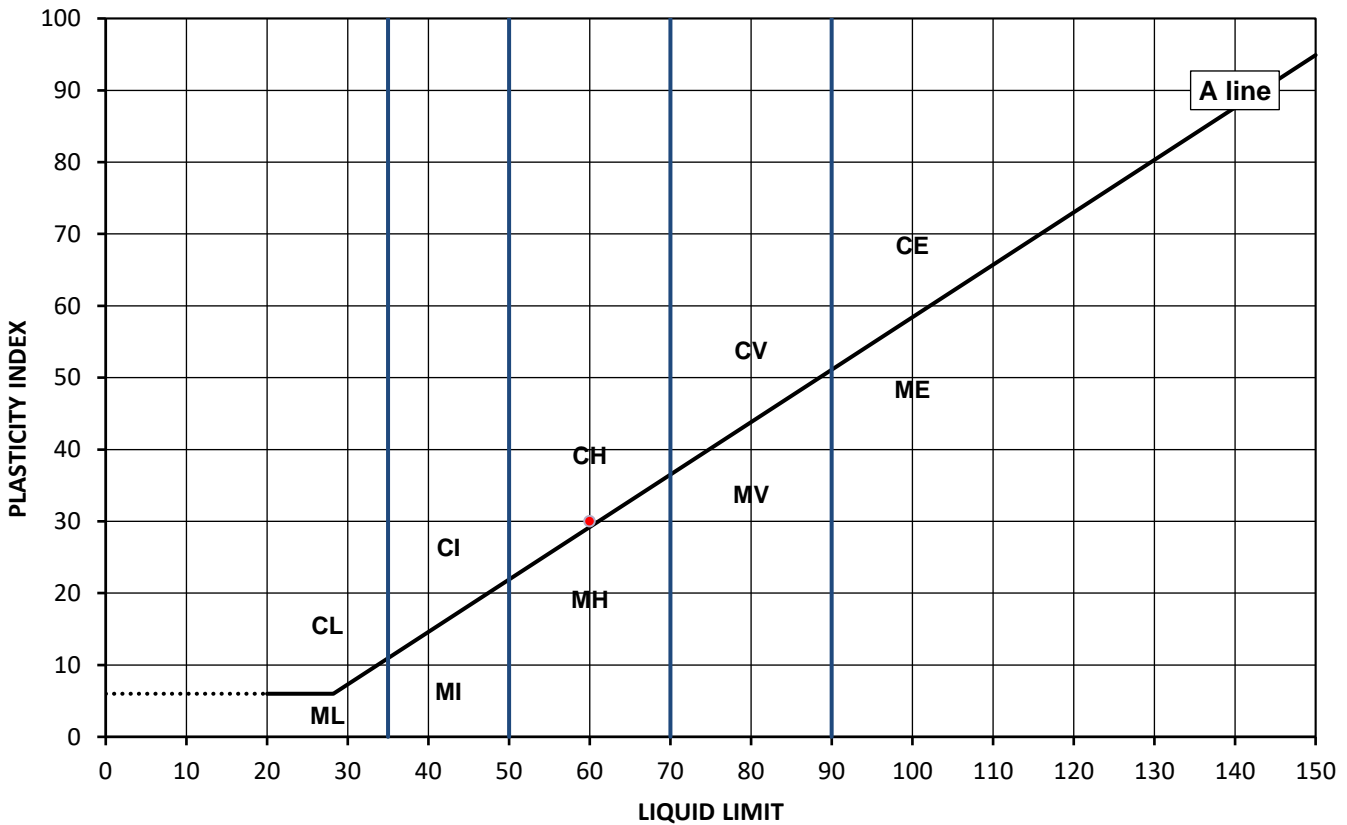
Test Results:

Laboratory Reference: 1339838
Hole No.: BH01-DO1
Sample Reference: Not Given
Soil Description: Brown slightly gravelly CLAY

Depth Top [m]: 0.40
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested after >425um removed by hand

As Received Moisture Content [%]	Liquid Limit [%]	Plastic Limit [%]	Plasticity Index [%]	% Passing 425µm BS Test Sieve
34	60	30	30	97



Legend, based on BS 5930:2015 Code of practice for site investigations

C	Clay	L	Low	Liquid Limit	below 35
M	Silt	I	Medium		35 to 50
		H	High		50 to 70
		V	Very high		70 to 90
		E	Extremely high		exceeding 90
	Organic	O	append to classification for organic material (eg CHO)		

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
for and on behalf of i2 Analytical Ltd GF 232.5

Opinions and interpretations expressed here in are outside of the scope of the UKAS Accreditation. This report may not be reproduced other than in full without the prior written approval of the issuing laboratory. The results included within the report are representative of the samples submitted for analysis. The analysis was carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Rudzka Staska, Poland.

Any assessment of compliance with specifications based the analytical results in a report take in to account no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies. An estimate of measurement uncertainty can be provided on request.



TEST CERTIFICATE

Liquid and Plastic Limits

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

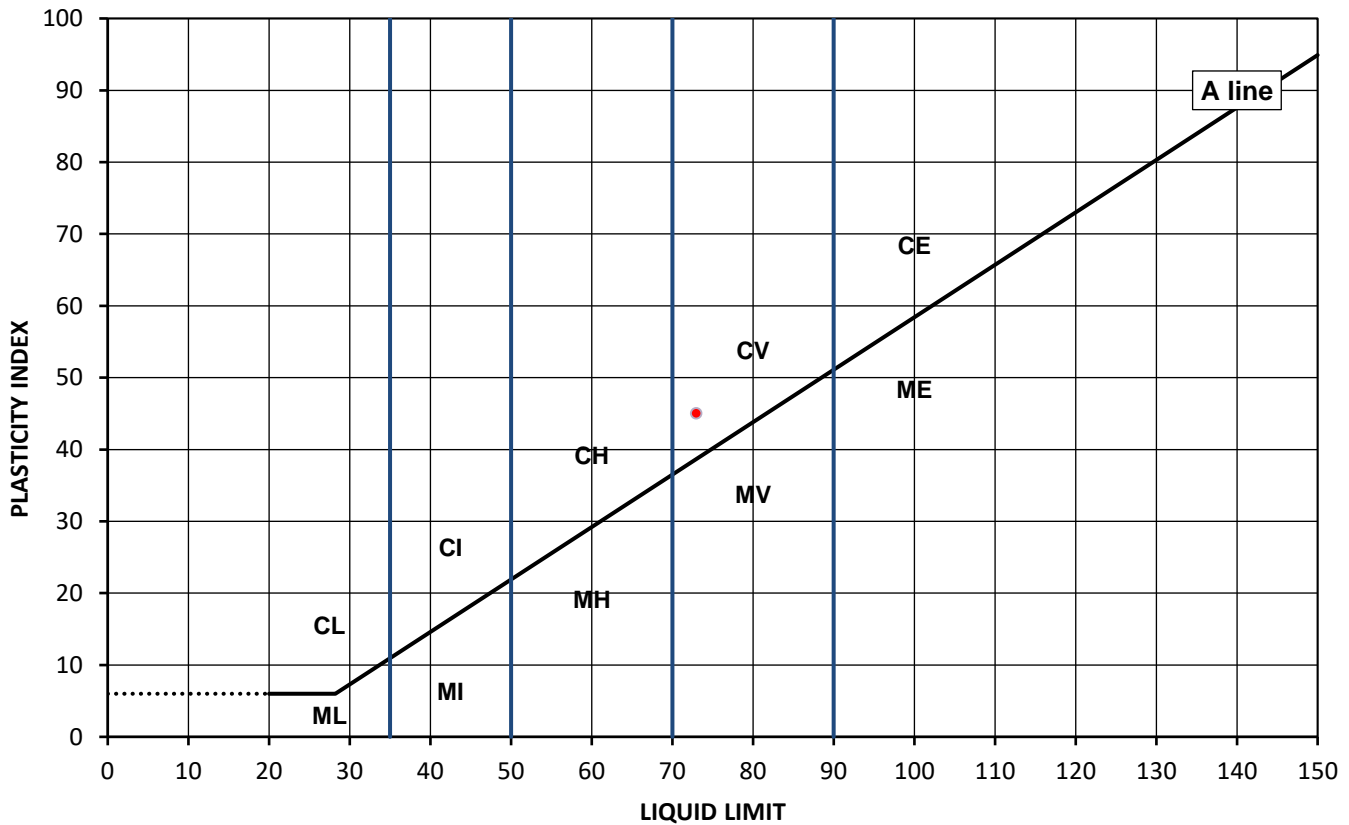
Test Results:

Laboratory Reference: 1339839
Hole No.: BH01-DO2
Sample Reference: Not Given
Soil Description: Brown mottled grey CLAY

Depth Top [m]: 1.80
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested in natural condition

As Received Moisture Content [%]	Liquid Limit [%]	Plastic Limit [%]	Plasticity Index [%]	% Passing 425µm BS Test Sieve
31	73	28	45	100



Legend, based on BS 5930:2015 Code of practice for site investigations

C	Clay	L	Low	Liquid Limit	below 35
M	Silt	I	Medium		35 to 50
		H	High		50 to 70
		V	Very high		70 to 90
		E	Extremely high		exceeding 90
	Organic	O	append to classification for organic material (eg CHO)		

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
for and on behalf of i2 Analytical Ltd GF 232.5

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TEST CERTIFICATE

Liquid and Plastic Limits

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

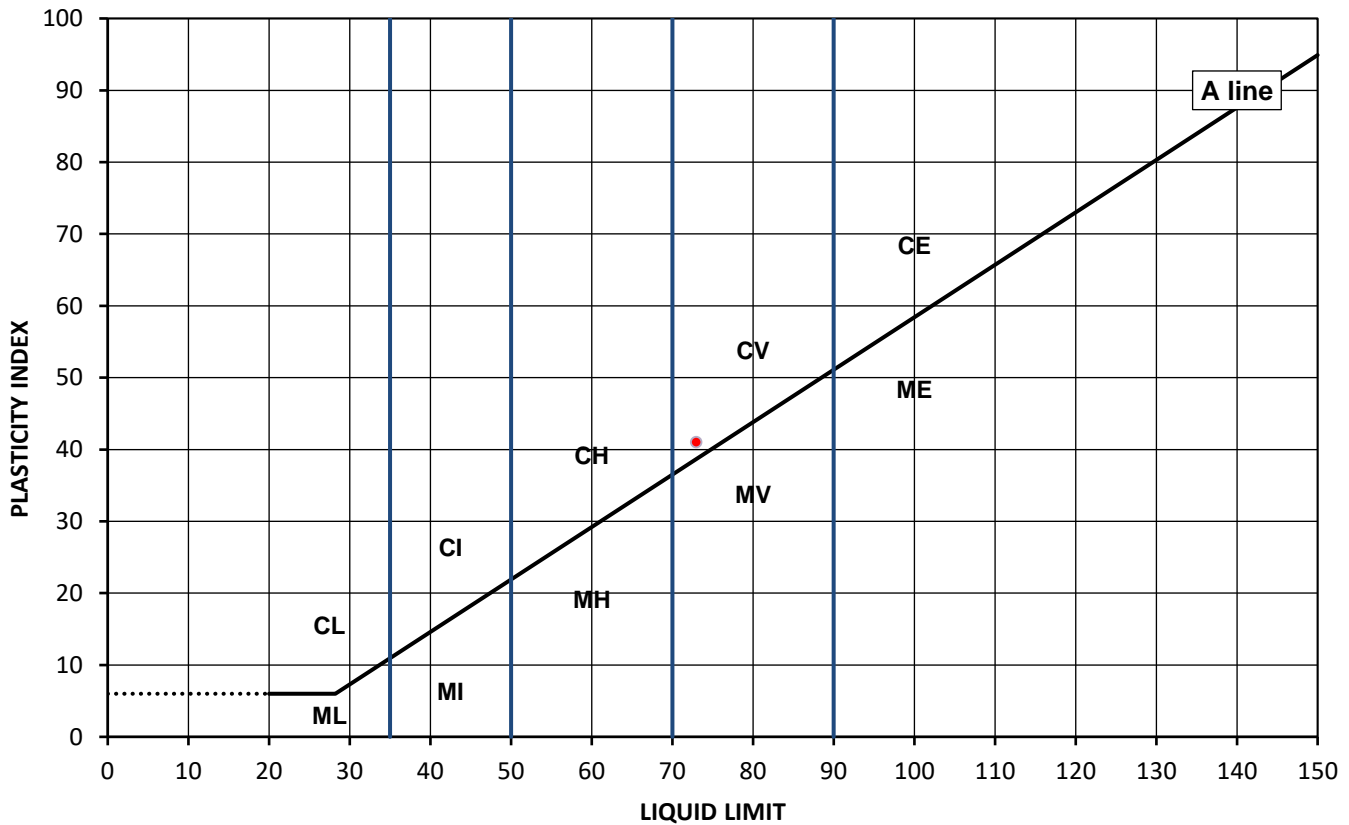
Test Results:

Laboratory Reference: 1339840
Hole No.: BH01-DO3
Sample Reference: Not Given
Soil Description: Brown CLAY

Depth Top [m]: 3.00
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested in natural condition

As Received Moisture Content [%]	Liquid Limit [%]	Plastic Limit [%]	Plasticity Index [%]	% Passing 425µm BS Test Sieve
28	73	32	41	100



Legend, based on BS 5930:2015 Code of practice for site investigations

C	Clay	L	Low	Liquid Limit	below 35
M	Silt	I	Medium		35 to 50
		H	High		50 to 70
		V	Very high		70 to 90
		E	Extremely high		exceeding 90
	Organic	O	append to classification for organic material (eg CHO)		

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
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TEST CERTIFICATE

Liquid and Plastic Limits

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

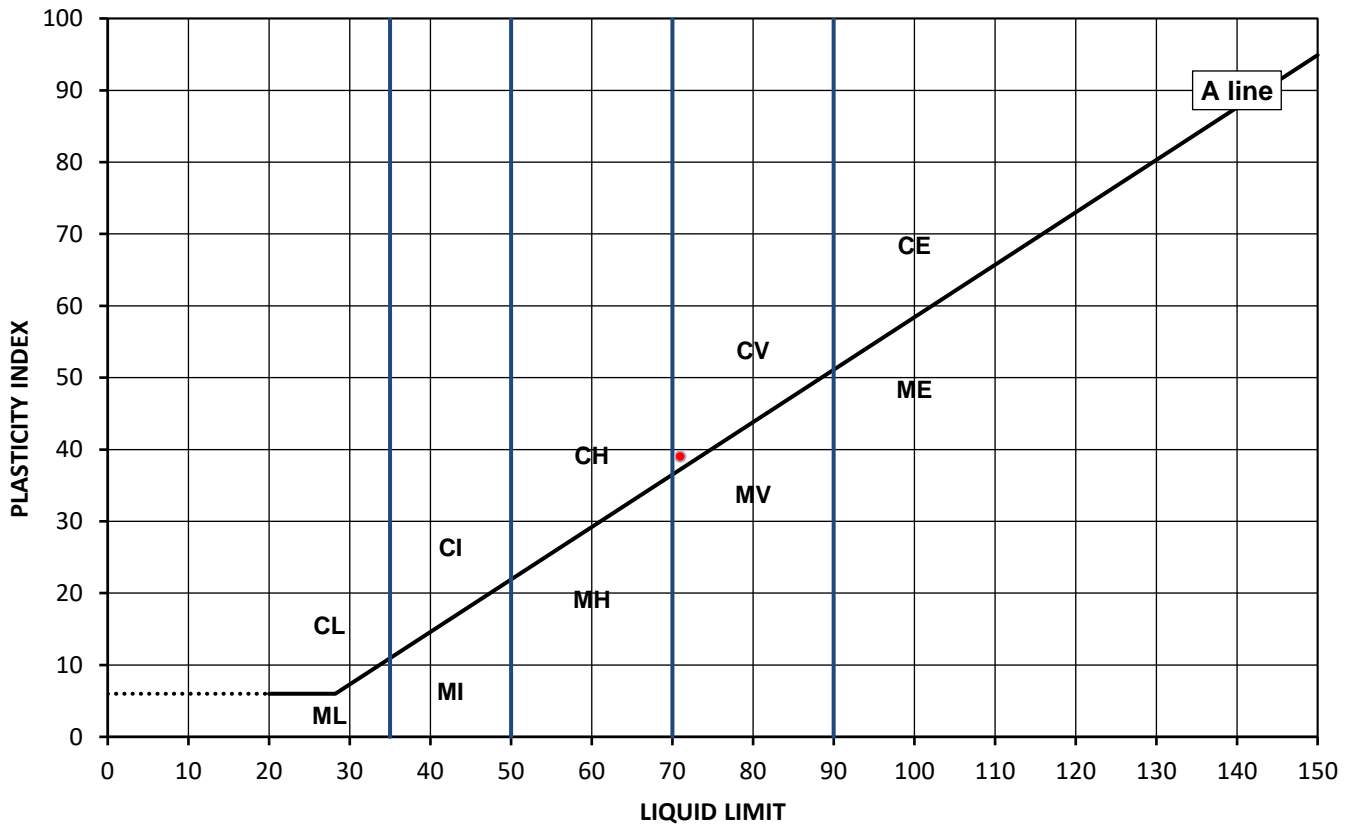
Test Results:

Laboratory Reference: 1339841
Hole No.: BH01-DO4
Sample Reference: Not Given
Soil Description: Brown CLAY

Depth Top [m]: 3.90
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested in natural condition

As Received Moisture Content [%]	Liquid Limit [%]	Plastic Limit [%]	Plasticity Index [%]	% Passing 425µm BS Test Sieve
27	71	32	39	100



Legend, based on BS 5930:2015 Code of practice for site investigations

C	Clay	L	Low	Liquid Limit	below 35
M	Silt	I	Medium		35 to 50
		H	High		50 to 70
		V	Very high		70 to 90
		E	Extremely high		exceeding 90
	Organic	O	append to classification for organic material (eg CHO)		

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
for and on behalf of i2 Analytical Ltd GF 232.5

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TEST CERTIFICATE

Liquid and Plastic Limits

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

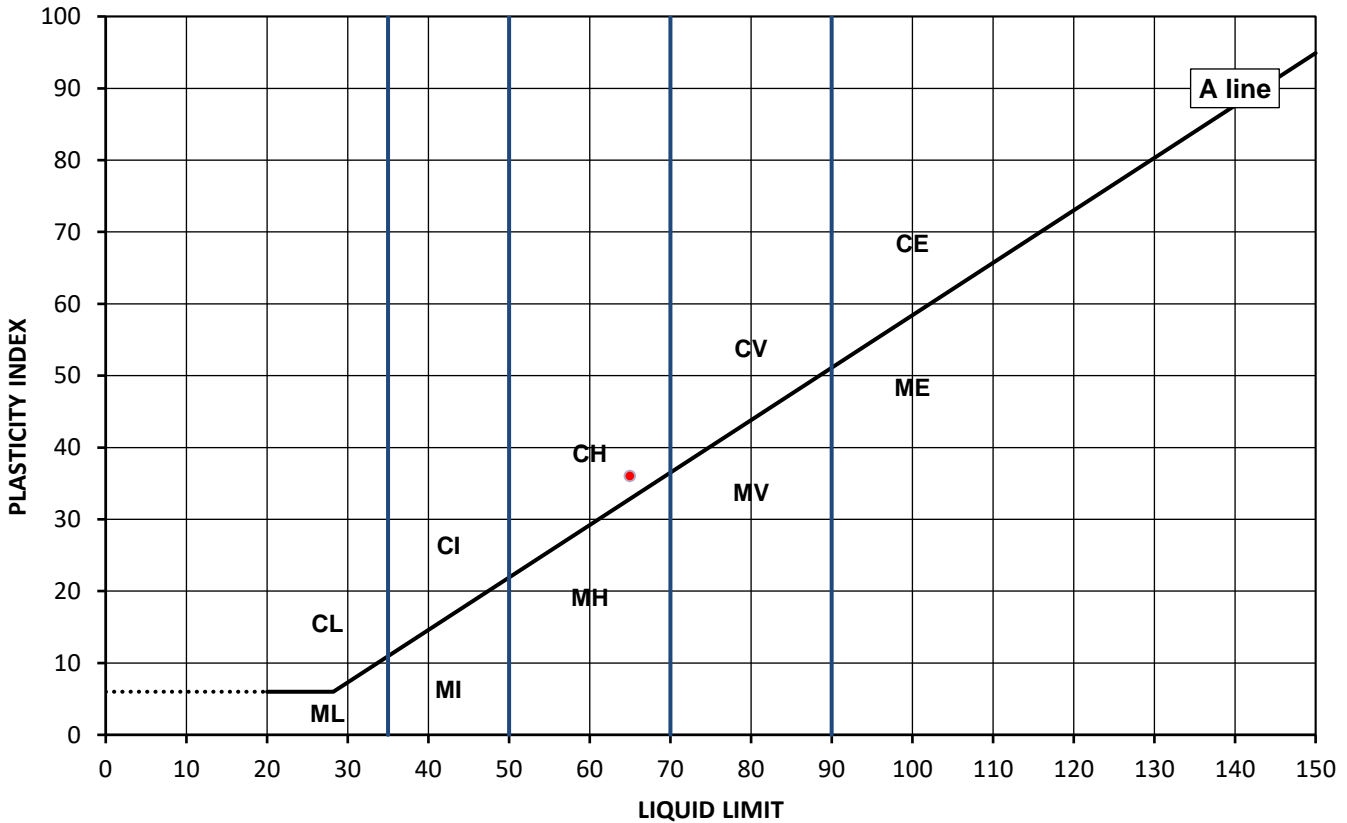
Test Results:

Laboratory Reference: 1339842
Hole No.: BH01-DO5
Sample Reference: Not Given
Soil Description: Brown CLAY

Depth Top [m]: 5.75
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested in natural condition

As Received Moisture Content [%]	Liquid Limit [%]	Plastic Limit [%]	Plasticity Index [%]	% Passing 425µm BS Test Sieve
29	65	29	36	100



Legend, based on BS 5930:2015 Code of practice for site investigations

C	Clay	L	Low	Liquid Limit	below 35
M	Silt	I	Medium		35 to 50
		H	High		50 to 70
		V	Very high		70 to 90
		E	Extremely high		exceeding 90
	Organic	O	append to classification for organic material (eg CHO)		

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
for and on behalf of i2 Analytical Ltd GF 232.5

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SUMMARY REPORT

Summary of Classification Test Results

i2 Analytical Ltd
7 Woodshots Meadow
Croxley Green Business Park
Watford Herts WD18 8YS



Tested in Accordance with:

Client: Maund GeoConsulting Ltd
Client Address: 20 Mortlake Avenue, Worcester,
WR5 1QT
Contact: Julian Maund
Site Name: 15 Lyndcroft Gardens, London NW6 1LB
Site Address: Not Given

MC by BS 1377-2: 1990: Clause 3.2; WC by BS EN 17892-1: 2014; Atterberg
by BS 1377-2: 1990: Clause 4.3, Clause 4.4 and 5; PD by BS 1377-2: 1990:
Clause 8.2

Client Reference: 19-67613
Job Number: 19-67613
Date Sampled: 18/10/2019
Date Received: 21/10/2019
Date Tested: 28/10/2019
Sampled By: Not Given

Test results

Laboratory Reference	Hole No.	Sample				Description	Remarks	MC %	WC %	Atterberg				Density			Total Porosity# %
		Reference	Depth Top m	Depth Base m	Type					% Passing 425um	LL %	PL %	PI %	bulk Mg/m3	dry Mg/m3	PD Mg/m3	
1339838	BH01-DO1	Not Given	0.40	Not Given	D	Brown slightly gravelly CLAY	Atterberg 1 Point	34		97	60	30	30				
1339839	BH01-DO2	Not Given	1.80	Not Given	D	Brown mottled grey CLAY	Atterberg 1 Point	31		100	73	28	45				
1339840	BH01-DO3	Not Given	3.00	Not Given	D	Brown CLAY	Atterberg 1 Point	28		100	73	32	41				
1339841	BH01-DO4	Not Given	3.90	Not Given	D	Brown CLAY	Atterberg 1 Point	27		100	71	32	39				
1339842	BH01-DO5	Not Given	5.75	Not Given	D	Brown CLAY	Atterberg 1 Point	29		100	65	29	36				

Note: # Non accredited; NP - Non plastic

Comments:

Approved: Dariusz Piotrowski
PL Geotechnical Laboratory Manager
Date Reported: 04/11/2019

Signed: Darren Berrill
Geotechnical General Manager
for and on behalf of i2 Analytical Ltd GF 234.7

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Julian Maund
Maund GeoConsulting Ltd
20 Mortlake Avenue
Worcester
WR5 1QT

e: julian.maund@gmail.com

i2 Analytical Ltd.
7 Woodshots Meadow,
Croxley Green
Business Park,
Watford,
Herts,
WD18 8YS

t: 01923 225404
f: 01923 237404
e: reception@i2analytical.com

Analytical Report Number : 19-67616

Project / Site name:	15 Lyndcroft Gardens, London NW6 1LB	Samples received on:	21/10/2019
Your job number:		Samples instructed on:	21/10/2019
Your order number:		Analysis completed by:	04/11/2019
Report Issue Number:	1	Report issued on:	04/11/2019
Samples Analysed:	2 soil samples		

Signed: 

Zina Abdul Razzak
Senior Quality Specialist

For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils	- 4 weeks from reporting
leachates	- 2 weeks from reporting
waters	- 2 weeks from reporting
asbestos	- 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies. An estimate of measurement uncertainty can be provided on request.



Analytical Report Number: 19-67616

Project / Site name: 15 Lyndcroft Gardens, London NW6 1LB

Lab Sample Number				1339846	1339847			
Sample Reference				BH01-DO1	BH01-DO2			
Sample Number				None Supplied	None Supplied			
Depth (m)				0.40	1.80			
Date Sampled				18/10/2019	18/10/2019			
Time Taken				None Supplied	None Supplied			
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1			
Moisture Content	%	N/A	NONE	23	20			
Total mass of sample received	kg	0.001	NONE	0.50	0.24			

General Inorganics

pH - Automated	pH Units	N/A	MCERTS	7.8	8.0			
Water Soluble SO ₄ 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.044	0.096			



Analytical Report Number : 19-67616

Project / Site name: 15 Lyndcroft Gardens, London NW6 1LB

* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
1339846	BH01-DO1	None Supplied	0.40	Light brown clay with brick.
1339847	BH01-DO2	None Supplied	1.80	Brown clay.



Analytical Report Number : 19-67616

Project / Site name: 15 Lyndcroft Gardens, London NW6 1LB

Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW) Process Water (PrW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In-house method based on BS1377 Part 2, 1990, Classification tests	L019-UK/PL	W	NONE
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In-house method based on BS1377 Part 3, 1990, Chemical and Electrochemical Tests	L099-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In-house method based on BS1377 Part 3, 1990, Chemical and Electrochemical Tests, 2:1 water:soil extraction, analysis by ICP-OES.	L038-PL	D	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

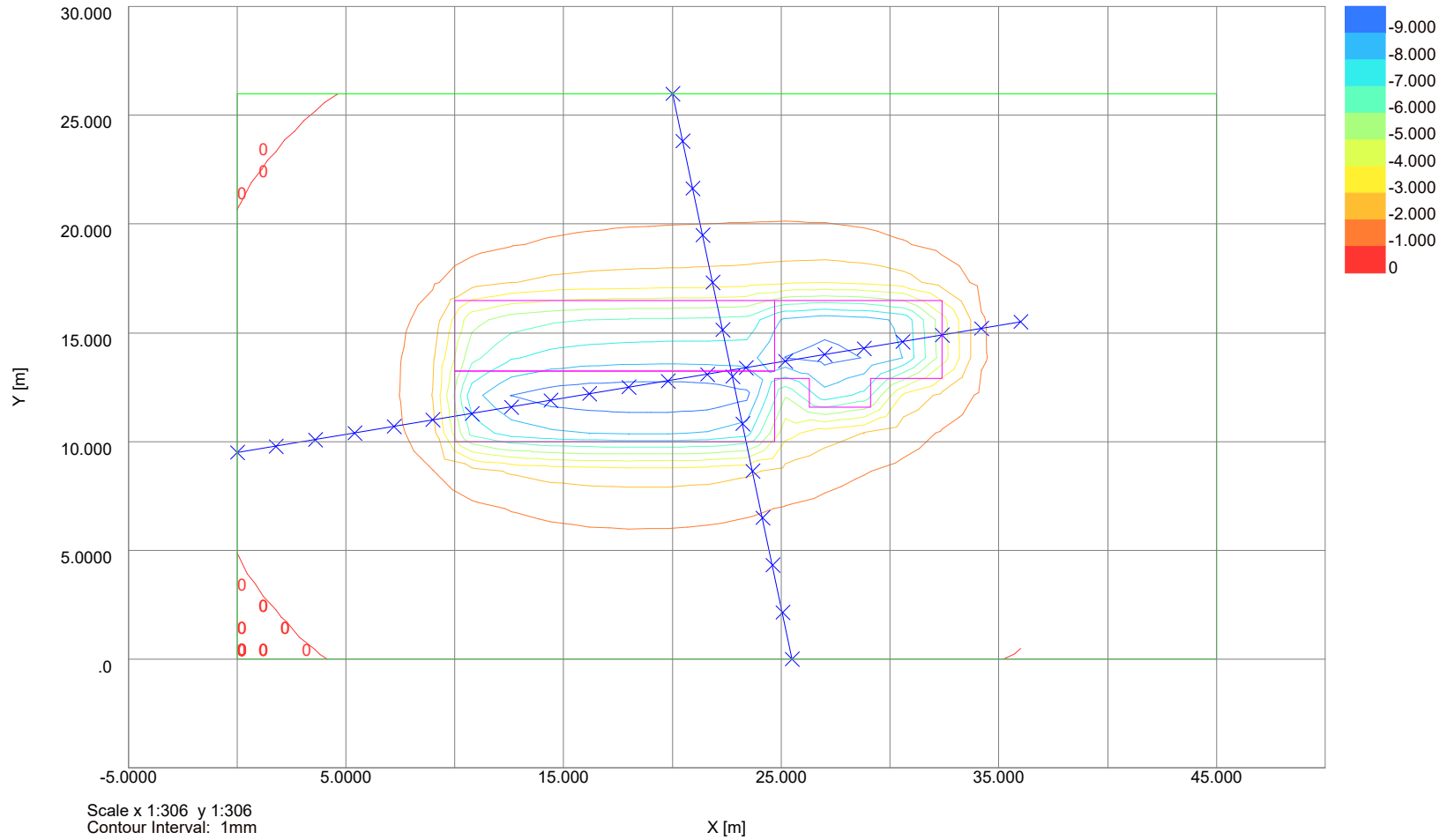
For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Appendix E PDisp Output

Job No.	Sheet No.	Rev.
MGC 29		
Drg. Ref.		
Made by JGM	Date 22 Nov 2019	Checked

Settlement Contours : Grid 1 at 70.0000m





Job No.	Sheet No.	Rev.
MGC 29		
Drg. Ref.		
Made by JGM	Date 22 Nov 2019	Checked

Titles

Job No.: MGC 29
 Job Title: 15 Lyncroft Gardens
 Sub-title: Excavation
 Calculation Heading:
 Initials: JGM
 Checker: jm
 Date Saved: 22 Nov 2019
 Date Checked: 23 Nov 2019
 Notes:
 File Name: Lyncroft excavation.pdd
 File Path: F:\OneDrive\Documents\Croft Structural Engineers\15 Lyncroft Gardens NW6 1LR\07-GIR Lyncroft\PDISP

History

Date	Time	By	Notes
19-Jul-2019	13:04	Maund Geo Consulting	New
19-Jul-2019	14:55	Maund Geo Consulting	
19-Jul-2019	15:12	Maund Geo Consulting	
19-Jul-2019	15:37	Maund Geo Consulting	
25-Jul-2019	13:07	Maund Geo Consulting	
25-Jul-2019	13:53	Maund Geo Consulting	
25-Jul-2019	14:21	Maund Geo Consulting	
02-Aug-2019	11:01	Maund Geo Consulting	
20-Nov-2019	21:02	Maund Geo Consulting	
20-Nov-2019	21:56	Maund Geo Consulting	
21-Nov-2019	20:35	Maund Geo Consulting	
22-Nov-2019	11:50	Maund Geo Consulting	
23-Nov-2019	14:25	Maund Geo Consulting	
23-Nov-2019	16:46	Maund Geo Consulting	Open

Analysis Options

General

Global Poisson's ratio: 0.20
 Maximum allowable ratio between values of E: 1.5
 Horizontal rigid boundary level: 45.00 [m OD]
 Displacements at load centroids: Yes
 GSA piled raft data : No

Elastic

Elastic : Yes
 Analysis: Boussinesq
 Stiffness for horizontal displacement calculations: Weighted average
 Using legacy heave correction factor: No

Consolidation

Consolidation : No

Soil Profiles/Soil Profile 1

Layer ref.	Name	Level at top	Number of intermediate displacement levels	Youngs Modulus : Top	Youngs Modulus : Btm.	Poissons ratio	Non-linear curve
1	london clay	70.000	25	15000.	140000.	0.50000	None

Non-linear Curve Coordinates - Non-linear Curve 1

Point strain Factor [%]

Soil Zones

Zone	Name	X min [m]	X max [m]	Y min [m]	Y max [m]	Profile
1	zone 1	0.0	45.000	0.0	26.000	Soil Profile 1

Polygonal Load Data

Load ref.	Name	Position : Level	Position : Polygon : Coords.	Position : Rect. : tolerance [%]	No. of Polygons	Value : Normal (local z) [kN/m²]
1	excavation full	70.00000	(10,10) (24.7,10) (24.7,12.9) (26.3,12.9) (26.3,11.6) (29.1,11.6) (29.1,12.9) (32.4,12.9) (32.4,16.5) (24.7,16.5) (24.7,13.3) (10,13.3) (10,10)	10.000	4	-60.000
2	excavation below cellar	70.00000	(10,13.3) (24.7,13.3) (24.7,16.5) (10,16.5) (10,13.3)	10.000	1	-34.000

Polygonal Loads' Rectangles

No.	Centre x	Centre y	Angle of local x from global X [Degrees]	Width x [m]	Depth y [m]
Load 1 : excavation full (Edge 1 optimal)					
1	17.35000	11.62500	0.0	14.700	3.2500
2	25.50000	14.70000	0.0	1.6000	3.6000
3	27.70000	14.05000	0.0	2.8000	4.9000
4	30.75000	14.70000	0.0	3.3000	3.6000
Load 2 : excavation below cellar (Edge 1 optimal)					
1	17.35000	14.87500	0.0	14.700	3.2500

Displacement Lines

Name	X1 [m]	Y1 [m]	Z1 [m]	X2 [m]	Y2 [m]	Z2 [m]	Intervals [No.]	Calculate	Detailed Results
Long section	0.00000	9.50000	70.00000	36.00000	15.50000	70.00000	20	Yes	Yes
Cross Section	25.50000	0.00000	70.00000	20.00000	26.00000	70.00000	12	Yes	Yes

Displacement Grids

Name	Extrusion:	X1	Y1	Z1	X2	Y2	Z2	Intervals	Extrusion:	Extrusion:	Calculate	Detailed
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**MAUND
GEO-CONSULTING LTD**

**15 Lyncroft Gardens
Excavation**

Job No. Sheet No. Rev.

MGC 29

Drq. Ref.

Made by
JGM

Date
22 Nov 2019

Checked

Ref.	Name	x	y	z	δz	Stress: Calc. Level	Stress: Vertical	Stress: Sum Princ.	Vert. Strain
		[m]	[m]	[mOD]	[mm]	[mOD]	[kN/m²]	[kN/m²]	[µ]
1	Grid	7.20000	24.26667	70.00000	-0.06667	69.710	-109.26E-6	-0.18299	5.5663E-6
1	Grid	9.00000	24.26667	70.00000	-0.10263	69.710	-154.41E-6	-0.22512	6.8462E-6
1	Grid	10.80000	24.26667	70.00000	-0.14045	69.710	-204.04E-6	-0.26910	8.1818E-6
1	Grid	12.60000	24.26667	70.00000	-0.17658	69.710	-249.92E-6	-0.31045	9.4378E-6
1	Grid	14.40000	24.26667	70.00000	-0.20804	69.710	-286.26E-6	-0.34562	10.506E-6
1	Grid	16.20000	24.26667	70.00000	-0.23321	69.710	-312.30E-6	-0.37306	11.340E-6
1	Grid	18.00000	24.26667	70.00000	-0.25172	69.710	-330.53E-6	-0.39293	11.944E-6
1	Grid	19.80000	24.26667	70.00000	-0.26392	69.710	-344.31E-6	-0.40616	12.346E-6
1	Grid	21.60000	24.26667	70.00000	-0.27011	69.710	-356.39E-6	-0.41353	12.569E-6
1	Grid	23.40000	24.26667	70.00000	-0.27006	69.710	-367.23E-6	-0.41489	12.610E-6
1	Grid	25.20000	24.26667	70.00000	-0.26263	69.710	-373.13E-6	-0.40860	12.417E-6
1	Grid	27.00000	24.26667	70.00000	-0.24621	69.710	-366.65E-6	-0.39195	11.910E-6
1	Grid	28.80000	24.26667	70.00000	-0.21979	69.710	-340.67E-6	-0.36281	11.025E-6
1	Grid	30.60000	24.26667	70.00000	-0.18421	69.710	-293.09E-6	-0.32154	9.7717E-6
1	Grid	32.40000	24.26667	70.00000	-0.14266	69.710	-230.05E-6	-0.27199	8.2675E-6
1	Grid	34.20000	24.26667	70.00000	-0.10006	69.710	-164.83E-6	-0.22067	6.7096E-6
1	Grid	36.00000	24.26667	70.00000	-0.06119	69.710	-110.07E-6	-0.17387	5.2884E-6
1	Grid	0.00000	26.00000	70.00000	0.03059	69.710	-16.101E-6	-0.062302	1.8971E-6
1	Grid	1.80000	26.00000	70.00000	0.02160	69.710	-22.840E-6	-0.076144	2.3183E-6
1	Grid	3.60000	26.00000	70.00000	0.00918	69.710	-32.403E-6	-0.093160	2.8360E-6
1	Grid	5.40000	26.00000	70.00000	-0.00688	69.710	-45.539E-6	-0.11360	3.4575E-6
1	Grid	7.20000	26.00000	70.00000	-0.02633	69.710	-62.585E-6	-0.13724	4.1764E-6
1	Grid	9.00000	26.00000	70.00000	-0.04829	69.710	-82.892E-6	-0.16315	4.9642E-6
1	Grid	10.80000	26.00000	70.00000	-0.07123	69.710	-104.55E-6	-0.18966	5.7699E-6
1	Grid	12.60000	26.00000	70.00000	-0.09333	69.710	-125.00E-6	-0.21471	6.5316E-6
1	Grid	14.40000	26.00000	70.00000	-0.11299	69.710	-142.24E-6	-0.23657	7.1961E-6
1	Grid	16.20000	26.00000	70.00000	-0.12914	69.710	-155.58E-6	-0.25418	7.7315E-6
1	Grid	18.00000	26.00000	70.00000	-0.14126	69.710	-165.41E-6	-0.26721	8.1276E-6
1	Grid	19.80000	26.00000	70.00000	-0.14925	69.710	-172.57E-6	-0.27571	8.3860E-6
1	Grid	21.60000	26.00000	70.00000	-0.15304	69.710	-177.69E-6	-0.27977	8.5093E-6
1	Grid	23.40000	26.00000	70.00000	-0.15239	69.710	-180.63E-6	-0.27916	8.4906E-6
1	Grid	25.20000	26.00000	70.00000	-0.14679	69.710	-180.11E-6	-0.27324	8.3100E-6
1	Grid	27.00000	26.00000	70.00000	-0.13572	69.710	-174.05E-6	-0.26114	7.9420E-6
1	Grid	28.80000	26.00000	70.00000	-0.11906	69.710	-160.68E-6	-0.24246	7.3738E-6
1	Grid	30.60000	26.00000	70.00000	-0.09753	69.710	-139.91E-6	-0.21779	6.6240E-6
1	Grid	32.40000	26.00000	70.00000	-0.07291	69.710	-114.02E-6	-0.18908	5.7515E-6
1	Grid	34.20000	26.00000	70.00000	-0.04764	69.710	-87.078E-6	-0.15918	4.8428E-6
1	Grid	36.00000	26.00000	70.00000	-0.02412	69.710	-63.026E-6	-0.13089	3.9828E-6

Results : Consolidation : Displacement Data : Grids

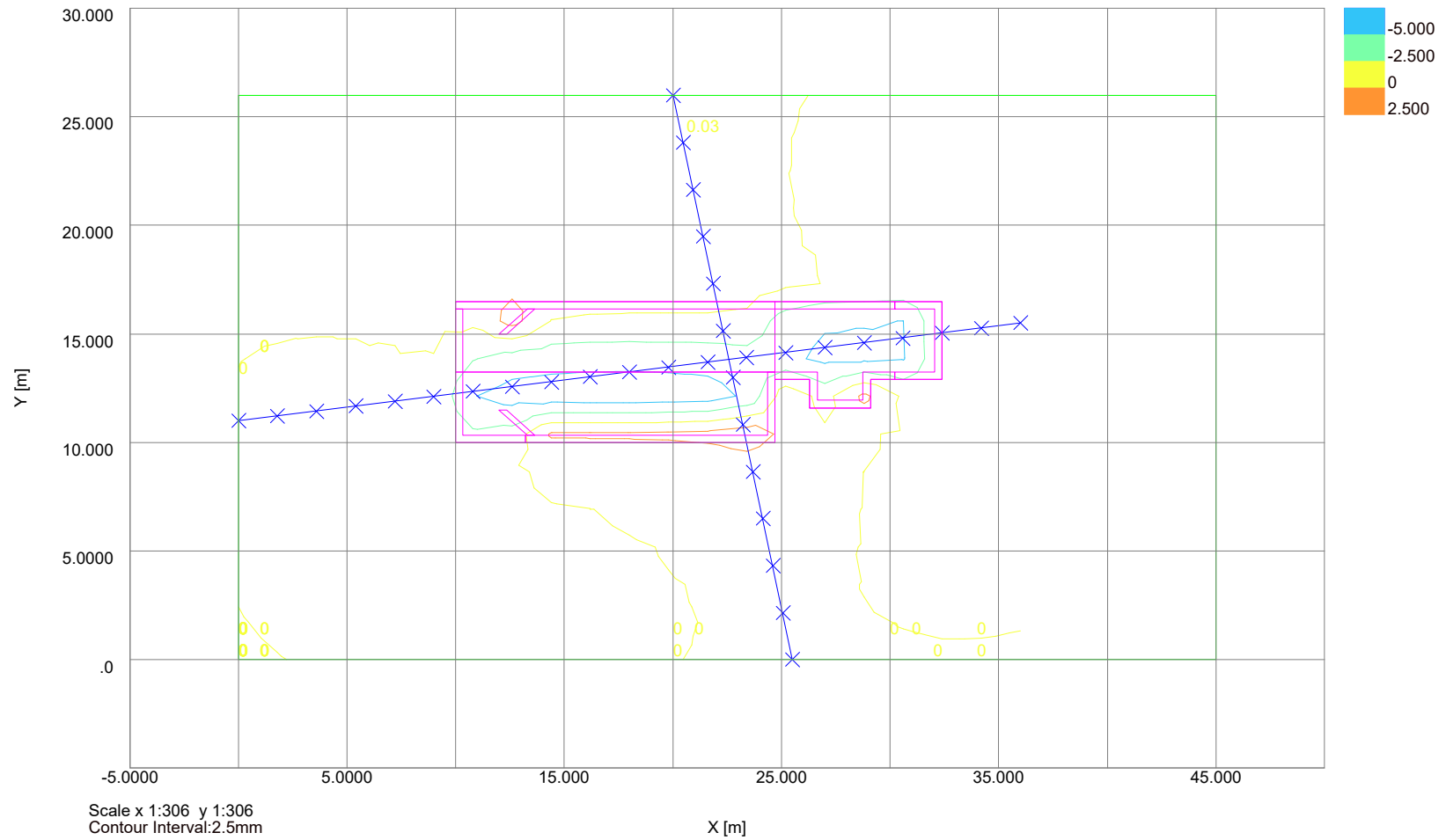
None

Results : Total : Displacement Data : Grids

None

Job No.	Sheet No.	Rev.
MGC 29		
Drg. Ref.		
Made by JGM	Date 21 Nov 2019	Checked

Settlement Contours : Grid 1 at 70.0000m





15 Lyncroft Gardens
Excavation and wall loads combined long term

Titles

Job No.: MGC 29
Job Title: 15 Lyncroft Gardens
Sub-title: Excavation and wall loads combined long term
Calculation Heading:
Initials: JGM
Checker: jm
Date Saved: 21 Nov 2019
Date Checked: 23 Nov 2019
Notes:
File Name: Lyncroft excavation and wall loads.pdd
File Path: F:\OneDrive\Documents\Croft Structural Engineers\15 Lyncroft Gardens NW6 ILR\07-CIR Lyncroft\PDISP

History

Date	Time	By	Notes
19-Jul-2019	13:04	Maund Geo Consulting	New
19-Jul-2019	14:55	Maund Geo Consulting	
19-Jul-2019	15:12	Maund Geo Consulting	
19-Jul-2019	15:37	Maund Geo Consulting	
25-Jul-2019	13:07	Maund Geo Consulting	
25-Jul-2019	13:53	Maund Geo Consulting	
25-Jul-2019	14:21	Maund Geo Consulting	
02-Aug-2019	11:01	Maund Geo Consulting	
20-Nov-2019	21:02	Maund Geo Consulting	
20-Nov-2019	21:56	Maund Geo Consulting	
21-Nov-2019	09:46	Maund Geo Consulting	
21-Nov-2019	20:37	Maund Geo Consulting	
22-Nov-2019	11:47	Maund Geo Consulting	
22-Nov-2019	12:10	Maund Geo Consulting	
23-Nov-2019	14:21	Maund Geo Consulting	
23-Nov-2019	16:51	Maund Geo Consulting	

Analysis Options

General

Global Poisson's ratio: 0.20
Maximum allowable ratio between values of E: 1.5
Horizontal rigid boundary level: 45.00 [m OD]
Displacements at load centroids: Yes
GSA piled raft data : No

Elastic

Elastic : Yes
Analysis: Boussinesq
Stiffness for horizontal displacement calculations: Weighted average
Using legacy heave correction factor: No

Consolidation

Consolidation : No

Soil Profiles Soil Profile 1

Layer ref.	Name	Level at top	Number of intermediate displacement levels	Youngs Modulus : Top	Youngs Modulus : Btm.	Poissons ratio	Non-linear curve
1	london clay	70.0000	25	11250. [kN/m ²]	140000. [kN/m ²]	0.50000	None

Non-linear Curve Coordinates - Non-linear Curve 1

Point	Strain [%]	Factor

Soil Zones

Zone	Name	X min [m]	X max [m]	Y min [m]	Y max [m]	Profile
1	zone 1	0.0	45.0000	0.0	26.0000	Soil Profile 1

Polygonal Load Data

Load ref.	Name	Position : Level	Position : Polygon	Coords. : Rect. tolerance	No. of Polygons	Value : Normal (local z)
1	excavation full	70.00000	(10,10) (24.7,10) (24.7,12.9) (26.3,12.9) (26.3,11.6) (29.1,11.6) (29.1,12.9) (32.4,12.9) (32.4,16.5) (24.7,16.5) (24.7,13.3) (10,13.3) (10,10)	10.0000	4	-50.000
2	wall 2 60 kN/m	70.00000	(30.2,16.1) (30.2,16.5) (24.7,16.5) (24.7,16.1) (30.2,16.1)	10.0000	1	176.00
3	wall 3 30 kN/m	70.00000	(30.2,16.1) (30.2,16.5) (32.4,16.5) (32.4,12.9) (30.2,12.9) (30.2,13.3) (32,13.3) (32,16.1) (30.2,16.1)	10.0000	3	86.000
4	wall 4 60 kN/m	70.00000	(30.2,12.9) (29.1,12.9) (29.1,11.6) (26.3,11.6) (26.3,12.9) (24.7,12.9) (24.7,13.3) (26.6,13.3) (26.6,11.9) (28.8,11.9) (28.8,13.3) (30.2,13.3) (30.2,12.9)	10.0000	5	176.00
5	bay A 60 kN/m	70.00000	(13.3,10.3) (13.3,11.5) (12,11.5) (13.3,10.3)	10.0000	11	176.00
6	bay b 60 kN/m	70.00000	(13.3,16.1) (13.7,16.1) (12.3,15) (12,15) (13.3,16.1)	10.0000	11	176.00
7	wall 6 30kN/m	70.00000	(10,16.1) (10,3,16.1) (10,3,10.3) (13.2,10) (10,10) (10,16.1)	10.0000	2	86.000
8	wall 5 100 kN/m	70.00000	(13.2,10) (24.7,10) (24.7,13.3) (24.4,13.3) (24.4,10.3) (13.2,10.3) (13.2,10)	10.0000	2	296.00
9	wall 1 100kN/m	70.00000	(10,16.5) (24.7,16.5) (24.7,16.1) (10,16.1) (10,16.5)	10.0000	1	296.00
10	excavation cellar	70.00000	(10,13.3) (24.7,13.3) (24.7,16.5) (10,16.5) (10,13.3)	10.0000	1	-24.000



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15 Lyncroft Gardens
Excavation and wall loads combined long term

Job No.	Sheet No.	Rev.
MGC 29		
Drg. Ref.		
Made by JGM	Date 21 Nov 2019	Checked

Load ref. Name Position : Level Position : Polygon : Coords. Position : Polygon Rectangles : Rect. : Rect. tolerance No. of Rectangles Value : Normal (Local z)

Polygonal Loads' Rectangles

No. Centre : Centre : Angle of Width x Depth y
x y local x
from
global X
[Degrees] [m] [m]

Load 1 : excavation full
(Edge 1 optimal)
1 17.35000 11.62500 0.0 14.700 3.2500
2 25.50000 14.70000 0.0 1.6000 3.6000
3 27.70000 14.05000 0.0 2.8000 4.9000
4 30.75000 14.70000 0.0 3.3000 3.6000

Load 2 : wall 2 60 kN/m
(Edge 1 optimal)
1 27.45000 16.32500 90.000 0.35000 5.5000
Load 3 : wall 3 30 kN/m
(Edge 2 optimal)
1 32.22500 14.70000 -90.000 2.9000 0.35000
2 31.30000 13.07500 -90.000 0.35000 2.2000
3 31.30000 16.32500 -90.000 0.35000 2.2000

Load 4 : wall 4 60 kN/m
(Edge 2 optimal)
1 29.65000 13.07500 -180.00 1.1000 0.35000
2 28.92500 12.42500 -180.00 0.35000 1.6500
3 27.70000 11.77500 -180.00 2.1000 0.35000
4 26.47500 12.42500 -180.00 0.35000 1.6500
5 25.50000 13.07500 -180.00 1.6000 0.35000

Load 5 : bay A 60 kN/m
(Edge 2 optimal)
1 13.62268 10.35868 138.50 0.052430 0.023190
2 13.56805 10.37605 138.50 0.052430 0.069570
3 13.51341 10.39342 138.50 0.052430 0.11595
4 13.45878 10.41079 138.50 0.052430 0.16233
5 13.40414 10.42816 138.50 0.052430 0.20871
6 12.82500 10.92500 138.50 1.4735 0.23190
7 12.24586 11.42184 138.50 0.052430 0.20871
8 12.19122 11.43921 138.50 0.052430 0.16233
9 12.13659 11.45658 138.50 0.052430 0.11595
10 12.08195 11.47395 138.50 0.052430 0.069570
11 12.02732 11.49132 138.50 0.052430 0.023190

Load 6 : bay b 60 kN/m
(Edge 1 optimal)
1 12.02732 15.00868 41.496 0.052430 0.023190
2 12.08195 15.02605 41.496 0.052430 0.069570
3 12.13659 15.04342 41.496 0.052430 0.11595
4 12.19122 15.06079 41.496 0.052430 0.16233
5 12.24586 15.07816 41.496 0.052430 0.20871
6 12.82500 15.57500 41.496 1.4735 0.23190
7 13.40414 16.07184 41.496 0.052430 0.20871
8 13.45878 16.08921 41.496 0.052430 0.16233
9 13.51341 16.10658 41.496 0.052430 0.11595
10 13.56805 16.12395 41.496 0.052430 0.069570
11 13.62268 16.14132 41.496 0.052430 0.023190

Load 7 : wall 6 30kN/m
(Edge 2 optimal)
1 10.17500 13.07500 0.0 0.35000 6.1500
2 11.77500 10.17500 0.0 2.8500 0.35000

Load 8 : wall 5 100 kN/m
(Edge 1 optimal)
1 18.77500 10.17500 0.0 11.150 0.35000
2 24.52500 11.62500 0.0 0.35000 3.2500

Load 9 : wall 1 100kN/m
(Edge 2 optimal)
1 17.35000 16.32500 0.0 14.700 0.35000

Load 10 : excavation cellar
(Edge 1 optimal)
1 17.35000 14.87500 0.0 14.700 3.2500

Displacement Lines

Name	X1	Y1	Z1	X2	Y2	Z2	Intervals	Calculate	Detailed
	[m]	[m]	[m]	[m]	[m]	[m]	[No.]	Results	Results
Long section	0.00000	11.00000	70.00000	36.00000	15.50000	70.00000	20	Yes	Yes
Cross Section	25.50000	0.00000	70.00000	20.00000	26.00000	70.00000	12	Yes	Yes

Displacement Grids

Name	Extrusion: Direction	X1	Y1	Z1	X2	Y2	Z2	Intervals Along Line [No.]	Extrusion: Distance	Extrusion: Intervals Along [No.]	Calculate	Detailed
		[m]	[m]	[m]	[m]	[m]	[m]		[m]		Results	Results
Grid	Global X	0.00000	0.00000	70.00000	-	26.00000	70.00000	15	36.00000	20	Yes	Yes

Results : Immediate : Load Centres : Polygonal

Ref.	Name	x	y	z	δz	Stress: Calc. Level	Stress: Vertical	Stress: Sum Princ.	Vert. Strain
		[m]	[m]	[mOD]	[mm]	[mOD]	[kN/m ²]	[kN/m ²]	[μ]
1	excavation full	21.74929	12.73088	70.00000	-6.18765	69.730	-49.353	-99.081	-0.0019449
2	wall 2 60 kN/m	27.45000	16.32500	70.00000	2.99353	69.730	74.098	77.061	0.0057674
3	wall 3 30 kN/m	31.66747	14.70000	70.00000	-4.60508	69.730	-46.273	-79.713	-0.0023472
4	wall 4 60 kN/m	27.53025	12.47215	70.00000	-3.46897	69.730	-46.210	-68.989	-0.0027656
5	bay A 60 kN/m	12.82500	10.92500	70.00000	0.15873	69.730	37.515	16.976	0.0037952
6	bay b 60 kN/m	12.82500	15.57500	70.00000	4.97538	69.730	65.509	87.161	0.0043431
7	wall 6 30kN/m	10.68167	12.15667	70.00000	-4.55011	69.730	-44.980	-76.647	-0.0023149
8	wall 5 100 kN/m	20.07274	10.50226	70.00000	0.12206	69.730	3.4036	30.190	-793.42E-6
9	wall 1 100kN/m	17.35000	16.32500	70.00000	11.17077	69.730	174.36	221.12	0.011991
10	excavation cellar	17.35000	14.87500	70.00000	-2.61881	69.730	-23.709	-44.621	-0.0010526

Results : Consolidation : Load Centres : Polygonal

None

Results : Total : Load Centres : Polygonal

None

Results : Immediate : Displacement Data : Lines

Ref.	Name	x	y	z	δz	Stress: Calc. Level	Stress: Vertical	Stress: Sum Princ.	Vert. Strain
		[m]	[m]	[mOD]	[mm]	[mOD]	[kN/m ²]	[kN/m ²]	[μ]



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Excavation and wall loads combined long term

Ref.	Name	x	y	z	dz	Stress: Calc. Level	Stress: Vertical	Stress: Sum Princ.	Vert. Strain
		[m]	[m]	[mOD]	[mm]	[mOD]	[kN/m ²]	[kN/m ²]	[μ]

Results : Consolidation : Displacement Data : Grids

None

Results : Total : Displacement Data : Grids

None