



E N V I R O N M E N T A L E N G I N E E R I N G P A R T N E R S H I P

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INTRODUCTION

Where projects are proposing to incorporate a below ground level basement, in accordance with the Camden Local Plan these projects are required to submit a drainage report where the following information is required:

- Identification of flood risk (by Akera Engineers)
- Assessment of existing run-off rates
- · Calculation of greenfield run-off rates
- Identification of measures, in line with the drainage hierarchy, to reduce run-off rates
- Calculation of proposed run-off rates

Existing Drainage Connection

Currently, the rainwater drainage plus the soil and waste drainage from the building are combined and discharge directly to the Thames Water Sewer located beneath the Frognal Gardens road, the drainage combines within an existing manhole located within the front of house private drive/car parking area.

Accordingly, as it is known that the existing connection to the sewer is via a 'combined' drain, which has been confirmed to be $\emptyset150$ mm. However, the drainage connection from 18A connects to the drainage from 18B ($\emptyset100$ mm pipework) before connecting to the sewer. The connection between the two buildings can be seen at the manhole located on the property of 18B.

Please see a recent CCTV report of the existing drainage system in the Appendix of this report.

It is assumed that the existing rainwater drainage system would have been designed using a rainfall intensity of 75mm/hour.

Drainage Hierarchy

- 1. Storage for later use
- 2. Infiltration
- 3. Attenuation via ponds/water features for gradual release
- 4. Attenuation via storage tanks for gradual release
- 5. Discharge direct to watercourse
- 6. Discharge to surface water sewer
- 7. Discharge to combined sewer



SURFACE AREA CALCULATION ñ EXISTING AND PROPOSED

Existing Areas m² GF 18.8 Drains to garden 131.0 Garden 47.9 71.1 Drains to street 30.2 Garden Roof 104.6

Total	403.6

161.2m² Garden

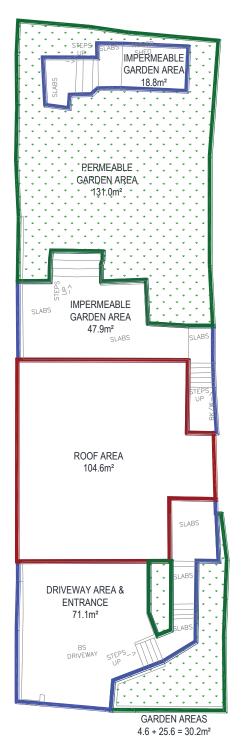
Impermeable 152.5

Permeable 180.0

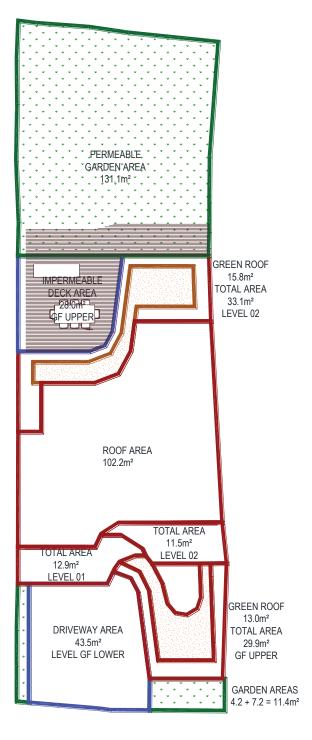
To street 71.1

Propose Areas m ²	d	
GF Lower	43.5	Drains to street
	11.4	Garden
GF Upper	131.1	Garden
	28.0	
	29.9	13.0m ² Green Roof
01	12.9	
02	33.1	15.8m ² Green Roof
	11.5	
Roof	102.2	
Total	403.6	
142.5m ² Gar	den	l
28.8m ² Green	Roof	
Impermeable	154.6]
Permeable	142.5]
Semi Permeable	63.0]
To street	43.5]

Existing



Proposed





2

SURFACE AREA CALCULATION ñ EXISTING AND PROPOSED

Existing Building:

To	tal		403.6m ²
c)	Discharges to street	=	71.1m ²
b)	Permeable area	=	180.0m ²
a)	Impermeable area	=	152.5m ²

Proposed Building:

Tot	tal		403.6m ²
f)	Discharges to street	=	43.5m^2
e)	Semi-permeable Green Roof area	=	63.0m ²
d)	Permeable area	=	142.5m ²
c)	Impermeable area	=	154.6m ²

As can be seen from table 6 within the CIBSE Guide KS11 the rainfall water retention will be 50% (TBC) which affectively increases the "permeable area" by 38 m 2 (63 m 2 ÷ 2 = 31.5 m 2 or 50%), therefore the new site areas simplified for run-off calculation purposes we can adjust the areas as follows:-

Proposed Building (Simplified):

To	tal		403.6m ²
h)	Discharges to street	=	43.5m ²
g)	Permeable area	=	174m² (142.5 + 31.5)
f)	Impermeable area	=	186.1m ² (154.6 + 31.5)

Substrate depth / mm	Vegetation	Average annual water retention / %	Average annual rainfall run off / %
20–40	Moss, sedums	40	60
40–60	Sedums and moss	45	55
60–100	Sedums, moss and herbs	50	50
100–150	Sedums, herbs and grass	55	45
150–200	Grass and herbs	60	40

Water retention in extensive green roofs (based on 650–800 mm annual rainfall). CIBSE KS11 – Green Roofs, Table 6.



RUN-OFF RATE CALCULATIONS

The rainfall intensity stipulated within Clause 3.8 of the March 2019 version of the 'Camden Planning Guidance - Water & Flooding' for a residential development the required 'Protection Years' are for a 100 year return period, using BS EN 12056-3 this results in a design intensity of 250mm/hour (0.069 litres/second/metre²).

	Run-off Calcu				
		Area m²	Run-off l/s		
Existing	Impermeable	152.5	10.52		
Exis	Permeable	180.0	12.42		
Proposed	Impermeable	186.1	12.84	22%	% Change
Prop	Permeable	174	12.01		

Rainfall intensity calculations for Based on Category 2 values from BS EN 12056 part 3

18a FROGNAL GARDENS, LONDON.

BASIC DATA		
Nearest geographical town	London	ı
Building life span required	100	years

Calculations data based on BS EN	1205	6 part 3	Category	2		
Protection years (T) (1.5 x life span)	150	years				
Fig. NB.6 value (return period in years)	4.5	(based	on 2min M5	5)		
From Table NB.1, fraction for 2 min. store	m = 1.0	0				
Therefore 2 min. M5 rainfall =	4.5	X	1	=	4.5	
Factor from Fig. NB.7 using protection ye	ears	150	(M5=2)	=	1.9	
Using return period year factor for 2 min	M5 =	1.85	х	4.5	=	8.325

Calculated Flowrates						
Rainfall intensity in mm/hour	30	Х	8.325	=	250	mm/hour
Run off in litres per second	0.069	per squa	are metre			



ATTENUATION TANK CAPACITY CALCULATIONS

In order to maintain the building's original rainwater discharge to the sewer any design for the attenuation and controlled release of rainwater shall not exceed the discharge rate of the original design. It is assumed that the rainfall intensity used for the original design was based on $75 \, \text{mm}$ per hour/m².

Therefore, following the guidance from the Camden Planning Guidance documents and methodology provided in the BRE Digest 365 – Soakaway Design, and The Wallingford Procedure, the following equations can be used to size the rainwater attenuation tank:

Run-off Calculations Based on original building design (75mm/Hr/m²)				
		Area m²	Run-off l/s	
	Impermeable	152.5	3.20	

maximum allowable discharge to sewer

NOMENCLATURE

 $MX-D_{min}$: X=return period (years)D=storm durat ion (min)

Z1: rainfall factor taken from figure 1 and table 1 – BRE Digest 365

Z2: growth factor taken from table 2 – BRE Digest 365

 $d = discharge \ rate(l/s)$

T = time(s)

 $I = inflow (m^3)$

 $0 = outflow(m^3)$

 $S = storage(m^3)$

EQUATIONS

$$M5-D_{min} = M5-60_{min\ rainfall} \times Z1$$

$$M10-D_{min} = M5-D_{min} \times Z2$$

 $O=d\times M10\text{-}D_{min}$

 $I = d \times T$

 $I_{140\%} = I \times 1.4$

 $S = I_{140\%} - O$

Area	Duration	Rainfall factor	M5 rainfalls	Growth factor	M10 year rainfall	Growth factor	M100 year rainfall	M100 year rainfall + 40%	Inflow	Inflow (@140%)	Outflow	Storage required
m ²		Z1	mm	Z2 (M10)	mm	Z2 (M100)	mm	mm	m ³	m³	m³	m ³
186.1	5 mins	0.38	7.6	1.19	9.0	1.96	14.9	20.9	2.8	3.9	1.0	2.9
	10 mins	0.53	10.6	1.22	12.9	2.00	21.2	29.7	3.9	5.5	1.9	3.6
	15 mins	0.64	12.8	1.24	15.9	1.96	25.1	35.1	4.7	6.5	2.9	3.7
	30 mins	0.81	16.2	1.24	20.1	2.00	32.4	45.4	6.0	8.4	5.8	2.7
	1 hour	1.00	20	1.24	24.8	2.03	40.6	56.8	7.6	10.6	11.5	0.0
	2 hours	1.20	24	1.22	29.3	2.01	48.2	67.5	9.0	12.6	23.1	0.0
	4 hours	1.42	28.4	1.19	33.8	1.97	55.9	78.3	10.4	14.6	46.1	0.0
	6 hours	1.57	31.4	1.17	36.7	1.96	61.5	86.2	11.5	16.0	69.2	0.0
	10 hours	1.74	34.8	1.14	39.7	1.92	66.8	93.5	12.4	17.4	115.3	0.0
	24 hours	2.16	43.2	1.13	48.8	1.86	80.4	112.5	15.0	20.9	276.7	0.0



PROTECTED TREE ROOT ZONE

At 18A Frognal Gardens at the rear of the property are a number of fully established trees, a soakaway, if used, must be located outside of the root protection zone.

Building Regulations Part H and good practice requires that any soakaway be located not closer than 5m to the nearest building and section 3.20 within the 'Camden Planning Guidance – Water & Flooding' states any infiltration measures located within 5m must be designed to avoid harm to the building, in accordance with the Building Regulations.

A survey of the existing root area shows that distance of the building from the root protection zone varies from approximately 2.4m to 5.2m, this provides little or no room for the inclusion of a soakaway system of any meaningful size.

Therefore, rainwater run-off and drainage shall be addressed using a combination of solutions to be discussed in the sections to follow.





RAINWATER INFILTRATION & ATTENUATION

The proposed architectural design shows extensive green roofing to be installed to some terrace and roof areas, however, it is noted that section 3.14 within the 'Camden Planning Guidance – Water & Flooding' states:

However, green roofs cannot be considered a permeable soil and should be assumed to be saturated at the point of intense storms (i.e. storms that are more intense than a 1 in 10 year storm). Due to the sporadic nature of water consumption, rainwater harvesting tanks should also be assumed to be full at the point of a storm event. Both of these systems are generally not intended to control peak run-off rate during critical events, and are mainly useful during medium and small events to capture run-off and thus reduce the volume of water entering the drainage system during these smaller events.

But section 3.15 also states:

Some rainwater harvesting tanks can incorporate an attenuation chamber/ overflow with controlled release, and green roofs can incorporate an attenuation layer ("blue roof") or be designed to be substantially thick, allowing the associated attenuation volume to be included within the modelling. The Council will consider inclusion of these SuDS in developments favourably due to their additional environmental benefits.

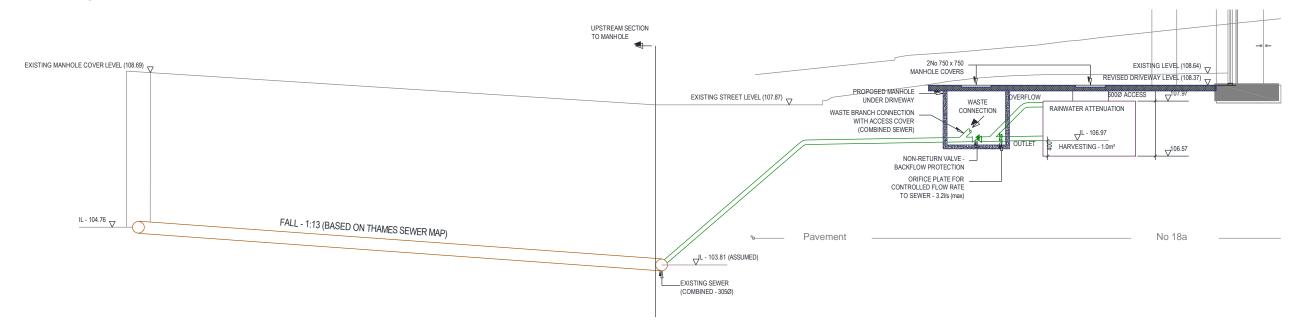
Therefore, the proposal for mitigating the impact of the proposed development to the local sewer network shall be through the attenuation and controlled release of surface water to the sewer. The discharge to the sewer shall be designed to meet the criteria used for the original building, therefore, any additional impact due to climate and increased surface run-off shall be dealt with on site.

The inclusion of several areas of "green roofing" shall serve to provide additional attenuation to the sewer, with the guidance provided in CIBSE Guide KS11, 50% of these areas have been included in the sizing of the attenuation tank.

In order to facilitate the sustainability ambitions of the project, and in line with the drainage hierarchy, it is proposed to maintain a level of water at the bottom of the tank for the provision of harvested rainwater to be reused for irrigation of the green roofs and landscape gardens.

This will require the addition of submersible pumps to the tank that will be serviceable from the manhole cover provided. The tank shall not include a mains water backup, therefore, at times when the tank is empty irrigation will need to come from a Category 5 protected mains water supply.







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CONCLUSIONS

Due to the revised layout of 18A Frognal Gardens the surface water run-off is expected to increase by 22% when compared with the volume currently coming from the building.

Due to this increase and in order to meet the expectations of the Camden Policy Guidance documents the following measures are proposed to relieve pressure on the local drainage system:

- Green roof attenuation to control the flow of rainwater from the building
- Rainwater attenuation and controlled release a hybrid underground system located to the front of the building that will be used to control the flow discharging to the combined sewer with additional harvesting capacity for reuse through irrigation.

With these measures in place we expect to mitigate the impact of any changes to the impermeable run-off surfaces whilst also addressing the future demands owing to climate change. The discharge rate from the attenuation tank shall be designed to be no worse than the original peak discharge for the current building's original design (based on 75mm/Hr/m²).





APPENDIX ñ CCTV SURVEY REPORT





Table of Contents

Project Name	Project Number	Project Date
04.09.19 18A Frognal Gardens, London	-	04/09/2019

Project Information	P-1
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Section: 3; MH2 > D/S MH2 (MH2X)	5
Section: 4; JUNCTION > D/S JN (JUNCTIONX)	7



Aqua-Jet Specialist Drainage Contractors Ltd

Yard 21 Hilton Ind Est, Sutton Lane, Hilton, Derbyshire, DE65 5FE Tel. 01283 730333 aquajetltd@aol.com

Project Information

Project Name Project Number Project Date
04.09.19 18A Frognal Gardens, London 04/09/2019

Client

Company: Environmental Engineering Partnership

Street: The Chapel House, High Street **Town or City:** West Wycombe, HP14 3AG

Site

Company: EEP

Street: 18A Frognal Gardens **Town or City:** London, NW3 6XA

Contractor

Company: Aqua-Jet Specialist Drainage Contractors Ltd

Contact: Rob Wilkinson

Street: Yard 21 Hilton Ind Est, Sutton Lane

Town or City: Hilton, Derbyshire, DE65 5FE

Phone: 01283 730333 Email: aquajetltd@aol.com



Section Inspection - 04/09/2019 - MH1X

Section	Inspection	on Date Time		Client`s Job Ref	Weather	Pre Cleaned	PLR	
1	1	04/09/19		Not Specified	No Rain Or Snow	Υ	MH1X	
Ope	Operator		icle	Camera	Preset Length	Legal Status	Alternative ID	
RR/MD		FJ17 ZDS		Flexi	Not Specified	Not Specified	Not Specified	

Town or Village:	London	Inspection Direction:	Downstream	Upstream Node: MH1		
Road:	18A Frognal Gardens	Inspected Length:	10.00 m	Upstream Pipe Depth:		
Location:		Total Length:	10.00 m	Downstream Node: MH2		
Surface Type:		Joint Length:	0.00 m	Downstream Pipe Depth:		
Use:	Combined		Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer		Dia/Height:	100 mm		
Year Constructed:			Pipe Material:	Cast iron		
Flow Control:	No flow control		Lining Type:	No Lining		
Inspection Purpose:			Lining Material:	No Lining		

Comments:

Scale:	1:87	Position [m]	Code	Observation	MPEG	Photo	Grade
	Depth: m MH1						
		0.00	МН	Start node type, manhole, reference number: MH1	00:00:00		
		0.00	WL	Water level, 0% of the vertical dimension	00:00:01		
		<u>0.10</u> S01	DEE	Attached deposits, encrustation from 12 o'clock to 12 o'clock, 5% cross-sectional area loss, start: RUST / CORROSION	00:00:06		
•							
		9.90 F01	DEE	Attached deposits, encrustation from 12 o'clock to 12 o'clock, 5% cross-sectional area loss, finish: RUST / CORROSION	00:01:22		2
	MH2	10.00	MHF	Finish node type, manhole, reference number: MH2	00:01:23		

		5	tructurai Defec	ts		Construction Features					
I		Service &	Operational Ob	oservations		Miscellaneous Features					
Ę	STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade	
	0	0.0	0.0	0.0	1.0	1	1.0	1.0	10.0	2.0	



Section Pictures - 04/09/2019 - MH1X

Section	Inspection Direction	PLR	Client`s Job Ref	Contractor`s Job Ref
1	Downstream	MH1X		



Attached deposits, encrustation from 12 o'clock to 12 o'clock, 5% cross-sectional area loss, start



Section Inspection - 04/09/2019 - A/MH1X

Section 2	Inspection 2	Date 04/09/19	Time	Client's Job Ref Not Specified	Weather No Rain Or Snow	Pre Cleaned Y	PLR A/MH1X	
	rator /MD		icle ZDS	Camera Flexi	Preset Length Not Specified	Legal Status Not Specified	Alternative ID Not Specified	

Town or Village:	London	Inspection Direction:	Upstream	Upstream Node:	A/MH1	
Road:	18A Frognal Gardens	Inspected Length:	7.50 m	Upstream Pipe Depth:		
Location:		Total Length:	7.50 m	Downstream Node:	MH1	
Surface Type:		Joint Length:	0.00 m	Downstream Pipe Depth:		
Use:	Combined		Pipe Shape:	Circular		
Type of Pipe:	Gravity drain/sewer		Dia/Height:	100 mm		
Year Constructed:			Pipe Material:	Cast iron		
Flow Control:	No flow control		Lining Type:	No Lining		
Inspection Purpose:			Lining Material:	No Lining		

Inspec	tion Purpo	se:			Lining Material: No Lir	ning		
Comm Recon	ents: imendatio	ns:						
Scale:	1:66	Position [m]	Code	Observation		MPEG	Photo	Grade
	Depth: m							
		0.00	MH	Start node type, man	hole, reference number: MH1	00:00:00		
		0.00	WL	Water level, 5% of th	e vertical dimension	00:00:01		
		<u>0.10</u> S01	DEE	Attached deposits, er o'clock, 5% cross-sec CORROSION	ncrustation from 12 o'clock to 1 ctional area loss, start: RUST /	2 00:00:03		
1								
		7.40 F01	DEE	Attached deposits, er o'clock, 5% cross-sec CORROSION	ncrustation from 12 o'clock to 1 ctional area loss, finish: RUST	2 00:01:22		2
	A/MH1	7.50	MHF		nhole, reference number: A/MH	11 00:01:22		
	Depth: m							
		Structural	Defects		Const	ruction Features		

	3	iructurai Delec	สร		Construction Features					
	Service &	Operational Ob	oservations		Miscellaneous Features					
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade	
0	0.0	0.0	0.0	1.0	1	1.0	1.1	8.0	2.0	



Section Pictures - 04/09/2019 - A/MH1X

Section	Inspection Direction	PLR	Client`s Job Ref	Contractor`s Job Ref
2	Unstream	A/MH1X		



A_MH1X_7069cf84-8557-4d21-aaf6-a844871a4e0a_2019090 6_100331_033.jpg, 00:00:03, 0.10 m
Attached deposits, encrustation from 12 o'clock to 12 o'clock, 5% cross-sectional area loss, start



				-			
Section	Inspection	Date	Time	Client's Job Ref	Weather	Pre Cleaned	PLR
3	3	04/09/19		Not Specified	No Rain Or Snow	Y	MH2X
Ope	rator	Veh	icle	Camera	Preset Length	Legal Status	Alternative ID
RR/MD		FJ17	ZDS	Flexi	Not Specified	Not Specified	Not Specified

Town or Village:	London	Inspection Direction:	Downstream	Upstream Node: MH2	
Road:	18A Frognal Gardens	Inspected Length:	7.30 m	Upstream Pipe Depth:	
Location:		Total Length:	7.30 m	Downstream Node: D/S MH2	
Surface Type:		Joint Length:	0.00 m	Downstream Pipe Depth:	
Use:	Combined		Pipe Shape:	Circular	
Type of Pipe:	Gravity drain/sewer		Dia/Height:	100 mm	
Year Constructed:			Pipe Material:	Vitrified clay pipe (i.e. all clayware)	
Flow Control:	ow Control: No flow control		Lining Type:	No Lining	
Inspection Purpose:			Lining Material:	No Lining	
Commonts:					

1:64	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m MH2						
	0.00	МН	Start node type, manhole, reference number: MH2: SURVEY THROUGH TOP ACCESS HOLE OF TRAP	00:00:00		
	0.00	WL	Water level, 5% of the vertical dimension	00:00:01		
	0.20	JN	Junction at 6 o'clock, diameter: 100mm: BOTTOM OUTLET OF TRAP	00:00:09		
	0.30	SC	Size changes, new size(s), 150mm high	00:00:15		
	1.80	CCJ	Crack, circumferential at joint from 8 o'clock to 3 o'clock	00:00:33		2
	2.70	LD	Line deviates down	00:00:41		
	6.80_	LR	Line deviates right	00:01:23		
	Depth: m	Depth: m MH2 0.00 0.20 0.30 1.80 2.70	Depth: m MH2 0.00 WL 0.20 JN 0.30 SC 1.80 CCJ 2.70 LD	Depth: m MH2 0.00 MH Start node type, manhole, reference number: MH2: SURVEY THROUGH TOP ACCESS HOLE OF TRAP WL Water level, 5% of the vertical dimension JN Junction at 6 o'clock, diameter: 100mm: BOTTOM OUTLET OF TRAP 0.30 SC Size changes, new size(s), 150mm high 1.80 CCJ Crack, circumferential at joint from 8 o'clock to 3 o'clock LD Line deviates down	Depth: m MH2 0.00 MH Start node type, manhole, reference number: MH2: SURVEY THROUGH TOP ACCESS HOLE OF TRAP 0.00 WL Water level, 5% of the vertical dimension 00:00:01 0.20 JN Junction at 6 o'clock, diameter: 100mm: BOTTOM OUTLET OF TRAP 0.30 SC Size changes, new size(s), 150mm high 0:00:01 1.80 CCJ Crack, circumferential at joint from 8 o'clock to 3 o'clock 0:00:33 2.70 LD Line deviates down 00:00:41	Depth: m MH2 O.00 MH Start node type, manhole, reference number: MH2: 00:00:00 SURVEY THROUGH TOP ACCESS HOLE OF TRAP O.00 WL Water level, 5% of the vertical dimension 00:00:01 JN Junction at 6 o'clock, diameter: 100mm: BOTTOM 00:00:09 OUTLET OF TRAP O.30 SC Size changes, new size(s), 150mm high 00:00:15 1.80 CCJ Crack, circumferential at joint from 8 o'clock to 3 o'clock 00:00:33 2.70 LD Line deviates down 00:00:41

Structural Defects						Construction Features				
Service & Operational Observations					Miscellaneous Features					
STR	No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
	1	10.0	1.4	10.0	2.0	0	0.0	0.0	0.0	1.0



Section Pictures - 04/09/2019 - MH2X

Section	Inspection Direction	PLR	Client`s Job Ref	Contractor`s Job Ref
3	Downstream	MH2X		



MH2X_99a95e93-e9bd-4658-b633-a5a8b0b76c35_20190906 _101510_209.jpg, 00:00:33, 1.80 m Crack, circumferential at joint from 8 o'clock to 3 o'clock



Section Inspection - 04/09/2019 - JUNCTIONX

				=			
Section	Inspection	Date	Time	Client`s Job Ref	Weather	Pre Cleaned	PLR
4	4	04/09/19		Not Specified	No Rain Or Snow	N	JUNCTIONX
Operator		Veh	icle	Camera	Preset Length	Legal Status	Alternative ID
RR/MD		FJ17	ZDS	Flexi	Not Specified	Not Specified	Not Specified

Town or Village:	London	Inspection Direction:	Downstream	Upstream Node: JUNCTION	
Road:	18A Frognal Gardens	Inspected Length:	11.00 m	Upstream Pipe Depth:	
Location:		Total Length:	50.00 m	Downstream Node: D/S JN	
Surface Type:		Joint Length:	0.00 m	Downstream Pipe Depth:	
Use:	Combined		Pipe Shape:	Circular	
Type of Pipe:	Gravity drain/sewer		Dia/Height:	300 mm	
Year Constructed:			Pipe Material:	Vitrified clay pipe (i.e. all clayware)	
Flow Control:	low Control: No flow control		Lining Type:	No Lining	
Inspection Purpose:			Lining Material:	No Lining	

Comments:

Scale:	1:435	Position [m]	Code	Observation	MPEG	Photo	Grade
	Depth: m						
(ONCTION	0.00	МН	Start node type, manhole, reference number: JUNCTION	00:00:00		
7	TA TA	0.00	WL	Water level, 5% of the vertical dimension	00:00:01		
		0.10	REM	General remark: POOR VIEW OF PIPE DUE TO SIZE	00:00:07		
		3.20	JN	Junction at 9 o'clock, diameter: 150mm	00:00:31		
		5.20	JN	Junction at 3 o'clock, diameter: 150mm	00:00:42		
		9.40	CN	Connection other than junction at 10 o'clock, diameter:	00:01:07		
		11.00	SA	150mm Survey abandoned: LIMIT OF SURVEY	00:01:17		
	 			•			
(50.00		End of pipe			
`	D/C IN						
	D/S JN Depth: m						
	•						

STR No. Def STR Peak

Service & Operational Observations

STR Mean

0.0

STR Total

1.0

0.0

SER Grade

1.0

Miscellaneous Features

0.0

SER Total

0.0

STR Grade | SER No. Def | SER Peak | SER Mean |

0.0

0

AquaJet

Manhole Information

Drainage Specialists

REFERENCE: MH I	DUTY: CB	REFERENCE: MH2	DUTY: CB			
DIAGRAM:		DIAGRAM:				
Buckdrop 100mm IL 600mm 100mm	M	Trap	Toomm Toomm			
DEPTH AT OUTLET: 1740MM	1	DEPTH AT OUTLET: 14-00	OMM			
MH SIZE 900mm × 500	OMM	MH SIZE 950m	m × 600mm			
MH MATERIAL Brick/Ren	العام	MH MATERIAL Brick				
OBSERVATIONS/ COMMENTS:		OBSERVATIONS/ COMMENTS:				
REFERENCE:	DUTY:	REFERENCE: DUTY:				
DIAGRAM:		DIAGRAM				
DEPTH AT OUTLET:		DEPTH AT OUTLET:				
MH SIZE		MH SIZE				
MH MATERIAL		MH MATERIAL				
OBSERVATIONS/ COMMENTS:		OBSERVATIONS/ COMMENTS:				

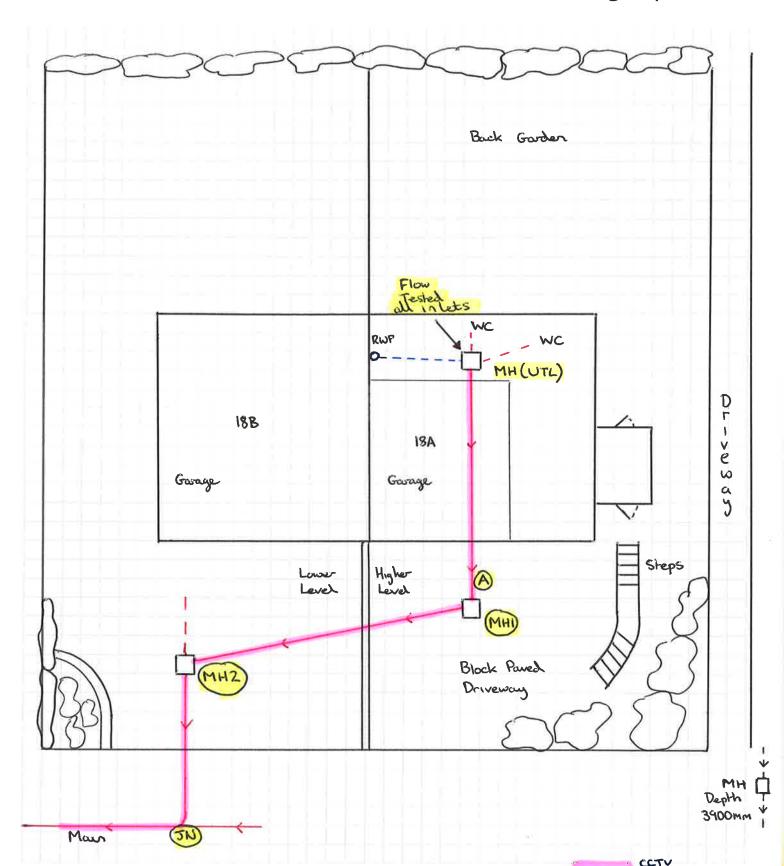
Jetting Vacuumation Camera Inspection Excavation & Repairs Septic Tanks

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Drainage Specialists



Jetting • Vacuumation • Camera Inspection • Excavation & Repairs • Septic Tanks

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