

106 KING HENRY'S ROAD - PRELIMINARY CALCULATIONS  
FOR B/A SUBMISSION.

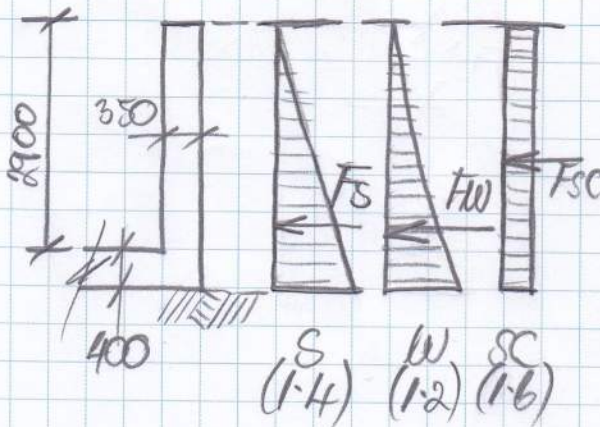
BASEMENT WALLS.

CHECK CONSERVATIVELY AS CANTILEVERING FROM THE  
BASEMENT END. THIS IS THE WORST CASE SCENARIO.

FROM SOILTECHNICS' GROUND INVESTIGATION REPORT:

- WATER TABLE IS BELOW BASEMENT.
- ANGLE OF SHEARING RESISTANCE =  $22^\circ$
- BULK DENSITY OF SOIL (UNSATURATED) =  $18 \text{ kN/m}^3$
- $K_A = 0.5$ ,  $K_0 = 1.0$  (ASSUME INTERPOLATED AT 0.7)
- DESIGN BEARING CAPACITY =  $125 \text{ kN/m}^2$ .

ASSUME CONSERVATIVE FLOODING SCENARIO WITH WATER  
UP TO GROUND LEVEL AROUND BASEMENT.



EFFECTIVE WEIGHT OF  
CANTILEVER =  $2.9 + 0.2 = 3.1 \text{ m}$ .

$$F_s = 1.4 (18 - 9.81) \times \frac{3.1^2}{2} \times 0.7 = 39 \text{ kN}$$

$$F_w = 1.2 (9.81) \left(\frac{3.1^2}{2}\right) = 57 \text{ "}$$

$$F_{sc} = 1.6 (1.5) (3.1) \times 0.7 = 8 \text{ "}$$

URS BM ON WALL :-

$$(39 \times 3.1/3) + (57 \times 3.1/3) + (5 \times 3.1/2) = \underline{107 \text{ kNm/m.}}$$

TRY T20 @ 150 EF VERTICALLY. ( $A_s = 2090 \text{ mm}^2/\text{m}$ )

ASSUME 50 COVER TO MAIN STEEL.

$$d = 350 - 50 - 20/2 = 290 \text{ mm.}$$

$$K = M / bd^2 f_{cu} = 10756 / 1000(290)^2 40 = 0.03$$

$$z = 280 \text{ mm}$$

$$A_{s \text{ req}} = M / 0.95 f_y z = 10756 / 0.95 (460) (280) = \underline{875 \text{ mm}^2/\text{m.}}$$

( $\ll 2090 \therefore \text{OK}$ )

CHECK  $\delta$  :-

$$\text{ACCODE } l/d = 3100/290 = 10.7$$

$$\left. \begin{array}{l} M/bd^2 = 1.27 \\ \rho_s = 127 \end{array} \right\} \rightarrow \text{TEN. MOD} = 1.85 \rightarrow \text{Acc. } l/d$$

$$100 A_s / bd = 0.72 \rightarrow \text{TEN. MOD} = 1.19 \rightarrow \underline{15.4 (> 10.7)}$$

→ WALL SECTION FINELY OK UNDER CONSERVATIVE SPAN CONFIGURATION (CHIMNEY), CONSERVATIVE BONDING (WATER @ G.L) USING NORMAL WEIGHT REBAR (T20 @ 150%).

BASEMENT S.M.B.

APPROX. WEIGHT OF BUILDING: -

ROOF.	1.5	KN/M <sup>2</sup>	(SRS)
1 <sup>st</sup> FLOOR.	2.5	"	"
GROUND FLOOR.	6	"	"
BASEMENT S.M.B.	13	"	"
	<u>23</u>	"	" (FROM FLOORS & ROOF)

EXTERNAL WALLS  $2.5 \text{ KN/M}^2 \times 6.5 \text{ M} = 16.3 \text{ KN/M}$  AROUND EDGE  
 +  $30 \text{ KN/M}$  FROM BASEMENT WALLS.  
 $47 \text{ KN/M}$  (SRS) LOAD  
AROUND PERIMETER.

INTERNAL WALLS, SAY  $1.5 \text{ KN/M}^2$  (SRS).

TOTAL WEIGHT  $\approx 24.8 \times (12 \times 10 \text{ M}) = 2976 \text{ KN.}$   
 +  $47 \times (2 \times (10 + 12)) = 2068 \text{ KN.}$   
 $5044 \text{ KN. (SRS)}$

UPLIFT FROM WATER APPROX.  $30 \times (10 \times 12) = 3600 \text{ KN.} (< 5044 \text{ KN.})$   
 (FRICTION IGNORED  $\therefore$  F.O.S.  
 ON BUOYANCY VERY LARGE).

CHECK BENDING & DEFLECTION OF SLAB: -

WATER PRESSURE ↑, SAY 30 kN/m<sup>2</sup> (SES)

DEAD LD. ↓

SLAB & SOLE =  $0.475 \times 24 = 11.4 \text{ kN/m}^2$

INTERNAL WINDS =  $1.5 \text{ kN/m}^2 \times 3 = 4.5$

FLOOR FINISHES, SAY 10/2 = 5

21 kN/m<sup>2</sup> (SES)

NET ULS UPWARD FORCE =  $1.2(30) - 21 = 15 \text{ kN/m}^2$

SPAN OF SLAB = 10 m.

BM =  $wl^2/10$  SAY =  $150 \text{ Nm/m}$

TRY T20 @ 150% ( $A_s = 2090 \text{ mm}^2/\text{m}$ )

$d = 400 - 50 - 10 = 340 \text{ mm}$

$K = M / bd^2 f_{cu} = 150 \text{ Eb} / 1000(340)^2(40) = 0.032$

$Z = 327 \text{ mm} \rightarrow A_s \text{ REQ} = 150 \text{ Eb} / 0.95(460)(327) = 1049 \text{ mm}^2/\text{m}$   
( $\ll 2090$ : OK)

CHECK  $\delta$ : -

TYPICAL  $l/d = 10000/340 = 29$

$M/bd^2 = 1.29$ ,  $f_s = 152 \text{ N/mm}^2 \rightarrow \text{TEN. Mod} = 1.79$

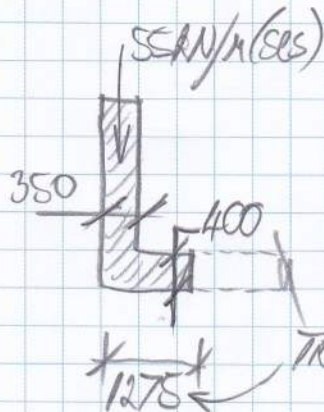
$100A_s/bd = 0.61 \rightarrow \text{COMP. Mod} = 1.16$

ALLOWABLE  $l/d = 23 \text{ SAY} \times 1.79 \times 1.16 = 47 (> 29 \text{ OK})$

BEARING PRESSURES:

APPROX. BUILDING WEIGHTS HAVE BEEN ASSESSED PREVIOUSLY, BUT A MORE ACCURATE BREAKDOWN FOR THE KEY SHORT TERM & LONG TERM STAGES FOR USE IN SETTLEMENT CALCULATIONS IS AS FOLLOWS:

SHORT TERM PERIMETER LOADS ON UNDERPINS ONLY = MAX. 55kN/m RUN FOR WORST CASE PARTY WALL.



TEMP. SHORT TERM GBP BEHEAVN  
UNDERPIN TOE =  $\frac{55}{1.275} = 43 \text{ kN/m}^2$  APPROX.

\* 1275 ← TEMP. UNDERPIN TOE WIDTH.

"SHORT TERM" MAY BE AROUND 3 MONTHS, BEFORE MAIN BASEMENT SLAB IS CAST.

LONG TERM PRESSURE BEHEAVN FOOTING. BASEMENT SLAB ASSUMING ALL LOADS DISTRIBUTED =  $\frac{35 \text{ kN/m}^2}{\text{APPROX.}}$   
(ON A 10M x 12M BASEMENT)