Basement Impact Assessment – Structural

Property Details 16 Frognal Gardens London, NW3

Client Information Holly Walk Developments Alan Harari 20 Holly Walk London NW3 6RA

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Hydrogeology Report	Land Stability Report
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-	03/10/18	First Issue
-	08/10/18	Minor alterations
1	21/12/18	Alterations to comments by Campbell Reith
2	16/09/19	Alterations to comments by Campbell Reith &
		Alterations to architectural layout
3	07/11/19	Alterations to Audit comments









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STRUCTURAL ENGINEERS



Executive (n	ion-technical) Summary
	The London Borough of Camden requires a Basement Impact Assessment (BIA) to be prepared for developments that include basements and lightwells. This document forms the main part of the BIA and gives details on the impact of surface water flow. The scheme design for the proposed subterranean structure is also included.
	This document should be used in conjunction with the Land Stability and the Groundwater BIA (GWPR2777/GIR/November 2019). These are separate reports and are referred to, where relevant, within this document.
	 This BIA follows the requirements contained within following; Camden Council's planning guidance CGP: Basements (March 2018) Guidance for Subterranean Development (GSD). Issue 01. November 2010. Ove Arup & Partners Camden Development Policy (DP) 27: Basements and Lightwells Camden Development Policy (DP): Water Camden Local Plan 2017; Policy A5 Basements and Policy CC3 Water and Flooding.
	In summary, the council will only allow basement construction to proceed if it does not: • cause harm to the built or natural environment and local amenity
	 result in flooding lead to ground instability.
Existing Site	The site is in north-west London area of Hampstead in the Borough of Camden. The site is of a rectangular shape on a light slope of Holly Walk and currently occupied by two blocks of garages. The full area of site is tarmac paved.
Proposed Development	The proposed development involves the demolition of the garages and construction of three storey high residential property in its place. The property compromises of the basement extending about 3.3m below the ground level from the position of trial holes BH1 & BH2 as well as underground garage to a formation depth of 2.5-2.6m below ground level.



	Figure 1: Aerial view with approx. site area indicated
Stage 1 – Screening	Refer to Ground and Water BIA report reference <mark>GWPR2777/GIR/ November</mark> 2019.
Stage 2 – Scoping	Refer to Ground and Water BIA report reference <mark>GWPR2777/GIR/ November</mark> 2019.
Stage 3 – Site Investigation and Study	Refer to Ground and Water BIA report reference <mark>GWPR2777/GIR/ November</mark> 2019.
Stage 4 – Impact Assessment	Refer to Ground and Water BIA report reference GWPR2777/GIR/ November 2019.



1. Site Inve	1. Site Investigation and Desk Study	
	This section identifies the relevant features of the site and its immediate surroundings, providing further scoping where required.	
	Desk Study and Walkover Survey	
	Site & Existing Property	
	The site is located in north-west London area of Hampstead in the Borough of Camden. The site is of a rectangular shape on a light slope of Holly Walk and currently occupied by two blocks of garages.	
	<u>Hardstanding</u>	
	The full area of site is tarmac paved.	
	Figure 2: Holly walk site view	
	Trees and Vegetation	
	Shrubs, but no trees on the site. Some trees on at the adjacent properties although the proposed works are outside of the tree protection areas.	











Drainage Asse	essment	
Hard standing	The hardstanding area will not c tarmac.	hange as the site currently is fully covered in
SUDS Assessment	From review of the existing and proposed hardstanding the increase will be? 0%	
	Percentage Increase < 5% Percentage Increase Between 5% to 10%	No SUDS to be incorporated into scheme

Ground Move	ement Assessment & Predicted Damage Category
	The design and construction methodology aim to limit damage to the existing building on the site, and to the neighbouring buildings, to Category 1 or lower as set out in CIRIA report C760 Table 6.4. For this development, suitable temporary propping during the construction phase will limit the amount of movement due to the basement works. This is described in the Basement Method Statement (appended). The ground movement assessment is contained within Ground and Water BIA report reference GWPR2777/GIR/ November 2019.



Mitigation Measures Ground Movement

A method statement, appended, has been formulated with Croft's experience of over 500 basements completed without error. As mentioned previously, the procedures described in this statement will mitigate the impacts that the construction of the basement will have on nearby properties.

The works must be carried out in accordance with the Party Wall Act and condition surveys will be necessary at the beginning and the end of the works. The Party Wall Approval procedure will reinforce the use of the proposed method statement and, if necessary, require it to be developed in more detail with more stringent requirements than those required at planning stage.

For complete list of mitigation measures refer to Ground and Water BIA report reference GWPR2777/GIR/ November 2019 and ground movement analysis and mitigation measures within.



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Monitoring of	Structures	
	In order to safeguard the existing structu basement construction, movement mor	ures during underpinning and new nitoring is to be undertaken.
Risk	Monitoring Level proposed	Type of Works.
Risk Assessment	Monitoring 4 Visual inspection and production of condition survey by Party Wall Surveyors at the beginning of the works and also at the end of the works. Visual inspection of existing party wall during the works. Inspection of the footing to ensure that the footings are stable and adequate. Vertical monitoring movement by standard optical equipment Lateral movement between walls by laser measurements Before the works begin, a detailed mon the implementation of the monitoring. T • Risk Assessment to determine le • Scope of Works • Applicable standards • Specification for Instrumentatio • Monitoring of movement • Reporting • Trigger Levels using a RED / AMI	Basements up to 4.5m deep in clays itoring report is required to confirm The items that this should cover are: evel of monitoring n BER / GREEN System
	Recommend levels are shown within the	e proposed monitoring statement.



Basement Des Consideration	sign & Construction Impacts and Initial Design
Foundation type	Reinforced concrete cantilevered retaining walls will form the new foundation of the property. The design of the retaining walls was calculated using software by TEDDS. The software is specifically designed for retaining walls and ensures that the construction is kept to a limit to prevent damage to the adjacent property. The overall stability of the walls is designed using K _a & K _p values, while the design of the wall structure uses K ₀ values. This approach minimises the level of movement from the concrete affecting the adjacent properties. The design also considers floatation as a risk. The design has accounted for the weight of the building and the uplift forces from the water. The weight of the building is greater than the uplift, resulting in a stable structure.
Intended use of structure and user requirements	Family/domestic use
Loading Requirements (EC1-1)	UDL kN/m²Concentrated Load kNDomestic Single Dwellings1.52.0
Part A3	Number of Storeys 3
collapse	Is the Building Multi Occupancy? No Class 1 Single occupancy houses not exceeding 4 storeys
Exposure and wind loading conditions	Is the Building Multi Occupancy? No Class 1 Single occupancy houses not exceeding 4 storeys Basic wind speed Vb = 21 m/s to EC1-2 Topography not considered significant.
Exposure and wind loading conditions Stability Design	Is the Building Multi Occupancy? No Class 1 Single occupancy houses not exceeding 4 storeys Basic wind speed Vb = 21 m/s to EC1-2 Topography not considered significant. The cantilevered walls are suitable for carrying the lateral loading applied from above.
Exposure and wind loading conditions Stability Design Lateral Actions	Is the Building Multi Occupancy? No Class 1 Single occupancy houses not exceeding 4 storeys Basic wind speed Vb = 21 m/s to EC1-2 Topography not considered significant. The cantilevered walls are suitable for carrying the lateral loading applied from above. Below ground level, the reinforced concrete retaining walls are designed to carry the lateral loading applied from above.



	retaining walls will be checked for resistance to the overturning force this				
	produces.				
	Lateral forces will be applied from:				
	Soil loads				
	Hydrostatic pressures Sureharge leading from behind the wall				
	Sucharge loading from bening me wai				
	These produce retaining wall thrust. This will be restrained by the opposing retaining wall.				
	Design overall stability to $K_{\alpha} \& K_{\alpha}$ values. Lateral movement necessary to				
Retained soil	achieve K_{α} mobilisation is height/500 (from Tomlinson). This is tighter than the				
Parameters	deflection limits of the concrete wall.				
	Has a soil investigation been carried out? Yes				
Water Table	Refer to Ground and Water BIA report reference GWPR2777/GIR/ November				
	2019.				
	Design temporary condition for water table level, If deeper than basement				
	ignore.				
	Design permanent condition for water table level:				
	If deeper than existing, design reinforcement for water table at full				
	basement depth to allow for local failure of water mains, drainage and				
	storm water. Global uplift forces can be ignored when the water table is				
	lower than the basement. BS8102 only indicates guidance.				
	lower than the basement. BS8102 only indicates guidance.				
	lower than the basement. BS8102 only indicates guidance.				
	lower than the basement. BS8102 only indicates guidance.				



Additional	Surcharge Loading				
loading requirements	The following will be applied as surcharge loads to the front retaining walls:				
	 10kN/m² if within 45° of road 5kN/m² if within 45° of Pavement Garden Surcharge 2.5kN/m² + 1 m of soil (if present above basement ceiling) 20kN/m² Surcharge for adjacent property 1.5kN/m² + 4kN/m² for concrete ground bearing slab 				
	<u>Highways loading:</u> The basement is within 5m of the pavement and within 5m of the public highway.				
	Adjacent Properties: All adjacent property footings within 45° to have additional geotechnical engineer's input. A line at 45° from the base of the neighbours' wall footing would be intersected by the basement retaining wall. This should be accounted for in the design.				
Mitigation Measures - Internal	To mitigate the risks associated with flooding, Croft would recommend the following mitigation measures:				
FIOODING	• A pumping mechanism will be installed for the proposed basement. There is a likelihood that this may fail and allow excess water to accumulate. If this were to occur, the build-up of water would be gradual and noticeable before it becomes a significant life-threatening hazard.				
	• The pumping system should be a dual mechanism to maintain operation in the event of a failure. This should include a battery backup and a suitable alarm system for warning purposes.				
	Route all electrical wiring at high level				
Mitigation Measures - Drainage and	The design of drainage and damp-proofing is not within the scope of this assessment and would not normally be expected to be part of the structural engineer's remit at detailed design stage.				
proofing	A common and anticipated detailed design stage approach is to use internal membranes (Delta or similar). These will be integral to the waterproofing of the basement. Any water from this will enter a drainage channel below the slab. This will be pumped and discharged into the exiting sewer system.				
	It is recommended that a waterproofing specialist is employed to ensure all				



	the water proofing requirements are met. The waterproofing specialist must name their structural waterproofer. The structural waterproofer must inspect the structural details and confirm that he is happy with the robustness. Due to the segmental construction nature of the basement, it is not possible to waterproof the joints. All waterproofing must be made by the waterproofing specialist. He should review the structural engineer's design stage details and advise if water bars and stops are necessary. The waterproofing designer must not assume that the structure is watertiaht.
	 To help reduce water flow through the joints in the segmental pins, the following measures should be applied: All faces should be cleaned of all debris and detritus Faces between pins should be needle hammered to improve key for bonding All pipe work and other penetrations should have puddle flanges or hydrophilic strips
Mitigation Measures - Localised Dewatering	Monitor water levels 1 month prior to starting on site and throughout the construction process. Localised dewatering to pins may be necessary.
Temporary Works	 Walls are designed to be temporarily stable. Temporary propping details will be required for the ground and this must be provided by the contractor. Their details should be forwarded to the design stage engineer. To demonstrate the feasibility of the works, a proposed basement construction method statement is appended. 1. Demolish existing structure 2. Excavate to formation level and prop as required (propping at base and head is recommended) 3. Construct basement and install drainage
	4. Construct above ground structure Prior to construction, temporary propping details will be required. This must be provided by the contractor. Their details should be forwarded to the structural engineer at detailed design stage.



Noise and	The contractor is to follow the good working practices and guidance laid down in the 'Considerate Constructors Scheme'.
Nuisance Control	
	The hours of working will be limited to those allowed; 8am to 5pm Monday to
	None of the practices cause undue noise that one would typically expect
	from a construction site (a conveyor beil typically fons at around 70ab).
	The site will be hoarded with 8' site hoarding to prevent access.
	The hours of working will further be defined within the Party Wall Act.
	The site is to be hoarded to minimise the level of direct noise from the site.
	Working in the basement generally requires hand tools to be used. The level
	of noise generally will be no greater than that of digging of soil. The noise is reduced and muffled by the works being undertaken underground. The
	level of noise from basement construction works is lower than typical ground
	level construction due to this.
СТМР	The council may require a Construction Traffic Management Plan (CTMP) to
	be produced. This is outside the brief of the Basement Impact Assessment
	and is not covered within Croft's brief.
	THENOUNDEDC



Appendix A: Structural Calculations

As part of the building control pack full calculations must be undertaken and provided at detailed design stage once planning permission is granted. The calculations must be completed to a recognised Standard (BS or Euro Codes). The calculations must take into account the findings of this report and the recommendations of the auditors.

The design must resist:

- Vertical loads from the proposed works and adjacent properties
- Lateral loads from wind, soil water and adjacent properties
- Loadings in the temporary condition
- All other applied loads on the building
- Uplift forces from hydrostatic effects and soil heave

The final proposed scheme must:

- Provide stability in the temporary condition to all forces
- Provide stability to all forces in the permanent condition

As part of the planning Croft structural engineers has considered some of the pertinent parts of the basement structure to ensure that it can be constructed. The following calculations are not a full set of calculations for the final design which must be provided for building regulations. The structural calculations we consider pertinent and included in this appendix for this development are:

1. Front basement foundation & retaining wall with highways loading as necessary

2. Party Wall foundation and retaining wall



Ref	nh Hr	olift										
310												
Wa	II DL	35	kN/m					Wall DL	35	kN/m		
	w=	0.35	m									
			soil depth	n abov e=	0	m						
		4		Span=	4.7	m	•		•			
			1									
										Water =	2.85	m
					H =		3.7	m			^	
			Slab Thic	kness =	0.35							
Heel=	0			Slab =	5.1							
-			4	_> ∢_			₩					
			Toe =	♥ 0.3	m						•	
			Toewidth=	1.5	m				soil uni	t weight=	18	kN/m ³
- 2	2				0	-	-					
Uplift Calc		<		R	()	_						
	1			>1	\sim							
Total Dead	Loac	<u> = </u>	Slab=	44.625	kN/m							
1	1-	Тое	and heel =	27.75	kN/m	1	1	T 1 1		A 1		
			Wall =	64.75					K	Δ		
			Soil=(0	+ 💛	~	0) x 2 +	0	= 1 \sL	0	16.835
	200	Total De	ead load =	207.125	kN/m							
<u>Total Uplift</u>	Force)=		153.9	kN/m			f.o.s.=	1.35	No Globo	al Uplift	
	5-1-1				7							

TYPICAL RC RETAINING WALL DESIGN

Loading

Cavity Wall Floor DL (lower & first floor) Roof DL Total Dead Load

Floor LL (lower & first floor) Roof LL Total Live Load $\label{eq:2.1} \begin{array}{l} \text{DLcavity} = 3.98 \text{kN/m}^2 \times 5.5 \text{m} = \textbf{21.890} \text{kN/m} \\ \text{DLfloor} = 2 \times 0.7 \text{kN/m}^2 \times 4.1 \text{m} \ / \ 2 = \textbf{2.870} \text{kN/m} \\ \text{DLroof} = 1.1 \text{kN/m}^2 \times 4.1 \text{m} \ / \ 2 = \textbf{2.255} \text{kN/m} \\ \text{DL} = \text{DLcavity} + \text{DLfloor} + \text{DLroof} = \textbf{27.015} \text{kN/m} \end{array}$

LLfloor = 2×1.5 kN/m² × 4.1m / 2 = **6.150**kN/m LLroof = 0.75kN/m² × 4.1m / 2 = **1.537**kN/m LL = LLfloor + LLroof = **7.687**kN/m

RETAINING WALL ANALYSIS

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.07



Retaining wall details			
Stem type	Cantilever		
Stem height	h _{stem} = 3700 mm		
Stem thickness	t _{stem} = 350 mm		
Angle to rear face of ster	n	α = 90 deg	
Stem density	$\gamma_{stem} = 25 \text{ kN/m}^3$		
Toe length	l _{toe} = 3000 mm		
Base thickness	t _{base} = 350 mm		
Base density	$\gamma_{\text{base}} = 25 \text{ kN/m}^3$		
Height of retained soil	h _{ret} = 3700 mm	Angle of soil surface	$\beta = 0 \deg$
Depth of cover	$d_{cover} = 0 mm$		
Height of water	h _{water} = 2500 mm		
Water density	γ _w = 9.8 kN/m ³		
Retained soil properties			
Soil type	Hard clay		
Moist density	γmr = 20 kN/m ³		
Saturated density	γ _{sr} = 20 kN/m ³		
Base soil properties			
Soil type	Hard clay		
Soil density	γ _b = 20 kN/m ³		
Loading details	CTDU	OTUD	
Variable surcharge load	Surcharge _Q = 10 kN/m ²		
Vertical line load at 3150	mm	P _{G1} = 27 kN/m	
X F - V	P _{Q1} = 7.7 kN/m		
ALL THE			



	◀─────3000────	→ 350 →	
	◄ 3150	►	
▲ 360+ ▲ 1700	Prop		V/m ² 75. % kN/m ²
calculate retaining wall g	7 kN/m ²	ET CTUR	
Base length	lbase = 3350 mm		
Saturated soil height	h _{sat} = 2500 mm		
Moist soil height	h _{moist} = 1200 mm		
Length of surcharge load	l _{sur} = 0 mm		
Vertical distance	x _{sur v} = 3350 mm		
Effective height of wall	h _{eff} = 4050 mm		
Horizontal distance	x _{sur h} = 2025 mm		
Area of wall stem	A _{stem} = 1.295 m ²	Vertical distance	x _{stem} = 3175 mm
Area of wall base	A _{base} = 1.173 m ²	Vertical distance	x _{base} = 1675 mm
Design approach 1			
Partial factors on actions	- Table A.3 - Combination	1	
Partial factor set	A1		
Permanent unfavourable	action	γ _G = 1.35	Permanent
favourable action	γ _{Gf} = 1.00		
Variable unfavourable ac	ction	γ _Q = 1.50	Variable
favourable action	$\gamma_{Qf} = 0.00$		
Partial factors for soil para	imeters – Table A.4 - Com	bination 1	
Soil parameter set	M1		



Angle of shearing resistan	се	$\gamma_{\phi'} = 1.00$	Effective
cohesion	γ _{C'} = 1.00		
Weight density	$\gamma_{\gamma} = 1.00$		
Poteinod coil proportion		Library ife	m Pamai factors summary
Design maist density	$y_{m}' = 20 k N / m^3$	Design saturated density	$y'' = 20 \text{kN}/m^3$
Dess seil properties		Design saterated density	
Design soil density	$m' = 20 \ (h) \ (m^3)$		
	γ6 - 20 κιν/Πο		
Soil coefficients		K	
Coeff.friction to back of v		$K_{\rm fr} = 0.325$	Cooff triation
bonogth base		$N_{\rm fb} = 0.325$	Coentinction
Active pressure coefficier	Nfbb - 0.325	$K_{\star} = 0.440$	
coefficient	[™] K⊳ = 2 280	NA - 0.440	
Overturning check			
Vertical forces on wall			-
Ioidi	Γ total_v – Γ stem + Γ base + ΓP_v +	- Fwater_v - Fwater_u - 88.7 KIN/I	T1
Horizontal forces on wall			
lotal	$F_{total_h} = F_{sur_h} + F_{sat_h} + F_{wate}$	r_h + Fmoist_h + Fexc_h = 151.5 k	(N/M
Overturning moments on	wall	OTUD	A. 1
Total	$M_{total_OT} = M_{sur_OT} + M_{sat_OT} -$	+ Mwater_OT + Mmoist_OT = 214. 3	s kNm/m
Restoring moments on wo			
Total	$M_{total_R} = M_{stem_R} + M_{base_R} + M_{base_R}$	+ M _{P_R} = 237 kNm/m	
Check stability against ov	verturning		
Factor of safety	FoS _{ot} = 1.106		
	PASS - Maximum restoring	moment is greater than ov	erturning moment
Bearing pressure check			
Vertical forces on wall			
Total	$F_{total_v} = F_{stem} + F_{base} + F_{P_v} +$	- F _{water_v} = 131.3 kN/m	
Horizontal forces on wall			
Total	$F_{total_h} = F_{sur_h} + F_{sat_h} + F_{wate}$	er_h + F _{moist_h} + F _{pass_h} = 151.5	kN/m
Moments on wall			
Total	$M_{total} = M_{stem} + M_{base} + M_{su}$	r + MP + Msat + Mwater + Mmc	_{ist} = 142 kNm/m
Check bearing pressure			
Propping force	F _{prop_base} = 151.5 kN/m		
Bearing pressure at toe	$q_{toe} = 60.7 \text{ kN/m}^2$	Bearing pressure at heel	$q_{heel} = 0 \text{ kN/m}^2$
Factor of safety	FoS _{bp} = 1.648		
PASS - All	owable bearing pressure e	exceeds maximum applied	l bearing pressure



Design approach 1

-			
Partial factors on actions	- Table A.3 - Combination	2	
Partial factor set	A2		
Permanent untavourable	action	γ _G = 1.00	Permanent
tavourable action	γ _{Gf} = 1.00		
Variable unfavourable ac	ction	γ _Q = 1.30	Variable
favourable action	γ _{Qf} = 0.00		
Partial factors for soil parc	ameters – Table A.4 - Comb	bination 2	
Soil parameter set	M2		
Angle of shearing resistan	ice	$\gamma_{\phi'} = 1.25$	Effective
cohesion	γ _{C'} = 1.25		
Weight density	$\gamma_{\gamma} = 1.00$		
		Library ite	m Partial factors summary
Retained soil properties			
Design moist density	γ _{mr} ' = 20 kN/m ³	Design saturated density	$\gamma_{\rm sr}' = 20 \ {\rm kN}/{\rm m}^3$
Base soil properties			
Design soil density	γ _b ' = 20 kN/m ³		
Soil coefficients			
Coeff.friction to back of v	vall	K _{fr} = 0.325	
Coeff.friction to front of w	vall	K _{fb} = 0.325	Coeff.friction
beneath base	K _{fbb} = 0.325	CTUD	
Active pressure coefficier	nt 🔵 📉 🖯	K _A = 0.440	Passive pressure
coefficient	K _P = 2.280	• · • · ·	
Overturning check			
Vertical forces on wall			
Total	$F_{total_v} = F_{stem} + F_{base} + F_{P_v}$	+ F _{water_v} - F _{water_U} = 88.7 kN/r	n
Horizontal forces on wall			
Total	Ftotal h = Fsur h + Fsat h + Fwate	er h + Fmoist h + Fexc h = 114.9 k	κN/m
Overturning memorie on	wall		
	$\Delta A_{\rm total} = \Delta A_{\rm total$	$+ \lambda \Lambda_{\rm max} + \lambda \Lambda_{\rm max} + \alpha - 165 \mu$	kNm/m
Restoring moments on wo	11		
Iotal	$M_{total_R} = M_{stem_R} + M_{base_R}$	+ M _{P_R} = 237 kNm/m	
Check stability against ov	verturning		
Factor of safety	FoS _{ot} = 1.432		
	PASS - Maximum restoring	moment is greater than ov	erturning moment
Bearing pressure check			
Vertical forces on wall			
Total	$F_{total_v} = F_{stem} + F_{base} + F_{P_v}$	+ F _{water_v} = 98.7 kN/m	
Horizontal forces on wall			
Total	$F_{total h} = F_{sur h} + F_{sat h} + F_{wate}$	er h + Fmoist h + Fpass h = 114.9	kN/m
-			-



Moments on wall

Total

 $M_{\text{total}} = M_{\text{stem}} + M_{\text{base}} + M_{\text{sur}} + M_{P} + M_{\text{sat}} + M_{\text{water}} + M_{\text{moist}} = 102.9 \text{ kNm/m}$

Check bearing pressure

Bearing pressure at toe	q _{toe} = 47.3 kN/m ²	Bearing pressure at heel	$q_{heel} = 0 \text{ kN/m}^2$
Factor of safety	FoS _{bp} = 2.113		
DACC AL	llowable bearing pressure.	avaa da maximuma analiaa	I bearing process

PASS - Allowable bearing pressure exceeds maximum applied bearing pressure

RETAINING WALL DESIGN

In accordance with EN1992-1-1:2004 incorporating Corrigendum dated January 2008 and the UK National Annex incorporating National Amendment No.1

Tedds calculation version 2.9.07

Concrete details - Table 3.1 - Strength and deformation characteristics for concrete

Concrete strength class	C32/40		
Char.comp.cylinder stren	igth	f _{ck} = 32 N/mm ²	Mean axial
tensile strength	f _{ctm} = 3.0 N/mm ²		
Secant modulus of elastic	city	E _{cm} = 33346 N/mm ²	Maximum
aggregate size	h _{agg} = 20 mm		
Design comp.concrete st	rength	f _{cd} = 18.1 N/mm ²	Partial factor γ_{C}
= 1.50			
Reinforcement details	ATDI	OTUD	
Characteristic yield streng	gth	f _{yk} = 500 N/mm ²	Modulus of
elasticity	E _s = 200000 N/mm ²	CIUN	
Design yield strength	f _{yd} = 435 N/mm ²	Partial factor	γs = 1.15
Cover to reinforcement			
Front face of stem	C _{sf} = 40 mm	Rear face of stem	c _{sr} = 50 mm
Top face of base	Cbt = 50 mm	Bottom face of base	C _{bb} = 75 mm









Library item: Rectangular single summary



Deflection control - Sec	tion 7.4		
Limiting span to depth r	atio	13.3	Actual span to
depth ratio	12.7		
	PASS - Spant	o depth ratio is less than def	lection control limit
Crack control - Section	7.3		
Limiting crack width	w _{max} = 0.3 mm	Maximum crack width	w _k = 0.191 mm
PASS - Maximum o	crack width is less than lin	niting crack widthRectangulo	ar section in shear -
Section 6.2			
Design shear force	V = 128.9 kN/m	Design shear resistance	V _{Rd.c} = 179.5
kN/m			
	PASS - Desig	gn shear resistance exceeds	design shear force
Horizontal reinforcemer	nt parallel to face of stem	- Section 9.6	
Min.area of reinforceme	entA _{sx.req} = 503 mm²/m	Max.spacing of reinforce	ement s _{sx_max} =
Trans reinforcement pro	wided	12 dia bars @ 200 c/c	
	Trans reinforcement or	ovided	$A_{syprov} = 565$
mm²/m			
PASS - Area	of reinforcement provided	d is areater than area of reini	orcement required
Check have design at t			
Check base design and	b = 250 mm		
Depin of section	11 – 350 11111	I OTI ID	
Rectangular section in f	lexure - Section 6.1		
Design bending momer	nt M = 198.8 kNm/m	K = 0.088	K' = 0.207
Contras-	κ'	> K - No compression reinfor	cement is required
Tens.reinforcement requ	uired	Abb.req = 1886 mm ² /m	
lens.reinforcement prov		20 dia.bars @ 100 c/c	
<u> </u>	lens.reinforcement pro	DVIDED	Abb.prov = 3142
mm²/m			
Min.area of reinforceme	entA _{bb.min} = 417 mm²/m	Max.area of reinforceme	ent A _{bb.max} =
14000 mm²/m			
PASS - Area	of reinforcement provided	a is greater than area of reini	
Crack control - Section	73		
Limiting crack width		Maximum crack width	$w_{t} = 0.259 \text{ mm}$
PASS - Maximum (crack width is less than lin	niting crack widthRectangula	r section in shear -
Section 6.2			
Design shear force	V = 95 8 kN/m	Desian shear resistance	V _{Pd c} = 199 7
kN/m		Designation resistance	VRU.C IOUN
	PASS - Desic	an shear resistance exceeds	desian shear force
Secondary - Increase		alian 0.2	
secondary transverse re			
Min.area of reinforceme	$EntAbx.req = 628 \text{ mm}^2/\text{m}$	max.spacing of reinforce	ement s _{bx_max} =
450 mm			



Trans.reinforcement provided

12 dia.bars @ 175 c/c

Trans.reinforcement provided

 $A_{bx.prov} = 646$

mm²/m

PASS - Area of reinforcement provided is greater than area of reinforcement required





Appendix B: Construction Programme

The Contractor is responsible for the final construction programme

Outline cor	Outline construction Program															
(For planning p	urpos	es on	ly)													
								Мо	nths							
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Planning																
approval																
Derailed																
Design																
Tender																
Party Walls																
Monitoring of																
Adjacent	1			1			1									
structures	A					N										
Enabling works	1				_											
Basement	0			0		-										
Construction			\geq					17								
Superstructure				Ċ				11								
construction			4													
	1				1	1.1	0	10			1.17		5/	1		
					- 1				1.5							



Appendix C: Structural Drawings

1:50 Basement Plan on A3 Showing Neighbouring basements if present1:50 Ground Floor plan on A3 Showing Neighbouring property1:50 Section on A3 Including section through Neighbouring Footings



CROFT STRUCTURAL ENGINEERS







Upper Ground Floor Plan Scale 1:50



Issued for Planning

3	07/08/2019	Alteration to plans to final		
2	24/07/2019	Alteration to plans		
1	08/10/2018	Basement plan added		
-	03/10/2018	First issue for comment		
Rev	Date	Amendments	\bigcirc	
Str En Client Projec Title :	Croft Structural DagineersClockshop Mews, r/o 60 Saxon Rd, London, SE25 5EH. 020 8684 4744 www.croftse.co.ukDubberClient: Mr. Alan HarariClient: Mr. Alan HarariProject: Holly WalkTitle : Structural Plans			
1806	18 Draw	pr as shown		
Dwg Numbe	or Rev	3 Date October '18	3	





Issued for Planning

3	07/08/2019	Alteration to plans to final architectural plans			
2	24/07/2019	Section updated			
1	08/10/2018	Section updated			
-	18/07/2018	First issue for comment			
Rev	Date	Amendments			
Clockshop Mews, r/o 60 Saxon Rd, London, SE25 5EH. 020 8684 4744 www.croftse.co.uk					
Client	: Mr. Ala	an Harari			
Proje	Project: Holly Walk				
Title : Existing and Proposed section 2-2					
Job Numbe	r Drawi	pr as shown			
Dwg Numbe	er Rev	3 Date October '18			



4	07/11/2019	Monitoring points a Holly Walk paveme
3	16/09/2019	Alterations follow Assessment repo
2	07/08/2019	Alteration to pla architectural pla
1	24/07/2019	Monitoring poin layout altered
-	21/12/2018	First issue for c
Rev	Date	Amendments

I			

G8

added along nent and highway Job Number Date Date Dec '18 Client: Mr. Alan Harari Croft Structural Engineers Clockshop Mews, r/o 60 Saxon Rd, London, SE25 5EH. 020 8684 4744 www.croftse.co.uk ing Movement ort Project: Holly Walk Dwg Number SD-22 lans to final 4 ans Title : Monitoring Plan Ch'kd Drawn nts added, pr Scale As shown comment @ A2



Appendix D: Utilities Search

https://www.linesearchbeforeudig.co.uk/website does a wide search of all utilities possibly present within the site and highlight only the ones present within the vicinity. In case of 16 Frognal Gardens electricity cables are the only utility present within site.

The desktop utility search was completed through <u>https://www.linesearchbeforeudig.co.uk/</u> and discovered that UK Power Networks have assets registered within the vicinity of the site.

UK Power Networks provided the following;

- 1. Maps of the area showing the electrical lines and/or electrical plant.
- 2. London Symbol Guide
- 3. Think before you dig guide



CROFT STRUCTURAL ENGINEERS



Registered Office: Newington House 237 Southwark Bridge Road London SE1 6NP Company: UK Power Networks (Operations) Limited

Registered in England and Wales No: 3870728

Our Ref: 14446595 Your Ref: 16 Frognal Gardens

Tuesday, 18 December 2018

Pawel Rogalewicz Clockshop Mews Clockshop Mews rear of 60 Saxon road london London E255EH

Dear Pawel Rogalewicz

Thank you for contacting us regarding UK Power Networks equipment at the above site. I have enclosed a copy of our records which show the electrical lines and/or electrical plant. I hope you find the information useful.

I have also enclosed a fact sheet which contains important information regarding the use of our plans and working around our equipment. Safety around our equipment is our number one priority so please ensure you have completed all workplace risk assessments before you begin any works.

Should your excavation affect our Extra High Voltage equipment (6.6 KV, 22 KV, 33 KV or 132 KV), please contact us to obtain a copy of the primary route drawings and associated cross sections.

If you have any further queries do not hesitate to contact us.

Plan Provision 0800 056 5866









Registered Office: Newington House 237 Southwark Bridge Road London SE1 6NP

Registered in England and Wales No: 3870728

This information is made available to you on the terms set out below. If you do not accept the terms of use set out in this fact sheet please do not use the plans and return them to UK Power Networks.

- 1. UK Power Networks does not warrant that the information provided to you is correct. You rely upon it at your own risk.
- 2. UK Power Networks does not exclude or limit its liability if it causes the death of any person or causes personal injury to a person where such death or personal injury is caused by its negligence.
- 3. Subject to paragraph 2 UK Power Networks has no liability to you in contract, in tort (including negligence), for breach of statutory duty or otherwise how for any loss, damage, costs, claims, demands, or expenses that you or any third party may suffer or incur as a result of using the information provided whether for physical damage to property or for any economic loss (including without limitation loss of profit, loss of opportunity, loss of savings, loss of goodwill, loss of business, loss of use) or any special or consequential loss or damage whatsoever.
- 4. The information about UK Power Networks electrical plant and/or electric lines provided to you belongs to and remains the property of UK Power Networks. You must not alter it in any respect.
- 5. The information provided to you about the electrical plant and/or electric lines depicted on the plans may NOT be a complete record of such apparatus belonging to UK Power Networks. The information provided relates to electric lines and/or electrical plant belonging to UK Power Networks that it believes to be present but the plans are not definitive: other electric lines and/or electrical plant may be present and that may or may not belong to UK Power Networks.
- 6. Other apparatus not belonging to UK Power Networks is not shown on the plan. It is your responsibility to make your own enquiries elsewhere to discover whether apparatus belonging to others is present. It would be prudent to assume that other apparatus is present.
- 7. You are responsible for ensuring that the information made available to you is passed to those acting on your behalf and that all such persons are made aware of the contents of this letter.
- 8. Because the information provided to you may not be accurate, you are recommended to ascertain the presence of UK Power Networks electric lines and/or electrical plant by the digging of trial holes. Trial holes should be dug by hand only.

Excavations must be carried out in line with the Health and Safety Executive guidance document HSG 47. We will not undertake this work. A copy of HSG 47 can be obtained from the Health and Safety Executives website.

All electric lines discovered must be considered LIVE and DANGEROUS at all times and must not be cut, resited, suspended, bent or interfered with unless specially authorised by UK Power Networks.

The electric line and electrical plant belonging to UK Power Networks remains so even when made dead and abandoned and any such electric line and/or electrical plant exposed shall be reported to UK Power Networks.

Where your works are likely to affect our electric lines and/or electrical plant an estimate of the price of any protective /diversionary works can be prepared by UK Power Networks Branch at Metropolitan House, Darkes Lane, Potters Bar, Herts. , EN6 1AG, telephone no. 0845 2340040



2|3



Registered Office: Newington House 237 Southwark Bridge Road London SE1 6NP

Registered in England and Wales No: 3870728

9 Any work near to any overhead electricity lines must be carried out by you in accordance with the Health and Safety Executive guidance document GS6 and the Electricity at Work Regulations.

The GS6 Recommendations may be purchased from HSE Books or downloaded from the Energy Networks Association's website.

If given a reasonable period of prior notice UK Power Networks will attend on site without charge to advise how and where "goal posts" should be erected. If you wish to use this service, in the first instance please telephone: 0845 6014516 between 08:30 and 17:00 Monday to Friday.

- 10. You are responsible for the security of the information provided to you. It must not be given, sold or made available upon payment of a fee to a third party.
- 11. If in carrying out work on land in, on, under or over which is installed an electric line and/or electrical plant that belongs to UK Power Networks you and/or anyone working on your behalf damages (however slightly) that apparatus you must inform immediately UK Power Networks by our emergency 24 hour three digit telephone number **105** providing;
 - your name, address and telephone number;
 - the date, time and place at which such damage was caused;
 - a description of the electric line and/or electrical plant to which damage was caused;
 - the name of the person whom it appears to you is responsible for that damage;
 - the nature of the damage.
- 12. The expression "UK Power Networks" includes UK Power Networks (EPN) plc, UK Power Networks (LPN) plc, UK Power Networks (SEPN) plc, UK Power Networks and any of their successors and predecessors in title.









FOR DETAILS OF ENLARGEMENTS SEE

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No starting the start st _____ · ÷. • • (J16) TQ 2685 SW-N (J16) TQ 2685 SW-N (J16) TQ 2685 SW-N (J16) TQ 2685 SW-N (J16) TQ 2685 SW-N











Network Records NetMAP Symbols Booklet - London

This symbol booklet is intended as a general guide only - some local variations of these symbols may be found.

Version 1.2

Released October 2010

Always check with your local Network Records office or the UK Power Networks server to ensure that you are using the most up to date copy of this booklet.Tel: 08000 565866

Index:-

Page no:

Contents:

Guidance notes. 1 2 The area covered by this guide. 3 Scenerv. Scenery (UK Power Networks use only-boxed red) 4 7 Primary distribution cables (EHV). 8 Secondary distribution cables (LV/HV). 9 Cable terminology. 10 Cable size abbreviations. 11 Cable ducts. Other NetMAP symbols. 12 15 Services. 17 Symbols used in cross sections. Abbreviations used in cross sections. 19 20 Typical plan and cross section representations: All areas: NetMAP/vector. All areas: composite raster style 1. Ex-Western area and Holborn: main and wavs. The City of London: single line. Finsbury and Shoreditch: multi-single line style 1. Ex-North Eastern area: HV/LV. Ex-North Eastern area: multi-single line style 2. Ex-North Eastern area: composite raster style 2. Regional NetMAP anomalies - general overview. 23 24 Region 1: ex-Western area. 25 Region 2: ex-Northern area. 27 Region 3: ex-North eastern area. 29 Region 4: ex-South Eastern area. 30 Region 5: ex Southern area.

Guidance notes.

Important notice:

If you do not understand the NetMAP record that you are using, please contact UK Power Networks Network Records for guidance **Tel: 08000 565866.**

- The position of apparatus shown on NetMAP is believed to be correct, but the original landmarks may have altered since the apparatus was installed.
- It must be assumed that there is at least one service to each property, lamp column, street sign etc. A separate record may be available.
- When excavations are to be carried out near Extra High Voltage (EHV) cables, further details must be obtained before commencement of work.
- Third party cables are not usually shown.
- When two or more maps are supplied for the same area, the maps must be read in conjunction with each other and with this symbol booklet.
- All LV cables are assumed to be 4 core, and all HV cables assumed to be 3 core unless otherwise stated.
- All Imperial cable sizes are assumed to be copper and all metric cable sizes are assumed to be aluminium unless otherwise stated.



Plan Provision Team Fore Hamlet Ipswich Suffolk IP3 8AA Tel: 08000 565866



Please see the anomalies map at the end of this safety booklet for greater map area detail, and a breakdown of the more significant anomalies within the London area.



Scenery for UK Power Networks use only - boxed in red					
NetMAP system	Scanned image	Description			
Inset Network – Contact xxxx IDNO for further information	Not applicable	Area of inset network - not the asset of UK Power Networks (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Proposed Cross Rail route (only visible to of UK Power Networks and their immediate contractors)			
	Not applicable	High pressure pipelines in the general vicinity (only visible to of UK Power Networks and their immediate contractors)			
Note: Pipelines are only viewable on NetMAP by UK Power Networks staff and their immediate contractors. Do not carry out any excavation without consent from the relevant agency - legally protected high pressure petroleum products pipeline route in the general vicinity - consult www.linewatch.co.uk for contacts and guidance. Pipeline contact numbers can also be found on the intranet – out of hours, contact our Control Centre.					
	Not applicable	Water - surface water (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Water - Source Protection Zone 1 (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Water - Source Protection Zone 2 (only visible to UK Power Networks and their immediate contractors)			
\checkmark	Not applicable	Water - Source Protection Zone 3 (only visible to UK Power Networks and their immediate contractors)			
sectio	n continued on nex	t page			

Scenery for UK Power Networks use only - boxed in red					
NetMAP system	Scanned image	Description			
	Not applicable	Historical - Scheduled Monuments (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Historical - Parks and Gardens (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Historical - Areas of Archaeological Potential (AAP) (only visible to UK Power Networks and their Immediate contractors)			
	Not applicable	Nature - Ramsar Wetlands of International Importance (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Nature - Special Area of Conservation (SAC) (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Nature - Special Protected Area (SPA) (only visible UK Power Networks and their immediate contractors)			
	Not applicable	Nature - Site of Special and Scientific Interest (SSSI) (only visible to UK Power Networks and their immediate contractors)			

Scenery for UK Power Networks use only - boxed in red					
NetMAP system	Scanned image	Description			
	Not applicable	Nature - Local Nature Reserve (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Nature - National Nature Reserve (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Nature - Area of Outstanding Natural Beauty (AONB) (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Nature - National Park (only visible to UK Power Networks and their immediate contractors)			
—	Not applicable	Fluid filled cables - very high sensitivity (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Fluid filled cables - high sensitivity (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Fluid filled cables - medium sensitivity (only visible to UK Power Networks and their immediate contractors)			
	Not applicable	Fluid filled cables - low sensitivity (only visible to UK Power Networks and their immediate contractors)			

Primary distribution cables					
NetMAP system	Scanned image	Description			
EHY CABLE Solid BHY CABLE Gas BHY CABLE Oil Cable stop Cable stop Shallow	—— BHV Coble Roote 259 —— Not applicable —— s—— s—— s—— s——	UK Power Networks route (11,000 , 22,000 to 132,000 volts) Oil/gas cable stop Part of UK Power Networks cable route where cover is less than normal			
		less than normal			

Secur	ndary distributio	on cables
NetMAP system	Scanned image	Description
(20kV) (11kV) (6.6kV)	.3 (AL) ³ /c .15 ³ /c .3 (AL) ³ /c .3 (AL) ³ /c .15 ⁴ /c .3 (AL) ³ /c .185 ⁴ /c .0225 ¹ /c Not applicable	HV cable (up to 20kV) 3 phase LV cable (230V or 400/230V) 1 or 2 phase LV cable (230V or 400/230V) Pilot or Telephone cable, often not shown in plan if running with other cables Fibre-optic cable Earth cable HV or LV cable in duct Duct route(s) not containing live cables

Cable terminology					
NetMAP system	Scanned image	Description			
PL PLS PLST or PLSW PLSTS PLSWS PLSW PLS PLST or PLSW PLST PLST PLST PLST PLST PLSW AI Cu WV CS PVC EPR XLPE SOL ax cx	PL PLS PLA PLTS PLDT PLWS PLBW LC & H LC & A LC & BA DSTA STA SWA AI Cu WV CS PVC EPR XLPE SOLIDAL TRIPLEX TRIPLEX	Paper Lead Paper Lead Served Paper Lead Served Paper Lead Steel Tape Served Paper Lead Steel Wire Served Paper Lead Steel Wire Served Paper Lead Bright Wire Lead Covered & Hessian Lead Covered & Armoured Lead Covered & Armoured Dauble steel tape armoured Steel Tape Armoured Steel Wire Armoured Aluminium Copper Waveconal Consac Polyvinyl Chloride Ethylene Propylene Rubber Cross Linked Polyethylene Solid Aluminium Triplex (copper)			

Cable size abbreviations				
NetMAP system	Scanned image	Description		
1c c/c t/c 4c 3c CNE	% % № ог Т/сс % % (см)	Single core. Concentric cores Triple concentric cores Four cores Three cores and concentric neutral — not of the Waveconal type		
2c s/c 3c DC P Pr	光 (or Tw) 光 禿 DC P Pr	Two cores (or twin) Split concentric cores Three cores Direct current Pilot Number of telephone pairs		



Other NetMAP symbols				
NetMAP system	Scanned image	Description		
<u>0.3 4c AL PLSWS</u> (Details also in cable attributes and/or section)	3 % (59)	Cable size (and year laid)		
4		Cable capped end		
.	SE PE	Cable pressure (or pot) end or signal end		
	── - * ^E ──→ ^E	Pressure/pot end & earth cable/electrode		
 • {	+ +- E	Earth rod (vertical) Earth rod (horizontal) Earth plate Earth plate or end		
		Bottle or trouser joint or combined crutch & pressure end - (CPE)		
MAIN SERV		Straight joints		
<u> </u>		Tee joints		
<u> </u>		Crutch (or spur) joints (CJ) straight & crutch joints combined (S&CJ)		
→ =		Double crutch (or spur) joint		
UT (Disconnected universal tee)		Sleeve		
section continued on next page				









Symbols used in cross sections				
NetMAP syste	m Scanned image	Description		
•	• •	Cable laid direct		
۲	۵ ۵	Cable laid in duct		
⊗	$\oslash \otimes$	Blocked duct (sometimes used for unidentified cables)		
0	$\circ \circ$	Single earthenware duct		
◯ 2¥" S	0	Single steel pipe		
		Square cable duct		
80	88	Group of circular ducts		
83	88	Group of circular ducts (Sykes)		
		Group of square ducts (Doulton)		
(ᄃᠣᠳᠮ᠐᠊ᠸ	Cable trough		
	000	Bitumen casing (Crompton)		
	<u>.</u>	Bitumen filled iron trough (Trunks)		
\otimes		Bitumen casing (Tri-case)		
section continued on next page				

Symbols used in cross sections NetMAP system Scanned image Description Protective slab Tiles. \frown Concrete slabs Steel plate Plastic tile tape — т/т Timber Timber 👝 777

Abbreviations used in cross sections				
NetMAP system	Scanned image	Description		
EW F A P S C WI F PRD Left blank — means NR E.V T/T N/A N/A-destination now only shown in cable attribute	E.W.D(s) or EW. F.P or F or F.D ASB or A P S.P or S C.I or C or C.I.P W.I F or F.D PRD D.N.K or D:NR N.R or N.R E.V.P or E.V T/T 3/62 or NOV 79 ABCD etc Please note: Ducts are assumed f - unless otherwise s	Earthenware ducts Fibre duct Asbestos Plastic or pitch fibre Steel Cast iron Wrought iron pipe Fibre duct Plastic Rigiduct Depth not known No record Everite pipe Tape Tile Date cable laid HV cable destination (See section sheet HV ref) to be 4"/100mm earthenware tated		







Regional NetMAP Anomalies - general overview:

The following pages explain the various major map style anomalies found within the London area. These styles are a legacy from the five individual London Electricity areas which were again formed from seventeen separately organised LEB districts. Areas with significant anomalies are shown in the following pages as cross-hatched areas. Areas with standard composite vector and raster layer information are shown as un-hatched areas.

<u>Cautionary note</u>: - any region or sub-region, either shaded or un-shaded, may contain some local anomalies not mentioned in the following pages – if in doubt, please contact the UK Power Networks Plan Provision team on telephone number 08701 963797.

All regions (1-5) will contain recently created composite vector (NetMAP/AutoCAD) data.

Recent work created using the NetMAP system and previously created using the AutoCAD system (as opposed to raster/scanned data) are recorded in the composite vector style shown on the UK Power Networks London area symbol sheet - see the first example on page 18 of this document. Recent data will be indicated by the existence of multi-coloured cables on the NetMAP system, but this may not be reflected on printed matter produced with a black and white printer. AutoCAD data looks similar to the coloured NetMAP data, but does not hold any cable 'attributes' when selected using the NetMAP system. These cables will be represented individually (multi-line representation). New NetMAP cross sections may be accessed electronically on the NetMAP system and are presented in printed format accompanied by a seven digit NetMAP identification number.



Region 1 ex-Western area

This region includes Westminster, Kensington, Chelsea, Hammersmith and Fulham. The region is covered by two map layer systems – **region 1(a)** mains and ways dual layer raster, and **region 1(b)** composite raster. The following explains this in greater detail.

Region 1(a) (hatched)

Mains and ways representation:

This system consists of two maps layers for the same area.

- i) The mains map shows all cable routes.
- ii) The ways map shows pipe and duct routes with cross sections.

There are some enlargement sheets, cross sections and jointing details. EHV routes are shown on either the mains or the ways map.

It is important that all these maps are read in conjunction with each other.

Caution: - It is also important to note that the kerb line detail on these maps is a dash/dot line, which on the majority of UK Power Networks Central (London) records would refer to an HV cable route. HV cables are shown as a solid line when laid direct and a dashed line when in a duct.



enlargement sheets in the Aberdeen Place area. Please note that the kerb line is shown as a dotted line and HV cables are shown as dash/dot lines.

Region 2 ex-Northern area

This region includes Islington, Hackney, the City of London and parts of Brent, Camden and Ealing. The region is covered by four map layer systems - **Region 2(a)** - mains and ways dual layer raster (Holborn area), **Region 2(b)** - single line representation (City of London), **Region 2(c)** - multi-single line representation (Finsbury and Shoreditch) and **Region 2(d)** - composite multi-line maps (all other areas). This following explains this in greater detail.

Region 2(a) (hatched)

Covers part of WC1 and WC2 (Holborn).

Mains and ways representation:

This system consists of two maps layers for the same area.

- i) The mains map shows all cable routes.
- ii) The ways map shows pipe and duct routes with cross sections.

Where needed, extra sheets have been added for enlargements, cross sections and jointing details. EHV routes are shown on the mains map layer.

It is important that all these maps are read in conjunction with each other.

Caution: - It is also important to note that the kerb line detail on these maps is a dash/dot line, which on the majority of UK Power Networks Central (London) records would refer to an HV cable route. HV cables are shown as a solid line when laid direct and a dashed line when in a duct.



Region 2(b) (hatched

Covers parts of postal areas EC1, EC2 and all of postal areas EC3 and EC4.

Single line representation maps:

Whenever possible, all the information is on one map layer .One line can represent any number of cables or ducts. It is therefore very important to use cross sections. In some cross sections details may be written and not drawn. In complex and redrawn areas, some detail may be drawn using multi-line representation. There are some enlargement sheets.

Region 2(c) (hatched 1)



Multi-single line representation (style 1) maps:

Whenever possible, all the information is on one map layer. When cables lay immediately above/below each other, it is shown as a single line. For example if six cables lay three on three, only three lines would indicate the six cables. If the cables were laid flat, six separate lines would be shown. It is therefore important not to assume that the lines drawn indicate the number of cables, at any point. **Cross sections must be used.**



Region 2(d) (un-hatched)

Covers all other postal areas in this region

Composite single layer (style 1) maps:

Whenever possible, all the information is on one map layer. There are some enlargement sheets.

Region 3 ex-North Eastern area

This region includes Tower Hamlets, Newham, Redbridge, Waltham Forest, Loughton (Epping) and Barking and Dagenham. This region is covered by three mapping systems.

Region 3(a) (hatched

Separate HV and LV representation maps:

This system consists of two maps layers for the same area.

- i) The HV map layer showing HV cables and duct routes.
- ii) The LV map layer showing LV cables and duct routes.

Cross sections for both HV and LV cable routes are shown on a separate sheet. EHV cable routes are shown on the HV map layer.

It is important that all these maps are read in conjunction with each other.



Region 3(b) (hatched



A combination of composite single layer (style 1) and multi-single line (style 2):

Whenever possible, all the information is on one map layer. There are some enlargement sheets. There is a combination of map styles used in this area. Some areas may be conventional multi-line line representation with many areas of multisingle line representation. In the multi-line areas each (live) cable is shown individually in plan. In the multi-single line map areas, there is a single line for each voltage type, with a single HV line and a single LV line representing more than one cable run of each voltage (when applicable). Therefore a cable run containing three HV cable and four LV cables will be represented by one HV line and one LV line.



Region 3(c) (hatched

A combination of composite single layer (style 2) and multi-single line (style 2):

Whenever possible, all the information is on one map layer. There are some enlargement sheets. In this area (postal code areas E1, E2, E3, E14 and part of E9), the cross sections are listed under each road name. It is therefore extremely important that you have the correct cross sections for the road you are working in.

There is a combination of map styles used in this area. Most areas are composite single layer (style 2) with some areas of multi-single line representation, as described in region 3(b).

Region 4 ex-South Eastern area

This region includes Lewisham, Greenwich, Bromley, Bexley and Dartford. Nearly all maps are drawn in one style – single layer composite raster/vector.

Region 4 (un-hatched)

Composite single layer (style 1) with a small number of mains and ways representation maps :

Mainly composite maps - whenever possible, all the information is on one map layer. There are some enlargement and cross section sheets. Some maps do not show single phase services unless they are long and deviating. There are however some maps drawn using the mains and ways style. These are rare, but please be aware that they exist.



Region 5 ex-Southern area

This region includes Southwark, Lambeth, Wandsworth, Merton, Kingston upon Thames and Richmond upon Thames. All maps are drawn to one style - single layer composite raster/vector.

Region 5 (un-hatched)

Composite single layer (style 1) maps:

Composite maps - whenever possible, all the information is on one map layer. There are some enlargement and cross section sheets. A small number of maps may not show services.







Cross Section



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served. Plans generated by DigSAFE Pro^{to}software provided by LinesearchbeforeUdig



Appendix E: Basement Method Statement



CROFT STRUCTURAL ENGINEERS



16 Frognal Gardens

1. Preamble

- 1.1. This method statement provides an approach that will allow the basement design to be correctly considered during construction. The statement also contains proposals for the temporary support to be provided during the works. The Contractor is responsible for the works on site and the final temporary works methodology and design on this site and any adjacent sites.
- 1.2. This method statement has been written by a Chartered Engineer. The sequencing has been developed using guidance from ASUC (Association of Specialist Underpinning Contractors). Croft Structural Engineers are an Associate Member of ASUC.
- 1.3. This method has been produced to allow for improved costings and for inclusion in the Party Wall Award. Final site conditions need there to be flexibility in the method statement: Should the site staff require alterations to the Method statement this is allowed once an alternative methodology, of the changes is provided, and an Addendum to the Party Wall Award will be required.
- 1.4. Contact Party Wall Surveyors to inform them of any changes to this method statement.
- 1.5. On this development, the approach is: demolish the existing garages, construct the underpin segments in pin sequence accordingly to the structural drawings, once the substructure RC shell is completed, erect superstructure.
- 1.6. Temporary props will be provided along the height of the pin in the temporary condition. Before the base is cast cross props are needed. The base/ground slab provides propping in the final condition. In the temporary condition, the edge of the slab is buttressed against the soil in the middle of the property. Also, the skin friction between the concrete base and the soil provides further resistance. The central soil mass is to be removed in portions (thirds but no greater than 8m) and cross propping subsequently added as the central soil ass is removed
- 1.7. A ground investigation has been undertaken. The soil present is Bagshot Formation.
- 1.8. The bearing pressures have been limited to 125kN/m².
- 1.9. It is not expected to encounter water table.
- 1.10. The structural waterproofer (not Croft) must comment on the proposed design and ensure that he is satisfied that the proposals will provide adequate waterproofing.
- 1.11. Provide engineers with concrete mix, supplier, delivery and placement methods two weeks prior to the first pour. Site mixing of concrete should not be employed apart from in small sections (less than 1m3). The contractor must provide a method on how to achieve site mixing to the correct specification. The contractor must undertake toolbox talks with staff to ensure site quality is maintained.

2. Enabling Works

2.1. The site is to be hoarded with ply board sheets, at least 2.2m high, to prevent unauthorised public access.



- 2.2. Licences for skips and conveyors should be posted on the hoarding.
- 2.3. Provide protection to public where conveyor extends over footpath. Depending on the requirements of the local authority, construct a plywood bulkhead over the pavement. Hoarding to have a plywood roof covering over the footpath, night-lights and safety notices.
- 2.4. No significant dewatering is expected. Localised removal of water may be required to deal with rain from perched water or localised water. This is to be dealt with by localised pumping. Typically achieved by a small sump pump in a bucket.
- 2.5. On commencement of construction, the contractor will determine the foundation type, width and depth. Any discrepancies will be reported to the structural engineer in order that the detailed design may be modified as necessary.

3. Basement Sequencing

- 3.1. Demolish existing garages.
- 3.2. Excavate area 600mm below external ground level.
- 3.3. Excavate first pin as per plans. (Follow methodology in Section 4)
- 3.4. Excavate second pin as per plans. (Follow methodology in Section 4)
- 3.5. Continue cantilevered wall formation around perimeter of basement following the numbering sequence on the drawings.
 - 3.5.1.Excavation for the next numbered sequential sections of underpinning shall not commence until at least 8 hours after drypacking of previous works. Excavation of adjacent pin to not commence until 48 hours after drypacking. (24hours possible due to inclusion of Conbextra 100 cement accelerator to dry pack mix). No more than
- 3.6. Cast base to internal wall.
- 3.7. Excavate and cast floor slab
 - 3.7.1.Excavate 1/3 of the middle section of basement floor. As excavation proceeds, place Slim Shore props at a maximum of 2.5m c/c across the basement. Locate props at a third of the height of the wall.





3.7.2.Continue excavating the next 1/3 and prop then repeat for the final 1/3.



- 3.7.3. Place below-slab drainage. Croft recommends that all drainage is encased in concrete below the slab and cast monolithically with the slab. Placing drainage on pea shingle below the slab allows greater penetration for water ingress.
- 3.7.4. Place reinforcement for basement slab.
- 3.7.5. Building Control Officer and Engineer are to be informed five working days before reinforcement is ready and invited for inspection.
- 3.7.6. Once inspected, pour concrete.
- 3.8. Provide structure to ground floor and water proofing to retaining walls as required. It is recommended to leave 3-4 weeks between completion of the basement and installing drained cavity. This period should be used to locate and fill any localised leakage of the basemen

Mitigation measures where potential soft spots encountered

- 4.1.1. Where soft spots are encountered, leave in trench sheets or alternatively back prop with precast lintels or sacrificial boards. If the soil support to the ends of the lintels is insufficient, then brace the ends of the PC lintels with 150x150 C24 timbers and prop with Acrows diagonally back to the ground.
- 4.1.2. Where voids are present behind the lintels or trench sheeting, grout voids behind sacrificial propping. Grout to be 3:1 sand/cement packed into voids.

4.1.3.Prior to casting, place layer of DPM between trench sheeting (or PC lintels) and new concrete. The lintels are to be cut into the soil by 150mm either side of the pin. A site stock of a minimum of 10 lintels should be present to prevent delays due to ordering.

ENGINEERS