

PROJECT 34 DELANCY STREET

REF:

OUT PUT

SUMMARY

DESCRIPTION

Unit Loading

BASEMENT RC RETAINING WALL

RC wall

300 mm Thick RC

Reinforcement

H20 - 200 i.e. A393 Mexh Double layer top & Bottom

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UNIT LOADINGS PITCHED TILE ROOF

	Slope = 30 °		Service loads
Descriptions			<u>Kn/m²</u>
Tiles			= 0.6
Battens and Felt			= 0.05
Rafters			= 0.08
12.5mm Plaster board, insulation and skim			= 0.17
		Sub Total =	<u>0.9</u>
Dead Loads on Plan =	0.90 x SEC (30)		Total = <u>1.07</u>
			Say = 1.10 A
Super =	0.60 (60 - 30)/ 30		= 0.60
			Say = <u>0.60</u> B
		TOTAL A + B =	1.70

FLAT ROOF

			Kn/m ²
Descriptions			<u>Kn/m²</u>
13MM Bitumen bedded chippings			= 0.20
3 Layers mineral surfaced felt			= 0.10
Composite roof boarding			= 0.10
Poldeck insulation			= 0.03
SW Joist			= 0.15
12mm Plaster board and skim			= 0.17
		Total =	<u>0.75</u>
Super			= <u>0.75</u>
		TOTAL =	1.50

LOFT SPACE

			Kn/m ²
SW Joists say 150 x 50			= 0.10
12.5 Plaster board			= 0.15
100 mm mineral wool			= 0.05
Total Dead Load			= <u>0.30</u>
Super			= <u>0.25</u>
		Total =	0.55

LOFT FLOOR

			Kn/m ²
SW Joists say 200 x 50			= 0.15
12.5 Plaster board			= 0.15
100 mm mineral wool			= 0.05
Total Dead Load			= <u>0.35</u>
Super			= <u>1.50</u>
		Total =	1.85

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STUD WALL WITH CLAY TILES

	<u>Kn/m²</u>
Clay tiles hung on battens	= 0.60
75 x 50 SW struts @ 400 C/C	= 0.10
Foil backed plaster board and skim coat	= 0.20
Insulation etc.	<u>= 0.10</u>
Total	= 1.00

TIMBER PARTITIONS

	<u>Kn/m²</u>
100 x 50 SW studs @ 400 C/C and noggins	= 0.10
Foil backed plaster board and skim coat	<u>= 0.30</u>
Total	0.40

WALLS

	<u>Kn/m²</u>
Cavity Walls 225mm	= 3.25
Solid wall 225mm	= 4.70
Block wall 100mm	= 1.40
250mm Block wall	= 2.80
150 mm block wall	= 1.90
200mm	= 2.50
Brick infill studwork	= 1.70

NOTE

The calculations are based upon the drawings supplied. Any alterations to these drawings or changes effect on site during construction, could materially effect sizes of structural members that have been designed and adopted on plan. The latter should be brought to the notice of the structural Engineer with specific instructions to prepare revised calculations to accommodate the changes made.

No inspection of the existing soil conditions has been made by the designer, and no information appertaining to the latter have been offered.

No work appertaining to the plans should be carried out until the plan and calculations have been examined by the local authority, and formal written approval obtained. Any works carried out before such approval is obtained, is done solely at contractors or owners risk.

The availability of new and or existing foundation to sustain the new loading arrangement should be verified on site and agreement reached with the Building Control Officer and appropriate changes made where applicable. The condition of all existing load bearing walls to be verified on site and agreement reached with Building Control Officer as to their suitability.

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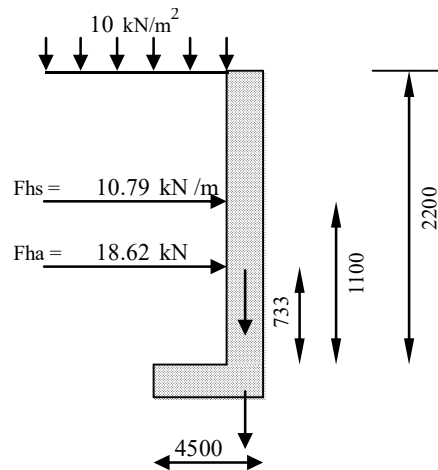
BASEMENT RC RETAINING WALL

$f_y = 460 \text{ N/mm}^2$
 $f_{cu} = 35 \text{ N/mm}^2$
 $H(\text{effective}) = 2.2 \text{ m}$
 Assume wall thickness, $D_w = 300 \text{ mm}$ To accommodate insulated wall above.
 Assume Base thickness, $D_b = 300 \text{ mm}$
 Angle of internal friction, $\phi = 20^\circ$
 Dry density of soil, $\gamma_d = 1600 \text{ Kg/m}^3 = 15.7 \text{ kN/m}^3$
 Coefficient of resistance, $\mu = 0.45$

Angle of internal friction between soil and wall face. $\delta = 0.00$ (worst conditions of soil fully saturated)

Table 1

K_a	=	$(1.0 - \sin 20) / (1.0 + \sin 20) = 0.49$	
Active H. Force, F_{ah}	=	$K_a * \gamma_d * H^2 / 2 = 0.49 * 15.7 * 2.20^2 / 2$	= 18.62 kN --- A
Over Turning Moment, M1	=	$P_{ah} * H / 3.0 = 18.62 * 2.2 / 3.0$	= 13.66 kN/m --- B
Super Imposed Load, ws	=	10.00 kN/m ²	
Active Horizontal Force, F_{hs}	=	$K_a ws H = 0.49 * 10.0 * 2.20$	= 10.79 kN --- C
Over Turning Moment, M2	=	$F_a * H / 2 = 10.79 * 2.20 / 2$	= 11.87 kNm
Total Horizontal Force, Fha	=	$18.62 + 10.79 = 29.41 \text{ kN}$	(B + D)
Factored force.	=	$29.41 * 2 = 58.82 \text{ kN}$	
Total Moment	=	$13.66 + 11.87 = 25.52 \text{ kNm}$	
Mu	=	$25.52 * 2 = 51.04 \text{ kNm}$	



SLIDE CHECK

Self wt. of RC wall	=	$2.20 * 0.30 * 24$	=	15.84 kN/m	
Wt. of wall above.	=		=	kN/m	Assume wall opening 30 %
Roof Load	=		=	kN/m	
Total	=		=	15.8 kN/m	A
Self wt. of Base	=	$4.50 * 0.30 * 24$	=	32.4 kN/m	B
			=		C
Total Wt. A + B + C	=		=	48.2 kN/m	
Frictional resistance force.	=	$0.45 * 4.50 * 48.24$	=	97.69 kN	
Sliding force	=		=	58.82 < 97.69	PASS

Additionally, floor to be resisted by the existing foundation of the rear exten.

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OVERTURNING CHECK

RESTRAINING MOMENT

Due to wall	= 15.84 * 4499.85	=	69 kNm	
Due to Base	= 32.40 * 2.25	=	72.9 kNm	
Due to Soil behind	= 0.00 * 2.40	=	0.0 kNm	
Total Restraining moment	= 68.90 + 72.90 + 0.00	=	142 > 51.04	PASS
Overturing Moment	=			

NOTE All horizontal forces will neutralized with opposite forces.

	RC WALL	
Mu kNm	51.0	
D _v	300 Base thickness	
f _{cu}	35	
f _y	460	
b	1000	
d	250	
b _v	1000	
K = M / (f _{cu} * b * d ²)	0.023	
l _a = 0.5 + (0.25 - K / 0.9) * 0.5 ≤ 0.95	0.95	
z = l _a * d	238	
A _{st} = M / (0.95 * f _y * z) mm ²	492	
Tension Reinforcement	H 20 - 200	
A _s Provided mm ²	1570 > 492 PASS	
A _s Ratio = 100 * A _s / (3.14 * D ² / 4)	4.44	
d'	Not req.	
d' / d	Not req.	
A _s ' req. = (k - 0.156) * f _{cu} * b * d ² / {0.95 * f _y (d - d')}		
Compression Reinforcement		
A _s ' Provided = mm ²		
A _s ' Ratio = 100 * A _s ' / (B * D)		
A _s ' Prov - A _s ' req.		
A _s Prov - A _s req.		
Shear V _s =	58.8	
v = V _s / (b * D) = 58,819 / (1000 * 300) =	0.20	
100 * A _s / (b _v * d) ≤ 3	0.63	
400 / d ≥ 1	1.60	
v _c = 0.79 {100 * A _s / (b _v * d)} ^{1/3} * (400 / d) ^{1/4} / γ _m	0.76 > 0.20 PASS	

Tablr 3.8

Provide;

RC wall 300 mm Thick RC

Reinforcement H20 - 200 i.e. A393 Mexh Double layer top & Bottom