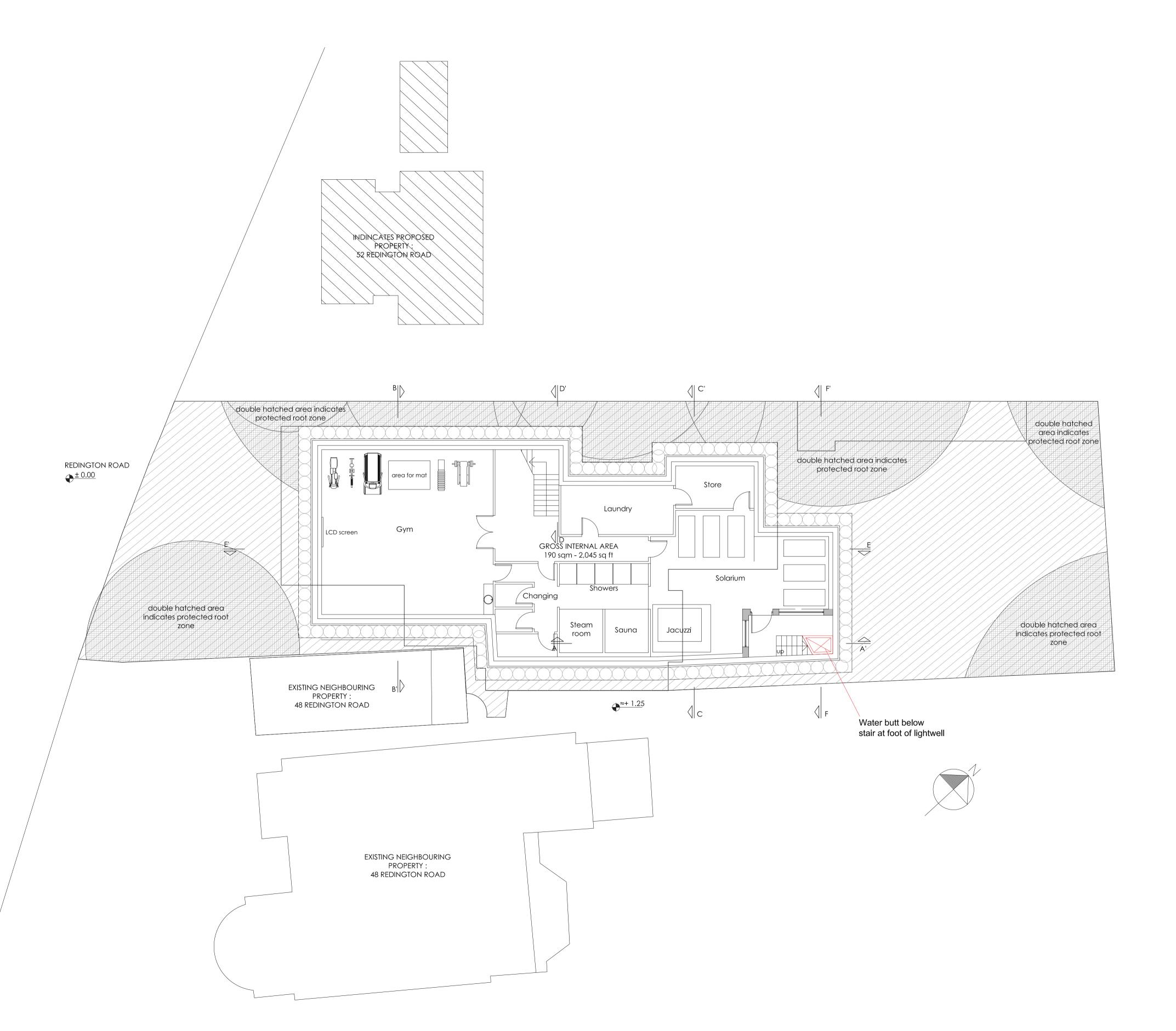
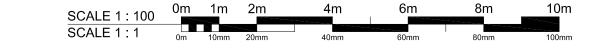
A 11.02.2014 PREPARED FOR PLANNING SUBMISSION B 14:03:18 Water butt indicated for Condition 8



PROPOSED BASEMENT LEVEL 2 PLAN



ISSUED FOR PLANNING ISSUE DATE: 11.02.2014



development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DONN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

DRAWING No.:

PROPOSED BASEMENT LEVEL 2 PLAN

E10-030/P0B

SCALE: 1:100@A1 / 1:200@A3

DRAWN: JK

DATE: FEB 2014 CHECKED: WTM DATE: FEB 2014

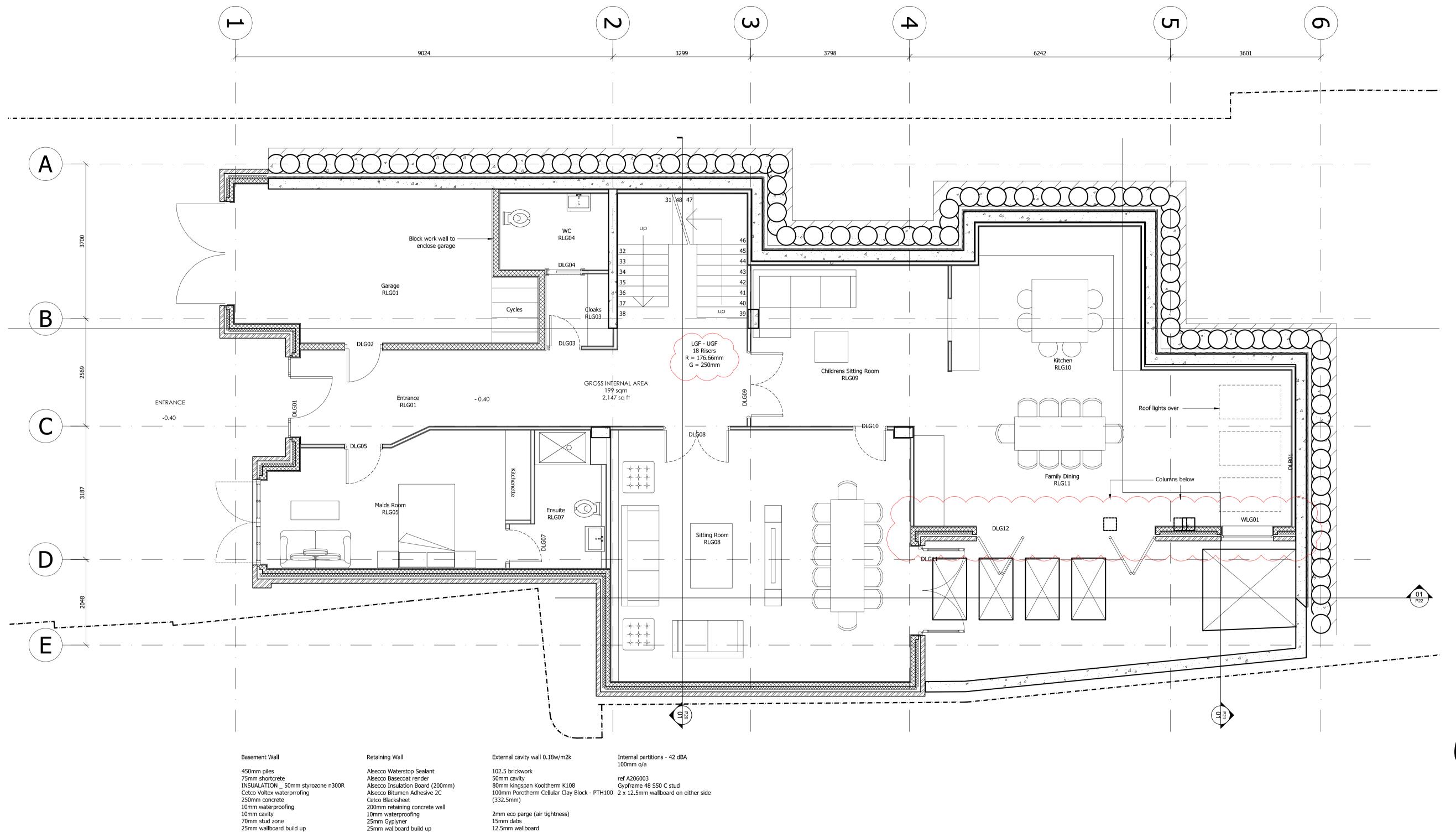
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REV. DATE REVISION

E 11.02.2014 PREPARED FOR PLANNING SUBMISSION
F 08.12.2017 STRUCTURAL INFORMATION UPDATED
G 22.12.2017 UPDATED AS CLOUDED AND SECTION LINES ADDED



Drawing Notes

O1 Internal walls to be non-load bearing MF partitions which will be part of a future fit-out works package.

Wall linings etc. also to be part of a future fit-out works package.

These layouts give indicative location and quantities for drainage services etc.

FOR COMMENT



development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DOWN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

PROPOSED LOWER GROUND FLOOR PLAN

DRAWING No.: E10-030/P01

SCALE: 1:50 @ A1

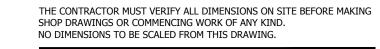
DRAWN: JK DATE: FEB 2014

CHECKED: WTM DATE: FEB 2014

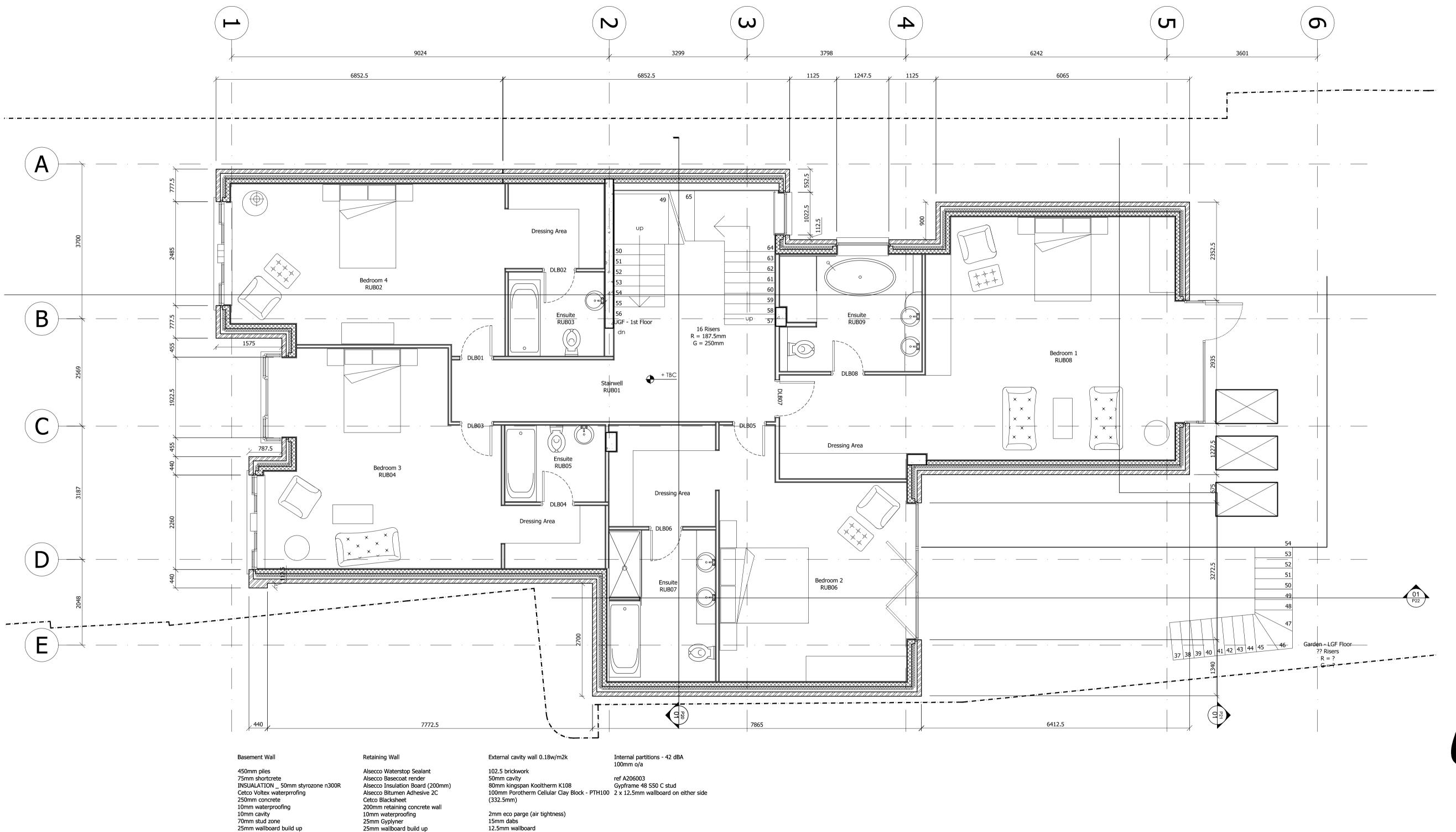
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- D 11.02.2014 PREPARED FOR PLANNING SUBMISSION 08.12.2017 STRUCTURAL INFORMATION UPDATED
- 02.06.2014 PREPARED FOR PLANNING SUBMISSION G 21.12.2017 UPDATED AS CLOUDED AND SECTION LINES



be part of a future fit-out works package. Wall linings etc. also to be part of a future fit-out works These layouts give indicative location and quantities for drainage services etc.

01 Internal walls to be non-load bearing MF partitions which will

Drawing Notes

FOR COMMENT



development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DOWN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

PROPOSED UPPER GROUND FLOOR PLAN

DRAWING No.: E10-030/P02

SCALE: 1:50 @ A1

DRAWN: JK

DATE: JUN 2014 CHECKED: WTM DATE: JUN 2014

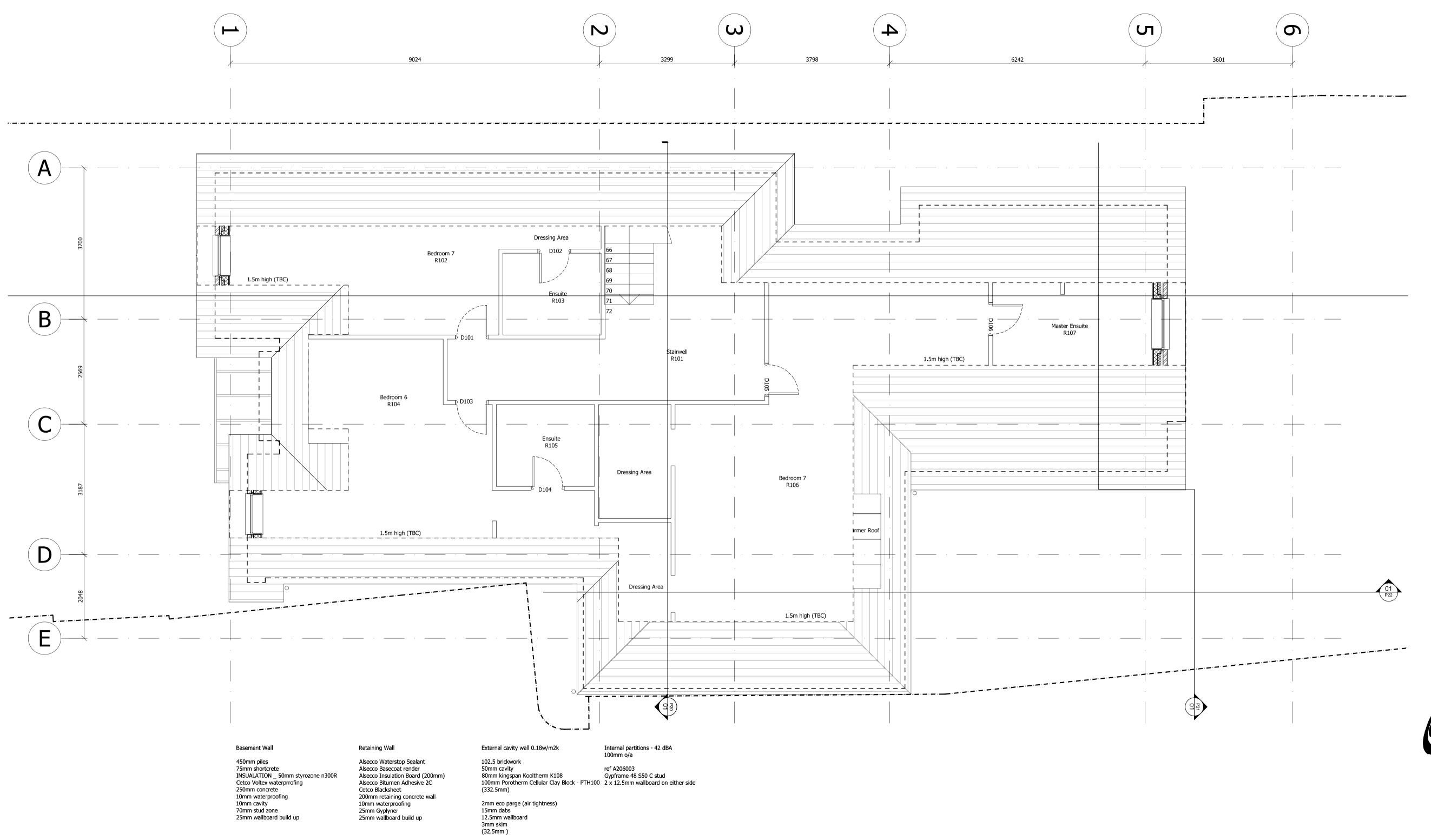
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REV. DATE REVISION

F 22.12.2017 STRUCTURAL INFORMATION UPDATED



Drawing Notes

01 Internal walls to be non-load bearing MF partitions which will be part of a future fit-out works package.

Wall linings etc. also to be part of a future fit-out works package.

These layouts give indicative location and quantities for drainage services etc.

FOR COMMENT



development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DOWN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

PROPOSED FIRST FLOOR PLAN

DRAWING No.: E10-030/P03

SCALE: 1:100 @ A1

CAMAN IX

DRAWN: JK DATE: FEB 2014

CHECKED: WTM DATE: FEB 2014

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 D
 11.02.2014
 PREPARED FOR PLANNING SUBMISSION

 E
 02.06.2014
 PREPARED FOR PLANNING SUBMISSION

 F
 22.12.2017
 STRUCTURAL INFORMATION UPDATED

 G
 09.01.2018
 LEVELS, TREE LINE AND BOUNDARY LINE UPDATED



Details of tree protection measures are all set-out within Crown Consulting Arboricultural Method Statement of Tree Protection Plan ref: CCL09882/TPP rev. 1 as approved under Condition 5 depptication reference 2017/5528/P. dated 7 December 2017).

FOR COMMENT



development consultants

HAMPSTEAD LONDON NW3 7RS

MARCUS DOWN 50 REDINGTON ROAD LONDON NW3 7RS

PROPOSED ROOF PLAN

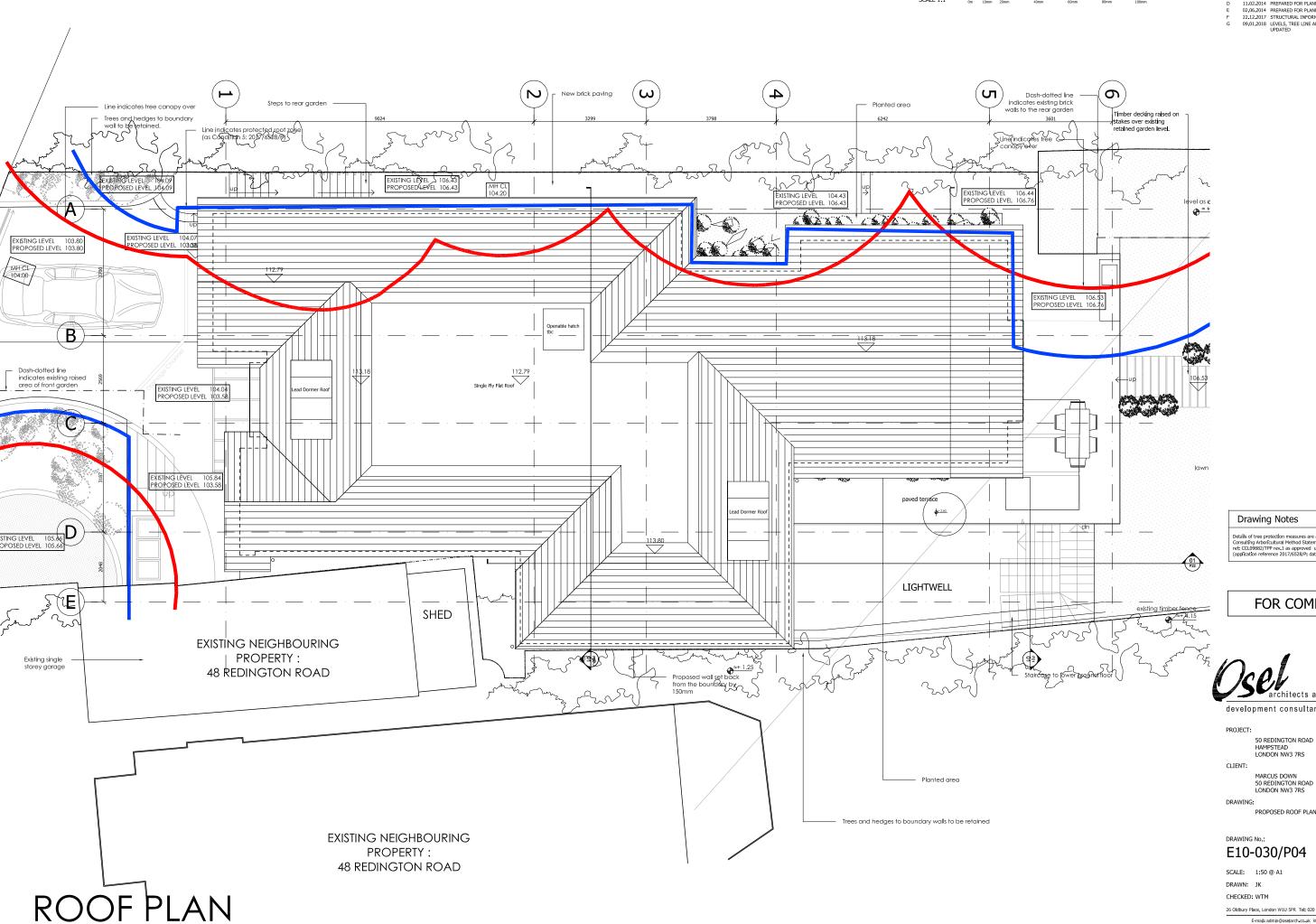
E10-030/P04

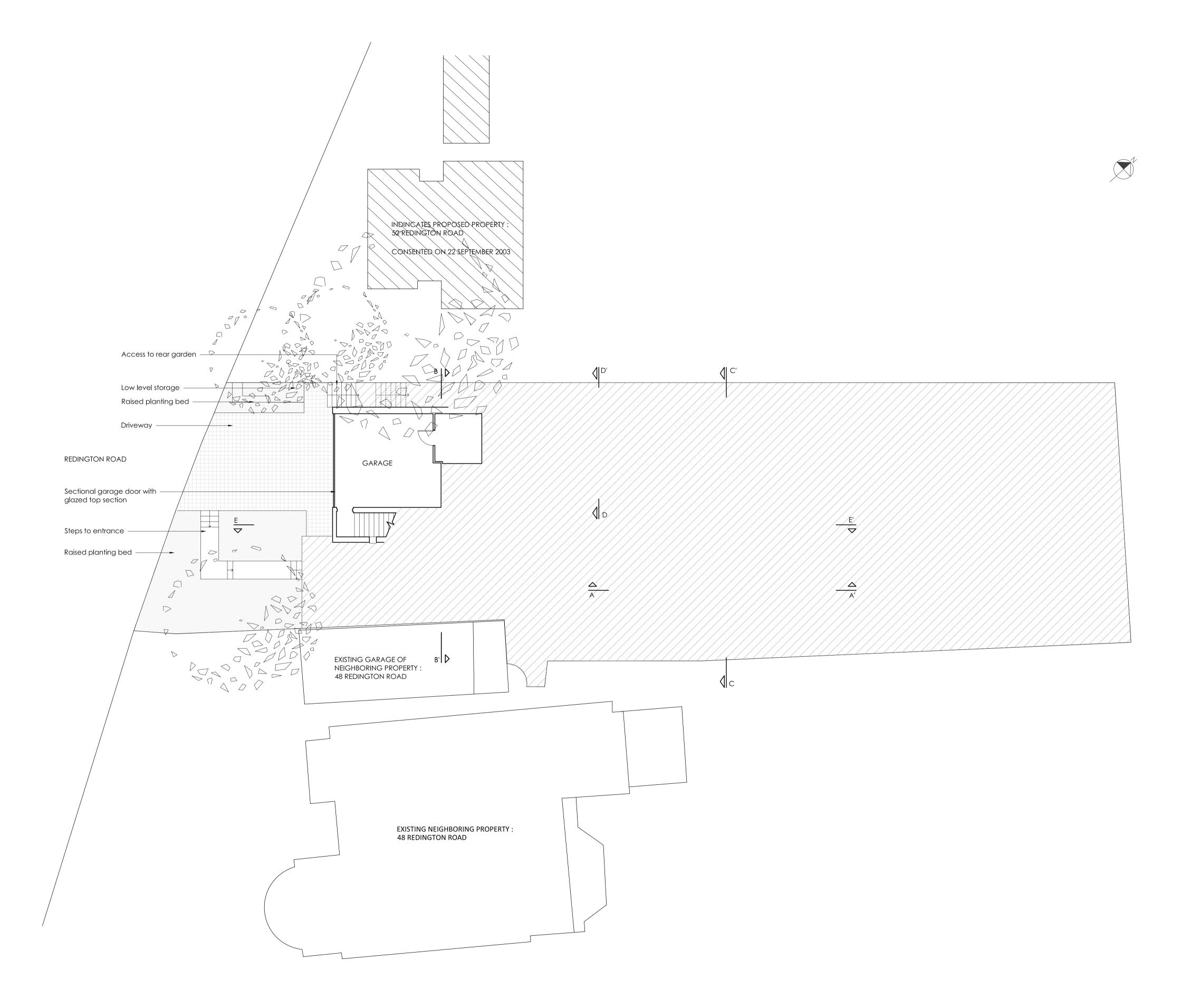
SCALE: 1:50 @ A1

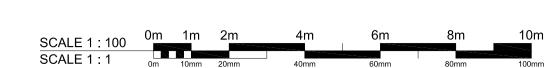
DATE: JUN 2014 DATE: JUN 2014

REV:

G









development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DONN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

EXISTING LOWER GROUND FLOOR PLAN

DRAWING No.: E10-030/S01

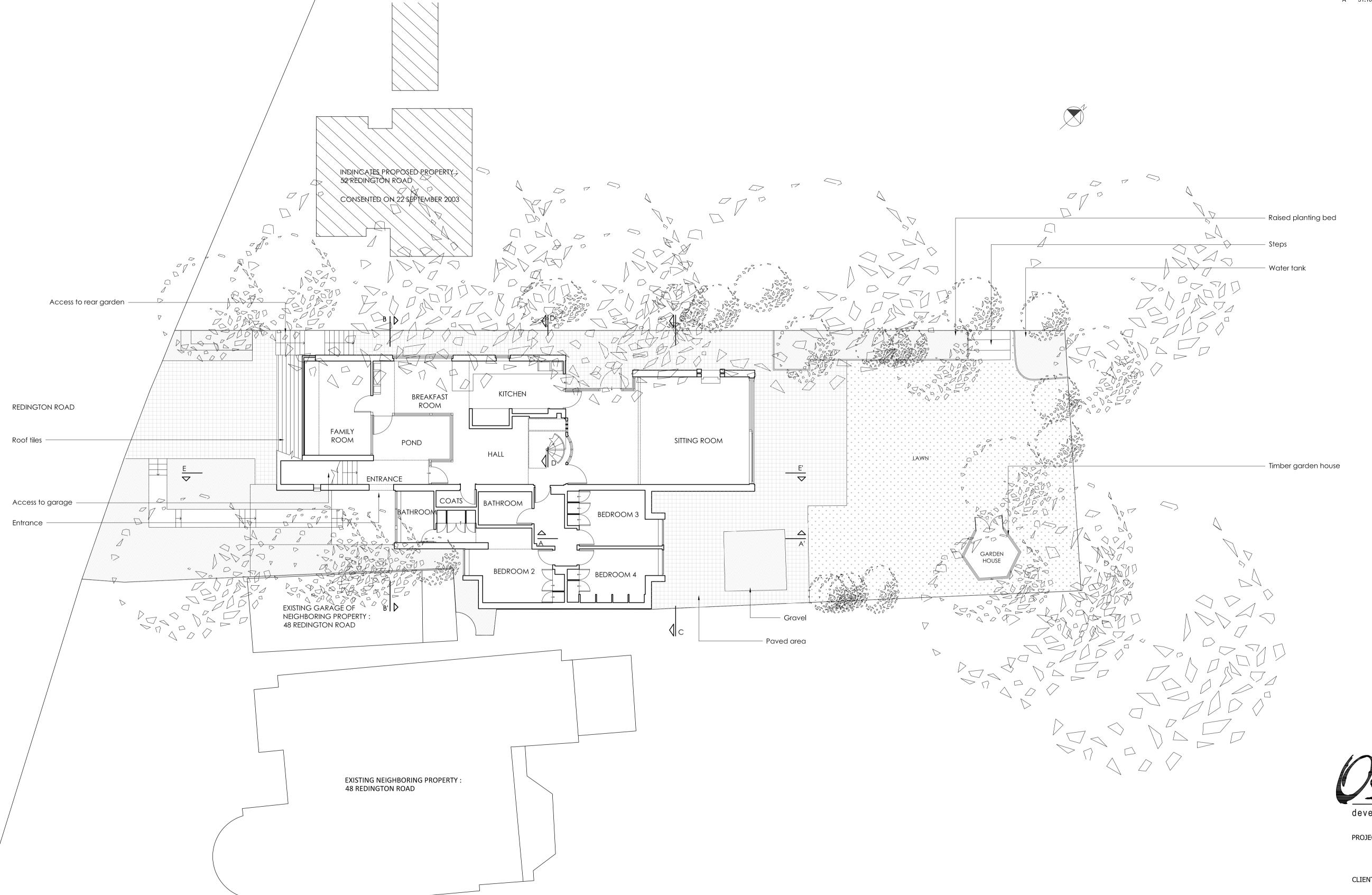
SCALE: 1:100@A1 / 1:200@A3

DRAWN: ZETA KACHRI DATE: AUG 2011
CHECKED: TERRY MONAN DATE: AUG 2011

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A 31.10.2011 SECTION LINES ADDED



development consultants

PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DONN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

EXISTING UPPER GROUND FLOOR PLAN

DRAWING No.: E10-030/S02

SCALE: 1:100@A1 / 1:200@A3

DRAWN: ZETA KACHRI

CHECKED: TERRY MONAN DATE: AUG 2011 26 Oldbury Place, London W1U 5PR Tel: 020 7224 2447 Fax: 020 7224 2997

DATE: AUG 2011

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A 31.10.2011 SECTION LINES ADDED





PROJECT:

50 REDINGTON ROAD HAMPSTEAD LONDON NW3 7RS

CLIENT:

MARCUS DONN 50 REDINGTON ROAD LONDON NW3 7RS

DRAWING:

SCALE 1 : 100 SCALE 1 : 1 EXISTING FIRST FLOOR PLAN

DRAWING No.: E10-030/S03

SCALE: 1:100@A1 / 1:200@A3

DRAWN: ZETA KACHRI DATE: AUG 2011
CHECKED: TERRY MONAN DATE: AUG 2011

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3	Geotechnical 8 Environmental Associates							Site 50 Redington Road, Hampstead, London, NW3 7RS	Bore Num BH	ber
Boring Meth Cable Percus			Diamete 60mm cas	r ed to 14.00m	Ground	Level (104.40	mOD)	Client Marcus Donn	Job Num J12	
		Locatio	on		Dates 08	3/03/201 9/03/201	12- 12	Engineer Michael Alexander Consulting Engineers	Shee 1/	et /2
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	De (n (Thick	pth n) (ness)	Description	Legen	Mater Mater
					104.30		(0.10) 0.10	Brick Paving		:
0.30	D1		DRY		104.10 104.00	E ((0.20) 0.30	Reinforced concrete		×
0.50	D2		DRY				(0.10) 0.40	Made Ground (brown clayey sand with occasional fragments of concrete)	× ×	× •
							(1.10)	Firm brown silty sandy CLAY with occasional layers of clayey sand and fine rootlets	×	×
1.20-1.65	SPT N=13		DRY	1,2/2,3,4,4	102.90	Ē		dayey Sand and fine rootlets	×	<u>.</u>
1.20-1.65	D3		DRY	1,2/2,0,4,4	102.90	E	1.50	Firm medium strength brown mottled orange-brown and	×	
1.75	D4		DRY					grey silty sandy CLAY with a layer of sand between 3.75 m and 4.0 m	× ×	-
2.00-2.45	U5		DRY	23 blows		<u> </u>			*	×
						E			×	<u>:</u>
							(0.50)		× × ·	
2.75	D6		DRY			F ((2.50)		×	<u>.</u> .
3.00-3.45 3.00-3.45	SPT N=16 D7		DRY DRY	3,3/4,4,4,4					* <u></u>	×
						E			×	-
0.75	D0	2.50	DDV			Ē			×	×
3.75 4.00-4.45	D8	3.50	DRY	30 blows	100.40	E	4.00	Stiff high strength and very high strength dark grey silty	×	=
4.00-4.43	03	3.30	DICI	30 blows				sandy CLAY with occasional partings of light grey fine sand and silt	× ×	×
						E			×	×
4.75	D10	3.50	DRY						×	-
5.00-5.45 5.00-5.45	SPT N=17 D11	3.50 3.50	DRY DRY	2,3/4,3,5,5					· × ·	-
3.00-3.43		3.30	DIXI			-			* <u> </u>	×
						E			×	÷
	5.0								× ×	
6.00	D12	5.00	DAMP			Ē			×	× ·
6.50-6.95	SPT N=19	6.00	DAMP	3,4/5,5,4,5					×	×
6.50-6.95	D13	6.00	DAMP	3,4/3,3,4,5		Ē			× × ·	Ξ.
									×	×
						E			×	×
7.50	D14	7.00	DRY			E			× × ×	
						Ē			· ×	× •
8.00-8.45	U15	7.50	DRY	34 blows					×	×
						Ē			×	-
						Ē			× × × ×	<u>. </u>
9.00	D16	8.00	DAMP	Moderate (1) at					×	<u>∵</u> ⊻1
5.00	D10	0.00	DAIVIE	9.00m, sealed at 13.50m.		E			×	<u>:</u>
9.50-9.95	SPT N=23	9.00	DAMP	3,5/5,6,6,6		E			× × × ×	
9.50-9.95	D17	9.00	DAMP						×	× ·
Remarks The OD level	ls shown should be	checked s	and are ar	pproximate		<u> </u>		Scale	Logg	jed
Two hours sp Standpipe in:	pent breaking out co stalled to a depth of	ncrete and 8.0 m	d hand di	gging a service pit an	id manhand	dling eq	Juipmer			
Three hours	demobilising equipn	nent and t	idying					1:50	HE No.) ——
								Figure	NO. 045 .BH ⁻	1

वा	Geotechnical & Environmental Associates	:				nhanger House Coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7F	RS	Borehole Number BH 1
Boring Meth Cable Percus		_	Diamete	r ed to 14.00m		Level (mOD) 104.40	Client Marcus Donn		Job Number J12045
		Location	n		Dates 08	3/03/2012- 9/03/2012	Engineer Michael Alexander Consulting Engineers		Sheet 2/2
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Vater Vater
						 			××
10.50	D18	9.00	9.00			<u></u>			× × ×
11.00-11.45 11.00-11.45	SPT N=24 D19	10.00 10.00	10.00 10.00	1,4/5,6,6,7					× × × ×
						<u>E</u>			× × ×
12.00	D20	12.00	10.00						× × ×
12.50-12.95	D21	12.20	11.00	08/03/2012:DRY					××
12.50-12.95	SPT N=25	12.20	11.00	09/03/2012:10.50m 10,5/5,6,7,7					× × × × × × × × × × × × × × × × × × ×
13.50	D22	13.50	11.00						× - × · · · · · · · · · · · · · · · · ·
14.00-14.45	U23	13.50	11.00	34 blows					× × ×
						E E E			× - ×
15.00	D24	14.00	14.00			(16.00)			× · · · · ·
15.50-15.95 15.50-15.95	SPT N=27	14.00	14.50 14.50	2,4/5,6,8,8					× × .
15.50-15.95	D25	14.00	14.50						× × × × × × × × × × × × × × × × × × ×
16.50	D26	14.00	14.50						× × ×
17.00-17.45	SPT N=26	14.00	15.00	5,5/6,6,7,7					× × ×
17.00-17.45	D27	14.00	15.00						× × × × × × × × × × × × × × × × × × ×
18.00	D28	14.00	15.00						×× ·
18.50-18.95	U29	14.00	15.50	45 blows					× × ×
				-					× × × × × × × × × × × × × × × × × × ×
19.25	D30	14.00	15.50						×
19.55-20.00 19.55-20.00	SPT N=31 D31	14.00 14.00	15.50 15.50	2,6/6,7,8,10 09/03/2012:15.50m					× × × ×
Remarks					84.40	20.00		Scale (approx)	Logged By
								1:50	БУ HD
								Figure N	0.
								J1204	5 .BH 1

1	Geotechnical & Environmental Associates				hanger Hous Coursers Road St Albans AL4 0PG	d s	Site 50 Redington Road, Hampstead, London, NW3 7RS	Numb BH	
Excavation Opendrive S		Dimens	ions		Level (mO 107.40	D)	Client Marcus Donn	Job Numb	
		Locatio	n	Dates 06	6/03/2012		Engineer Michael Alexander Consulting Engineers	Sheet	
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thicknes	ss)	Description	Legen	Water
0.25 0.50 1.00-1.45 1.20 1.70 2.00-2.45 2.10 2.60 3.00-3.45 3.00 4.00-4.45 4.00 4.50 5.00-5.45 5.00 6.00-6.45 6.00 6.50 6.80 7.00-7.45 7.30 7.60 7.90 8.00-8.45	D1 D2 SPT N=14 D3 D4 SPT N=16 D5 D6 SPT N=18 D7 D8 SPT N=19 D9 D10 SPT N=30 D11 D12 SPT N=17 D13 D14 D15 SPT N=28 D16 D17 D18 SPT N=30		3,3/3,4,4,3 2,2/3,4,4,5,5 2,3/4,4,5,6 3,4/6,7,8,9 2,3/3,4,5,5 4,6/5,6,7,10	107.38 187:18 106.40 106.10 105.40 102.00 100.70 99.90 99.70 99.40	0.0 (0.20	12 (2) (2) (2) (2) (3) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4	Paving Slab Concrete Sand sub-base Medium-dense light orange-brown mottled grey slightly clayey SAND with occasional rootlets Light brown fine SAND Light brown mottled orange-brown and grey clayey SAND with rootlets Stiff brown mottled orange-brown and grey slity CLAY with occasional partings of orange-brown fine sand and slit, rare calcareous nodules. Rootlets encountered to a depth of 3.0 m - desiccated soil enocuntered to a depth of about 3.0 m Stiff brown mottled orange-brown silty CLAY with occasional partings of orange-brown fine sand and silt, rare calcareous nodules Stiff dark grey silty CLAY with occasional partings of orange-brown fine sand and silt Medium dense brown mottled orange-brown and grey clayey SAND Grey silty fine SAND with occasional partings of orange-brown fine sand and silt Brown mottled orange-brown and grey clayey SAND Complete at 8.00m		
The OD leve Groundwate	els shown should be or not encountered du stalled to a depth of	ring drillin	and are approximate Ig				Scale (approx) 1:50 Figure I J120	HD No.	·

GE	Geotechnical & Environmental Associates				hanger House Coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 71	RS	Number BH 3	
Excavation Drive-in Wir	Method ndow Sampler	Dimension	ns		Level (mOD) 107.40	Client Marcus Donn		Job Number J12045	
		Location		Dates 08	8/03/2012	Engineer Michael Alexander Consulting Engineers		Sheet 1/1	_
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.50	D1				(0.85)	Made Ground (greyish brown clayey sand with occ fragments of brick, concrete and charcoal)	casional		
1.00	D2			105.80	(0.75)	Light orange-brown clayey SAND. Rootlets encou a depth of 1.2 m Terminated at 1.60m	ntered to		
Remarks The OD leve	els shown should be	checked and	are approximate				Scale (approx)	Logged By	
Groundwate	er not encountered			ampling equip	oment - boreh	ole relocated 0.5 m to the east	1:50	HD	
							Figure N J1204	o. 45.BH 3	

GE	Geotechnical & Environmental Associates				hanger House coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7RS	Number BH 3A
Excavation Drive-in Win	Method adow Sampler	Dimensi	ions		Level (mOD) 107.40	Client Marcus Donn	Job Number J12045
		Location	1	Dates 08	3/03/2012	Engineer Michael Alexander Consulting Engineers	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend Nate
0.30 0.60 0.85 1.50 2.00 2.50 3.00 3.50 3.90 4.20 4.80 5.50 6.00	D1 D2 D3 D4 D5 D6 D7 D8 D9 D10 D11 D12 D13	Depth (m)	Field Records	106.60 105.70 103.70 103.45 103.30 102.80	(0.80)	Made Ground (greyish brown sandy clay with rootlets, occasional fragments of brick, concrete and charcoal) Light orange-brown clayey SAND Stiff light orange-brown silty CLAY with occasional partings of fine sand and silt. Rootlets encountered to a depth of 3.0 m - desiccated soil encountered to a depth of about 3.0 m Light orange-brown clayey SAND Stiff light orange-brown silty CLAY with occasional partings of fine sand and silt Light orange-brown clayey SAND Stiff light orange-brown silty CLAY with occasional partings of fine sand and silt Light brown mottled orange-brown fine SAND Complete at 6.00m	Legend *A
Remarks						Scale	Logged
The OD leve Groundwate	els shown should be der not encountered du nstalled to a depth of (ring drilling	nd are approximate g			Scale (approx)	Logged By
						Figure	

GE	Geotechnical & Environmental Associates				hanger House coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7R	RS	Number BH 4	
Excavation Drive-in Win	Method dow Sampler	Dimension	ns		Level (mOD) 107.20	Client Marcus Donn		Job Number J12045	
		Location		Dates 08	5/03/2012	Engineer Michael Alexander Consulting Engineers		Sheet 1/1	
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.30 0.60 0.75 1.00 1.40 1.65	D1 D2 D3 D4 D5 D6			105.95 105.40	(0.05)	Made Ground (brown silty clay with occasional fragcharcoal and brick) Light orange-brown gravelly fine SAND Brown mottled orange-brown clayey SAND with gr. Rootlets encountered to a depth of 0.8 m 'Stiff light orange-brown silty CLAY with occasiona of fine sand and silt - desiccated soil Terminated at 1.80m	avel. Il partings	X X X	
The OD leve Groundwate	els shown should be or not encountered tompleted as coupleted as coupl			ampling equip	oment - boreh	ole relocated 1.0 m to the west	Scale (approx) 1:50 Figure N	HD HD.	

GE	Geotechnical & Environmental Associates				hanger House Coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7RS	Number BH 4A
Excavation Drive-in Win	Method dow Sampler	Dimension	ns		Level (mOD) 107.20	Client Marcus Donn	Job Number J12045
		Location		Dates 15	5/03/2012	Engineer Michael Alexander Consulting Engineers	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend X
0.40 0.65 1.00 1.70 2.50 3.00 3.50 4.00 4.50 5.00 5.50 5.90	D1 D2 D3 D4 D5 D6 D7 D8 D9 D10 D11 D12	(m)		106.70 186:49 105.65	(0.50)	Made Ground (greyish brown sandy clay with fine rootlets and occasional fragments of brick and charcoal) Light orange-brown mottled grey clayey SAND with rare gravel Light orange-brown mottled grey slightly clayey fine SAND 'Stiff' brown silty CLAY with occasional partings of light orange-brown fine sand and silt. Rootlets encountered to a depth of about 2.0 m Firm becoming stiff brown silty CLAY with occasional partings of light orange-brown fine sand and silt. Rootlets encountered to a depth of about 2.0 m Firm becoming stiff brown silty CLAY with occasional pockets of fine sand and silt with a layer of fine sand between 5.6 m and 5.8 m Complete at 6.00m	
Remarks The OD leve Groundwate	els shown should be or not encountered	checked and	are approximate		<u> </u>	Scale (approx	HD

तु	Geotechnical 8 Environmental Associates	k I				nanger House oursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7RS	Borehole Number BH5
Boring Meth			Diamete 0mm cas	r ed to 11.00m		Level (mOD 07.40	Client Marcus Donn	Job Number J12045
		Location	n			/03/2014- /03/2014	Engineer Michael Alexander Consulting Engineers	Sheet 1/3
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness	Description)	Legend &
0.30	D1				107.05	 (0.35) 0.35	Made Ground (dark brown slightly clayey silty sand with occasional flint gravel, rare partings of orange-brown sand, fine rootlets and abundant fragments of brick and ash)	
0.50	B2						Medium dense orange-brown clayey silty SAND with fine rootlets	
1.20-1.65 1.20-1.65	CPT N=11 B3			1,1/2,2,3,4	105.15			×
								- - x - x
1.75	D4			1 2/2 2 4 4		<u> </u>		×
2.00-2.45 2.00-2.45	CPT N=14 B5			1,2/3,3,4,4	105.15	2.25	Firm locally soft medium strength orange-brown silty sandy CLAY with alternate bands of orange-brown clayey sand	××
2.75	D6							××
3.00-3.45	U7							× × · · ·
						<u>-</u>		× ×
						<u> </u>		× - ×
3.75	D8							× × · · ·
4.00-4.45	CPT N=16			2,2/3,4,4,5		<u></u>		· × ·
4.00-4.45	D9							××
						<u></u>		× × ·
4.75	D10							××
5.00-5.45	D11			03/03/2014:DRY		<u> </u>		×
5.00-5.45	CPT N=18			04/03/2014:DRY 2,3/4,4,5,5				× × × × × × × × × × × × × × × × × × ×
6.00	D12					(8.25)		× × × ×
6 50 6 05	SPT N=18			22/4 4 5 5		(8.25)		××
6.50-6.95 6.50-6.95	D13			2,3/4,4,5,5				×
								×
						E E		× - ×
7.50	D14					<u> </u>		× - ×
								× × -
8.00-8.45	D15			Water strike(1) at 8.00m.		<u></u>		× — × \
8.00-8.45	SPT N=13	8.00		8.00m. 1,2/2,3,4,4		Ē		× _ ×
								× × ·
						<u>-</u>		××
9.00	D16					<u> </u>		× - ×
	007			4.0/0 : : -				× × -
9.50-9.95 9.50-9.95	SPT N=16 D17			1,2/3,4,4,5		<u> </u>		××
								× - · ·
Hand-dug se	equipment and rig to	of 1.20 m (position 60 minut	(8 hours)	,		Scale (approx)	Logged By
Dismantled r	rig and equipment (8 stalled to a depth of	hours)					1:50	HD
							Figure	No.
							J12	045.BH5

I	Geotechnical & Environmental Associates	k I				hanger House Coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7RS	Bore Num Bh	
Boring Meth Cable Percus			Diamete Omm cas	r ed to 11.00m		Level (mOD)	Client Marcus Donn	Job Num J12	
		Locatio	n			3/03/2014- 5/03/2014	Engineer Michael Alexander Consulting Engineers	Shee 2	et 2/3
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness	Description	Leger	Mater br
						(8.25)		×	×
10.50	D18				96.90	10.50	Stiff high strength dark grey silty very sandy CLAY with occasional partings of light grey fine sand and silt	×	×
11.00-11.45	U19							× × × × × × × × × × × × × × × × × × ×	×
12.00	D20							× · · · · · · · · · · · · · · · · · · ·	×
12.50-12.95 12.50-12.95	SPT N=25 D21	11.00		2,4/5,5,7,8				× · · · · · · · · · · · · · · · · · · ·	× × × × × × × × × × × × × × × × × × ×
13.50	D22							×	× .
14.00-14.45	U23			Water strike(2) at 14.00m.				× × × × × × × × × × × × × × × × × × ×	×.
15.00	D24							× × × × × × × × × × × × × × × × × × ×	×
15.50-15.95 15.50-15.95	SPT N=25 D25	11.00		2,4/5,6,7,7				× · · · · · · · · · · · · · · · · · · ·	×
16.50	D26							× × × × × × × × × × × × × × × × × × ×	×
17.00-17.45 17.00-17.45	SPT N=20 D27	11.00		4,4/5,,7,8				× × × × × × × × × × × × × × × × × × ×	× · · · · · · · · · · · · · · · · · · ·
18.00	D28					- (14.50	,	× × × × × × × × × × × × × × × × × × ×	×
18.50-18.95 18.50-18.95	SPT N=26 D29	11.00		3,5/6,7,6,7				× × × × × × × × × × × × × × × × × × ×	× · · · · · · · · · · · · · · · · · · ·
19.50	D30	14.00		E 6/6 7 7 9				× · · · · · · · · · · · · · · · · · · ·	× × × ×
20.00-20.45 Remarks	SPT N=28	11.00		5,6/6,7,7,8		<u> </u>	S (an	cale Logo prox) By	ged
							1	:50 HI	
							"	J12045.BH	5

तुः	Geotechnical & Environmental Associates					hanger House coursers Road St Albans AL4 0PG	Site 50 Redington Road, Hampstead, London, NW3 7R	S	Boreh Number	
Boring Meth		1	Diamete	r ed to 11.00m		Level (mOD)	Client Marcus Donn		Job Numbe	
									J1204	
		Locatio	n		Dates 03 05	8/03/2014- 5/03/2014	Engineer Michael Alexander Consulting Engineers		Sheet 3/3	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
20.00-20.45	D31			Water strike(3) at 20.00m.					× × × × × × × × × × × × × × × × × × ×	. ∇ 3
21.00	D32								× × × ×	
21.50-21.95 21.50-21.95	CPT N=30 D33	11.00		2,4/5,8,8,9					× × × × × × × × × × × × × × × × × × ×	
22.50	D34					(14.50)			××	
23.00-23.45 23.00-23.45	CPT N=32 D35	11.00		2,5/7,8,8,9					× × × × × × × × × × × × × × × × × × ×	
24.00	D36					<u>-</u> - - - - - - - - -			× × × × × × × × × × × × × × × × × × ×	=
24.55-25.00 24.55-25.00	CPT N=35 D37	11.00		5,5/8,8,9,10		<u>-</u>			× × ×	_
Romarks				04/03/2014:15.00m	82.40		Complete at 25.00m		*	
Remarks								Scale (approx)	Logge By	∗d
								1:50	HD	
								Figure N J120	ю. 45.ВН5	



DRAINAGE STRATEGY REPORT

FOR

50 REDINGTON ROAD LONDON NW3 7RS

CLIENT: MR MARCUS DONN PROJECT NO: P3940

ISSUE 1.1 – Updated to reflect CCTV Results 19 JANUARY 2018



1.00 INTRODUCTION

- 1.01 It is proposed to demolish the existing house at 50 Redington Road NW3 7RS and construct a replacement house. The proposed works are covered by Planning Application 2014/4531/P
- 1.02 Michael Alexander Consulting Engineers have been instructed by the Client, Mr Marcus Donn to prepare a Drainage Strategy for the project. This amended version of the report reflects the findings of the CCTV report. Revised clauses are highlighted in the margin.
- 1.03 This report has been prepared by Isaac Hudson MA(Cantab.) MEng CEng MIStructE Calculations in respect of allowable discharge rates and attenuation volumes have been prepared by Rupert Evans MSc CEnv C.WEM MCIWEM PIEMA

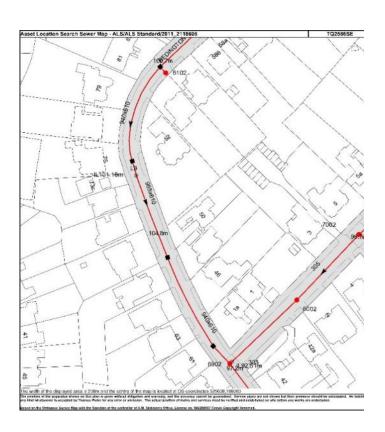
2.00 DRAINAGE STRATEGY REQUIREMENTS

- 2.01 Planning permission reference 2014/4531/P (Planning Condition 8) sets out the following requirements of the Sustainable Urban Drainage System: "(the) system shall be based on a 1:100 year event with 30% provision for Climate Change and demonstrating greenfield levels of runoff"
- 2.02 Furthermore, the scheme is required to be compliant with Code for Sustainable Homes level 4. Mandatory requirements are: -
 - The peak runoff rate and annual runoff volume is not greater for development than for predevelopment.
 - The system must also be designed to prevent flooding of property in the instance of local drainage failure.

3.00 DESK STUDY & INVESTIGATIONS

3.01 The Thames Water Asset Location Search Sewer Map was obtained.

It shows a combined sewer within Redington Road, at a depth of circa 4.8-5.2m below ground level.





3.02 A CCTV survey has been commissioned. The report by Chelmer Global Ltd, reference CDL/9623 is included in Appendix D.

The aim of the CCTV survey was both to establish the existing layout but also to enable assessment of the condition of the drainage network where it is to be retained in the new scheme.

4.00 IMPERMEABLE AREA CALCULATIONS

4.01 In its current condition the site is predominantly covered by the house and its surrounding hard landscaping, with a lawn to the rear and small soft landscaped areas either side of the drive to the front garden



4.02 For the proposed replacement house, the increased footprint is offset by the reduction in hard landscaped areas. The areas of building and hard landscaping have been calculated and are summarised on the drawings included in Appendix A.

5.00 RAINWATER STORAGE CALCULATIONS

- 5.01 A specialist consultant, Evans River & Coastal, has been appointed to: -
 - calculate the allowable discharge rates based on the criteria set out in the Planning Condition and described in clause 2.01 of this report
 - generate a required attenuation volume to enable the discharge criteria to be met.

The calculations and findings are given in Appendix B

5.02 The results of the analysis are that between 43m³ and 49m³ of attenuation is required. For the purposes of the drainage strategy the worst case of 49m³ has been adopted.

6.00 CONSTRAINTS ON DRAINAGE SOLUTION

6.01 The Drainage Strategy has been developed to meet the requirements described above. A number of alternative options have been considered but the adopted solution has been informed by the constraints described below.



- 6.02 The soil conditions have been determined by geotechnical investigations (boreholes and trial pits). The soils were found to be the strata known as the 'Claygate Member' which is a clay based soil with partings and bands of sand. Whilst there are parts of the soil which are locally permeable the areas are not well interlinked so overall the soil how as a low permeability. As such the use of soakaways, permeable paving or other approaches based on infiltration are not appropriate for this site.
- 6.03 The site is constrained by the boundaries so there is limited space for new drainage runs or shallow extensive attenuation features outside the footprint of the building. Furthermore there are areas where groundworks need to be minimised due to the presence of tree roots.
- 6.04 The level of the public sewer limits the depth of any attenuation to enable a gravity connection.

7.00 DRAINAGE STRATEGY

- 7.01 The Drainage Strategy is described on the drawings included in Appendix C
- 7.02 The Foul drainage from the 1st floor, Upper Ground and Lower Ground floors will generally run externally or are distributed at high level within Basement Level 1. These will then fall under gravity to an external Foul manhole within the front drive.
 - Foul drainage for Basement Levels 1 and 2 will be collected in a sump beneath Basement Level 2. These will then be pumped up to the external Foul manhole within the front drive.
- 7.03 The waterproofing to the basement levels will include 'Cavity Drainage' to the floor of Basement Level 2 and the walls of both basement levels. Any collected water from this system will be collected in a separate Cavity Drainage sump within the basement and pumped to the Foul manhole within the front drive.
- 7.04 Surface water will be collected from external hard landscaped areas and the new building's roof, using existing drainage runs where it is possible to retain them. The Surface Water drainage system will be attenuated to limit discharge to the required levels.
- 7.05 Attenuation will be provided using a cellular modular storage system such as Hydro International 'Stormell'. This has been adopted due to the limited site area available for attenuation and the ability to stack the cells.

Each cell has a capacity of 1.5m^3 and hence the required number of cells = 49/1.5 = 32.7 cells. Due to the inherent storage in the drains etc, this is rounded down to 32 cells.





- 7.06 The existing drainage connection will be reused for the foul drainage.
 - In respect of the surface water drainage, it is not possible to reuse the existing connection as the invert level of the last manhole is above the level of the base of the cellular attenuation. Therefore a new connection to the combined sewer in Redington Road will be made for the surface water.
- 7.07 The existing surface water drainage system does not have attenuation, and the collected impermeable areas for existing and proposed conditions are similar. Hence the Code for Sustainable Homes requirement for not increasing the volume of run-off is easily met by the proposals.
- 7.08 There is a low risk of flooding to the property in the case of Drainage System failure.
 - If there is ponding to external areas, water will be prevented from entering the building by the landscaping levels design and appropriate threshold details. There will be upstands to all lightwells. For the foul drainage the sump will be fitted with a backup pump and there will be an alarm/SMS alert in case of failure.
- 7.09 The existing defects of the retained sections of the drainage system, as highlighted in Chelmer report CDL/9623 will be addressed prior to handover of the completed project. A CCTV survey of the full system will also be carried out prior to practical completion to ensure no damage to the drainage has occurred during construction
- 7.10 The maintenance regime for the drainage systems will be documented in accordance with best practice. Maintenance of the system will be carried out by specialist contractors at the appropriate frequency.
 - In respect of the drainage attenuation, the detailed design and installation will minimise the maintenance requirements. A catchpit chamber immediately upstream of the cellular storage will help to reduce the risk of siltation, as will the laying of pipes to achieve self-cleansing velocities. The maintenance regime will involve checking the catchpit chamber and the hydrobrake after 3 months, and then further checks at 6 monthly intervals.



APPENDIX A

Impermeable Area Drawing



APPENDIX B

Calculation of Storage Requirements



Email from Rupert Evans of Evans River & Coastal, dated 7th December 2017: -

Greenfield Runoff

The contributing impermeable area across the proposed site has been calculated as 416.6 sq m. Runoff from the contributing area is assumed to be 100% (i.e. 100% PIMP and no infiltration into the ground) and permeable parts of the site such as garden areas will be profiled so that they do not enter the drainage system. Therefore, in accordance with Section 24.2 of CIRIA 753 permeable areas will not contribute to the drainage system and have therefore not been included in the Greenfield runoff rate calculations.

In order to quantify the equivalent Greenfield runoff rate for the contributing area, the methodology outlined within the document entitled *The Revitalised Flood Hydrograph Model ReFH2 Technical Guidance* has been adopted. The document states that Table 24.1 of CIRIA 753 prefers FEH Methods over the IoH 124 Method, as they are more accurate when calculating peak flows within small catchments and plot scale Greenfield runoff calculations.

The ReFH2.2 software has been integrated within the Microdrainage software Version 2017.1.2 with rescaling abilities for sites below 50 ha. The method also uses the more up-to-date FEH13 Point rainfall data (which replaces the FEH99 data) which have been imported into the Microdrainage software Version 2017.1.2 from the FEH Web Service as well as the catchment descriptors.

In the Microdrainage software the AREA was modified to represent the contributing area area and a winter storm profile was used. Figure 1 shows that the equivalent 1 in 1 year runoff rate is 0 l/s (i.e. too low to be recorded by the software) and the 1 in 2 year runoff rate (i.e. similar to QBAR) is 0.1 l/s. When considering 2 l/s/ha, the runoff rate is 0.08 l/s.



Evans Rivers & Costal Ltd					Page 1
19 St Andrews Avenue	Greenf	ield	runoff rat	e	
Thorpe St Andrew	12.000				4
Norwich NR7 ORG					
Date 07/12/2017 20:03	Design	ed by	ruperter	1	MICIO
File	Checke	- 37			Drainage
Micro Drainage			rol 2017.1	.2	
RAS	H2 Rural Run	off Da	ak Flows		
	355				
Return Perio	Inpu	10		2	
FEH Rainfal				2013	
Site	Location		GB 52563	8 186082	
	Data Type			Point	
	Season			Winter	
	Country Engla	nd/Wal	es/Northern		
	Area (ha)			0.042	
103	SAAR (mm)			664	
	BFIHOST			0.683	
	FARL			0.000	
	SPRHOST			0.000	
URBE:	XT (2000)			0.0000	
	Resul	Lts			
	Return Period	Rural	Urban		
	(Years)	(1/s)	(1/s)		
	User	0.1	0.1		
	Q1	0.0			
	Q2		0.1		
		0.1			
		0.1			
		0.2			
		0.2			
		0.2			
		0.2			
		0.3			
	Q1000	0.4	0.4		

Figure 1: Greenfield runoff rate equivalent (Source: Microdrainage Version 2017.1.2)

Allowable Discharge Rate

The DEFRA/EA document entitled *Rainfall runoff management for developments* dated 2013, and BS8582:2013 advise that the post-development site should aim to try and replicate the undeveloped state and that for Greenfield sites, the peak runoff rate from the developed site for the 1 in 1 year event and 1 in 100 year event should be constrained to the equivalent peak Greenfield runoff rate to minimise the impact on the receiving watercourse.

The guidance states that when considering volume control, the volume discharged from the site for the 1 in 100 year, 6 hour event is constrained to the equivalent volume associated with the Greenfield condition.

The aforementioned guidance and Section 24.10 of CIRIA 753 states that where the additional volume from the development cannot be used or disposed of on-site (e.g. through infiltration or rainwater harvesting) such as in this case, to avoid an increased runoff volume from developed areas into the sewer system, this volume should be discharged at a very low rate.

Therefore, the guidance recommends that:

a) The additional volume resulting from the development (i.e. long term storage volume) should be discharged at a rate of 2 l/s/ha (or less); or



b) <u>ALL</u> the runoff for the 1 in 100 year event from the site should be discharged at a rate of 2 l/s/ha or QBAR (whichever is greater).

In order to provide effective attenuation from the site it is proposed that the runoff from the proposed site will be discharged in accordance with criterion b) above. Therefore, the allowable discharge from the site equates to 0.1 l/s.

Attenuation

The Microdrainage – *Quick Storage Estimate* function has been used in order to determine the volume of storage required based on the discharge rate of 0.1 l/s and contributing area of 416.6 sq m. The model was run for the climate change (30%) 1 in 100 year event as required by Planning Condition 8. The result can be seen on Figure 2.

