

SITE INVESTIGATION FACTUAL REPORT

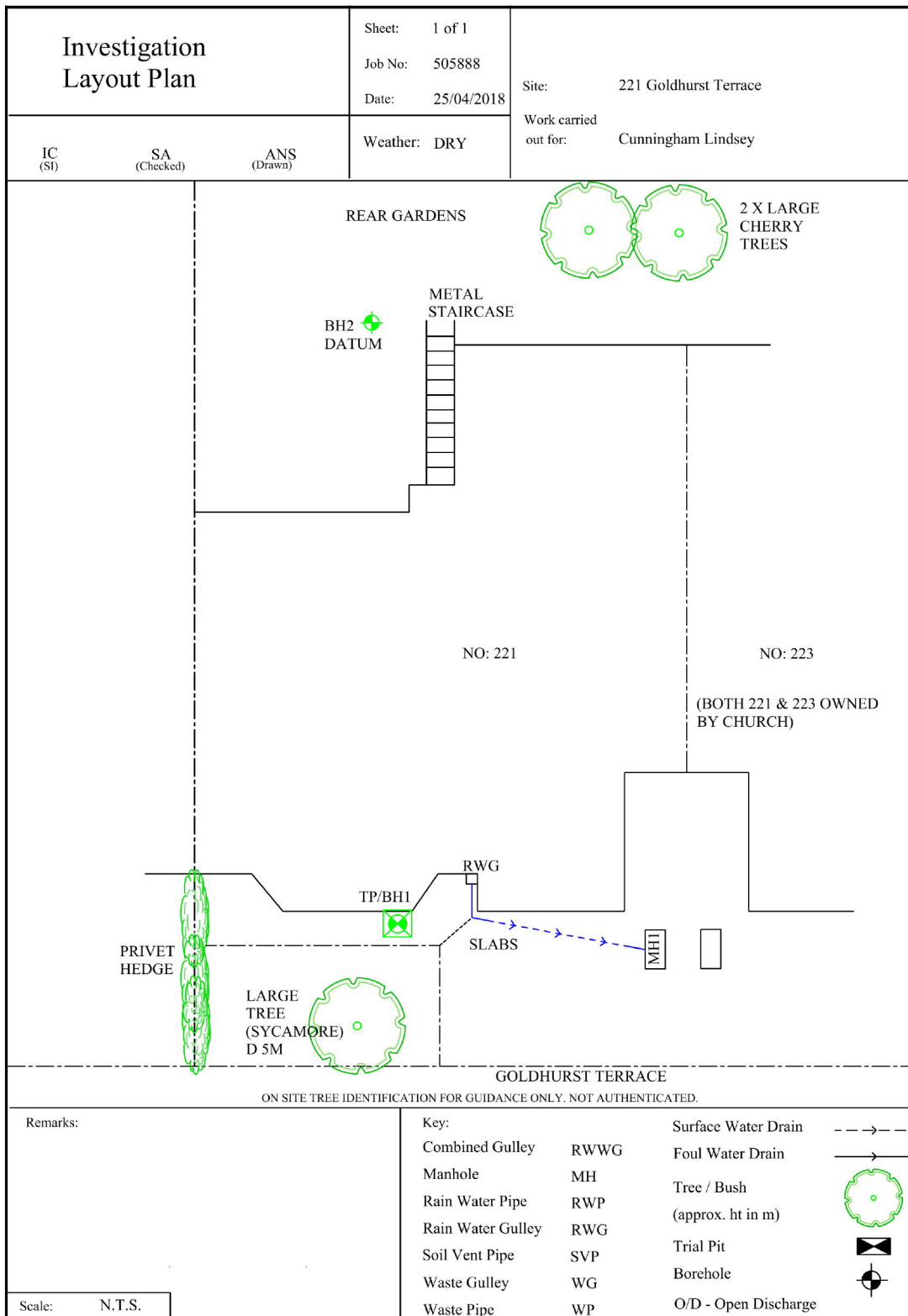
Report No: 505888
Client: Cunningham Lindsey - Morley (Leeds)
Site: 221 Goldhurst Terrace
Client Ref: [REDACTED]
Date of Visit: 25/04/18



Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys



CET is the trading name of CET Structures Ltd
Registered in England No. 02527130



TEST REPORT: Trial Pit

REPORT NUMBER: C351157 / 1097.1.1.1

TRIAL PIT REF: Tp1 1of1

CLIENT: Cunningham Lindsey

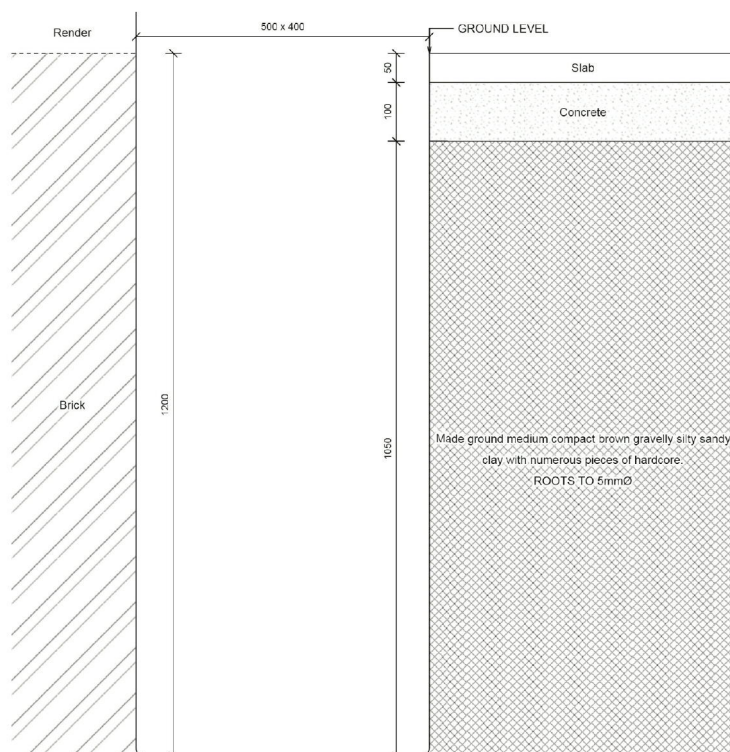
JOB NO: 505888

EXCAVATION METHOD: Hand tools

DATE: 25/04/2018

SITE: 221 Goldhurst Terrace, NW6 3EP

WEATHER: Clear



For Strata below 1200mm see Bore Hole log

TP abandoned at 1200mm due to depth. BH carried out in back of trial pit. u/s of foundation not found ,no soil sample taken.

Key:

D Small disturbed sample J Jar sample
B Bulk disturbed sample V Pilcon vane (kPa)
W Water sample M Mackintosh probe
TDTD Too dense to drive

Remarks:
Amended report.This test report supersedes test report version 1

For and on behalf of CET
Scott Alger - Lab

Report Format:



Approved Signatory
26-Apr-18

CET is the trading name for CET Structures Limited.
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Report version 2

Page 1 of 1

Borehole		1	Sheet: 1 of 1 Job No: 505888 Date: 25/04/2018 Ground Level:		Site: 221 Goldhurst Terrace Client: Cunningham Lindsey - Morley (Leeds)
Boring Method:	Rotary Auger				
Diameter (mm):	100	Weather:	dry		
Depth	Soil Description				Thickness Legend
(m)					Depth Type Result
0.00	See Trial Pit				1.20 
1.20	MADEGROUND medium compact brown silty sandy clay with pieces of hardcore				0.30 
1.50	Stiff orange-brown silty CLAY with gravel				0.30 
1.80	Stiff brown-grey veined silty CLAY				0.20 
2.00	Very Stiff brown-grey veined silty CLAY with claystone nodules				3.00 
5.00	End of BH				140+
Remarks: BH ends at 5.0m .BH dry and open on completion,no roots observed below 2.2m.					Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive
					To Max Depth Dia (m) (mm) 2.20 1 4.50 D 5.00 DV 140+
Logged:	IC	SA	Checked:	Approved:	Version V1.0 28/01/16 N.T.S.

Borehole		2			Sheet: 1 of 1 Job No: 505888 Date: 25/04/2018	Site: 221 Goldhurst Terrace Client: Cunningham Lindsey - Morley (Leeds)								
Boring Method: Hand Auger		Weather: dry		Ground Level:										
Diameter (mm):	75													
Depth (m)	Soil Description					Thickness Legend Depth Type Result								
0.00	MADEGROUND medium compact brown silty sandy clay with gravel and brick pieces					0.80 								
0.80	Stiff brown-grey veined silty CLAY with claystone nodules					1.70 								
2.50	Very Stiff brown-grey veined silty CLAY with claystone nodules					3.30 								
5.80	End of BH					140+								
Remarks: BH ends at 5.8m, too stiff/hard to hand auger. BH dry and open on completion, no roots observed. No access for drill. Datum installed at 5.8m.					Key: D - Disturbed Sample B - Bulk Sample W - Water Sample Roots J - Jar Sample Roots V - Pilcon Shear Vane (kPa) Roots M - Mackintosh Probe Depth to Water (m) TDTD - Too Dense To Drive									
Logged:	IC	SA	Checked:	Approved:	Version V1.0 28/01/16	To Max Depth Dia (m) (mm) <table border="1"> <tr><td></td><td></td></tr> <tr><td></td><td></td></tr> <tr><td></td><td></td></tr> <tr><td></td><td></td></tr> </table>								
					N.T.S.									

Laboratory Summary Results

Our Ref: 5058888

Date Sampled: 25/04/18

Location : 221, Goldhurst Terrace, London, NW6 3EP

Date Received : 26/04/18

Client: Cunningham Lindsey - Morley (Leeds)

Date Tested : 26/04/18

Address: Subsidence Scanning Centre, Ground Floor, Fountain Court, LS27 0JG

Date of Report : 01/05/18

TP/BH No	Sample Ref		Type	Moisture Content (%) [11]	Soil Fraction > 0.425mm (%) [12]	Liquid Limit (%) [13]	Plastic Limit (%) [14]	Plasticity Index (%) [15]	Liquidity * Index (%) [6]	Modified * Plasticity Index (%) [7]	Soil * Class (%) [8]	Filler Paper Contact Time (h)	Soil Sample Suction (kPa) [9]	Oedometer Strain (%) [10]	Estimated Heave Potential (Dd) (mm) [11]	In situ * Shear Vane Strength (kPa) [12]	Organic * Content (%) [13]	pH * Value (%) [14]	Sulphate Content * (g/l)			Soil Class (%) [16]
	TP/BH	Depth (m)																	SO ₃ [15]	SO ₄ [16]		
1	1.5	D	24	18	65	21	44	0.06	36	CH												
	2.0	D	34	<5																		
	2.5	D	30	<5	74	26	48	0.08	48	CV												
	3.0	D	32	<5																		
	3.5	D	33	<5	78	26	52	0.14	52	CV												
	4.0	D	34	<5																		
	4.5	D	33	<5																		
5.0	D	34	<5																			

Test Methods / Notes

[1] BS 1377: Part 2: 1990, Test No 3.2

[2] Estimated if <5%, otherwise measured

[3] BS 1377: Part 2: 1990, Test No 4.4

[4] BS 1377: Part 2: 1990, Test No 5.3

[5] BS 1377: Part 2: 1990, Test No 5.4

[6] BRE Digest 240: 1993

[7] BS 5930: 1981: Figure 31 - Plasticity Chart for the classification of fine soils

[8] In-house method S10 adapted from BRE IP 493

[9] In-house Test Procedure S17a: One Dimensional Swell/Shrink Test

[10] Estimated Heave Potential (Dd)

[11] Values of shear strength were determined in situ by CPT using a Picon hand vane or Geotest vane (GV).

[12] BS 1377: Part 3: 1990, Test No 4

[13] BS 1377: Part 2: 1990, Test No 9

[14] BS 1377: Part 3: 1990, Test No 5.6

[15] SO₄ = 1.2 x SO₃

Key

D Disturbed sample (small)

B Disturbed sample (bulk)

U Undisturbed sample

W Groundwater sample

EXP Essentially Non-Plastic by inspection

US Underlain by Foundation



8618

Our Ref: 505888

Location : 221, Goldhurst Terrace, London, NW6 3EP

Client: Cunnigham Lindsey - Morley (Leeds)

Address: Subsidence Scanning Centre, Ground Floor, Fountain Court, LS27 0JG

Date Sampled : 25/04/18

Date Received : 26/04/18

Date Tested : 26/04/18

Date of Report : 01/05/18

Laboratory Testing Results

TP/BH No.	Sample Ref.		Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity * Index [5]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (h)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated Heave Potential (Dd) (mm) [10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH * Value [13]	Sulphate Content * (g / l)			* Class [16]
	Depth (m)																		SO ₃ [14]	SO ₄ [15]		
BH2	U/S 1.00		D	35	<5	77	25	52	0.19	52	CV					77						
	1.5		D	30	<5	73	23	50	0.15	50	CV					85						
	2.0		D	32	<5											99						
	2.5		D	34	<5	77	29	48	0.10	48	CV					> 140						
	3.0		D	34	<5											> 140						
	3.5		D	30	<5	77	26	51	0.07	51	CV					> 140						
	4.0		D	32	<5											> 140						
	4.5		D	30	<5											> 140						
5.0		D	32	<5											> 140							

Test Methods / Notes

[1] BS 1377 : Part 2 : 1990, Test No 3.2

[2] Estimated if <5%, otherwise measured

[3] BS 1377 : Part 2 : 1990, Test No 4.4

[4] BS 1377 : Part 2 : 1990, Test No 5.3

[5] BS 1377 : Part 2 : 1990, Test No 5.4

[6] BRE Digest 240 : 1993

[7] BS 5930 : 1981 : Figure 31 - Plasticity Chart for the classification of fine soils

[8] In-house method SRA adapted from BS 1377 4.5.5

[9] In-house Test Procedure S17a, One Dimensional Swell/Strain Test

[10] Estimated Heave Potential (Dd)

[11] Values of shear strength were determined in situ by CET using a Placom hand vane or Geotom vane (GV).

[12] BS 1377 : Part 3 : 1990, Test No 4

[13] BS 1377 : Part 2 : 1990, Test No 9

[14] BS 1377 : Part 3 : 1990, Test No 5.6

[15] SO₄ = 1.2 x SO₃

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D Disturbed sample (small)

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ENP Essentially Non-Plastic by inspection

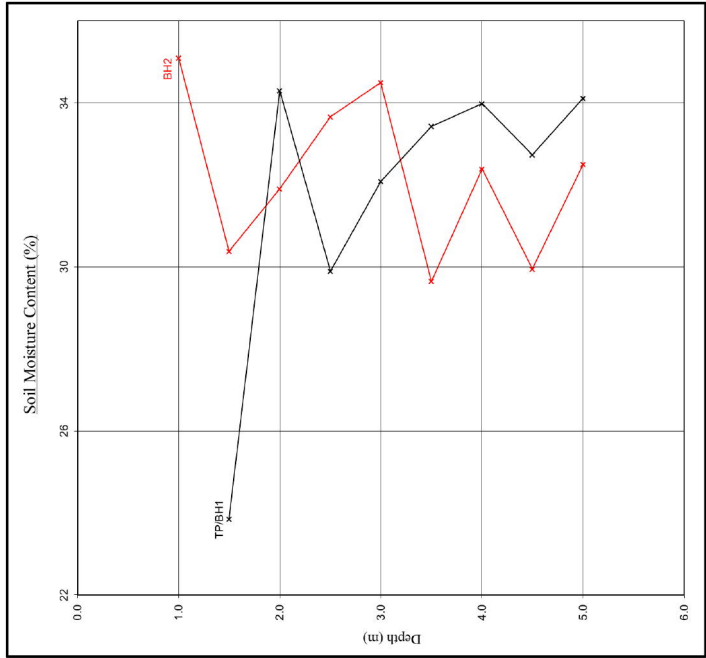
US Underside of Foundation

UKAS TESTING

8618

Moisture Content Profiles

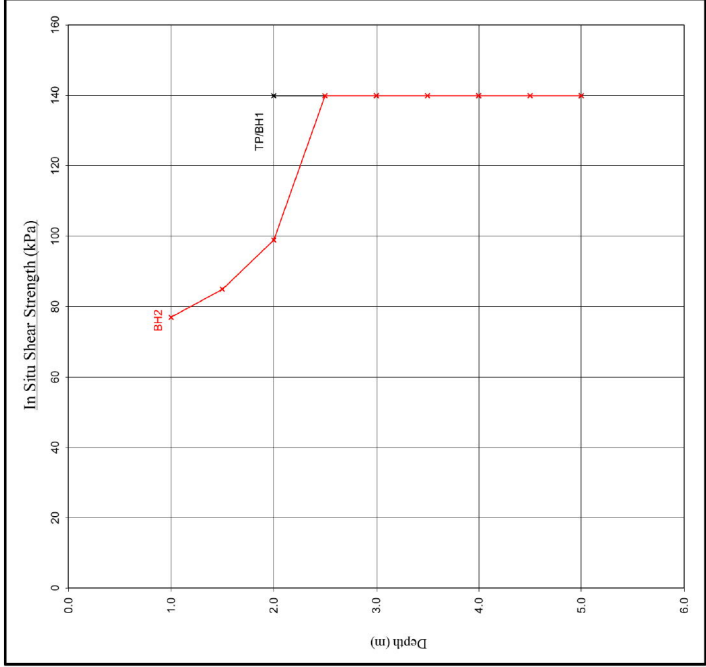
Our Ref: 503888
Location: 221, Goldhurst Terrace, London, NW6 3EP
Work carried out for: Cunningham Lindsey - Morley (Leeds)



Notes
1. If plotted, 0.4 LL and PI+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Shear Strength Profiles

Date Sampled: 25/04/18
Date Received: 26/04/18
Date Tested: 26/04/18
Date of Report: 01/05/18



Note
1. Unless otherwise stated, values of Shear Strength were determined in situ by CET using a Picon Hand Vane the calibration of which is limited to a maximum reading of 140 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

Site: **221 Goldhurst Terrace, NW6,**
Work carried
out for: **Cunningham Lindsey**

Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.

The results were as follows -

<u>Trial pit/ Borehole number</u>	<u>Root diameter (mm)</u>	<u>Tree, shrub or climber from which root originates</u>	<u>Result of starch test</u>
BH1 (to 2.2m)	2 mm	Acer spp. * 2 roots	Positive

* Plus 3 other decayed roots the same.

Acer spp. are maples, including sycamore, Norway maple, and Japanese maples.

MDM

RJS

Head of Laboratory Services : M D Mitchell B.Sc. (Hons), M.Phil.

Plant Anatomist : Dr G S Turner B.Sc. (Hons), M.Sc., Ph.D

Plant Anatomist : Dr R J Shaw B.Sc. (Hons), Ph.D

Consultant: Dr M P Denne B.Sc. (Hons), M.Sc., Ph.D.

Registered in England. No 3256771, Registered Office:

To: Cunningham Lindsey - Morley (Leeds)
Subsidence Scanning Centre
Ground Floor
Fountain Court
West Yorkshire
LS27 0JG

Our Ref: **505888**

Your Ref: **6667535**

Date: **28-Apr-18**

From: Gavin Catheline

ESTIMATE

Site:- 221 Goldhurst Terrace

Item

1.0	Location	Rvg 1 downstream - Run 1
	Shared System	No
	Condition Grade	B
	Drain Serviceability	Unserviceable
	Work Spec	Excavate and replace gully + 1 meter of pipework downstream. From excavation Hmaj to clear and line downstream to Mh 1 with super flex liner.

Notes

Repairs to shared runs and off boundary pipe-work may be the responsibility of the water authority.

Condition Grade

A - Structurally sound with no leakage evident.
B - Cracks and fractures observed.
C - Structurally unsound

Quotation is binding only if accepted within 28 days from date of issue and is subject to our Standard Terms and Conditions
The price qualification notes, stated on the drainage solutions schedule of rates, apply to this quotation.
CET Structures Ltd undertakes to return to site free of charge to carry out remedial work to the drainage repairs set out above for a period of 2 months from the date of this invoice. The company standard charge rates will apply to the visit should the work requested be unrelated to the said repairs.

ESTIMATING & COSTING SHEET - DOMESTIC DRAINAGE

Site:- 221 Goldhurst Terrace

Client :- Cunningham Lindsey - Morley (Leeds)
Attention of:- Gavin Catheline

Client ref	6667535
Job Number :-	505888
Insurer	Allianz Insurance
Date:-	28-Apr-18
Recommendation	1

Item No	Description	Unit	Quantity
Rwg 1 downstream - Run 1			
1.0	Emergency Drain Blockage Clearance		
1.1	Unblock drain 8mm-6pm - First 1/2 Hour	Item	
1.2	Unblock drain 8mm-6pm- Subsequent 1/2 Hour	Item	
1.3	Unblock drain 6pm-midnight	Item	
1.4	Unblock drain 6pm-midnight - Subsequent 1/2 hour	Item	
2.1	CCTV Surveys		
2.2	Undertake CCTV survey 8mm-6pm (up to 3 hours)	Item	
2.3	Additional 1/2 hr survey charge	Item	
3.0	Replacing Underground Drainage		
3.1	Gullies		
3.2	Take out and replace gully (100mm outlet)	Item	1
3.3	Take out and replace rodding point (100mm outlet)	Item	
3.4	Bends/junctions		
3.5	Excavate and replace rest bend (100mm outlet)	Item	
3.6	Excavate and replace rest bend (150mm outlet)	Item	
3.7	Excavate and replace junction/bend (100mmØ), Excavation depth 0-1m.	Item	2
3.8	Excavate and replace junction/bend (150mmØ), Excavation depth 0-1m.	Item	
3.9	Excavate and replace junction/bend (100mmØ), Excavation depth 1-1.5m.	Item	
3.10	Excavate and replace junction/bend (150mmØ), Excavation depth 1-1.5m.	Item	
3.11	Excavate and replace junction/bend (100mmØ), Excavation depth 1.5-2.0m.	Item	
3.12	Excavate and replace junction/bend (150mmØ), Excavation depth 1.5-2.0m.	Item	
3.13	Pipes		
3.14	Excavate trench and replace 100mmØ pipework, Excavation depth 0-1m, First 10m.	m	1
3.15	Excavate trench and replace 150mmØ pipework, Excavation depth 0-1m, First 10m.	m	
3.16	Excavate trench and replace 100mmØ pipework, Excavation depth 0-1m.	m	
3.17	Excavate trench and replace 150mmØ pipework, Excavation depth 0-1m.	m	
3.18	Excavate trench and replace 100mmØ pipework, Excavation depth 1-1.5m, First 10m.	m	
3.19	Excavate trench and replace 150mmØ pipework, Excavation depth 1-1.5m, First 10m.	m	
3.20	Excavate trench and replace 100mmØ pipework, Excavation depth 1-1.5m.	m	
3.21	Excavate trench and replace 150mmØ pipework, Excavation depth 1-1.5m.	m	
3.22	Excavate trench and replace 100mmØ pipework, Excavation depth 1.5-2.0m, First 10m.	m	
3.23	Excavate trench and replace 150mmØ pipework, Excavation depth 1.5-2.0m, First 10m.	m	
3.24	Excavate trench and replace 100mmØ pipework, Excavation depth 1.5-2.0m.	m	
3.25	Excavate trench and replace 150mmØ pipework, Excavation depth 1.5-2.0m.	m	
3.26	Surface Reinstatement of Trenches		
3.27	Excavate through and reinstate turf.		
3.28	Excavate through and replace concrete paving slabs	m	
3.29	Excavate through and replace block paving	m	
3.30	Excavate through and reinstate plain concrete, maximum thickness 100mm.	m	2
3.31	Excavate through and reinstate plain concrete, thickness 100- 200mm.	m	
3.32	Excavate through and reinstate reinforced concrete, maximum thickness 100mm.	m	
3.33	Excavate through and reinstate reinforced concrete, thickness 100-200mm.	m	
3.34	Excavate through and reinstate Tarmac - Cold rolled	m	
3.35	Excavate through and reinstate Tarmac - Hot rolled	m	
3.36	Reinstatement of crazy paving	m	
4.0	Lining		
4.1	Set up lining rig for drain lining including first 3m of lining per run, for 100mm or 150mm	Item	1
4.2	Line 100mmØ drain	m	
	Super Flex Liner 100mm drain	m	2
4.3	Line 150mmØ drain	m	
	Super Flex Liner 150mm drain	m	
4.4	Post lining CCTV survey	no	1
4.5	Minimum lining charge	Item	
4.6	Root cutting of drain prior to lining	hr	
4.7	Set up lining rig for patch lining	Item	
4.8	Patch line 100mmØ drain	no	
4.9	Patch line 150mmØ drain	no	
4.10	Post patch lining CCTV survey	Item	
4.11	Minimum patch lining charge	Item	
4.12	Re-open lateral branch up to 2m length, pipe up to 150mm	no	
4.13	Re-open lateral branch over 2m length, pipe up to 150mm	no	1
	Epoxy resin	no	
5.0	Miscellaneous		
5.1	Excavation and backfill of soakaway (1m3) with stone	Item	
5.2	% Uplift on disbursements and suppliers charges	%	
5.3	Daywork - Hourly labour rate	hr	
5.4	Minimum project value	Item	
5.5	Confined space equipment	Item	1
5.6			
5.7			
5.8			
6.0	Additional items		
6.1	De-scaling (fat/grime)	hr	1
6.2	De-scaling (scale using chain flails)	hr	
6.3	Gully surround	item	
6.4	Manhole works (up to 1.2m)	item	
6.6	Oversize soakaway (1.5m3)	item	
6.7	Soakaway >1.5m3	item	
6.8	Waste disposal	m	2
6.9	Shoring	m	0
Total Estimate Price For Recommendation Number			1.0
			0.00

Note: Subject to the attached Terms and Conditions

A - When calculating prices, all measurements are rounded up

C - Every effort will be made to match existing surfaces where disturbed although this cannot be guaranteed

G - Daywork rates do not include for materials that are charged at cost plus 25%

KEY: ne = not exceeding, eo = extra over rate, m = linear metre, nr = number, hr = hour

B - Depths are taken to the base of excavations

D - All rates exclude VAT

F - The above rates are subject to re-measurement

E - Depths are taken to the base of excavations

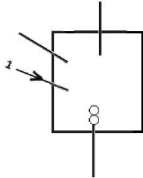
Coding Sheet		Sheet:		Site:	221 Goldhurst Terrace			
		Job No.:	505888					
		Date:	25/04/18	Client:	Cunningham Lindsey - Morley (Leeds)			

Run:	1								
From:		Rwg	Invert Level:		Direction:	D/S			
To:		MH1	Invert Level:		Function:	S/W			
Pipe Material:		VC	Pipe Dia:	100					
Water/Pressure Test:			Drain Break-In:	Yes	Gully Condition:	Poor			
Distance (m)	Code	Clock Ref at to	Dia mm	Intrusion % mm	Shared Run:	No			
					If Shared How:				
0.00	ST				Remarks	Surface Material	Length (m)		
0.00	LD				Line deviates down				
0.20	JDL				Joint displaced large	slabs			
0.60	GO				Line levels				
0.60	LL				Line deviates left				
1.20	FC	12	12		Fracture circumferential				
1.20	JDM				Joint displaced medium				
1.20	RFJ				Roots fine at joint				
1.80	CC	12	12		Crack circumferential				
1.80	RFJ				Roots fine at joint				
2.20	LD				slight				
2.50	JDM				Joint displaced medium				
2.60	CC	12	12		Crack circumferential				
2.60	RFJ				Roots fine at joint				
3.10	CC	12	12		Crack circumferential				
3.60	CC	12	12		Crack circumferential				
4.10	GO				Line levels				
4.50	FH				reached mh1				
Comments:									
too steep to push from mh1 (2.0m deep)									

Manhole Details	Sheet:	1 of 1	Site:	221 Goldhurst Terrace
	Job No.:	505888		
	Date:	25/04/18	Client:	Cunningham Lindsey - Morley (Leeds)

MH:- MH1

Depth:- 2000 (mm)



Chamber Dimension:- 600 / 450 (mm)

Depths of run if different to invert level:-

Run	Depth (mm)

Manhole Condition:- Good

Reasons for poor condition.

MH:-

Depth:- (mm)



Chamber Dimension:- / (mm)

Depths of run if different to invert level:-

Run	Depth (mm)

Manhole Condition:-

Reasons for poor condition.

MH:-

Depth:- (mm)



Chamber Dimension:- / (mm)

Depths of run if different to invert level:-

Run	Depth (mm)

Manhole Condition:-

Reasons for poor condition.

Key



Interceptor



Internal Back Drop.



External Back Drop.

Additional Comments for Poor Condition