

LLFA COMMENT / QUERY TRACKER - 11.12.2019

INTRODUCTION

This tracker has been compiled to provide a detailed response to comments received by Camden Planning from the Lead Local Flood Authority in relation to the proposed development.

EXECUTIVE SUMMARY

The comments were discussed and informally resolved at a meeting between Coyle Kennedy, LBH and the LLFA on 26th November 2019. This tracker introduces the additional information being presented, which is in the form of a Response to Further information request by Coyle Kennedy issued on 9th December 2019 and an updated SuDS Proforma (to London Sustainable Drainage forum requirements) with associated updated supporting calculations.

The SuDS proposals for this development now deliver Greenfield run-off rates, in accordance with Camden Policy.

LLFA COMMENT / QUERY TRACKER

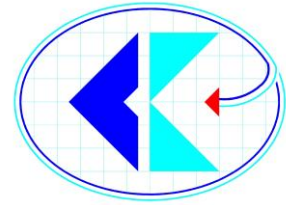
LLFA 2ND COMMENTS	APPLICANT RESPONSE	LLFA 3RD COMMENTS	LBH RESPONSE
<p>Plan shows:</p> <ul style="list-style-type: none"> 50 m³ surface water attenuation 15 m³ swale 3 m³ underground attenuation tank Flow controlled rate from driveway 5 l/s <p>Issue i: the indicated storage adds to 68 m³, a shortfall of 73.5 m³.</p> <p>Issue ii: No technical details indicated for the blue-green roofs (storage volume, area/depth, void %, flow controls, discharge rates etc.)</p> <p>Issue iii: no information on freeboard requirements</p>	<p>Issue i: There is a total storage volume of approximately of 141.5m³, which is made up as follows:</p> <ul style="list-style-type: none"> 15m³ in the swale 50m³ in the attenuation tank 76m³ in green and blue roof storage <p>Issue ii: The green/blue roofs will be designed by a specialist supplier/designer and it is unreasonable to request such details at this time as any specialist supplier/designer is unlikely to carry out a design without first receiving an order and deposit.</p> <p>Issue iii: The proposed house levels for the houses on the east side of the site are in excess of 1m above the level of Millfield Lane. From this it is clear there is more than ample freeboard provided.</p>	<p>Issue i: Noted. The blue-green roof storage information the Drainage Report. At least the Site Drainage Layout should be updated with the figures that make up the 76 m³. Further action requested</p> <p>Issue ii: Adequate information should be submitted at planning stage, based on outline designs generated by the drainage consultant. This is to show the LPA/LLFA how the proposed drainage measures, rates and volumes could be achieved in the scheme. Final detailed design information is not expected at this stage; it is accepted details may change, but the potential to achieve the stated aims should be demonstrated. Further action requested</p> <p>Issue iii: This request related to the SuDS freeboards as per original comment: "Provide the maximum volume each of the SuDS features can accommodate, as well as freeboard requirements in order to verify if the 141.5 m³ of attenuation required can be accommodated and indicate the discharge rate from the impermeable driveway to the combined sewer. Further action requested</p>	<p>Issue i: <i>The roof designs will be undertaken by a specialist designer in due course.</i></p> <p><i>At this stage a conservative assumption might be made as follows:</i></p> <p><i>Assumed Approx. green/blue roof area</i></p> <p><i>Plots 1 to 3 - 80 m² each</i> <i>Plots 4 & 5 - 135 m² each</i> <i>Assumed equivalent depth water retained 150mm</i> <i>(3 x 80) + (2 x 135) = 510 m²</i> <i>510 m² x .15m = 76.5 m³</i></p> <p><i>NB. Coyle Kennedy have provided a further assessment (attached) based upon revised areas that suggests 125m³ of blue/green roof storage to be potentially available.</i></p> <p>Issue ii: <i>Coyle Kennedy have provided additional information (attached) to demonstrate how the drainage measures will be achieved.</i></p> <p>Issue iii: <i>Coyle Kennedy have indicated a surplus storage capacity of some 50m³. Given that there is no potential for freeboard within the filled swale and the attenuation tank this can be accommodated within the roof designs as a freeboard of some 60mm. (see attached CK detail)</i></p>

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<p>The Micro Drainage tables show storm sewer design results and foul sewerage design.</p> <p>Issue iv: we should expect, for each return period:</p> <ul style="list-style-type: none"> - summary of results - rainfall details / time area diagram - model details / storage structure / depth-flow relationship outflow control 	<p>The details of the storage structures are outlined in Coyle Kennedy's drawings to a level of detail which is reasonable for this stage of the planning/design.</p>	<p>We are seeking copies of the MicroDrainage calculations which will have been carried out by the consultant in order to generate the storage proposals. This is your evidence to support the proposed attenuation volumes and discharge rates. I can provide example copies on request from other schemes but the consultant would be aware of the items requested.</p> <p>Further action requested</p>	<p>Issue iv:</p> <p><i>See attached updated LSD Proforma and calcs to demonstrate storage required to achieve</i></p> <ul style="list-style-type: none"> <i>A. Greenfield Rates - 161m³</i> <i>B. 50% Betterment - 143m³</i> <p><i>(it is noted that the previous estimate for 50% betterment was 141.5m³, and also that the 190m³ storage proposed by Coyle Kennedy will accommodate the Camden Policy to limit post development discharge to greenfield rates)</i></p>

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<p>The reference states “...as agreed with Mr. Bob Warnock on site on the 10 May 2018.” .</p> <p>Issue v: We should seek written confirmation from the landowner</p>	<p>[Not addressed in ‘Response to Further information request – 55 Fitzroy Park’]</p>	<p>Further action requested</p>	<p>Issue v: We met on Bob Warnock of the City of London (and his then Hydrology advisor) on 1st May 2018 (not 10th May) and discussed how best to deal with the water that runs across Millfield Lane.</p> <p><i>I think we mistakenly understood at the time that the CoL were indicating a preference that they wished to see it removed and dealt with by means of a pipe installed under the carriageway of the lane.</i></p> <p><i>On 4th October 2018 we re-iterated to the CoL that we had no strong feelings on the matter but wished to accommodate whichever of the following options the CoL felt to be the most appropriate. 1) Leave as is 2) Replace with a pipe or 3) Replace with a more formal surface stone/concrete “ford/channel”.</i></p> <p><i>Unfortunately, the CoL were unable to provide any clarification but have since indicated that they have commissioned an independent hydrological consultant.</i></p> <p><i>On 26th October 2018 the CoL wrote that they would not support a request to discharge water onto the Heath, but again were unable to clarify what they wished to happen with the existing discharge. However, it would seem somewhat unlikely that the historic discharge of an ancient watercourse would require CoL permission to flow. (Not the least because the flow must be seen as an important contribution to the wetland area of the nature reserve, and, ultimately, to the Highgate Pond chain.)</i></p>

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LLFA 2ND COMMENTS	APPLICANT RESPONSE	LLFA 3RD COMMENTS	LBH RESPONSE
<p>The reference states landscaping without providing further details.</p> <p>Issue vi: Final details may be conditioned, but we should ask for additional information to clarify the measures in outline.</p>	<p><i>[Not addressed in 'Response to Further information request – 55 Fitzroy Park']</i></p>	<p>Further action requested</p>	<p>Issue vi:</p> <p><i>The landscaping details will be finalised in due course to provide the flood defence required. The required measures were outlined in the Hydrological & Hydrogeological assessment LBH4480 Ver 2.0 section 7.8 (page 31), where it was indicated that local landscaping would be used to direct any surface water flooding away from Plots 4& 5 and direct this to the designated overland flood route immediately to the north of Plot 5. (see attached additional information in the form of a plan and sketch sections by Coyle Kennedy)</i></p>

Response to Further information request – 55 Fitzroy Park



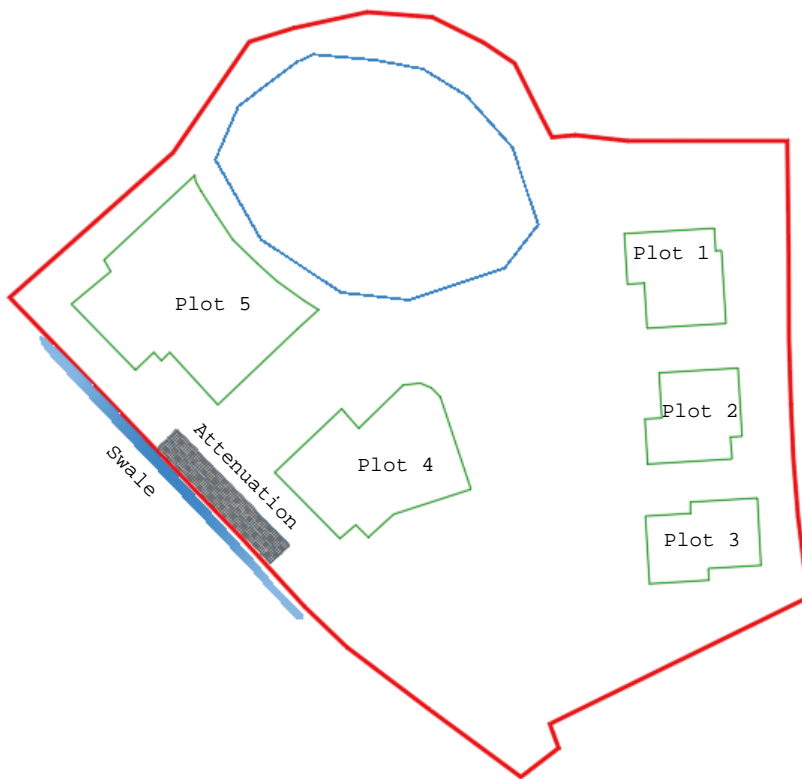
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There is a total storage volume of approximately of 141.5m³, which is made up as follows:



Make up of attenuation volumes:

Swale 46m x 1.0m effective deptg filled with stone (33% voids) = 15m³
attenuation tank – 18 x 3 x 95% voids = 50m³

Blue and Green roof

Plot 1 - 101m² of roof by 150mm deep = 15.2m³
Plot 2 - 98m² of roof by 150mm deep = 14.7m³
Plot 3 - 95m² of roof by 150mm deep = 14.3m³
Plot 4 - 214m² of roof by 150mm deep = 32.1m³
Plot 5 - 329m² of roof by 150mm deep = 49.4m³

Total = 190 m³

In relation to the storage volume there is a freeboard of 50m³.

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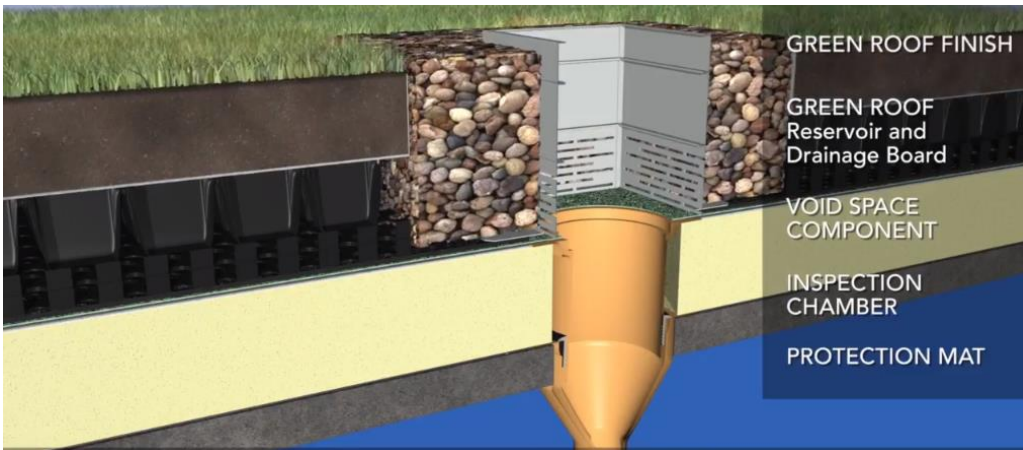
CKCE Limited
Registered in England 11566522

Directors:
Brian Coyle BE CEng MIEI MStructE
Tadhg Kennedy BE MEngSc CEng MIEI MStructE

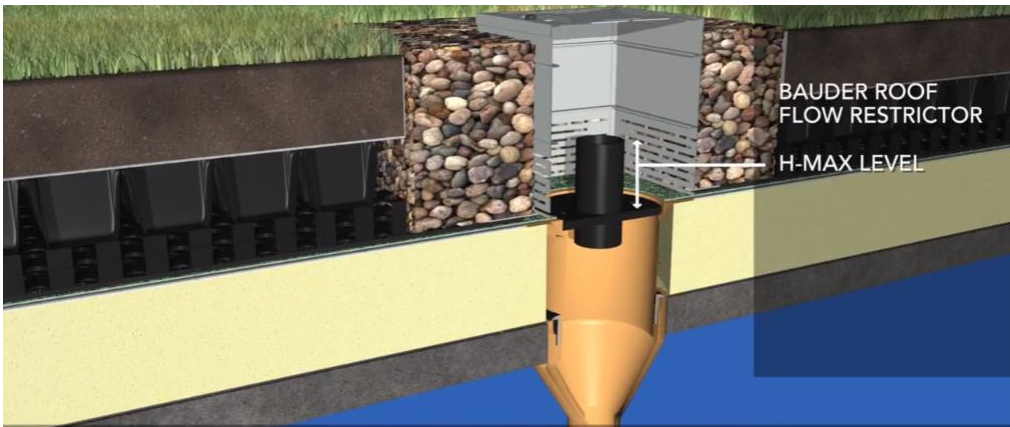


Issue ii

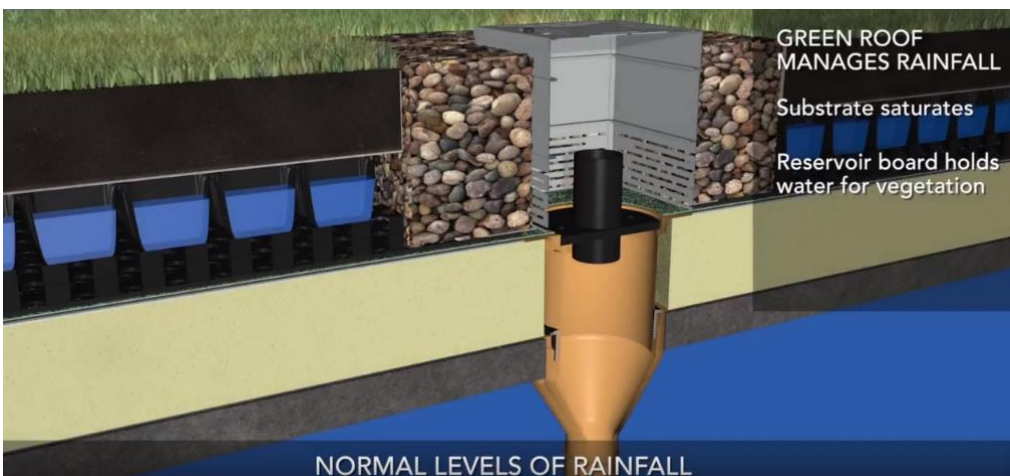
Blue and Green roof Process



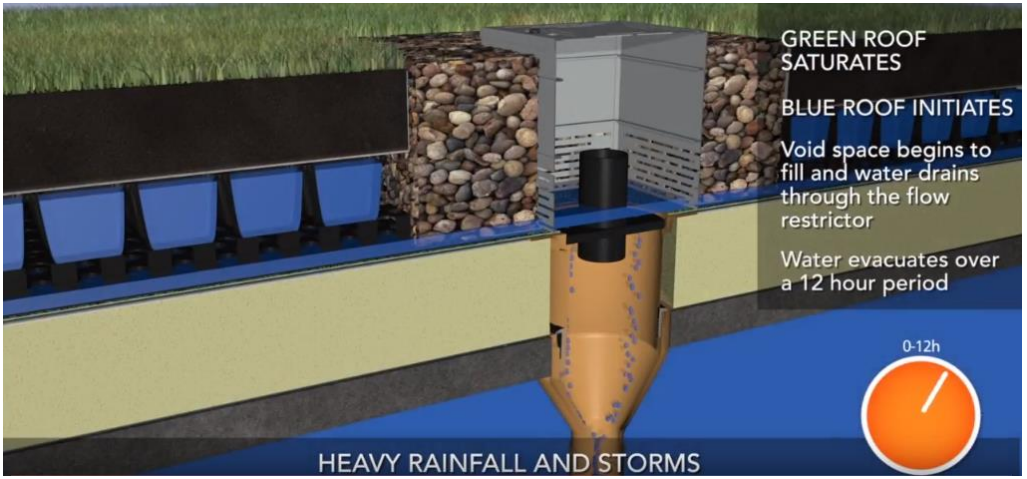
Green/Blue roof makeup



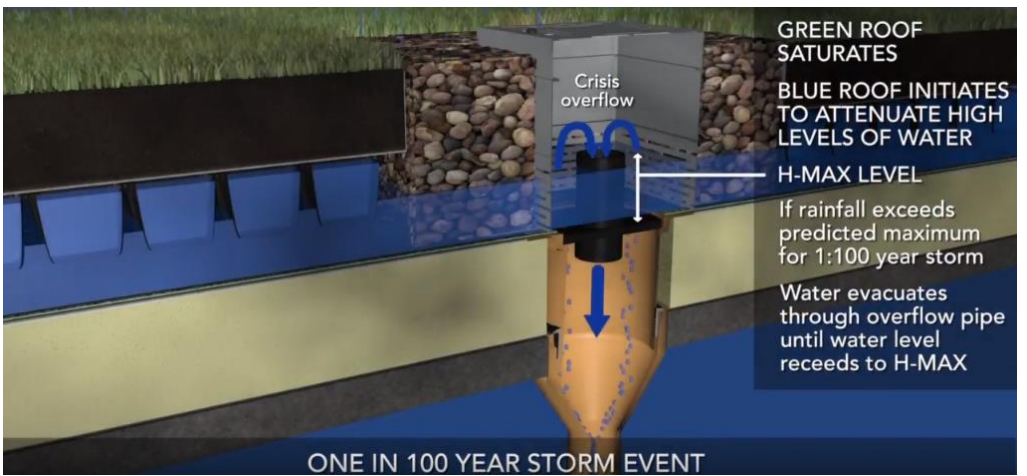
Maximum attenuation level



Normal levels of rainfall – substrate saturates, reservoir holds water for vegetation – no discharge



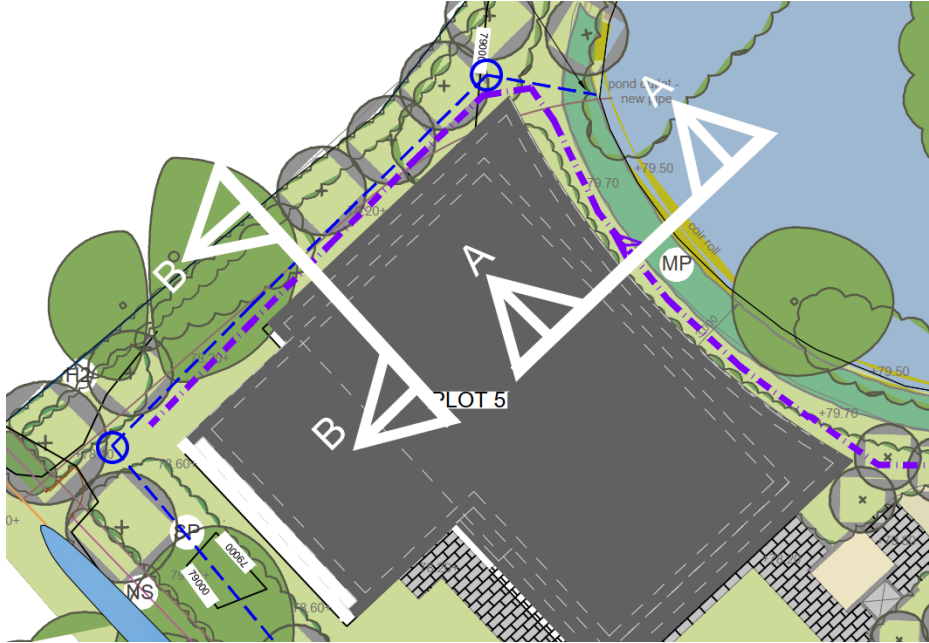
Heavy rainfall – water overflows the storage cells into the void space and drains through the flow restrictor attenuating the discharge



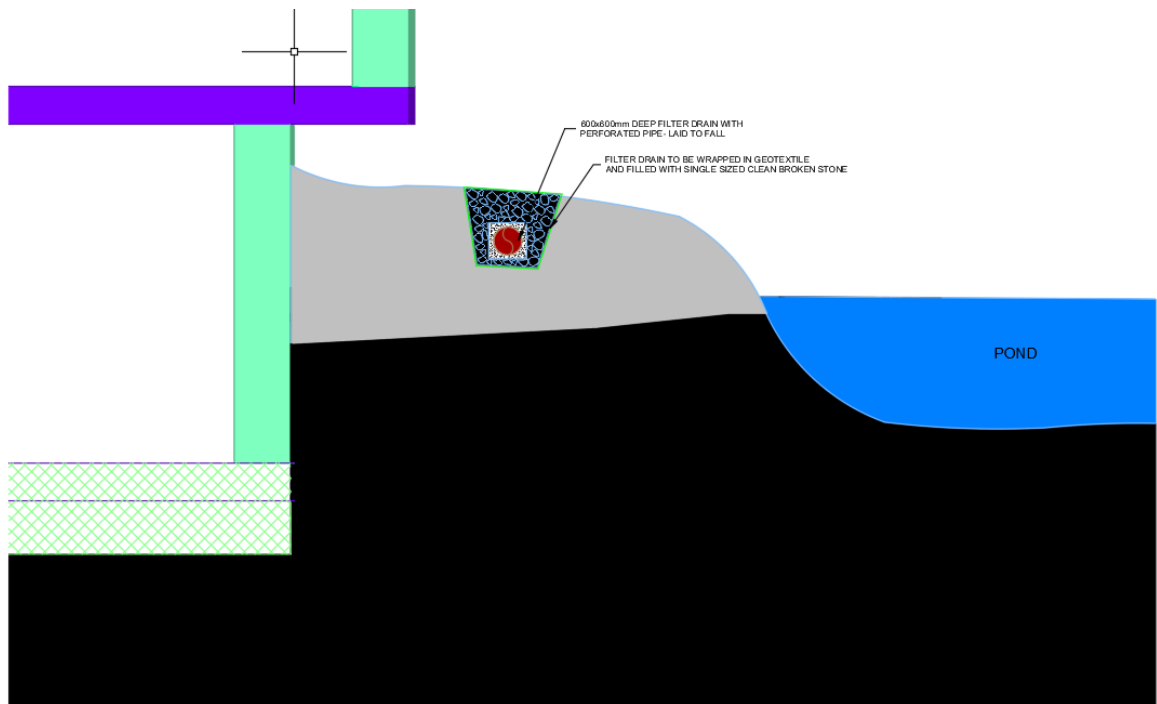
1:100 year event – if the rainfall exceeds the maximum 1:100 rainfall event the excess rainfall discharges the via the overflow pipe at high level – the remainder of the water is attenuated as normal.

Landscaping detail – typical detail shown adjacent to plot 5

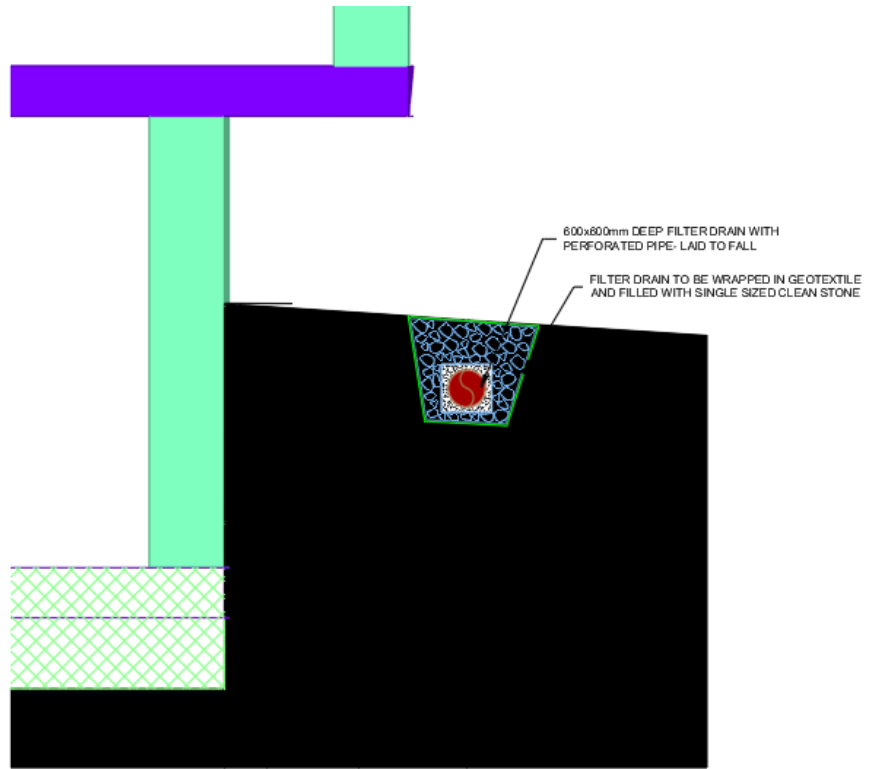
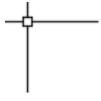
The details below show the intent of the landscaping/drainage adjacent to divert the water away from the buildings to the attenuation/swale area.



Layout of plot 4



Section A-A



Section B-B

Tadhg Kennedy
COYLE KENNEDY
CHARTERED ENGINEERS

1. Project & Site Details	Project / Site Name (including sub-catchment / stage / phase where appropriate)	55 FITZROY PARK
	Address & post code	55 FITZROY PARK Camden N6 6JA
	OS Grid ref. (Easting, Northing)	E 527780
		N 186940
	LPA reference (if applicable)	
	Brief description of proposed work	Demolition of large house and construction of a five smaller houses
	Total site Area	5075 m ²
	Total existing impervious area	1092 m ²
	Total proposed impervious area	947 m ²
	Is the site in a surface water flood risk catchment (ref. local Surface Water Management Plan)?	No
	Existing drainage connection type and location	Combined Sewer beneath Fitzroy Park
	Designer Name	Tadhg Kennedy
	Designer Position	Consulting Engineer
Designer Company	Coyle Kennedy	

2. Proposed Discharge Arrangements	2a. Infiltration Feasibility		
	Superficial geology classification	downwash	
	Bedrock geology classification	London Clay	
	Site infiltration rate	1.E-09 m/s	
	Depth to groundwater level	Varies	
	Is infiltration feasible?	No (Infiltration will be permitted but not relied upon)	
	2b. Drainage Hierarchy		
		<i>Feasible (Y/N)</i>	<i>Proposed (Y/N)</i>
	1 store rainwater for later use	Y	Y
	2 use infiltration techniques, such as porous surfaces in non-clay areas	No (Infiltration will be permitted but not relied upon)	
	3 attenuate rainwater in ponds or open water features for gradual release	Y	Y
	4 attenuate rainwater by storing in tanks or sealed water features for gradual release	Y	Y
	5 discharge rainwater direct to a watercourse	Y	Y
	6 discharge rainwater to a surface water sewer/drain	N	N
	7 discharge rainwater to the combined sewer.	Y	Y
2c. Proposed Discharge Details			
Proposed discharge location	Combined Sewer beneath Fitzroy Park and pond/natural watercourse system		
Has the owner/regulator of the discharge location been consulted?	Yes		

4. Supporting Information	4a. Discharge & Drainage Strategy	<i>Page/section of drainage report</i>
	Infiltration feasibility (2a) – geotechnical factual and interpretive reports, including infiltration results	Hydrological & Hydrogeological Impact Assessment LBH4480 Ver 2.0 July 2018
	Drainage hierarchy (2b)	Section 8.2 p32
	Proposed discharge details (2c) – utility plans, correspondence / approval from owner/regulator of discharge location	TBN
	Discharge rates & storage (3a) – detailed hydrologic and hydraulic calculations	Appended
	Proposed SuDS measures & specifications (3b)	See Coyle Kennedy Drainage reports
	4b. Other Supporting Details	<i>Page/section of drainage report</i>
	Detailed Development Layout	See Coyle Kennedy Drainage reports
	Detailed drainage design drawings, including exceedance flow routes	See Coyle Kennedy Drainage reports
	Detailed landscaping plans	See Coyle Kennedy Drainage reports
	Maintenance strategy	TBN
	Demonstration of how the proposed SuDS measures improve:	Hydrological & Hydrogeological Impact Assessment LBH4480 Ver 2.0 July 2018
	a) water quality of the runoff?	Section 9 p37
	b) biodiversity?	
	c) amenity?	

GREENFIELD RUNOFF



Catchment Area: 5075sqm 0.508ha
PO Code : N6 6JA
Hydrological Region: 6 *From Wallingford on-line tool*
SAAR: 625mm *From Wallingford on-line tool*
SOIL type: 4 *From Wallingford on-line tool*
SPR: 0.47 *Derived as follows:*

SOIL	Sand	Clayey Sand	Sandy Clay	Clay	Rock
	1	2	3	4	5
SPR	0.1	0.3	0.37	0.47	0.53

From Wallingford on-line tool using IH 124 Method

Qbar: 216.97 *Calculated from SPR and SAAR*

Greenfield Peak

Run-off Rate:	Growth curve Factor
1 in 1 184.4 l/sec	0.85
1 in 30 520.7 l/sec	2.40
1 in 100 692.1 l/sec	3.19
1 in 200 811.5 l/sec	3.74

Qbar: 2.20 l/sec

Greenfield

Peak Run-off Rate:

1 in 1	1.87 l/sec
1 in 30	5.29 l/sec
1 in 100	7.03 l/sec
1 in 200	8.24 l/sec

National Non-Statutory Guidance:

For greenfield developments, the peak runoff rate from the development to any highway drain, sewer or surface water body for the 1 in 1 year rainfall event and the 1 in 100 year rainfall event should never exceed the peak greenfield runoff rate for the same event.

For developments which were previously developed, the peak runoff rate from the development to any drain, sewer or surface water body for the 1 in 1 year rainfall event and the 1 in 100 year rainfall event should be as close as reasonably practicable to the greenfield runoff rate from the development for the same rainfall event, but should never exceed the rate of discharge from the development prior to redevelopment for that event.

Where reasonably practicable, for greenfield development, the runoff volume from the development to any highway drain, sewer or surface water body in the 1 in 100 year, 6 hour rainfall event should never exceed the greenfield runoff volume for the same event.

Where reasonably practicable, for developments which have been previously developed, the runoff volume from the development to any highway drain, sewer or surface water body in the 1 in 100 year, 6 hour rainfall event must be constrained to a value as close as is reasonably practicable to the greenfield runoff volume for the same event, but should never exceed the runoff volume from the development site prior to redevelopment for that event.

SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
GREENFIELD RUNOFF	
Sheet 1 of 8	
Project Reference: LBH 4599	
Date: 13/12/2019	Rev: 1
Client: Turner Stokes / Springer	

LBHGEO

RAINFALL PEAK INTENSITY (i)

M5-60 : 20 *From Wallingford Fig A1*
 r: 0.42 *From Wallingford Fig A2*

D Duration		Z1	M5-D
5min	5min	0.38	7.6mm
10min	10min	0.55	11.0mm
15min	15min	0.65	13.0mm
30min	30min	0.75	15.0mm
1hr	60min	1.00	20.0mm
2hr	120min	1.20	24.0mm
4hr	240min	1.40	28.0mm
6hr	360min	1.60	32.0mm
10hr	600min	1.70	34.0mm
24hr	1440min	2.20	44.0mm
48hr	2880min	2.50	50.0mm

D Duration		M5-D	M1-D	M2-D	M3-D	M4-D	Z2	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	7.6mm	0.62	0.79	0.89	0.97	1.02	1.02	1.19	1.36	1.43	1.79
10min	10min	11.0mm	0.61	0.79	0.90	0.97	1.03	1.03	1.22	1.41	1.49	1.91
15min	15min	13.0mm	0.61	0.79	0.90	0.97	1.03	1.03	1.22	1.41	1.49	1.91
30min	30min	15.0mm	0.62	0.81	0.90	0.97	1.03	1.03	1.24	1.44	1.53	1.99
1hr	60min	20.0mm	0.64	0.81	0.90	0.97	1.03	1.03	1.24	1.45	1.54	2.03
2hr	120min	24.0mm	0.64	0.81	0.90	0.97	1.03	1.03	1.24	1.45	1.54	2.03
4hr	240min	28.0mm	0.66	0.82	0.91	0.97	1.03	1.03	1.24	1.44	1.53	2.01
6hr	360min	32.0mm	0.68	0.83	0.91	0.97	1.03	1.03	1.22	1.42	1.51	1.97
10hr	600min	34.0mm	0.68	0.83	0.91	0.97	1.03	1.03	1.22	1.42	1.51	1.97
24hr	1440min	44.0mm	0.70	0.84	0.92	0.97	1.02	1.02	1.19	1.38	1.47	1.89
48hr	2880min	50.0mm	0.72	0.85	0.93	0.98	1.02	1.02	1.17	1.34	1.42	1.81

D Duration		M5-D	M1-D	M2-D	M3-D	M4-D	MT-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	7.6mm	4.7mm	6.0mm	6.8mm	7.4mm	7.8mm	7.8mm	9.0mm	10.3mm	10.8mm	13.6mm
10min	10min	11.0mm	6.7mm	8.7mm	9.9mm	10.7mm	11.3mm	11.3mm	13.4mm	15.5mm	16.4mm	21.0mm
15min	15min	13.0mm	7.9mm	10.3mm	11.7mm	12.6mm	13.4mm	13.4mm	15.9mm	18.3mm	19.4mm	24.8mm
30min	30min	15.0mm	9.3mm	12.2mm	13.5mm	14.6mm	15.5mm	15.5mm	18.6mm	21.6mm	22.9mm	29.9mm
1hr	60min	20.0mm	12.8mm	16.2mm	18.0mm	19.4mm	20.6mm	20.6mm	24.8mm	29.0mm	30.9mm	40.6mm
2hr	120min	24.0mm	15.4mm	19.4mm	21.6mm	23.3mm	24.7mm	24.7mm	29.8mm	34.8mm	37.0mm	48.7mm
4hr	240min	28.0mm	18.5mm	23.0mm	25.5mm	27.2mm	28.8mm	28.8mm	34.7mm	40.3mm	42.9mm	56.3mm
6hr	360min	32.0mm	21.8mm	26.6mm	29.1mm	31.0mm	33.0mm	33.0mm	39.0mm	45.4mm	48.4mm	63.0mm
10hr	600min	34.0mm	23.1mm	28.2mm	30.9mm	33.0mm	35.0mm	35.0mm	41.5mm	48.3mm	51.5mm	67.0mm
24hr	1440min	44.0mm	30.8mm	37.0mm	40.5mm	42.7mm	44.9mm	44.9mm	52.4mm	60.7mm	64.5mm	83.2mm
48hr	2880min	50.0mm	36.0mm	42.5mm	46.5mm	49.0mm	51.0mm	51.0mm	58.5mm	67.0mm	71.0mm	90.5mm

D Duration		M1-D	M2-D	M3-D	M4-D	Intensity i	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	56.5mm/hr	72.0mm/hr	81.2mm/hr	88.5mm/hr	93.0mm/hr	108.5mm/hr	124.0mm/hr	130.1mm/hr	163.2mm/hr
10min	10min	0.17hr	40.3mm/hr	52.1mm/hr	59.4mm/hr	64.0mm/hr	68.0mm/hr	80.5mm/hr	93.1mm/hr	98.3mm/hr	126.1mm/hr
15min	15min	0.25hr	31.7mm/hr	41.1mm/hr	46.8mm/hr	50.4mm/hr	53.6mm/hr	63.4mm/hr	73.3mm/hr	77.5mm/hr	99.3mm/hr
30min	30min	0.50hr	18.6mm/hr	24.3mm/hr	27.0mm/hr	29.1mm/hr	30.9mm/hr	37.2mm/hr	43.2mm/hr	45.8mm/hr	59.7mm/hr
1hr	60min	1.00hr	12.8mm/hr	16.2mm/hr	18.0mm/hr	19.4mm/hr	20.6mm/hr	24.8mm/hr	29.0mm/hr	30.9mm/hr	40.6mm/hr
2hr	120min	2.00hr	7.7mm/hr	9.7mm/hr	10.8mm/hr	11.6mm/hr	12.4mm/hr	14.9mm/hr	17.4mm/hr	18.5mm/hr	24.4mm/hr
4hr	240min	4.00hr	4.6mm/hr	5.7mm/hr	6.4mm/hr	6.8mm/hr	7.2mm/hr	8.7mm/hr	10.1mm/hr	10.7mm/hr	14.1mm/hr
6hr	360min	6.00hr	3.6mm/hr	4.4mm/hr	4.9mm/hr	5.2mm/hr	5.5mm/hr	6.5mm/hr	7.6mm/hr	8.1mm/hr	10.5mm/hr
10hr	600min	10.00hr	2.3mm/hr	2.8mm/hr	3.1mm/hr	3.3mm/hr	3.5mm/hr	4.1mm/hr	4.8mm/hr	5.1mm/hr	6.7mm/hr
24hr	1440min	24.00hr	1.3mm/hr	1.5mm/hr	1.7mm/hr	1.8mm/hr	1.9mm/hr	2.2mm/hr	2.5mm/hr	2.7mm/hr	3.5mm/hr
48hr	2880min	48.00hr	0.6mm/hr	0.8mm/hr	0.8mm/hr	0.9mm/hr	0.9mm/hr	1.1mm/hr	1.3mm/hr	1.3mm/hr	1.7mm/hr
48hr	2880min	48.00hr	0.8mm/hr	0.9mm/hr	1.0mm/hr	1.0mm/hr	1.1mm/hr	1.2mm/hr	1.4mm/hr	1.5mm/hr	1.9mm/hr

SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
RAINFALL PEAK INTENSITY	
Sheet 2 of 8	
Project Reference: LBH 4599	
Date: 13/12/2019	Rev: 1
Client: Turner Stokes / Springer	

GREENFIELD PEAK RUNOFF

Hydrological

Region: 6

From Wallingford on-line tool

Qbar: 2.20 l/sec

			Run-Off Q								
D Duration			M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
10min	10min	0.17hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
15min	15min	0.25hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
30min	30min	0.50hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
1hr	60min	1.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
2hr	120min	2.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
4hr	240min	4.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
6hr	360min	6.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
10hr	600min	10.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
24hr	1440min	24.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec
48hr	2880min	48.00hr	1.87 l/sec	1.94 l/sec	2.23 l/sec	2.53 l/sec	2.82 l/sec	3.57 l/sec	4.33 l/sec	5.29 l/sec	7.03 l/sec

			Run-Off Volume								
D Duration			M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	0.6 m3	0.6 m3	0.7 m3	0.8 m3	0.8 m3	1.1 m3	1.3 m3	1.6 m3	2.1 m3
10min	10min	0.17hr	1.1 m3	1.2 m3	1.3 m3	1.5 m3	1.7 m3	2.1 m3	2.6 m3	3.2 m3	4.2 m3
15min	15min	0.25hr	1.7 m3	1.7 m3	2.0 m3	2.3 m3	2.5 m3	3.2 m3	3.9 m3	4.8 m3	6.3 m3
30min	30min	0.50hr	3.4 m3	3.5 m3	4.0 m3	4.5 m3	5.1 m3	6.4 m3	7.8 m3	9.5 m3	12.6 m3
1hr	60min	1.00hr	6.7 m3	7.0 m3	8.0 m3	9.1 m3	10.1 m3	12.8 m3	15.6 m3	19.0 m3	25.3 m3
2hr	120min	2.00hr	13.5 m3	14.0 m3	16.1 m3	18.2 m3	20.3 m3	25.7 m3	31.2 m3	38.1 m3	50.6 m3
4hr	240min	4.00hr	27.0 m3	27.9 m3	32.1 m3	36.4 m3	40.6 m3	51.4 m3	62.4 m3	76.1 m3	101.2 m3
6hr	360min	6.00hr	40.4 m3	41.9 m3	48.2 m3	54.5 m3	60.9 m3	77.1 m3	93.6 m3	114.2 m3	151.7 m3
10hr	600min	10.00hr	67.4 m3	69.8 m3	80.3 m3	90.9 m3	101.5 m3	128.4 m3	155.9 m3	190.3 m3	252.9 m3
24hr	1440min	24.00hr	161.7 m3	167.4 m3	192.8 m3	218.2 m3	243.6 m3	308.2 m3	374.2 m3	456.7 m3	607.0 m3
48hr	2880min	48.00hr	323.5 m3	334.9 m3	385.6 m3	436.4 m3	487.1 m3	616.5 m3	748.4 m3	913.3 m3	1213.9 m3

SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
GREENFIELD PEAK RUNOFF	
Sheet 3 of 8	
Project Reference: LBH 4599	
Date: 13/12/2019	Rev: 1
Client: Turner Stokes / Springer	

EXISTING PEAK RUNOFF

C_v: 0.48 Volumetric Run-Off Coefficient
C_R: 1.3 Routing Coefficient

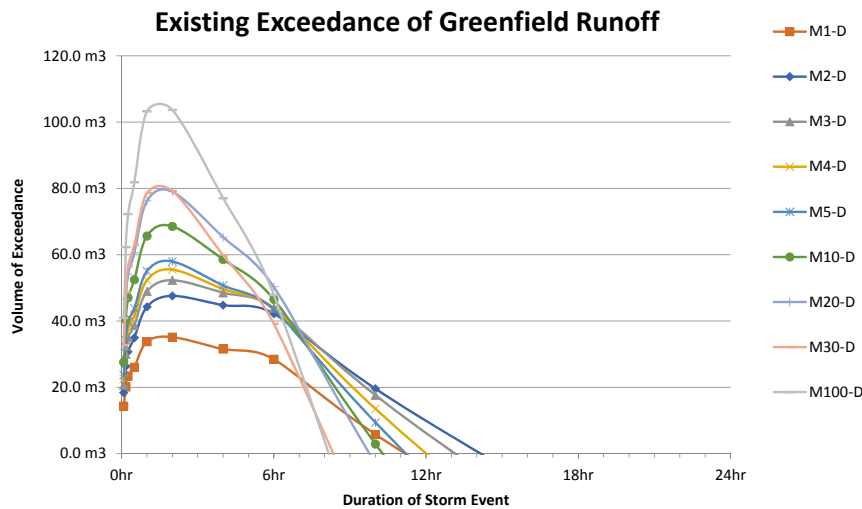
			Run-Off Q									
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D	
5min	5min	0.08hr	49.7 l/sec	63.4 l/sec	71.4 l/sec	77.8 l/sec	81.8 l/sec	95.5 l/sec	109.1 l/sec	114.5 l/sec	143.6 l/sec	
10min	10min	0.17hr	35.4 l/sec	45.9 l/sec	52.3 l/sec	56.3 l/sec	59.8 l/sec	70.8 l/sec	81.9 l/sec	86.5 l/sec	110.9 l/sec	
15min	15min	0.25hr	27.9 l/sec	36.1 l/sec	41.2 l/sec	44.4 l/sec	47.1 l/sec	55.8 l/sec	64.5 l/sec	68.2 l/sec	87.4 l/sec	
30min	30min	0.50hr	16.4 l/sec	21.4 l/sec	23.8 l/sec	25.6 l/sec	27.2 l/sec	32.7 l/sec	38.0 l/sec	40.3 l/sec	52.5 l/sec	
1hr	60min	1.00hr	11.3 l/sec	14.3 l/sec	15.8 l/sec	17.1 l/sec	18.1 l/sec	21.8 l/sec	25.5 l/sec	27.2 l/sec	35.7 l/sec	
2hr	120min	2.00hr	6.8 l/sec	8.6 l/sec	9.5 l/sec	10.2 l/sec	10.9 l/sec	13.1 l/sec	15.3 l/sec	16.3 l/sec	21.4 l/sec	
4hr	240min	4.00hr	4.1 l/sec	5.0 l/sec	5.6 l/sec	6.0 l/sec	6.3 l/sec	7.6 l/sec	8.9 l/sec	9.4 l/sec	12.4 l/sec	
6hr	360min	6.00hr	3.2 l/sec	3.9 l/sec	4.3 l/sec	4.6 l/sec	4.8 l/sec	5.7 l/sec	6.7 l/sec	7.1 l/sec	9.2 l/sec	
10hr	600min	10.00hr	2.0 l/sec	2.5 l/sec	2.7 l/sec	2.9 l/sec	3.1 l/sec	3.6 l/sec	4.2 l/sec	4.5 l/sec	5.9 l/sec	
24hr	1440min	24.00hr	1.1 l/sec	1.4 l/sec	1.5 l/sec	1.6 l/sec	1.6 l/sec	1.9 l/sec	2.2 l/sec	2.4 l/sec	3.0 l/sec	
48hr	2880min	48.00hr	0.7 l/sec	0.8 l/sec	0.9 l/sec	0.9 l/sec	0.9 l/sec	1.1 l/sec	1.2 l/sec	1.3 l/sec	1.7 l/sec	

			Run-Off Volume									
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D	
5min	5min	0.08hr	14.9 m3	19.0 m3	21.4 m3	23.3 m3	24.5 m3	28.6 m3	32.7 m3	34.3 m3	43.1 m3	
10min	10min	0.17hr	21.2 m3	27.5 m3	31.4 m3	33.8 m3	35.9 m3	42.5 m3	49.1 m3	51.9 m3	66.5 m3	
15min	15min	0.25hr	25.1 m3	32.5 m3	37.1 m3	39.9 m3	42.4 m3	50.2 m3	58.0 m3	61.3 m3	78.6 m3	
30min	30min	0.50hr	29.5 m3	38.5 m3	42.8 m3	46.1 m3	48.9 m3	58.9 m3	68.4 m3	72.5 m3	94.5 m3	
1hr	60min	1.00hr	40.5 m3	51.3 m3	57.0 m3	61.4 m3	65.2 m3	78.5 m3	91.8 m3	97.7 m3	128.6 m3	
2hr	120min	2.00hr	48.6 m3	61.6 m3	68.4 m3	73.7 m3	78.3 m3	94.2 m3	110.2 m3	117.3 m3	154.3 m3	
4hr	240min	4.00hr	58.5 m3	72.7 m3	80.7 m3	86.0 m3	91.3 m3	110.0 m3	127.7 m3	136.0 m3	178.2 m3	
6hr	360min	6.00hr	68.9 m3	84.1 m3	92.2 m3	98.3 m3	104.4 m3	123.6 m3	143.9 m3	153.4 m3	199.6 m3	
10hr	600min	10.00hr	73.2 m3	89.4 m3	98.0 m3	104.4 m3	110.9 m3	131.4 m3	152.9 m3	162.9 m3	212.1 m3	
24hr	1440min	24.00hr	97.5 m3	117.0 m3	128.2 m3	135.2 m3	142.1 m3	165.8 m3	192.3 m3	204.4 m3	263.3 m3	
48hr	2880min	48.00hr	114.0 m3	134.6 m3	147.3 m3	155.2 m3	161.5 m3	185.3 m3	212.2 m3	224.8 m3	286.6 m3	

			Exceedance of Greenfield Run-Off Volume									
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D	
5min	5min	0.08hr	14.4 m3	18.4 m3	20.8 m3	22.6 m3	23.7 m3	27.6 m3	31.4 m3	32.8 m3	41.0 m3	
10min	10min	0.17hr	20.1 m3	26.4 m3	30.0 m3	32.3 m3	34.2 m3	40.4 m3	46.5 m3	48.7 m3	62.3 m3	
15min	15min	0.25hr	23.4 m3	30.8 m3	35.0 m3	37.7 m3	39.9 m3	47.0 m3	54.1 m3	56.6 m3	72.3 m3	
30min	30min	0.50hr	26.1 m3	35.0 m3	38.7 m3	41.5 m3	43.9 m3	52.5 m3	60.6 m3	63.0 m3	81.9 m3	
1hr	60min	1.00hr	33.8 m3	44.3 m3	49.0 m3	52.3 m3	55.1 m3	65.7 m3	76.2 m3	78.7 m3	103.3 m3	
2hr	120min	2.00hr	35.2 m3	47.6 m3	52.3 m3	55.5 m3	58.0 m3	68.6 m3	79.0 m3	79.2 m3	103.7 m3	
4hr	240min	4.00hr	31.6 m3	44.8 m3	48.6 m3	49.6 m3	50.7 m3	58.6 m3	65.3 m3	59.9 m3	77.1 m3	
6hr	360min	6.00hr	28.5 m3	42.2 m3	44.0 m3	43.8 m3	43.5 m3	46.6 m3	50.3 m3	39.2 m3	47.9 m3	
10hr	600min	10.00hr	5.8 m3	19.6 m3	17.6 m3	13.5 m3	9.4 m3	2.9 m3	-3.0 m3	-27.3 m3	-40.8 m3	
24hr	1440min	24.00hr	-64.2 m3	-50.4 m3	-64.6 m3	-83.0 m3	-101.4 m3	-142.4 m3	-181.9 m3	-252.3 m3	-343.6 m3	
48hr	2880min	48.00hr	-209.5 m3	-200.3 m3	-238.4 m3	-281.2 m3	-325.6 m3	-431.2 m3	-536.2 m3	-688.5 m3	-927.4 m3	

C_v:
 Catchment Area: 5075sqm 100%
 Permeable: 3983sqm 78%
 Impermeable: 1092sqm 22%

 0.48



SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
EXISTING PEAK RUNOFF	
Sheet 4 of 8	
Project Reference: LBH 4599	
Date: 13/12/2019	Rev: 1
Client: Turner Stokes / Springer	

POST- DEVELOPMENT PEAK RUNOFF + CC

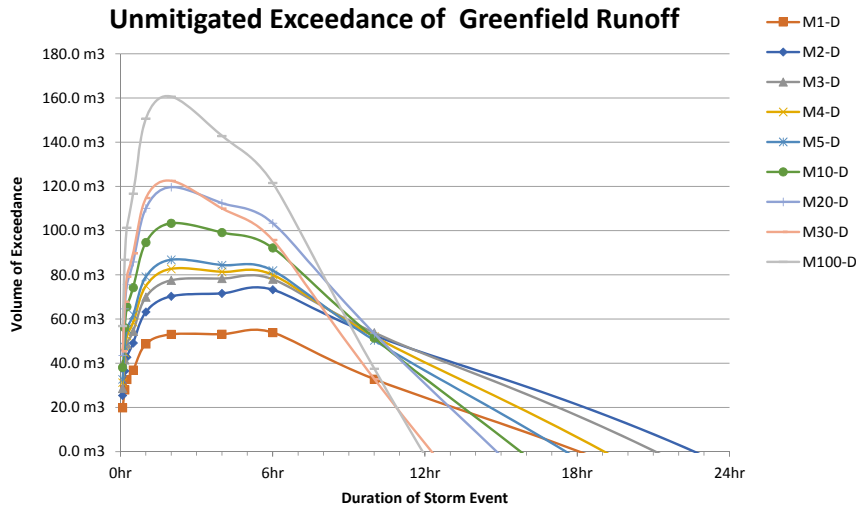
C_v: 0.47 *Volumetric Run-Off Coefficient* Climate Change Allowance: 40%
C_R: 1.3 *Routing Coefficient*

D Duration			Run-Off Q											
	M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D					
5min	5min	0.08hr	68.1 l/sec	86.8 l/sec	97.8 l/sec	106.5 l/sec	112.0 l/sec	130.7 l/sec	149.4 l/sec	156.7 l/sec	196.6 l/sec			
10min	10min	0.17hr	48.5 l/sec	62.8 l/sec	71.5 l/sec	77.1 l/sec	81.9 l/sec	97.0 l/sec	112.1 l/sec	118.4 l/sec	151.8 l/sec			
15min	15min	0.25hr	38.2 l/sec	49.5 l/sec	56.4 l/sec	60.7 l/sec	64.5 l/sec	76.4 l/sec	88.3 l/sec	93.3 l/sec	119.6 l/sec			
30min	30min	0.50hr	22.4 l/sec	29.3 l/sec	32.5 l/sec	35.0 l/sec	37.2 l/sec	44.8 l/sec	52.0 l/sec	55.2 l/sec	71.9 l/sec			
1hr	60min	1.00hr	15.4 l/sec	19.5 l/sec	21.7 l/sec	23.4 l/sec	24.8 l/sec	29.9 l/sec	34.9 l/sec	37.2 l/sec	48.9 l/sec			
2hr	120min	2.00hr	9.2 l/sec	11.7 l/sec	13.0 l/sec	14.0 l/sec	14.9 l/sec	17.9 l/sec	21.0 l/sec	22.3 l/sec	29.3 l/sec			
4hr	240min	4.00hr	5.6 l/sec	6.9 l/sec	7.7 l/sec	8.2 l/sec	8.7 l/sec	10.5 l/sec	12.1 l/sec	12.9 l/sec	16.9 l/sec			
6hr	360min	6.00hr	4.4 l/sec	5.3 l/sec	5.8 l/sec	6.2 l/sec	6.6 l/sec	7.8 l/sec	9.1 l/sec	9.7 l/sec	12.7 l/sec			
10hr	600min	10.00hr	2.8 l/sec	3.4 l/sec	3.7 l/sec	4.0 l/sec	4.2 l/sec	5.0 l/sec	5.8 l/sec	6.2 l/sec	8.1 l/sec			
24hr	1440min	24.00hr	1.5 l/sec	1.9 l/sec	2.0 l/sec	2.1 l/sec	2.3 l/sec	2.6 l/sec	3.0 l/sec	3.2 l/sec	4.2 l/sec			
48hr	2880min	48.00hr	0.9 l/sec	1.1 l/sec	1.2 l/sec	1.2 l/sec	1.3 l/sec	1.5 l/sec	1.7 l/sec	1.8 l/sec	2.3 l/sec			

D Duration			Run-Off Volume											
	M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D					
5min	5min	0.08hr	20.4 m3	26.0 m3	29.3 m3	32.0 m3	33.6 m3	39.2 m3	44.8 m3	47.0 m3	59.0 m3			
10min	10min	0.17hr	29.1 m3	37.7 m3	42.9 m3	46.3 m3	49.1 m3	58.2 m3	67.2 m3	71.1 m3	91.1 m3			
15min	15min	0.25hr	34.4 m3	44.5 m3	50.7 m3	54.7 m3	58.1 m3	68.8 m3	79.5 m3	84.0 m3	107.7 m3			
30min	30min	0.50hr	40.3 m3	52.7 m3	58.5 m3	63.1 m3	67.0 m3	80.6 m3	93.7 m3	99.3 m3	129.4 m3			
1hr	60min	1.00hr	55.5 m3	70.2 m3	78.0 m3	84.1 m3	89.3 m3	107.5 m3	125.7 m3	133.8 m3	176.0 m3			
2hr	120min	2.00hr	66.6 m3	84.3 m3	93.7 m3	100.9 m3	107.2 m3	129.0 m3	150.9 m3	160.6 m3	211.2 m3			
4hr	240min	4.00hr	80.1 m3	99.5 m3	110.5 m3	117.8 m3	125.0 m3	150.5 m3	174.8 m3	186.1 m3	244.0 m3			
6hr	360min	6.00hr	94.3 m3	115.2 m3	126.3 m3	134.6 m3	142.9 m3	169.3 m3	197.0 m3	210.0 m3	273.3 m3			
10hr	600min	10.00hr	100.2 m3	122.4 m3	134.1 m3	143.0 m3	151.8 m3	179.8 m3	209.3 m3	223.1 m3	290.4 m3			
24hr	1440min	24.00hr	133.5 m3	160.3 m3	175.5 m3	185.1 m3	194.6 m3	227.0 m3	263.3 m3	279.8 m3	360.6 m3			
48hr	2880min	48.00hr	156.1 m3	184.3 m3	201.6 m3	212.5 m3	221.1 m3	253.6 m3	290.5 m3	307.8 m3	392.4 m3			

D Duration			Exceedance of Greenfield Run-Off Volume											
	M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D					
5min	5min	0.08hr	19.9 m3	25.5 m3	28.7 m3	31.2 m3	32.8 m3	38.1 m3	43.5 m3	45.4 m3	56.9 m3			
10min	10min	0.17hr	28.0 m3	36.5 m3	41.6 m3	44.7 m3	47.4 m3	56.0 m3	64.6 m3	67.9 m3	86.9 m3			
15min	15min	0.25hr	32.7 m3	42.8 m3	48.7 m3	52.4 m3	55.5 m3	65.6 m3	75.6 m3	79.2 m3	101.3 m3			
30min	30min	0.50hr	37.0 m3	49.2 m3	54.5 m3	58.5 m3	61.9 m3	74.2 m3	85.9 m3	89.8 m3	116.8 m3			
1hr	60min	1.00hr	48.8 m3	63.3 m3	70.0 m3	75.0 m3	79.2 m3	94.7 m3	110.1 m3	114.8 m3	150.7 m3			
2hr	120min	2.00hr	53.1 m3	70.3 m3	77.6 m3	82.8 m3	86.9 m3	103.3 m3	119.7 m3	122.5 m3	160.7 m3			
4hr	240min	4.00hr	53.2 m3	71.6 m3	78.3 m3	81.4 m3	84.5 m3	99.2 m3	112.5 m3	110.0 m3	142.9 m3			
6hr	360min	6.00hr	53.9 m3	73.3 m3	78.1 m3	80.0 m3	82.0 m3	92.2 m3	103.5 m3	95.8 m3	121.6 m3			
10hr	600min	10.00hr	32.9 m3	52.6 m3	53.8 m3	52.1 m3	50.4 m3	51.4 m3	53.4 m3	32.8 m3	37.5 m3			
24hr	1440min	24.00hr	-28.2 m3	-7.2 m3	-17.3 m3	-33.1 m3	-49.0 m3	-81.2 m3	-110.9 m3	-176.9 m3	-246.4 m3			
48hr	2880min	48.00hr	-167.4 m3	-150.6 m3	-184.0 m3	-223.9 m3	-266.0 m3	-362.8 m3	-457.9 m3	-605.5 m3	-821.6 m3			
														160.7 m3

C _v :		
Catchment Area:	5075sqm	100%
Permeable Garden:	4128sqm	81%
Impermeable:	947sqm	19%
		<u>0.77</u>
		0.47



SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
POST-DEV. PEAK RUNOFF+CC	
Sheet 6 of 8	
Project Reference: LBH 4599	
Date: 13/12/2019	Rev: 1
Client: Turner Stokes / Springer	

POST- DEVELOPMENT & SOURCE MITIGATION PEAK RUN-OFF + CC STORAGE

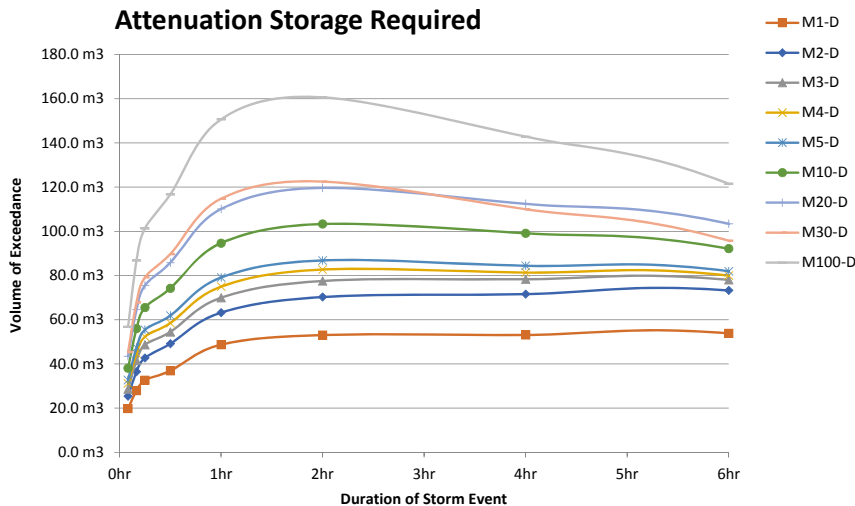
Proposed Discharge Rates: Greenfield x 1

			INFLOW								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	20.4 m3	26.0 m3	29.3 m3	32.0 m3	33.6 m3	39.2 m3	44.8 m3	47.0 m3	59.0 m3
10min	10min	0.17hr	29.1 m3	37.7 m3	42.9 m3	46.3 m3	49.1 m3	58.2 m3	67.2 m3	71.1 m3	91.1 m3
15min	15min	0.25hr	34.4 m3	44.5 m3	50.7 m3	54.7 m3	58.1 m3	68.8 m3	79.5 m3	84.0 m3	107.7 m3
30min	30min	0.50hr	40.3 m3	52.7 m3	58.5 m3	63.1 m3	67.0 m3	80.6 m3	93.7 m3	99.3 m3	129.4 m3
1hr	60min	1.00hr	55.5 m3	70.2 m3	78.0 m3	84.1 m3	89.3 m3	107.5 m3	125.7 m3	133.8 m3	176.0 m3
2hr	120min	2.00hr	66.6 m3	84.3 m3	93.7 m3	100.9 m3	107.2 m3	129.0 m3	150.9 m3	160.6 m3	211.2 m3
4hr	240min	4.00hr	80.1 m3	99.5 m3	110.5 m3	117.8 m3	125.0 m3	150.5 m3	174.8 m3	186.1 m3	244.0 m3
6hr	360min	6.00hr	94.3 m3	115.2 m3	126.3 m3	134.6 m3	142.9 m3	169.3 m3	197.0 m3	210.0 m3	273.3 m3
10hr	600min	10.00hr	100.2 m3	122.4 m3	134.1 m3	143.0 m3	151.8 m3	179.8 m3	209.3 m3	223.1 m3	290.4 m3
24hr	1440min	24.00hr	133.5 m3	160.3 m3	175.5 m3	185.1 m3	194.6 m3	227.0 m3	263.3 m3	279.8 m3	360.6 m3
48hr	2880min	48.00hr	156.1 m3	184.3 m3	201.6 m3	212.5 m3	221.1 m3	253.6 m3	290.5 m3	307.8 m3	392.4 m3

			OUTFLOW								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	0.6 m3	0.6 m3	0.7 m3	0.8 m3	0.8 m3	1.1 m3	1.3 m3	1.6 m3	2.1 m3
10min	10min	0.17hr	1.1 m3	1.2 m3	1.3 m3	1.5 m3	1.7 m3	2.1 m3	2.6 m3	3.2 m3	4.2 m3
15min	15min	0.25hr	1.7 m3	1.7 m3	2.0 m3	2.3 m3	2.5 m3	3.2 m3	3.9 m3	4.8 m3	6.3 m3
30min	30min	0.50hr	3.4 m3	3.5 m3	4.0 m3	4.5 m3	5.1 m3	6.4 m3	7.8 m3	9.5 m3	12.6 m3
1hr	60min	1.00hr	6.7 m3	7.0 m3	8.0 m3	9.1 m3	10.1 m3	12.8 m3	15.6 m3	19.0 m3	25.3 m3
2hr	120min	2.00hr	13.5 m3	14.0 m3	16.1 m3	18.2 m3	20.3 m3	25.7 m3	31.2 m3	38.1 m3	50.6 m3
4hr	240min	4.00hr	27.0 m3	27.9 m3	32.1 m3	36.4 m3	40.6 m3	51.4 m3	62.4 m3	76.1 m3	101.2 m3
6hr	360min	6.00hr	40.4 m3	41.9 m3	48.2 m3	54.5 m3	60.9 m3	77.1 m3	93.6 m3	114.2 m3	151.7 m3
10hr	600min	10.00hr	67.4 m3	69.8 m3	80.3 m3	90.9 m3	101.5 m3	128.4 m3	155.9 m3	190.3 m3	252.9 m3
24hr	1440min	24.00hr	161.7 m3	167.4 m3	192.8 m3	218.2 m3	243.6 m3	308.2 m3	374.2 m3	456.7 m3	607.0 m3
48hr	2880min	48.00hr	323.5 m3	334.9 m3	385.6 m3	436.4 m3	487.1 m3	616.5 m3	748.4 m3	913.3 m3	1213.9 m3

			ATTENUATION STORAGE REQUIRED TO MEET PROPOSED DISCHARGE RATE								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	19.9 m3	25.5 m3	28.7 m3	31.2 m3	32.8 m3	38.1 m3	43.5 m3	45.4 m3	56.9 m3
10min	10min	0.17hr	28.0 m3	36.5 m3	41.6 m3	44.7 m3	47.4 m3	56.0 m3	64.6 m3	67.9 m3	86.9 m3
15min	15min	0.25hr	32.7 m3	42.8 m3	48.7 m3	52.4 m3	55.5 m3	65.6 m3	75.6 m3	79.2 m3	101.3 m3
30min	30min	0.50hr	37.0 m3	49.2 m3	54.5 m3	58.5 m3	61.9 m3	74.2 m3	85.9 m3	89.8 m3	116.8 m3
1hr	60min	1.00hr	48.8 m3	63.3 m3	70.0 m3	75.0 m3	79.2 m3	94.7 m3	110.1 m3	114.8 m3	150.7 m3
2hr	120min	2.00hr	53.1 m3	70.3 m3	77.6 m3	82.8 m3	86.9 m3	103.3 m3	119.7 m3	122.5 m3	160.7 m3
4hr	240min	4.00hr	53.2 m3	71.6 m3	78.3 m3	81.4 m3	84.5 m3	99.2 m3	112.5 m3	110.0 m3	142.9 m3
6hr	360min	6.00hr	53.9 m3	73.3 m3	78.1 m3	80.0 m3	82.0 m3	92.2 m3	103.5 m3	95.8 m3	121.6 m3
10hr	600min	10.00hr	32.9 m3	52.6 m3	53.8 m3	52.1 m3	50.4 m3	51.4 m3	53.4 m3	32.8 m3	37.5 m3
24hr	1440min	24.00hr	-28.2 m3	-7.2 m3	-17.3 m3	-33.1 m3	-49.0 m3	-81.2 m3	-110.9 m3	-176.9 m3	-246.4 m3
48hr	2880min	48.00hr	-167.4 m3	-150.6 m3	-184.0 m3	-223.9 m3	-266.0 m3	-362.8 m3	-457.9 m3	-605.5 m3	-821.6 m3

ATTENUATION STORAGE REQUIRED: 53.9 m3 73.3 m3 78.3 m3 82.8 m3 86.9 m3 103.3 m3 119.7 m3 122.5 m3 160.7 m3



SuDs CALCULATIONS	
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Client: Turner Stokes / Springer	

POST- DEVELOPMENT & SOURCE MITIGATION PEAK RUN-OFF + CC STORAGE

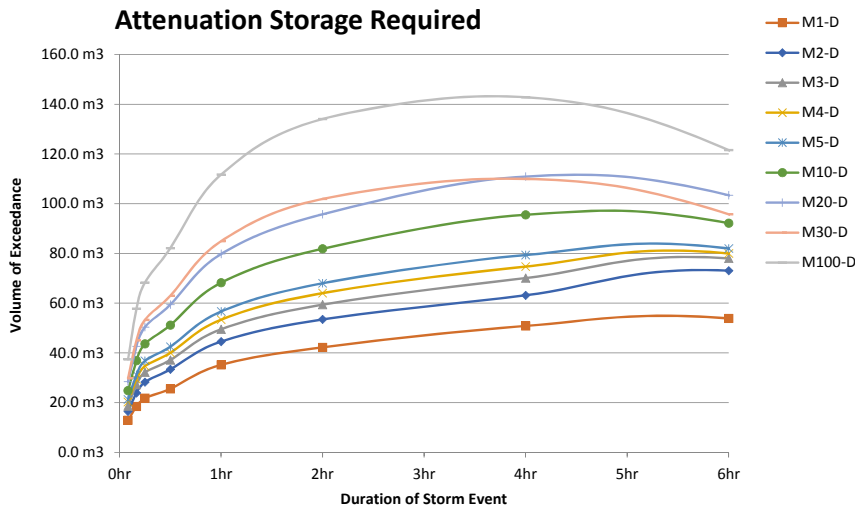
Proposed Discharge Rate: Existing x 50% (or greenfield where this is greater)

			INFLOW								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	20.4 m3	26.0 m3	29.3 m3	32.0 m3	33.6 m3	39.2 m3	44.8 m3	47.0 m3	59.0 m3
10min	10min	0.17hr	29.1 m3	37.7 m3	42.9 m3	46.3 m3	49.1 m3	58.2 m3	67.2 m3	71.1 m3	91.1 m3
15min	15min	0.25hr	34.4 m3	44.5 m3	50.7 m3	54.7 m3	58.1 m3	68.8 m3	79.5 m3	84.0 m3	107.7 m3
30min	30min	0.50hr	40.3 m3	52.7 m3	58.5 m3	63.1 m3	67.0 m3	80.6 m3	93.7 m3	99.3 m3	129.4 m3
1hr	60min	1.00hr	55.5 m3	70.2 m3	78.0 m3	84.1 m3	89.3 m3	107.5 m3	125.7 m3	133.8 m3	176.0 m3
2hr	120min	2.00hr	66.6 m3	84.3 m3	93.7 m3	100.9 m3	107.2 m3	129.0 m3	150.9 m3	160.6 m3	211.2 m3
4hr	240min	4.00hr	80.1 m3	99.5 m3	110.5 m3	117.8 m3	125.0 m3	150.5 m3	174.8 m3	186.1 m3	244.0 m3
6hr	360min	6.00hr	94.3 m3	115.2 m3	126.3 m3	134.6 m3	142.9 m3	169.3 m3	197.0 m3	210.0 m3	273.3 m3
10hr	600min	10.00hr	100.2 m3	122.4 m3	134.1 m3	143.0 m3	151.8 m3	179.8 m3	209.3 m3	223.1 m3	290.4 m3
24hr	1440min	24.00hr	133.5 m3	160.3 m3	175.5 m3	185.1 m3	194.6 m3	227.0 m3	263.3 m3	279.8 m3	360.6 m3
48hr	2880min	48.00hr	156.1 m3	184.3 m3	201.6 m3	212.5 m3	221.1 m3	253.6 m3	290.5 m3	307.8 m3	392.4 m3

			OUTFLOW								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	7.5 m3	9.5 m3	10.7 m3	11.7 m3	12.3 m3	14.3 m3	16.4 m3	17.2 m3	21.5 m3
10min	10min	0.17hr	10.6 m3	13.8 m3	15.7 m3	16.9 m3	17.9 m3	21.2 m3	24.6 m3	26.0 m3	33.3 m3
15min	15min	0.25hr	12.6 m3	16.3 m3	18.5 m3	20.0 m3	21.2 m3	25.1 m3	29.0 m3	30.7 m3	39.3 m3
30min	30min	0.50hr	14.7 m3	19.2 m3	21.4 m3	23.0 m3	24.5 m3	29.5 m3	34.2 m3	36.3 m3	47.3 m3
1hr	60min	1.00hr	20.3 m3	25.7 m3	28.5 m3	30.7 m3	32.6 m3	39.3 m3	45.9 m3	48.9 m3	64.3 m3
2hr	120min	2.00hr	24.3 m3	30.8 m3	34.2 m3	36.9 m3	39.1 m3	47.1 m3	55.1 m3	58.6 m3	77.1 m3
4hr	240min	4.00hr	29.3 m3	36.4 m3	40.3 m3	43.0 m3	45.7 m3	55.0 m3	63.8 m3	76.1 m3	101.2 m3
6hr	360min	6.00hr	40.4 m3	42.1 m3	48.2 m3	54.5 m3	60.9 m3	77.1 m3	93.6 m3	114.2 m3	151.7 m3
10hr	600min	10.00hr	67.4 m3	69.8 m3	80.3 m3	90.9 m3	101.5 m3	128.4 m3	155.9 m3	190.3 m3	252.9 m3
24hr	1440min	24.00hr	161.7 m3	167.4 m3	192.8 m3	218.2 m3	243.6 m3	308.2 m3	374.2 m3	456.7 m3	607.0 m3
48hr	2880min	48.00hr	323.5 m3	334.9 m3	385.6 m3	436.4 m3	487.1 m3	616.5 m3	748.4 m3	913.3 m3	1213.9 m3

			ATTENUATION STORAGE REQUIRED TO MEET PROPOSED DISCHARGE RATE								
D	Duration		M1-D	M2-D	M3-D	M4-D	M5-D	M10-D	M20-D	M30-D	M100-D
5min	5min	0.08hr	13.0 m3	16.5 m3	18.6 m3	20.3 m3	21.3 m3	24.9 m3	28.4 m3	29.8 m3	37.4 m3
10min	10min	0.17hr	18.5 m3	23.9 m3	27.2 m3	29.4 m3	31.2 m3	36.9 m3	42.7 m3	45.1 m3	57.8 m3
15min	15min	0.25hr	21.8 m3	28.3 m3	32.2 m3	34.7 m3	36.9 m3	43.7 m3	50.5 m3	53.3 m3	68.3 m3
30min	30min	0.50hr	25.6 m3	33.4 m3	37.2 m3	40.0 m3	42.5 m3	51.2 m3	59.5 m3	63.0 m3	82.2 m3
1hr	60min	1.00hr	35.2 m3	44.6 m3	49.5 m3	53.4 m3	56.7 m3	68.3 m3	79.8 m3	85.0 m3	111.7 m3
2hr	120min	2.00hr	42.3 m3	53.5 m3	59.5 m3	64.1 m3	68.0 m3	81.9 m3	95.8 m3	101.9 m3	134.1 m3
4hr	240min	4.00hr	50.9 m3	63.2 m3	70.1 m3	74.8 m3	79.4 m3	95.6 m3	111.0 m3	110.0 m3	142.9 m3
6hr	360min	6.00hr	53.9 m3	73.1 m3	78.1 m3	80.0 m3	82.0 m3	92.2 m3	103.5 m3	95.8 m3	121.6 m3
10hr	600min	10.00hr	32.9 m3	52.6 m3	53.8 m3	52.1 m3	50.4 m3	51.4 m3	53.4 m3	32.8 m3	37.5 m3
24hr	1440min	24.00hr	-28.2 m3	-7.2 m3	-17.3 m3	-33.1 m3	-49.0 m3	-81.2 m3	-110.9 m3	-176.9 m3	-246.4 m3
48hr	2880min	48.00hr	-167.4 m3	-150.6 m3	-184.0 m3	-223.9 m3	-266.0 m3	-362.8 m3	-457.9 m3	-605.5 m3	-821.6 m3

ATTENUATION STORAGE REQUIRED: 53.9 m3 73.1 m3 78.1 m3 80.0 m3 82.0 m3 95.6 m3 111.0 m3 110.0 m3 142.9 m3



SuDs CALCULATIONS	
Project: 55 FITZROY PARK	
STORAGE REQUIREMENTS	
Sheet 8 of 8	
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