169 West End Lane, NW6 2LH Basement Impact Assessment 35409.R.002.A.G.RJM May 2018

# Basement Impact Assessment

Using pro forma 1v0

169 West End Lane, NW6 2LH Basement Impact Assessment 35409.R.002.A.G.RJM May 2018

# 169 West End Lane NW6 2LH

Basement Impact Assessment
Planning reference (TBC)

For

LAD Construction Limited 35409.R.002.A.G.RJM May 2018

# **Revisions & additional material**



All documents within a BIA submission should clearly indicate the site address and planning reference number (where known).

Document numbers, revision numbers and dates of submission should be provided to ensure that only current documents are considered during the planning and BIA audit processes.

### **Document History and Status**

Revision	Date	Purpose/Status	File Ref	Author	Check	Review
[Type here]	[Type here]	[Type here]	[Type here]	[Type here]	[Type here]	[Type here]
1	May 2018	For Planning	35409.R.002.A.G.RJM	RJM	DDC	

### **Document Details**

Last saved	01/01/1601 00:00
Path	Document1
Author	RJM
Project Director	RJM
Project Num <b>ber</b>	35409
Project Name	169 West End Lane
Planning Reference No.	ТВС

# Additional supporting documents

Please note – the review process will be quicker if these are submitted as Word documents or searchable PDFs.

Date	Version	Produced by
May 2018	35409.R.001	Report on Desk Study & Site Investigation

### Please list all revisions here: 1

Date	Version	Produced by



All assessment and supporting documents should be referenced to ensure their inclusion in the planning and BIA audit processes.

# Contents

1.0	Non-Technical Summary	5
2.0	Introduction	7
2.1 2.2 2.3	. Sources of Information	8 8 9
3.0	Desk Study	11
3.1 3.2 3.3 3.4 3.5	<ul> <li>Geology</li> <li>Hydrogeology</li> <li>Hydrology, Drainage and Flood Risk</li> </ul>	11 11 11 11 12
4.0	Screening	13
4.2 4.3 4.4	. Surface Water and Flooding	13 14 15
5.0	Scoping	16
5.1 5.2 <b>Boo</b>	•	16 Error!
	. Example 3	Error!
5.4	okmark not defined. . Example 4 okmark not defined.	Error!
6.0	Site Investigation/Additional Assessments	17
6.1 6.2	· · · · · · · · · · · · · · · · · · ·	17 20
7.0	Construction Methodology/Engineering Statements	21
7.1. 7.2. 7.3. 7.4.	<ul> <li>Outline Temporary and Permanent Works Proposals</li> <li>Ground Movement and Damage Impact Assessment (if required)</li> </ul>	21 21 22 22
8.0	Basement Impact Assessment	23
8.2 8.3 8.4	. Hydrogeology and Groundwater Flooding	24 24 25

### **Appendices**

### Appendix 1: Desk Study References

- Site Location Plan
- Hampstead Heath Map (LB Camden GHHS figure 13)
- Hampstead Heath Surface Water Catchments & Drainage (LB Camden GHHS figure 14)

- WW2 Bomb Damage Map Extract
- Watercourses (LB Camden GHHS figure 11)
- Camden Surface Water Features (LB Camden GHHS figure 12)
- Geological Map Extract, BGS (Geology of Britain Viewer)
- Flooded Streets Map (LB Camden GHHS Figure 15)
- Slope Angle and Worked Ground Map (LB Camden GHHS figure 16)
- Areas of landslide potential (LB Camden GHHS figure 17)
- Flood Risk Map, Environment Agency (Surface Water Flood Risk)
- Groundwater Source Protection Zone (LB Camden GHHS Figure 8)
- Transport Infrastructure (LB Camden GHHS Figure 18)
- Historical Water Courses (LB Camden CGHHS Figure 11)

Appendix 2: Site Investigation Data

Appendix 3: Existing and Proposed Development Drawings

Appendix 4: Ground Movement and Damage Impact Assessment (Not Included)

Appendix 5: Structural Engineer's Statement and Calculations (Not Included)

Appendix 6: Arboricultural Report/Other Reports (Not required)

Appendix 7: Utility and Infrastructure Consultations (Thames Water Utilities Drawings)

# **Non-Technical Summary**

- 1.1.1. The site location is 169 West End Lane, London, NW6 2LH
- 1.1.2. The current site arrangement is a 4.5 storey (including basement with same footprint as ground floor) corner plot at the junction of West End Lane and Sherriff Road, approximately 40m south of West Hampstead Station. Lightwells extend from the southern side of the basement with grilled covers at ground floor level, which are flush with the adjoining pavement.
  - The existing basement is currently empty with some supporting walls and was previously used for storage. At ground floor level there is a dry cleaning shop and there is residential accommodation in the floors above.
- 1.1.3. The proposed development comprises a basement refurbishment to provide a residential flat. Around 70% of the existing lightwells will be retained, with steps providing access via the lightwell to the new basement flat. The remaining 30% of existing lightwell will be incorporated as an extension to the existing basement.
  - The existing floor level of the basement will be reduced from 8.24mAOD to 7.815mAOD, to provide additional headroom for a basement flat, and this will require excavation of the existing floor and underpinning of the existing walls.
- 1.1.4. The following assessments are presented:
  - Desk Study & Site Investigation
  - Screening
  - Scoping
  - Additional evidence/assessments
    - Site investigation
    - Infrastructure Drawings (Sewers)
    - Annotated extracts from Camden Geological, Hydrogeological and Hydrological Study
  - Impact Assessment

- 1.1.5. The authors of the assessments are Richard Moore, an Engineering Geologist with qualifications:

  BSc MSc FGS CGeol and more than 25 years experience of site investigation and construction, including a number of basements in Camden.
  - We have also consulted Jennifer Sturman, a Civil Engineer with qualifications CEng and MICE with more than 25 years experience and Darren Cook, a Structural Engineer with qualifications BEng, CEng, MIStructE, also with more than 25 years experience.
- 1.1.6. The ground and groundwater conditions beneath the site are such that the existing building is founded directly onto London Clay. A small quantity (<50mm depth) of perched water was found locally on top of the London Clay by investigations and it is anticipated that this is probably seasonal or a result of seepage from drains and sewers in the vicinity of the basement.
- 1.1.7. The construction methods proposed are hit-and -miss underpinning to deepen the foundations below the basement walls, followed by excavation of the existing floor slab and underlying clay, prior to forming the replacement basement floor as a ground bearing slab, incorporating a proprietary heave mitigation layer underneath.
- 1.1.8. A structural monitoring strategy to control the works and impacts to neighbouring structures will comprise condition surveys in advance of the works and precise levelling of monitoring studs mounted at representative locations on the existing property and party walls for the duration of the works.
- 1.1.9. The BIA has assessed land stability and the impacts of the proposed development on neighbouring structures are expected to be not greater than Category 1.
- 1.1.10. The BIA has identified no significant stability impacts associated with the proposed development.
- 1.1.11. The BIA has identified no potential hydrological impacts.
- 1.1.12. The BIA has identified no potential hydrogeological impacts
- 1.1.13. The BIA has identified a low flood risk for the proposed development.

# 2.0 Introduction

The purpose of this assessment is to consider the effects of a proposed basement development at 169 West End Lane, NW6 2LH on the local hydrology, geology and hydrogeology and potential impacts to neighbours and the wider environment. The site location is presented in Appendix 1.

The BIA approach follows current planning procedure for basements and lightwells adopted by LB Camden and comprises the following elements (CPG Basements):

- Desk Study;
- Screening;
- Scoping;
- Site Investigation, monitoring, interpretation. A ground movement assessment is not included because the scoping identified no specific issues.
- Impact Assessment

### 2.1. Authors

2.1.1. The BIA has been *authored/reviewed/approved by* Richard Moore, an Engineering Geologist with qualifications: BSc MSc FGS CGeol and more than 25 years experience of site investigation and construction.

We have also consulted Jennifer Sturman, a Civil Engineer with qualifications CEng and MICE with more than 25 years experience and Darren Cook, a Structural Engineer with qualifications BEng, CEng, MIStructE, also with more than 25 years experience.

### 2.2. Sources of Information

The Camden Geological, Hydrogeological and Hydrological Study - Guidance for Subterranean Development (produced by Arup, 2010) — is referred to throughout this document as CGHHS.

The following baseline data have been referenced to complete the BIA in relation to the proposed development:

- Site walkover/Description- Refer to Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018, provided in Appendix 2.
- Current/historical mapping- Refer to Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018, summarised in Appendix 2;
- Geological mapping- Refer to Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018, summarised in Appendix 2;
- Hydrogeological data- Refer to extracts from CGHHS provided in Appendix 1 and Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018, summarised in Appendix 2;
- Current/historical hydrological data- Refer to extracts from CGHHS provided in Appendix 1 and Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018, summarised in Appendix 2;
- Flood risk mapping- Refer to extracts from CGHHS provided in Appendix 1 and Groundsure Report provided with Report on Desk Study and Site Investigation, Ref: 35409.R.001.RJM, May 2018. Summarised in Appendix 2;
- LB Camden, Strategic Flood Risk Assessment (produced by URS, 2014);
- LB Camden, Floods in Camden, Report of the Floods Scrutiny Panel (2013);
- LB Camden, Planning Guidance (CPG) Basements (March 2018);
- LB Camden, Camden Geological, Hydrogeological and Hydrological Study (CGHHS) Guidance for Subterranean Development (produced by Arup, 2010);

- LB Camden, Local Plan Policy A5 Basements (2017);
- LB Camden's Audit Process Terms of Reference;
- ASUC Guidelines: Guidelines on safe and efficient basement construction directly below or near to existing structures

### 2.3. Existing and Proposed Development

- 2.3.1. The Application site is located at the junction of West End Lane and Sherriff Road. The site is not within a wider hillside setting and the immediate area surrounding the site is level. Slope angles in the vicinity of the site are less than 7deg. This is confirmed by a review of Ordnance Survey mapping and by reference to GHHS Figure 16.
- 2.3.2. The site slope angle is estimated to 0deg. to 1deg. The ground level at the site is approximately 54.0m
- 2.3.3. Description of current property/structures on site. The condition of the structures is stable with little to no evidence of historical movement
- 2.3.4. Description of adjacent properties/structures. The existing basement floor level is approximately 2.2m below ground level (51.8m) with foundation depth at 0.45m (51.35m) to 0.70m (51.1m) below existing basement floor level. The lightwell floor level is approximately 0.35m below existing basement floor level (51.45m).
- 2.3.5. None of the neighbouring buildings are Listed.
- 2.3.6. There are no gardens or trees directly bordering the site. The northern flank adjoins No171 along its length and the western flank adjoins the next door property on Sherriff Road. The southern and eastern sides face out onto Sherriff Road and West End Lane respectively. The nearest tree is a 9m high Elder located, 5m north in a private garden. (See photographs provided in Appendix 2).

A review of the geotechnical test results indicates that the formation soils underlying 169 West End Lane are unaffected by tree roots (i.e. Natural Moisture Content is greater than Liquid Limit +40% and Plastic Limited +2%).

Therefore, no further consideration is given to protection of trees or the effects of trees.

- 2.3.7. Adjacent infrastructure consists of the public footway to the south and east.
  - The mains sewer connection is marked by a manhole in the existing lightwell floor. Thames Water asset plans have been obtained and are provided in Appendix 1 for reference.
- 2.3.8. There is no major underground infrastructure present beneath/close to the site. (Refer to our summary of the Groundsure Report in Appendix 2 and CGHHS Figure 18 provided in *Appendix* 1).
- 2.3.9. Existing and Proposed development drawings are presented in Appendix 3.

- 2.3.10. The proposed development will utilise hit-and-miss underpinning methods to increase the foundation depths prior to excavation to form a new floor slab.
- 2.3.11. The outline construction programme will be developed in due course.
- 2.3.12. To conclude, the proposed improvements to the existing basement and lightwell are in full compliance with London Borough of Camden A5 Basement Requirements summarised below.

### **Note: Policy A5 Basement Requirements**

The siting, location, scale and design of basements must have minimal impact on, and be subordinate to, the host building and property. Basement development should:

- not comprise of more than one storey;
- not be built under an existing basement;
- not exceed 50% of each garden within the property;
- be less than 1.5 times the footprint of the host building in area;
- extend into the garden no further than 50% of the depth of the host building measured from the principal rear elevation;
- not extend into or underneath the garden further than 50% of the depth of the garden;
- be set back from neighbouring property boundaries where it extends beyond the footprint of the host building; and
- avoid the loss of garden space or trees of townscape or amenity value.

# 3.0 Desk Study

### 3.1. Site History

### 3.1.1. Summary of Historical Map Findings

On the first supplied map dated 1865, the site was an undeveloped plot of land, partly wooded.

From around 1894, a post office was depicted on site until around 1953 when the post office was no longer denoted. The structure does not appear to have changed significantly in this time.

The site is located in a city setting where there are a number of historic and current off site sources of contamination. However none of these are expected to represent a significant hazard to the subject site.

### 3.2. Geology

3.2.1. The British Geology Survey (BGS) map of the area (reference) geological mapping provided with the Groundsure Report confirms that the site is underlain by London Clay and this is confirmed on Figures 3, 4, 5, 6, 7, 16 and 17 of GHHS.

The geology on site has been confirmed in hand dug pits excavated in April 2018 and reported in Section 6 of this report and in the Report on Desk Study & Site Investigation provided in Appendix 2.

### 3.3. Hydrogeology

- 3.3.1. The geology underlying the site is classified as Unproductive Strata related to the underlying London Clay.
- 3.3.2. LB Camden data indicates the site is not within a groundwater source protection zone.

### 3.4. Hydrology, Drainage and Flood Risk

Relevant reference information is provided in:

- Watercourses, GHHS Figure 11
- Surface Water Features, GHHS Figure 12
- Hampstead Heath Surface Water Catchments and Drainage, GHHS Figure 14
- LB Camden, Strategic Flood Risk Assessment (Figure 6)
- Flood Map, GHHS Figure 12

Additional reference material is available from other sources e.g. Environment Agency.

- 3.4.1. The site is not located within 250m radius of surface water features.
- 3.4.2. The site is not located within 250*m of a historical water course.*
- 3.4.3. The site is not within the catchment of the Hampstead Heath Pond Chain, which is more than 1km north of the site.
- 3.4.4. The site surface area will not change. Refer to Appendix 3 for further details of the existing site layout.
- 3.4.5. The proposed surface area will be mostly hardstanding i.e. the same as existing. Refer to Appendix 3 for proposed site layout plans. The site run-off will not be increased by this development.
- 3.4.6. The site is classified as a very low risk of surface water flooding
- 3.4.7. The site is not within a Critical Drainage Area.

### 3.5. Other Information

The available information provided and referenced in this report is considered sufficient for this assessment.

# 4.0 Screening

# 4.1 Hydrogeology

### 4.1.1. A screening process has been undertaken and the findings are described below.

Question	Response	Details
1a. Is the site located directly above an aquifer?	No	London Clay is an Unproductive Strata.
1b. Will the proposed basement extend beneath the water table surface?	No	Trial pits dry with exception of minimal perched water on top of London Clay.
2. Is the site within 100mof a watercourse, well (used / disused) or potential spring line?	No	Refer to Appendix 1/ CGHHS.
3. Is the site within the catchment of the pond chains on Hampstead Heath?	No	Refer to Appendix 1/ CGHHS.
4. Will the proposed basement development result in a change in the proportion of hard surfaced / paved areas?	No	Refer to Appendix 3.
5. As part of site drainage, will more surface water (e.g. rainfall and run-off) than at present be discharged to the ground (e.g. via soakaways and/or SUDS)?	No	Refer to Appendix 3. No change to surface areas.
6. Is the lowest point of the proposed excavation (allowing for any drainage and foundation space under the basement floor) close to, or lower than, the mean water level in any local pond (not just the pond chains on Hampstead Heath) or spring line?	No	Site is located >1km south of the Hampstead Heath Pond Chains.

# 4.2. Slope Stability

Question	Response	Details
1. Does the existing site include slopes, natural or man-made greater than 7 degrees (approximately 1 in 8)?	No	Refer to Appendix 1/ CGGHS.
2. Will the proposed re-profiling of landscaping at the site change slopes at the property boundary to more than 7 degrees (approximately 1 in 8)?	No	Refer to Appendix 1/CGHHS & Appendix 3.
3. Does the development neighbour land, including railway cuttings and the like, with a slope greater than 7 degrees (approximately 1 in 8)?	No	Refer to Site Walkover & Appendix 1.
4. Is the site within a wider hillside setting in which the general slope is greater than 7 degrees (approximately1 in 8)?	No	Refer to Site Walkover & Appendix 1.
5. Is the London Clay the shallowest strata at the site?	No	Refer to Appendix 2- Property if founded o London Clay with some Made Ground above formation level.
6. Will any trees be felled as part of the development and/or are any works proposed	No	Refer to Appendix 2.

within any tree protection zones where trees are to be retained?		
7. Is there a history of seasonal shrink-swell subsidence in the local area and/or evidence of such effects at the site?`	No	There is a moderate risk highlighted by the Desk Study Report. Refer to findings in Appendix 2.
8. Is the site within 100m of a watercourse or a potential spring line?	No	Refer to Appendix 1/CGHHS.
9. Is the site within an area of previously worked ground?	No	Refer to Appenndix 1 & 2.
10. Is the site within an aquifer. If so, will the proposed basement extend beneath the water table such that dewatering may be required during construction?	No	London Clay is an Unproductive Strata.
11. Is the site within 50m of the Hampstead Heath Ponds?	No	Refer to Appendix 1. Site is more than 1KM south of the ponds.
12. Is the site within 5m of a highway or pedestrian right of way?	No	Refer to Appendix 1.
13. Will the proposed basement significantly increase the differential depth of foundations relative to neighbouring properties?	No	Refer to Appendix 1. Sketch of basement of 171 West End Lane.
14. Is the site over (or within the exclusion zone of) any tunnels, e.g. railway lines?	No	Refer to Appenndix 1. Site location plan & infrastructure detail map.

# 4.3. Surface Water and Flooding

Question	Response	Details
1. Is the site within the catchment of the ponds chains on Hampstead Heath?	No	The site more than 1km south of the pond chains.
2. As part of the proposed site drainage, will surface water flows (e.g. volume of rainfall and peak run-off) be materially changed from the existing route?	No	Refer to Appendix 1 & Appendix 2.
3. Will the proposed basement development result in a change in the proportion of hard surfaced / paved external areas?	No	The footprint of the building is not changing significantly. Refer to Appendix 3- Existing & proposed site layout plans.
4. Will the proposed basement result in changes to the profile of the inflows (instantaneous and long-term) of surface water being received by adjacent properties or downstream watercourses?	No	Refer to Appendix 1. There are no watercourses in close proximity to 169 West End Lane.
5. Will the proposed basement result in changes to the quality of surface water being received by adjacent properties or downstream watercourses?	No	Refer to Appendix 1. There are no watercourses in close proximity to 169 West End Lane.
6. Is the site in an area identified to have surface water flood risk according to either the Local Flood Risk Management Strategy or the Strategic Flood Risk Assessment or is it at risk from flooding, for example because the proposed basement is below the static water level of nearby surface water feature.	No	Refer to Appendix 1 & Desk Study data. No surface water features in close proximity to 169 West End Lane.

### 4.4. Non-Technical Summary of Screening Process

- **4.4.1.** The screening process is intended to identify issues to be carried forward to scoping for further assessment: Screening has **NOT** identified any issues to be carried forward to scoping for further assessment.
- 4.4.2. Other potential concerns considered within the screening process have been demonstrated to be not applicable or not significant when applied to the proposed development.

# 5.0 Scoping

The following issues have been brought forward from the Screening process for further assessment:

5.1.1. Screening has **NOT** identified any issues to be carried forward to scoping for further assessment.

# 6.0 Site Investigation/Additional Assessments

### 6.1. Site Investigation

### 6.1.1 Investigations

5No hand dug pits were excavated to expose the existing foundation details around the footprint of the basement and lightwell.

Beneath foundation formation level, a proportion of the holes were extended to greater depth by hand augering to confirm the underlying strata.

Laboratory testing has been carried out to classify the soil in terms of its index properties and potential effects on buried concrete. The results are attached in Appendix C.

### 6.1.2 Ground Conditions

Below the floor slab, which is typically not much more than 100-150mm thickness, there is generally a mixed fill consisting of clayey brick and stone which extends down to the base of the foundations at around 0.45m to 0.80m below existing slab level. Below this, the ground conditions are typically firm to stiff brown London CLAY.

Groundwater was encountered only as perched water on top of the London Clay in TP1 Section A-A and in TP5. The other holes were dry.

### 6.1.3 Existing Foundations

### Basement

Trial Pit TP1 in the north east corner of the existing basement confirmed a 2 brick corbel at 0.30m below existing basement slab level. In Section B-B under the West End Lane frontage, the corbel was founded on a 150mm layer of compacted brick fill over a 50mm layer of dry-packed crushed stone/lean mix concrete. In Section A-A, below the boundary with 171 West Lane, there was a 400mm thick layer of dry-packed gravel of brick and stone. Groundwater was struck in Trial Pit TP1 Section A-A but not in TP1 Section B-B. It is concluded that the water is perched within the gravel footing.

TP2 is on the southern side of the basement and encountered a 3 brick corbel resting on a 120mm concrete strip/pad foundation, placed on a 200mm layer of brick fill. No groundwater was encountered.

TP4 is located in the north west corner of the basement, at the foot of the stairs from ground floor. TP4 encountered a 300mm wide brick footing at basement floor level which extended to a depth of 0.27m below floor level and was founded on a 180mm thick layer of lime cemented brick fill. No groundwater was encountered.

In all cases, *insitu* firm to stiff or stiff brown London Clay was encountered at the base of the compacted brick fill foundations.

### Lightwell

Trial Pits TP3 and TP5 were excavated within the existing lightwell vaults.

TP3 was similar to TP1 and TP2 and consisted of a 3-brick corbel founded on a 400mm thick layer of lime cemented brick fill. A small amount of seepage occurred at the top of the London Clay and settled as 20mm of perched water on top of the *insitu* London Clay.

TP5 consisted of a single brick corbel resting on a 130mm thick layer of lime cemented brick fill. The clay in TP5 was more damp than in the other four trial pits and we suggest that this is (a) because there was an overhead pipe dripping persistently and (b) because the manhole within the lightwell connects to the mains sewers around this location.

### 6.1.4 Foundations & Retaining Walls

It is considered that the most appropriate foundation solution for the basement will be to underpin the existing walls so that that they are founded in the stiff London Clay at sufficient depth to accommodate the replacement ground floor slab.

It is suggested that hit-and-miss underpinning methods will be effective. It is possible that a limited amount of perched water may be encountered locally, but it should be feasible to dewater any underpins which encounter perched water by sump pumping. The groundworker undertaking the excavation and underpinning works should not commence any excavations until the necessary equipment is provided on site.

It is not anticipated that significant heave will be encountered but we advise that heave pressures should be calculated pro rata based on 10KN/m2 per metre of depth excavated and we recommend that a proprietary heave mitigation layer is provided underneath the floor slab.

The works should therefore be programmed to ensure that the basement floor slab is constructed without delay following excavation.

Ground gas is not identified as a significant issue at this site and we advise that tanking designed in accordance with BS8102 should provide sufficient mitigation against any residual ground gas risk.

The following parameters are considered appropriate for the design of any retaining walls to be incorporated in the scheme:

Parameter	London Clay		
φ'	21.50		
C,	1.0 KN/m <sup>2</sup>		
χ'	18.5 KN/m²		

### 6.1.5 Chemical Attack on Buried Concrete

Laboratory testing of soils has determined the following criteria for buried concrete in contact with the soils and localised perched groundwater at this site.

Design Sulphate Class: DS3
ACEC Classification: AC4

Groundwater should be considered as mobile across the site.

### 6.2. Additional Assessments

- Ivert - duto Secher 3.5

Provision of incomplete information or assessment will result in the BIA not being accepted as compliant with the relevant policies. This will result in either rejection of the planning application or the requirement to re-submit a policy compliant BIA. In addition to delaying the planning process, this is likely to incur additional costs for the Applicant.

### Where required, these may include:

- Arboricultural report;
- Condition Surveys;
- Asset Owner's Correspondence (e.g. TFL, Thames Water, etc);
- Flood Risk Assessment;
- Surface Water Drainage Strategy / SUDS Assessment;
- Others.

Note that all potential impacts identified during Screening and Scoping will need sufficient additional assessment to demonstrate that residual impacts are policy compliant.

Provision of incomplete information or assessment will result in the BIA not being accepted as compliant with the relevant policies. This will result in either rejection of the planning application or the requirement to re-submit a policy compliant BIA. In addition to delaying the planning process, this is likely to incur additional costs for the Applicant.

# 7.0 Construction Methodology/ Engineering Statements

### 7.1. Outline Geotechnical Design Parameters



Design parameters should be presented in accordance with the GHHS, Appendix G3. Note that for the purposes of Planning, all assessments should be reasonably conservative, in accordance with the relevant policies and Audit Terms of Reference.

7.1.1. Reasonably conservative geotechnical parameters have been determined, based on the site investigation data presented (in Section 6.1.4 and Appendix 2) and relevant technical guidance (as referenced in para 2.2 of this BIA).

The following parameters are considered appropriate for the design of any retaining walls to be incorporated in the scheme:

Parameter	London Clay
φ'	21.50
C,	1.0 KN/m²
χ'	18.5 KN/m <sup>2</sup>

### 7.2. Outline Temporary and Permanent Works Proposals

A description of temporary and permanent works should be provided, including sequencing and propping works. Sketches, drawings and outline retaining wall calculations are required. It should be demonstrated that bearing capacity is adequate. Settlements / heave / uplift should be calculated and be accounted for in the structural design. All resulting risks and impacts should be assessed and mitigated.

- **7.2.1.** The works proposals include....description of works, including:
  - Foundations hit-and-miss underpinning.
  - Retaining walls hit-and-miss underpinning.
  - Temporary works will include propping and localised sump pumping to remove shallow perched water where encountered on top of the London Clay.
  - Permanent structure Hit-and-miss underpinning and replacement floor slab
  - Drainage strategy/SUDS proposals- The site will be positively drained to existing sewers.
  - Flood risk mitigation measures The site is not identified as being at risk of flooding.

### 7.3. Ground Movement and Damage Impact Assessment

7.3.1. Due to the scale of the scheme and the findings of the BIA Screening (Section 4), a Ground Movement Assessment (GMA) has not been carried out. As it is reasonable to assume that movements will fall within Burland Range Category 0 (Negligible) and 1 (Very slight).

### 7.4. Control of Construction Works

- 7.4.1. The construction works will be closely controlled in accordance with *ASUC Guidelines*:

  Guidelines on safe and efficient basement construction directly below or near to existing structures
- 7.4.2. A structural monitoring strategy will be implemented to confirm that movements/damage impacts fall within the limits of the Burland Damage Category 0 to 1.

# 8.0 Basement Impact Assessment



The purpose of this assessment is to consider the effects of a proposed development on the local hydrology, geology and hydrogeology and impacts to neighbouring structures / properties.

### **Policy A5 Basement Requirements**

The Council will require applicants to demonstrate that proposals for basements:

- do not harm neighbouring properties, including demonstrating that the scheme poses a risk of damage to neighbouring properties no higher than Burland Scale 1 'very slight';
- avoid adversely affecting drainage and run-off or causing other damage to the water environment;
- avoid cumulative impacts;
- provide satisfactory landscaping, including adequate soil depth;
   and
- do not prejudice the ability of the garden to support trees where they are part of the character of the area.

### 8.1.1. The Conceptual Site Model (CSM) is...described below

The CSM should describe the changes to the site e.g. the current / proposed structural arrangements and levels relating existing and proposed levels to the ground model and, in the context of the ground and groundwater conditions, clearly illustrating potential risks, impacts, mitigation activities, residual impacts etc ideally including text and sketches.

- The proven ground conditions are, London Clay at formation level, with some Made Ground above.
- Groundwater level is not present, except for a small amount of perched water (<20mm) on top of the London Clay.
- The site is flat.
- The existing building is founded at 0.45m to 0.80m below existing basement slab level.
- The proposed development will be founded at approximately 51.05mAOD
- The depths of neighbouring foundations/basements are understood to be of similar depth.

- The distance to the footpath is 0.0m. The highway is approximately 5m away.
- There are no adjacent tunnels/utilities.
- No potential impacts have been identified.
- Proposed mitigation will be provided by undertaking monitoring & following a CMP &
   MS prepared by the structural engineer.
- No residual impacts are identified.

### 8.2. Land Stability/Slope Stability



Conclusions of assessment should be clearly presented here.

- 8.2.1. The site investigation has identified a suitable founding stratum of 52.05m.
- 8.2.2. The risk of movement and damage to this development due to shrink and swell of the London Clay) is *negligible to slight*.
- 8.2.3. The BIA has concluded that there will not be risk(s) or stability impact(s) to the development and/or adjacent sites due to slopes.

### 8.3. Hydrogeology and Groundwater Flooding



Conclusions of assessment should be clearly presented here.

- 8.3.1. The BIA has concluded there is a very low risk of groundwater flooding.
- 8.3.2. The BIA has concluded there are no impacts to the wider hydrogeological environment.

## 8.4. Hydrology, Surface Water Flooding and Sewer Flooding



Conclusions of assessment should be clearly presented here.

- 8.4.1. The BIA has concluded there is a low risk of surface water/sewer flooding.
- 8.4.2. The BIA has concluded there are no impacts to the wider hydrological environment.

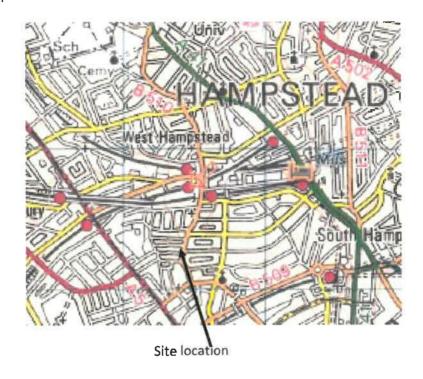
Appendices to be provided, as required

### Appendix 1: Desk Study References

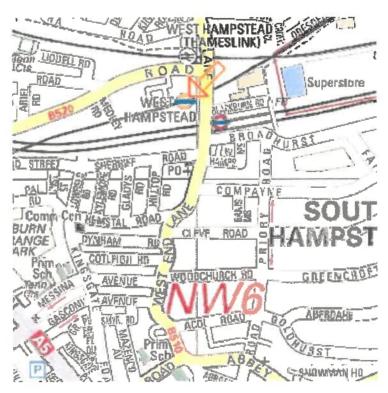
### **Site Location Plan**

Other mapping/data as required to evidence screening assessments, for instance:

Site Location



### **Site Location**

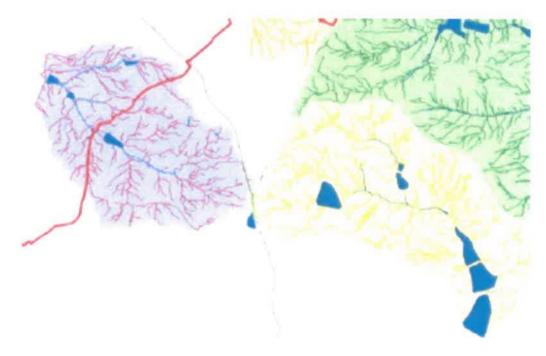


Hampstead Heath Map (LB Camden GHHS figure 13)



NOTE: The site lies more than 1km south

Hampstead Heath Surface Water Catchments & Drainage (LB Camden GHHS figure 14)



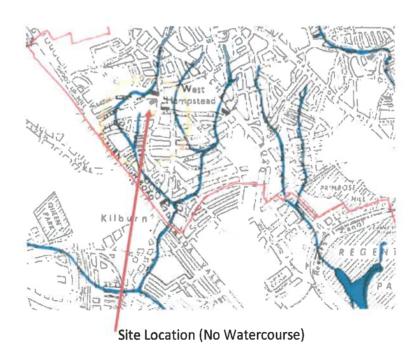
NOTE: The site lies more than 1km south

### WW2 Bomb Damage Map Extract

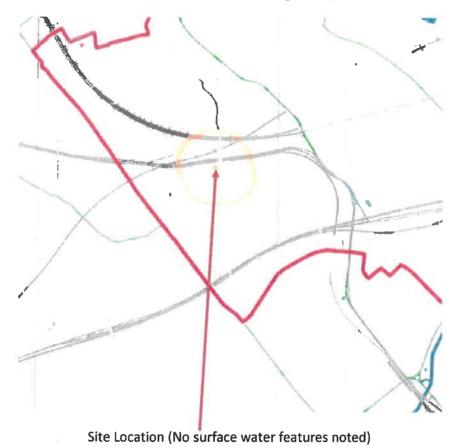


Site location (No damage indicated)

### Watercourses (LB Camden GHHS figure 11)



Camden Surface Water Features (LB Camden GHHS figure 12)



Geological Map Extract, BGS (Geology of Britain Viewer)

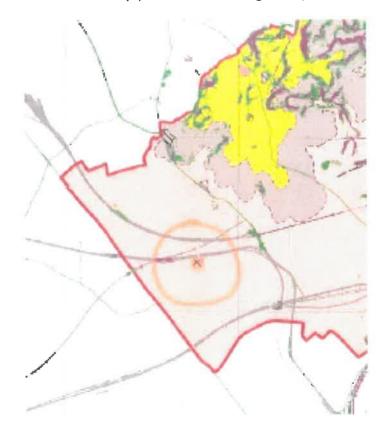


Flooded Streets Map (LB Camden GHHS Figure 15)

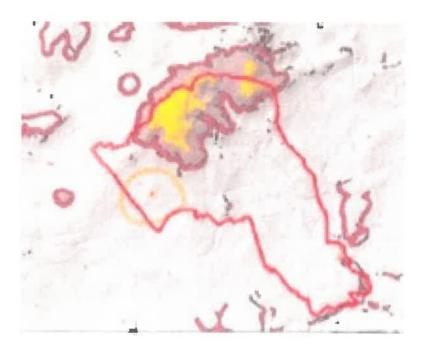


The highlighted streets are known to have flooded in 1975 and 2002 in West End Lane but it is understood that remedial actions described in `Flood in Camden` has reduced the risk of reoccurrence.

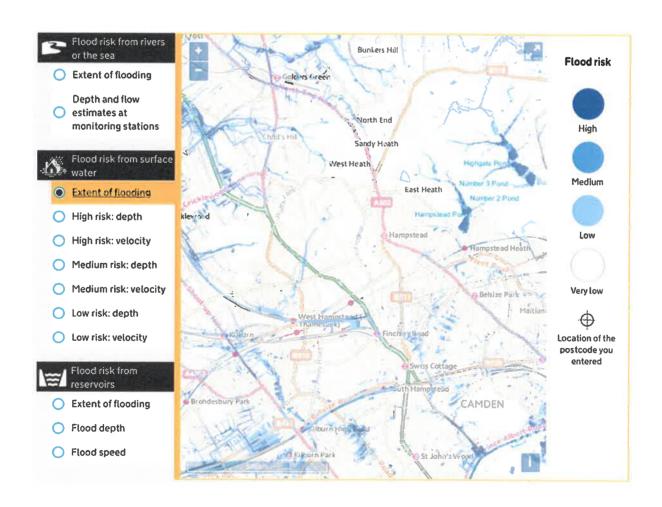
Slope Angle and Worked Ground Map (LB Camden GHHS figure 16)



Areas of landslide potential (LB Camden GHHS figure 17)



Flood Risk Map, Environment Agency (Surface Water Flood Risk)



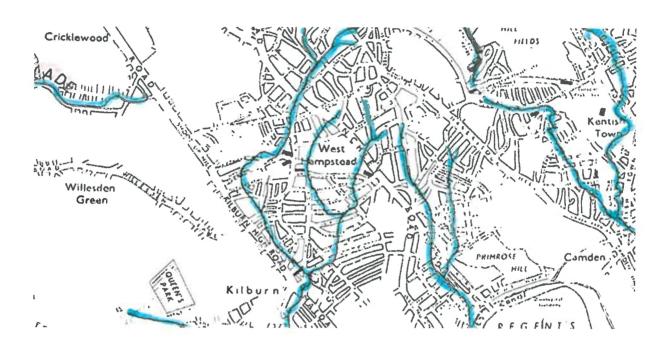
Groundwater Source Protection Zone (LB Camden GHHS Figure 8) e.g



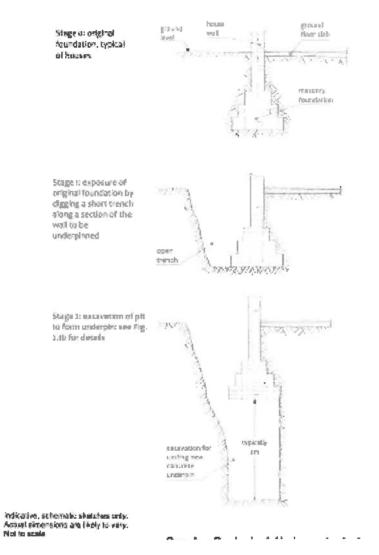
Transport Infrastructure (LB Camden GHHS Figure 18)



Historical Water Courses (LB Camden GHHS Figure 11)

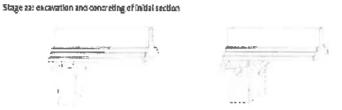


### • Typical underpinning construction sequence (LB Camden GHHS Figure 19)

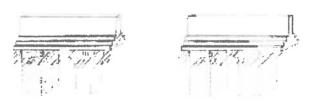




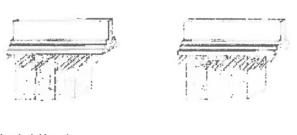
 Underpinning construction sequence with `hit and miss` pattern (LB Camden GHHS Figure 20)



Stage aix excavation and concreting of another section, not adjacent to first one



Stage 10: excavation and concreting of an intermediate section, so form configurate rows of underpin



kránczów a, authorna ko okonich pa proky Arthuak chroninikow a sow Skudy iko wary. Mot no oczake



10

# **Appendix 2: Site Investigation Data**

Extract taken from KHPL Desk Study-

#### 3. SITE INVESTIGATION

#### 3.1 Investigations

5No hand dug pits were excavated to expose the existing foundation details around the footprint of the basement and lightwell.

Beneath foundation formation level, a proportion of the holes were extended to greater depth by hand augering to confirm the underlying strata.

Laboratory testing has been carried out to classify the soil in terms of its index properties and potential effects on buried concrete. The results are attached in Appendix C.

## 3.2 Ground Conditions

Below the floor slab, which is typically not much more than 100-150mm thickness, there is generally a mixed fill consisting of clayey brick and stone which extends down to the base of the foundations at around 0.45m to 0.80m below existing slab level. Below this, the ground conditions are typically firm to stiff brown London CLAY.

Groundwater was encountered only as perched water on top of the London Clay in TP1 Section A-A and in TP5. The other holes were dry.

#### 3.3 Existing Foundations

#### **Basement**

Trial Pit TP1 in the north east corner of the existing basement confirmed a 2 brick corbel at 0.30m below existing basement slab level. In Section B-B under the West End Lane frontage, the corbel was founded on a 150mm layer of compacted brick fill over a 50mm layer of dry-packed crushed stone/lean mix concrete. In Section A-A, below the boundary with 171 West Lane, there was a 400mm thick layer of dry-packed gravel of brick and stone. Groundwater was struck in Trial Pit TP1 Section A-A but not in TP1 Section B-B. It is concluded that the water is perched within the gravel footing.



11

TP2 is on the southern side of the basement and encountered a 3 brick corbel resting on a 120mm concrete strip/pad foundation, placed on a 200mm layer of brick fill. No groundwater was encountered.

TP4 is located in the north west corner of the basement, at the foot of the stairs from ground floor. TP4 encountered a 300mm wide brick footing at basement floor level which extended to a depth of 0.27m below floor level and was founded on a 180mm thick layer of lime cemented brick fill. No groundwater was encountered.

In all cases, *insitu* firm to stiff or stiff brown London Clay was encountered at the base of the compacted brick fill foundations.

## Lightwell

Trial Pits TP3 and TP5 were excavated within the existing lightwell vaults.

TP3 was similar to TP1 and TP2 and consisted of a 3-brick corbel founded on a 400mm thick layer of lime cemented brick fill. A small amount of seepage occurred at the top of the London Clay and settled as 20mm of perched water on top of the *insitu* London Clay.

TP5 consisted of a single brick corbel resting on a 130mm thick layer of lime cemented brick fill. The clay in TP5 was more damp than in the other four trial pits and we suggest that this is (a) because there was an overhead pipe dripping persistently and (b) because the manhole within the lightwell connects to the mains sewers around this location.



#### 3.4 **Foundations & Retaining Walls**

It is considered that the most appropriate foundation solution for the basement will be to underpin the existing walls so that that they are founded in the stiff London Clay at sufficient depth to accommodate the replacement ground floor slab.

It is suggested that hit-and-miss underpinning methods will be effective. It is possible that a limited amount of perched water may be encountered locally, but it should be feasible to dewater any underpins which encounter perched water by sump pumping. The groundworker undertaking the excavation and underpinning works should not commence any excavations until the necessary equipment is provided on site.

It is not anticipated that significant heave will be encountered but we advise that heave pressures should be calculated pro rata based on 10KN/m2 per metre of depth excavated and we recommend that a proprietary heave mitigation layer is provided underneath the floor slab.

The works should therefore be programmed to ensure that the basement floor slab is constructed without delay following excavation.

Ground gas is not identified as a significant issue at this site and we advise that tanking designed in accordance with BS8102 should provide sufficient mitigation against any residual ground gas risk.

The following parameters are considered appropriate for the design of any retaining walls to be incorporated in the scheme:

London Clay	
21.5°	
1.0 KN/m²	
18.5 KN/m <sup>2</sup>	
	21.5 <sup>0</sup> 1.0 KN/m <sup>2</sup>

#### Chemical Attack on Buried Concrete

Laboratory testing of soils has determined the following criteria for buried concrete in contact with the soils and localised perched groundwater at this site.

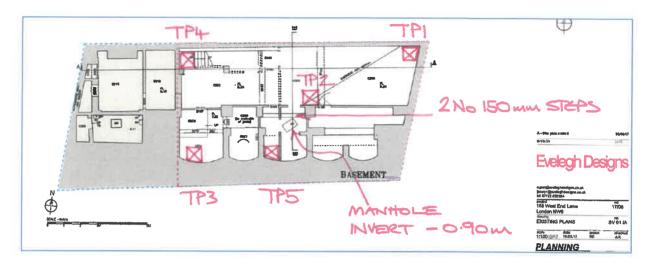
Design Sulphate Class: DS3

ACEC Classification: AC3

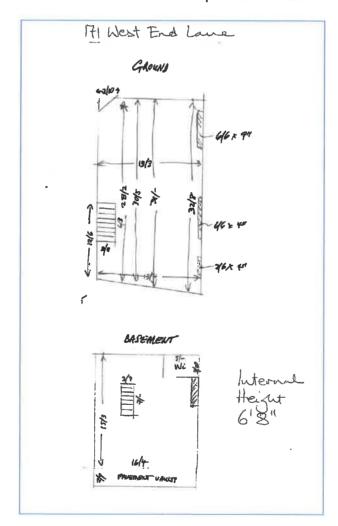
Groundwater should be considered as mobile across the site.



13

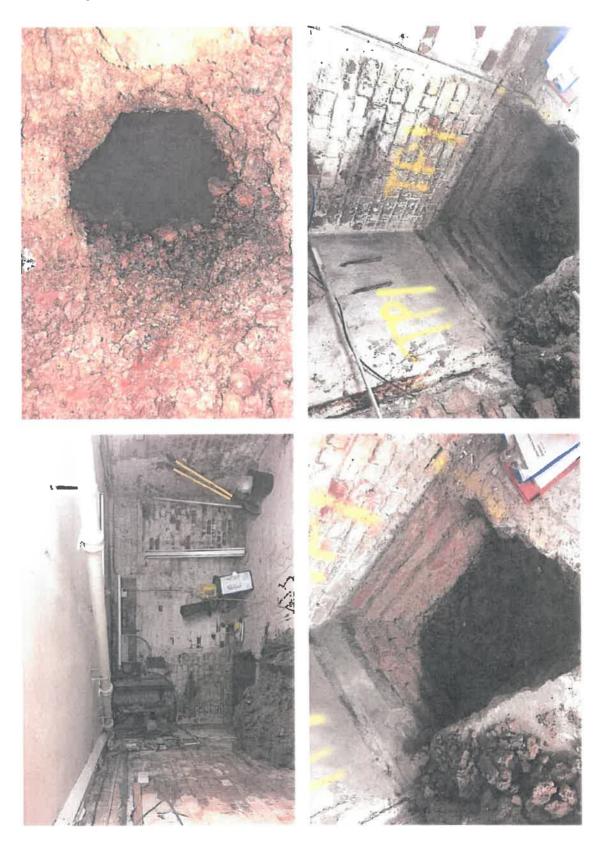


2.1 169 West End Lane Trial pit Location Plan.



2.2 Sketches of 171 West End Lane.





169 West End Lane – Trial Pit 1 (TP1)







169 West End Lane – Trial Pit 1 (TP1)







169 West End Lane – Trial Pit 3 (TP3)

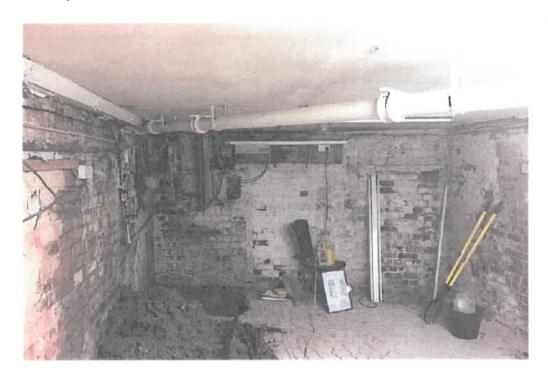






169 West End Lane – Trial Pit 4 (TP4)







169 West End Lane- Services mounted on existing basement ceiling.







169 West End Lane – Trial Pit 5 (TP5)







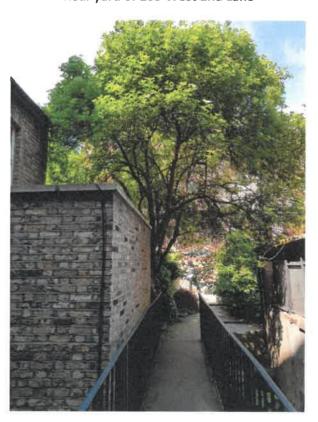


171 West End Lane- Existing basement & General Views.





Rear yard of 169 West End Lane



Trees noted adjacent to 169 West End Lane.



B B - B	& PARTNERS LTD RUCTURAL, CIVIL AL ENGINEERS	☐ Ashford 01	20 8232 1888 1233 502255 1924 269785 1827 307691	ISO 9001 REGISTERED FIRM
Project: 169 WE	STEND LINE	NWG	Job Ref: 35409	Page No.
Designed by:	Date:	Checked by:		Date: APR 18
than Programming and Common Action Action and Common Action Actio	PIT TPI ~	- Pun	A-A	B-B
MASONR	1 200 y	EXISTING BAS	EMENT FLOO	DE LEVEL
SALVEL OF BRICK OF	Dey 0001		- 7 Gloui	Disombal
	Firm to s	shiff brown	CUM.	
SECTIO	N B-B			
MASSNI	130 CONCE	7	inny rase look lev	MENT
150 BRICK F ON 50 DRY	PACK : : : : !	tiff brown (	CLAM	





KNAPP HICKS & PARTNERS LTD CONSULTING STRUCTURAL, CIVIL & GEOTECHNICAL ENGINEERS ☐ Brentford 020 8232 1888 ☐ Ashford 01233 502255

☐ Ossett 01924 269785 ☐ Tamworth 01827 307691

ISO 9001 REGISTERED FIRM

Project: 169 WESTEND LANE NW6 Job Ref: 35409 Page No.

Designed by: Date: Checked by: Date: APR'18

TRIAL PIT TP2.

MAJONRY

0.26

CONCRETE

0.60

BRICK FILL

0.80

Shff dry CLM





☐ Brentford 020 8232 1888 ☐ Ashford 01233 502255

☐ Ossett 01924 269785 ☐ Tamworth 01827 307691



Project: 169 WEST END	THE NE	16	Job Ref: 35409	Page No.
Designed by:	Date:	Checked by:		Date: APR 18

TRIAL AT TP3

LIME CEMBRIED

BY CLOSE STOR BELLOW BASEMENT FLOOR

LIME CEMBRIED

BY CLOSE STORED OF CLOSE BRICK WITTER

BY CON BY OF CLOSE

BY DEAD AND SITTY CLOSE

STORE BY DOWN SITTY CLOSE

O'SI

STORE BY DOWN

\* LOCALLY WITH CONCRETE OF IRRECULAR EXTENT + DEPTH





KNAPP HICKS & PARTNERS LTD CONSULTING STRUCTURAL, CIVIL & GEOTECHNICAL ENGINEERS ☐ Brentford 020 8232 1888 ☐ Ashford 01233 502255 ☐ Ossett 01924 269785

☐ Tamworth 01827 307691

ISO 9001 REGISTERED FIRM

Project: 169 WEST END LANE NW6 Job Ref: 35409 Page No.

Designed by: Date: Checked by: Date: APR 18

MAJONEY 300

BLEWEYS

MAJONEY

MIXED FILL

0'45

STIFF dry brown CLAY.



**KNAPP HICKS & PARTNERS LTD** CONSULTING STRUCTURAL, CIVIL. & GEOTECHNICAL ENGINEERS

☐ Brentford 020 8232 1888

☐ Ashford 01233 502255
☐ Ossett 01924 269785 ☐ Tamworth 01827 307691



Project: 169 WEST T	NA LANE	NWG	Job Ref: 35409	Page No.
Designed by:	Date:	Checked by:		Date: AAR 18

TRIAL AIT TP5

Soft to Firm damp CLM Becomes shap at Offm.

IR LOCALLY WITH CONCRETE OF IRREGULAR EXTENT & DEFITH





Peter Baxter Associates Laboratories A subsidiary of Peter Baxter Associates



Client: Knapp Hicks and Partners Report No: 1073/02/MC1

Address:

Prospect House

Your Ref: 35409G

1 Highpoint Business Village Henwood, Ashford

Report Date: 25/05/2018

TN24 8DH

Client Contact: Site:

Mr Richard Moore **West End Lane** 

Test Requested: Test Method:

Determination of Moisture Content, Liquid, Plastic Limits & Plasticity Index

BS 1377-2: 1990, Test Nos. 3.2; 4.4 (1 point LL); 5.3; 8 5.4

Sample Details:

Sampled and submitted by: Date Sampled: Date Received;

Client 19/04/2018

20/04/2018

24/04/2018

#### TEST RESULTS:

Laboratory	Client Referen	Client Reference			P.L	PJ	% Retained	Condition	Sample
Reference	BH/WS/TP	Ref.	(%)	(%)	(%)	(%)	on 425µm sleve	of Test	Type
1073/02/01	TP1 @ 0.85m	N/A	33	70	24	46	0	Netural	D
1073/02/03	TP1 @ 0.75m	N/A	33		-		-		D
1073/02/04	TP1 @ 0.85m	N/A	31				_	-	D
1073/02/05	TP1 @ 0.90m	N/A	31		-		_		D
1073/02/06	TP1 @ 1.00-1.25m	N/A	31	71	25	46	0	Netural	D
1073/02/07	TP1 @ 1.50-1.55m	N/A	29			-	-		D

#### Visual Descriptions:

Laboratory Reference	Client Reference	Description	
1073/02/01	TP1 @ 0.66m	Brown motified grey CLAY with selenite crystals	
1073/02/03	TP1 @ 0.75m	Brown mottled grey CLAY with selenite crystals	
1073/02/04	TP1 @ 0.85m	Brown CLAY	
1073/02/05	TP1 @ 0.90m	Brown mottled grey CLAY with selenite crystals	
1073/02/06	TP1 @ 1.00-1,25m	Brown mottled grey CLAY with selenite crystals	
1073/02/07	TP1 @ 1.50-1.55m	Brown CLAY with scienite crystals	

Note: All samples received for this job shall be disposed of after 28 days of this report.

.....END OF TEST REPORT.....

Kwaku Baah - Laboratory Manager

For and on behalf of PBA Laboratories

Page 1 of 1

Form: R2

Director Mr P Baxter CEng MICE

Registered Address Beaufort House, Rochester, Kent ME2 4FB

Company Registration 3028997





A subsidiary of Peter Baxter Associates



Client:

Knapp Hicks and Partners

Report No:

1073/02/MC2

Address:

Prospect House 1 Highpoint Business Village Hernwood, Ashford

Your Ref:

35409G

Report Date: 25/05/2018

TN24 8DH

Mr Richard Moore

Client Contact: 5ite:

West End Lane

Test Requested: Test Method:

Determination of Moisture Content, Liquid, Plastic Limits & Plasticity Index 88 1377-2: 1990, Test Nos. 3.2; 4.4 (1 point LL); 5.3; & 5.4

Sample Details:

Sampled and submitted by: Date Sampled: Date Received:

Client 19/04/2018

Date Tested:

20/04/2018

24/04/2018

#### TEST RESULTS:

Laboratory	Client Referer	100	MC	LL	P.L	P.I	% Retained	Condition	Sample
Reference	BH/W9/TP	Ref.	(%)	(%)	(%)	(%)	on 425µm sleve	of Test	Type
1073/02/08	TP3 @ 0.50-0.80m	N/A	33	-	-	_	-		D
1073/02/09	TP4 @ 0.45-0.60m	N/A	31		-		-		D
1073/02/10	TP4 @ 0.60-0.80m	N/A	30		-				D
1073/02/11	TP4 @ 1.16m	NA	29	-					D
1073/02/12	TP4 @ 1.25-1.55m	N/A	29	-	_	-	-		D

#### Visual Descriptions:

Laboratory Reference	Client Reference	Description	
1073/02/08	TP3 @ 0.50-0.80m	Brown mottled grey CLAY with selenite crystals	
1073/02/09	TP4 @ 0.45-0.60m	Brown mottled gray CLAY with selenite crystals	
1073/02/10	TP4 @ 0.60-0.80m	Brown mottled grey CLAY with selenite crystals	
1073/02/11	TP4 @ 1.15m	Brown mottled grey CLAY with selenite crystals	
1073/02/12	TP4 @ 1.25-1.55m	Brown mottled grey CLAY with selenite crystals	

Note: All samples received for this job shall be disposed of after 28 days of this report.

.....END OF TEST REPORT.....

Kwaku Baah - Laboratory Manager

For and on behalf of PBA Laboratories

Page 1 of 1

Form: R2

Director Mr P Baxter CEng MICE

Registered Address Beaufort House, Rochester, Kent MEZ 4FB

Company Registration 3028997





A subsidiary of Peter Baxter Associates Kestner Works

Report No: 1073/02/CH1

Your Rat 35409G Report Date: 25/05/2018



Knapp Hicks and Partners

Prospect House 1 Highpoint Business Village Hernwood, Ashfurd Kent TN24 BDH

Cliant Contact: Site:

Mr Richard Moore West End Lane

Test Requested: Test Method;

Determination of pH Value and Sulphate Content BS 1377-3: 1990, Clauses 5.5 & 9.5

Sample Detalls:

TEST RESULTS:

Laboratory	Client Reference		Soil Sulphates as 60 <sub>4</sub>		Water Sulphates as \$04	ulphates		Passing Passing Zoun test	Description				
Reference	BH/WS	Sample Ref	Total (%)	Water Soluble	(ar.)	<b>,</b>	CLASS* sieve (%)						Description
1073/02/01	TP1 @ 0.65m	N/A	-	2.1		6.7	DS-3	100	Brown motited grey CLAY with selentin crystals				
1073/02/02	TP1 @ 0.70m	N/A	-	0.0		6.2	D5-1	N/A	Water				

<sup>\*</sup> Classification based on Tables C1 & C2: BRE Special Digest 1:2005

.....END OF TEST REPORT.....

Kwaku Baah - Laboratory Manager

For and on behalf of PBA Laboratories

Page 1 of 1

Form: R10





Peter Baxter Associates Laboratories A subsidiary of Peter Baxter Associates Kestner Works Bredgar Road Gillingham

www.peterbaxterassociates.co.uk E. info@peterbaxterassociates.co.uk T.+44 (0) 1634 234332 / 717974

Report No: 1073/02/CH2

Your Ref: 35409G Report Date: 25/05/2018



Client:

Knepp Hicks and Partners

Address:

Prospect House 1 Highpoint Susiness Villege Herwood, Ashford Kent TN24 6DH

Client Contact: Site:

Mr Richard Moore West End Lane

Test Requested: Yest Method:

Determination of pH Value and Sulphate Content

BS 1377-3: 1990, Clauses 5.5 & 9.5

Samole Details:

Sampled and submitted by: Date Sampled: Onte Received: Date Tested:

Client 19/04/2018 20/04/2018 25/04/2018

TEST RESULTS:

Laboratory	Client Reference		Soli Sulphates as 80 <sub>4</sub>		Water Sulphales as \$04	lphates		Pessing 2mm test sieve	Description
Reference	BH/WS	Sample Ref	Total (%)	Wester Soluble (p/L)	(g/L)		CLASS*	(%)	
1073/02/09	TP4 @ 0.45-0.50m	N/A		1.9		6.8	DS-3	100	Brown mottled grey CLAY with selector crystals
1073/02/11	TP4 @ 1,15m	N/A	•	2.5	-	6.8	DS-3	100	Brown moltied grey CLAY with selentia crystals

<sup>\*</sup> Clessification based on Tables C1 & C2: BRE Special Digest 1:2006

.....END OF TEST REPORT.....

Kwaku Bash - Laboratory Manager

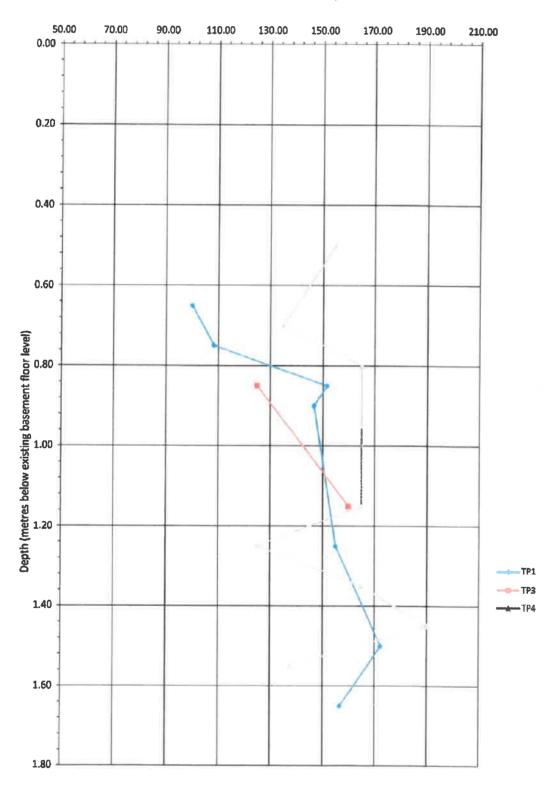
For and on bahalf of PBA Laboratories

Page 1 of 1

Form: R10

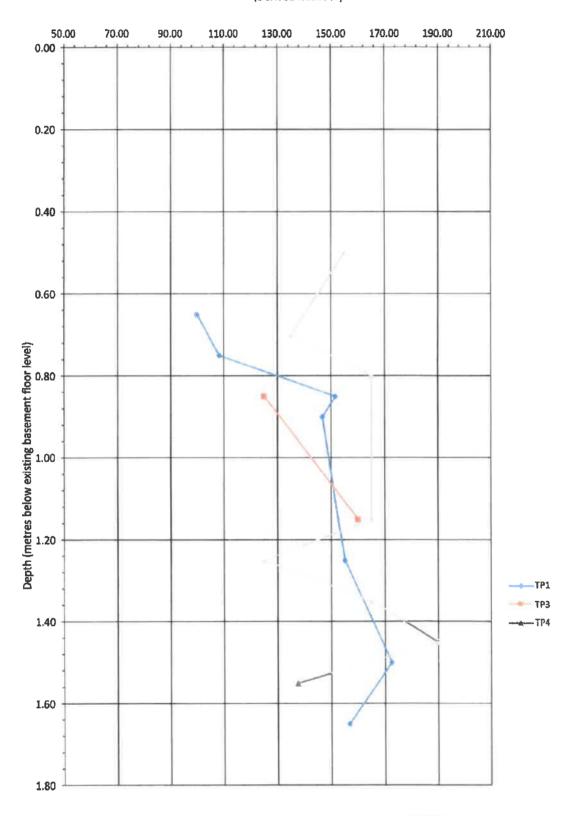


# 169 West End Lane, London, NW6 Undrained Shear Strength (Kn/m2) (Derived from PP)





# 169 West End Lane, London, NW6 Undrained Shear Strength (Kn/m2) (Derived from PP)





**Appendix 3: Existing and Proposed Development Drawings** 







# Lower Ground Floor

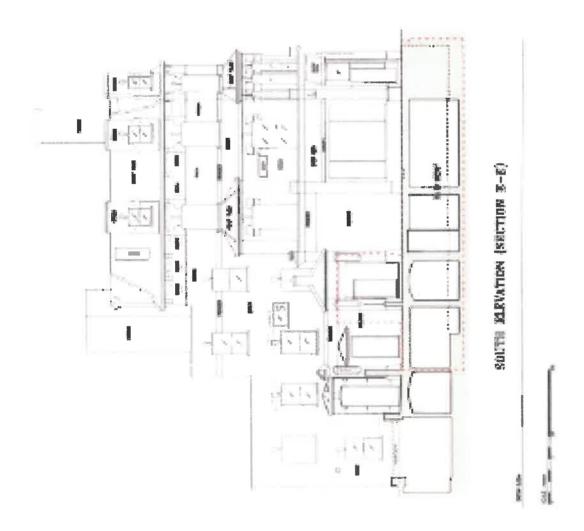
# **Ground Floor**

	1717	Hasi E	ind Lu	óé, MVI		
TOT	PAL IHM	ER44	L MLDX	39 AREA : 64 2 BO WETHES	AV6	
	1	!	4		SCALE 1:100 (mast 1:1250)	$\rightarrow$

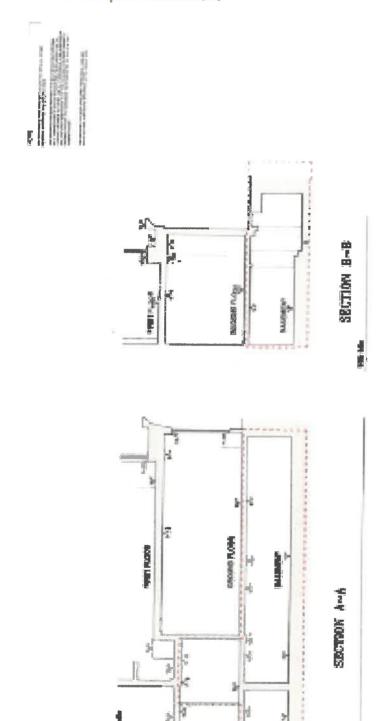










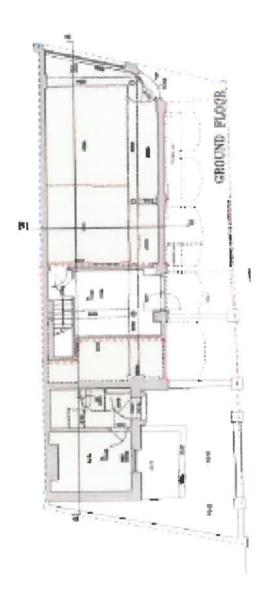


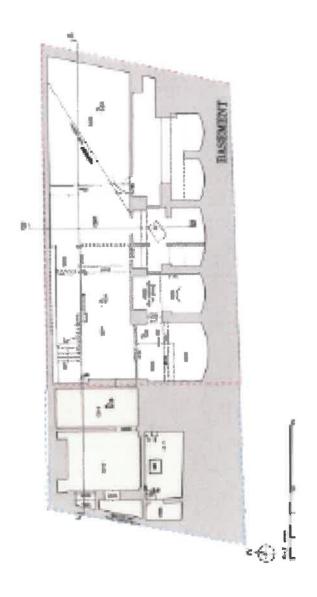








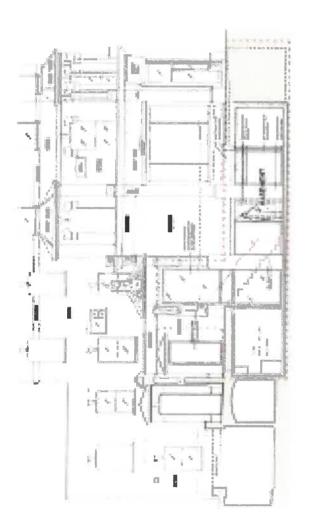


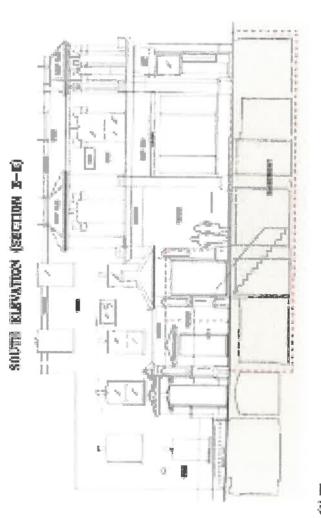






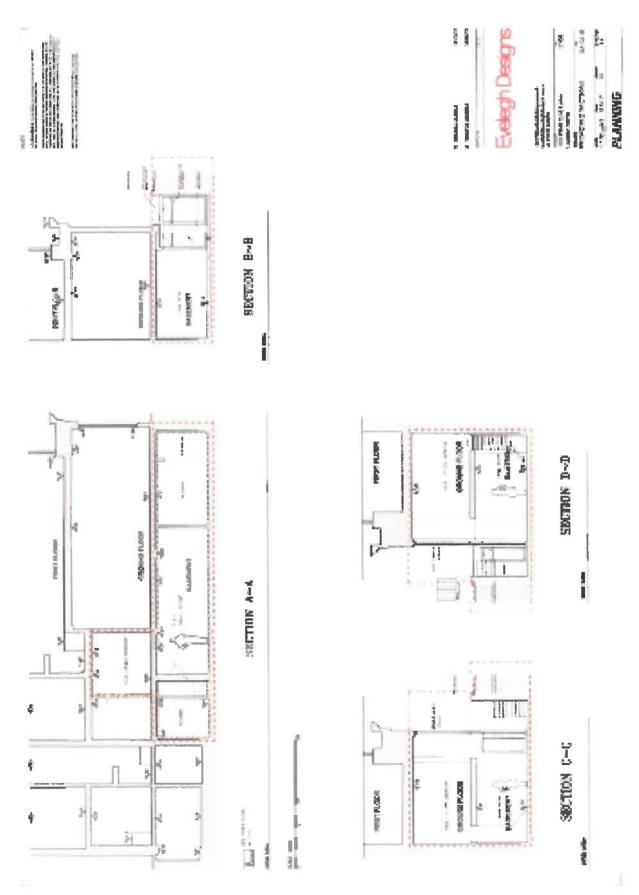






SUMME BENNION







# **Appendix 4: Ground Movement and Damage Impact Assessment**

**NOT REQUIRED- REFER TO SCREENING IN SECTION 4** 



# **Appendix 5: Structural Engineer's Statement and Calculations**

NOT REQUIRED- REFER TO SCREENING IN SECTION 4



# Appendix 6: Arboricultural Report/Other Reports (as required)

**NOT REQUIRED- REFER TO SCREENING IN SECTION 4** 



# **Appendix 7: Utility and Infrastructure Consultations**

# Asset location search



Knapp Hicks & Partners Ltd Unit 1 Highpoint Business Vil

ASHFORD TN24 8DH

Search address supplied

WEST END LANE

169

West End Lane London NW6 2LH

Your reference

35409G

Our reference

ALS/ALS Standard/2018\_3789044

Search date

6 May 2018

#### Keeping you up-to-date

Founded in A College being mississipped antennal to a conjugate, and more. The benefit of the notice of a black community and advanced in the new party of the found in the new party of the new party of the found in the new party of the new part

An absence of the design provings into injury methods and a relative of the war. When the analysis and injury to assess the coloring details of the analysis of the coloring details of the analysis of the coloring details and the coloring design of the coloring details and the coloring details are coloring details and the coloring details are coloring details and the coloring details and the coloring details and the coloring details are coloring details.



Thames Weter utilities Ltd Property Searches, PD Box 3189, Slough St 1 #WW DX 151280 Slough 13



searches@thameswater.co.co. www.thameswater.propertysearches.co.co.



0845 070 5146







Search address supplied: WEST END LANE, 169, West End Lane, London, NW6 2LH

Dear Sir / Madam

An Asset Location Search is recommended when undertaking a site development. It is essential to obtain information on the size and location of clean water and sewerage assets to safeguard against expensive damage and allow cost-effective service design.

The following records were searched in compiling this report: - the map of public sewers & the map of waterworks. Thames Water Utilities Ltd (TWUL) holds all of these.

This searchprovides maps showing the position, size of Thames Water assets close to the proposed development and also manhole cover and invert levels, where available.

Please note that none of the charges made for this report relate to the provision of Ordnance Survey mapping information. The replies contained in this letter are given following inspection of the public service records available to this company. No responsibility can be accepted for any error or omission in the replies.

You should be aware that the information contained on these plans is current only on the day that the plans are issued. The plans should only be used for the duration of the work that is being carried out at the present time. Under no circumstances should this data be copied or transmitted to parties other than those for whom the current work is being carried out.

Thames Water do update these service plans on a regular basis and failure to observe the above conditions could lead to damage arising to new or diverted services at a later date.

#### **Contact Us**

If you have any further queries regarding this enquiry please feel free to contact a member of the team on 0845 070 9148, or use the address below:

Thames Water Utilities Ltd Property Searches PO Box 3189 Slough SL1 4WW

Email: searches@thameswater.co.uk

Web: www.thameswater-propertysearches.co.uk

**Camden** 



#### **Waste Water Services**

Please provide a copy extract from the public sewer map.

Enclosed is a map showing the approximate lines of our sewers. Our plans do not show sewer connections from individual properties or any sewers not owned by Thames Water unless specifically annotated otherwise. Records such as "private" pipework are in some cases available from the Building Control Department of the relevant Local Authority.

Where the Local Authority does not hold such plans it might be advisable to consult the property deeds for the site or contact neighbouring landowners.

This report relates only to sewerage apparatus of Thames Water Utilities Ltd, it does not disclose details of cables and or communications equipment that may be running through or around such apparatus.

The sewer level information contained in this response represents all of the level data available in our existing records. Should you require any further Information, please refer to the relevant section within the 'Further Contacts' page found later in this document.

#### For your guidance:

- The Company is not generally responsible for rivers, watercourses, ponds, culverts
  or highway drains. If any of these are shown on the copy extract they are shown for
  information only.
- Any private sewers or lateral drains which are indicated on the extract of the public sewer map as being subject to an agreement under Section 104 of the Water industry Act 1991 are not an 'as constructed' record. It is recommended these details be checked with the developer.

## **Clean Water Services**

Please provide a copy extract from the public water main map.

Enclosed is a map showing the approximate positions of our water mains and associated apparatus. Please note that records are not kept of the positions of individual domestic supplies.

For your information, there will be a pressure of at least 10m head at the outside stop valve. If you would like to know the static pressure, please contact our Customer Centre on 0800 316 9800. The Customer Centre can also arrange for a full flow and pressure test to be carried out for a fee.

Thomas William Ltd., Property Searches, PO Box 3189, Slough SL1 4WW. DX 151280 Slough 13 T 0845 070 9148 E marches of the reverse course 1 may be presented to the control of the control

Page 3 of 12



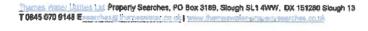


## For your guidance:

- Assets other than vested water mains may be shown on the plan, for information only.
- If an extract of the public water main record is enclosed, this will show known public
  water mains in the vicinity of the property. It should be possible to estimate the
  likely length and roule of any private water supply pipe connecting the property to
  the public water network.

## Payment for this Search

A charge will be added to your suppliers account.







#### Further contacts:

## Waste Water queries

Should you require verification of the invert levels of public sewers, by site measurement, you will need to approach the relevant Thames Water Area Network Office for permission to lift the appropriate covers. This permission will usually involve you completing a TWOSA form. For further information please contact our Customer Centre on Tel: 0845 920 0800. Alternatively, a survey can be arranged, for a fee, through our Customer Centre on the above number.

If you have any questions regarding sewer connections, budget estimates, diversions, building over issues or any other questions regarding operational issues please direct them to our service desk. Which can be contacted by writing to:

Developer Services (Waste Water) Tharnes Water Clearwater Court Vastern Road Reading RG1 8DB

Tel:

0800 009 3921

Email:

developer services@thameswater co.uk

#### Clean Water queries

Should you require any advice concerning clean water operational issues or clean water connections, please contact:

Developer Services (Clean Water) Thames Water Clearwater Court Vastern Road Reading RG1 8DB

Tel:

0800 009 3921

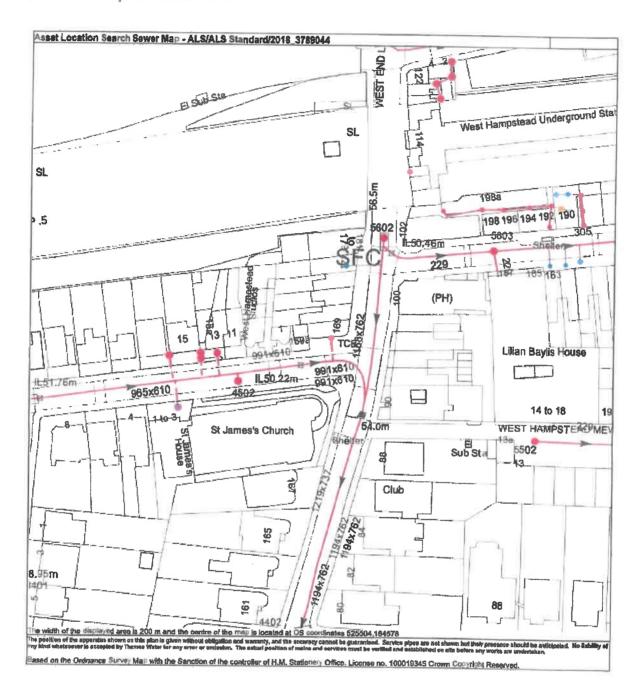
Email:

developer, services@thameswater co.uk

Property Searches, PO Box 3169, Slough SL1 4WW, DX 151280 Sough 13 T 0845 070 9148 E

Page 5 of 12





Pagu 6 of 12



MB. Levels quoted in metres Ordnance Newlyn Daturn. The value -9999,00 indicates that no survey information is available

Manhole Reference	Manhole Cover Level	Manhole Invert Level
581G	n/a	n/a
561F	nra	n/a
561E	nía	nia
56CD	nis	nia
S6CC	nia	sa/a
56CI	nja	rufe
56¢B	n/a	nte
561A	n/a	rofe .
61B	nia	pia
#BG	n/a	trin
4BE	n/s	gia
is be	กร	ria
K8H	nia	n/a
504	min	n/e
151A	nia	n/a
S05	s/a	m/a
507	n/a	nia
506	nia	rife
502	nia	hía
51A	nti	nfa
61C	n/a	ata
682	55.69	43.65
61D	n/a	n/a
6DC	n/a	nia
6DB	n/a	nia
803	83.54	49.43
ADA.	n/a	n/a
EC)	n/a	n/a
502	52.33	50.48
6DD	ola	
	1770	m/a

The position of the apparatus whome on the plant is given without obligation and warranty, and the accuracy cannot be generated. Service pipes are not not shown but their presence should be articipated. No faintify of any kind whatevever is accepted by Thomas Water for any error or omission. The actual position of notine and corridors result be verified and application on this before any works are benefit or the property of th

the rock superstanting strategies to assive time sensetiate statement carriedate of the receiption of the statement of the receiption of t

den

Towas Wiles.

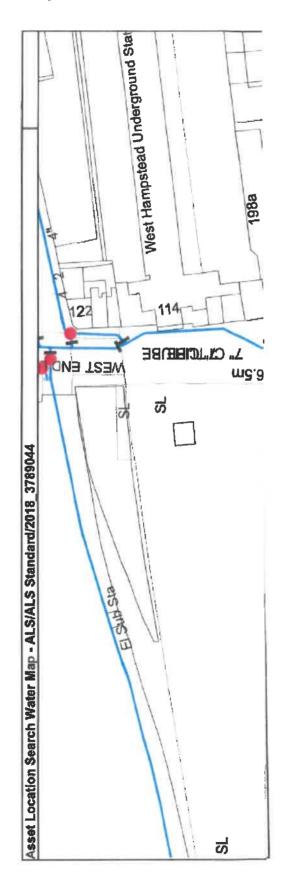
| Property Scarring, Pro Box 3149, Slough 9L1 4W, DX 151289 Slough 13
| Ones 5070 8148 E

 Most private place are not shown on our plans, se in the past, this whomselon not been recorded.

1) All louds essentiated with the plans ene to Crohemee Dealm Novidyn.
2) All measurements on the plans are metho.
3) Arrows (on gravity field sowers) or flectos (on risking methal) indicate.

Page 8 of 12

ALS Sewer Map Key





# Moter | ALS Water Map Key

# Water Pipes (Operated & Maintained by Thames Water)

- Distribution Main: The most common pipe shown on water maps. With few exceptions, domestic connections are only made to distribution mains.
- Trunk Mahri. A main carrying water from a source of supply to a treatmentplant or reservoir or from one treatment plant or reservoir to another. Also a main transferting weter in bulk to smaller water mains used for supplying individual contomers. İs

Other (Proposed)

Booster Stations

General PurposeValve

-

Pressure Control/alve Customer Valve

Air Valve

0

Operational Sites

Service Reservoir Pumping Station

4

Shaft Inspection Theatmen! Works

0

Single Hydrant

Meters -

Hydrants

×

0

- Supply Main: A supply main indicates that the water main is used as a supply for a single property or group of properties. ACRES E
- Fire Main: Where a pipe is used as a fire supply, the word FIRE will be displayed along the pipe. 1
- supplies waier for a single property or group of properties and final quantity of water passing through the pipe is matered even though there may be no meter symbol shown. Metered Pipe: A metered main indicates that the pipe in question THEFT
- Transmission Tunnel: A very large diameter water pipe. Most functs are build very deep tradenground. These pipes are not expected infect the structural integrity of buildings shown on the map provided.
- Proposadiffain: A main that is still in the pleaningstages or in the proposad main and its process of being laid. More details of the proposad main and its reference rumber are generally included near the main.

# Other Symbols

Symbol indicating what happens at the and of L. a woter main.

End Items

Blank Flange

Water Tower

Unknown

٥

Data Logger	
0	

Capped End	Emptying Pit	Undefined End	Marifold	Cuatomer Supply
	0	<b>(3</b> )	<b>MP3</b>	

аютег в	Supply
3	Ē

# Other Water Pipes (Not Operated or Mainlained by Thomas Water)

Private Blake, Incident final tine verter main in question le not owned by Themes Water, These mains mentally have text ensectated with them indicating the districtor and owner of the pipe.

11E0mm (3'8")

30Pmm - 600mm (12" - 24")

900mm (3°)

DEPTH BELOW GROUND

PIPE DIAMETER Up to 300mm (12") Themse Water Utilines Ltd., Properly Searches, PO Box 3189, Stough St.1 4W, DX 151260 Stough 13

T 0845 GT0 9148 E searchean them

1200mm (4°) SBOrner and bigger (24" plus)



#### **Terms and Conditions**

All sales are made in accordance with Thames Water Utilities Limited (TWUL) standard terms and conditions unless previously agreed in writing.

- 1. All goods remain in the property of Thames Water Utilities Ltd until full payment is received.
- 2. Provision of service will be in accordance with all legal requirements and published TWUL policies.
- All Invoices are strictly due for payment 14 days from due date of the invoice. Any other terms must be accepted/agreed in writing prior to provision of goods or service, or will be held to be invalid.
- Thames Water does not accept post-dated cheques-any cheques received will be processed for payment on date of receipt.
- 5. In case of dispute TWUL's terms and conditions shall apply.
- Penalty Interest may be invoked by TWUL in the event of unjustifiable payment delay. Interest charges will be in line with UK Statute Law 'The Late Payment of Commercial Debts (Interest) Act 1998'.
- 7. Interest will be charged in fine with current Court Interest Charges, if legal action is taken.
- 8. A charge may be made at the discretion of the company for increased administration costs.

A copy of Thames Water's standard terms and conditions are available from the Commercial Billing Team (cashoperations@thameswater.co.uk).

We publish several Codes of Practice including a guaranteed standards scheme. You can obtain copies of these leaflets by calling us on 0800 316 9800

If you are unhappy with our service you can speak to your original goods or customer service provider. If you are not satisfied with the response, your complaint will be reviewed by the Customer Services Director. You can write to her at: Thames Water Utilities Ltd. PO Box 492, Swindon, SN38 8TU.

If the Goods or Services covered by this invoice falls under the regulation of the 1991 Water Industry Act, and you remain dissatisfied you can refer your complaint to Consumer Council for Water on 0121 345 1000 or write to them at Consumer Council for Water, 1st Floor, Victoria Square House, Victoria Square, Birmingham, B2 4AJ.

#### Ways to pay your bill

Credit Card	BACS Payment	Telephone Banking	Cheque
Call 0845 070 9148 quoting your invoice number starting CBA or ADS / OSS	Account number 90478703 Sort code 60-00-01 A remittance advice must be sent to: Thames Water Utilities Ltd., PO Box 3189, Slough SL1 4WW. or email as.biffing@thameswater.	By calling your bank and quoting: Account number 90478703 Sort code 60-00-01 and your involce number	Made payable to 'Thames Water Utilities Ltd' Wite your Thames Water account number on the back. Send to: Thames Water Utilities Ltd., PO Box 3189, Slough 51 4WW or by DX to 151280 Slough 13

Thames Water Utilities Ltd Registored in England & Wales No. 2366661 Registered Office Clearwater Court, Vastern Rd, Reading, Barks, RG1 &DB.







#### Search Code

#### IMPORTANT CONSUMER PROTECTION INFORMATION

This search has been produced by Thames Water Property Searches, Clearwater Court, Vastern Road, Reading RG1 8DB, which is registered with the Property Codes Compliance Board (PCCB) as a subscriber to the Search Code. The PCCB independently monitors how registered search firms maintain compliance with the Code.

#### The Search Code:

- provides protection for homebuyers, sellers, estate agents, conveyancers and mortgage lenders who
  rely on the information included in property search reports undertaken by subscribers on residential
  and commercial property within the United Kingdom
- sets out minimum standards which firms compiling and selling search reports have to meet
- promotes the best practise and quality standards within the industry for the benefit of consumers and property professionals
- enables consumers and property professionals to have confidence in firms which subscribe to the code, their products and services.

By giving you this information, the search firm is confirming that they keep to the principles of the Code. This provides important protection for you.

## The Code's core principles

Firms which subscribe to the Search Code will:

- · display the Search Code logo prominently on their search reports
- · act with integrity and carry out work with due skill, care and diligence
- at all times maintain adequate and appropriate insurance to protect consumers
- · conduct business in an honest, fair and professional manner
- · handle complaints speedily and fairly
- ensure that products and services comply with industry registration rules and standards and relevant laws
- · monitor their compliance with the Code

## Complaints

If you have a query or complaint about your search, you should raise it directly with the search firm, and if appropriate ask for any complaint to be considered under their formal internal complaints procedure. If you remain dissatisfied with the firm's final response, after your complaint has been formally considered, or if the firm has exceeded the response timescales, you may refer your complaint for consideration under The Property Ombudsman scheme (TPOs). The Ombudsman can award compensation of up to £5,000 to you if he finds that you have suffered actual loss as a result of your search provider failing to keep to the Code,

Please note that all queries or complaints regarding your search should be directed to your search provider in the first instance, not to TPOs or to the PCCB.

#### TPOs Contact Details

The Property Ombudsman scheme Milford House 43-55 Milford Street Salisbury Wiltshire SP1 2BP Tel: 01722 333306

Fax: 01722 332296 Email: admin@tpos.co.uk

You can get more information about the PCCB from www.propertycodes.org.uk

PLEASE ASK YOUR SEARCH PROVIDER IF YOU WOULD LIKE A COPY OF THE SEARCH CODE

Tosts Water Maries 1, Property Searches, PO Box 3189, Slough SL1 4W. DX 151280 Slough 13 T0845 070 9148E\_surchase Camerosias.co.uk I www.thamasarata-property-serches.co.uk

Page 12 of 12



