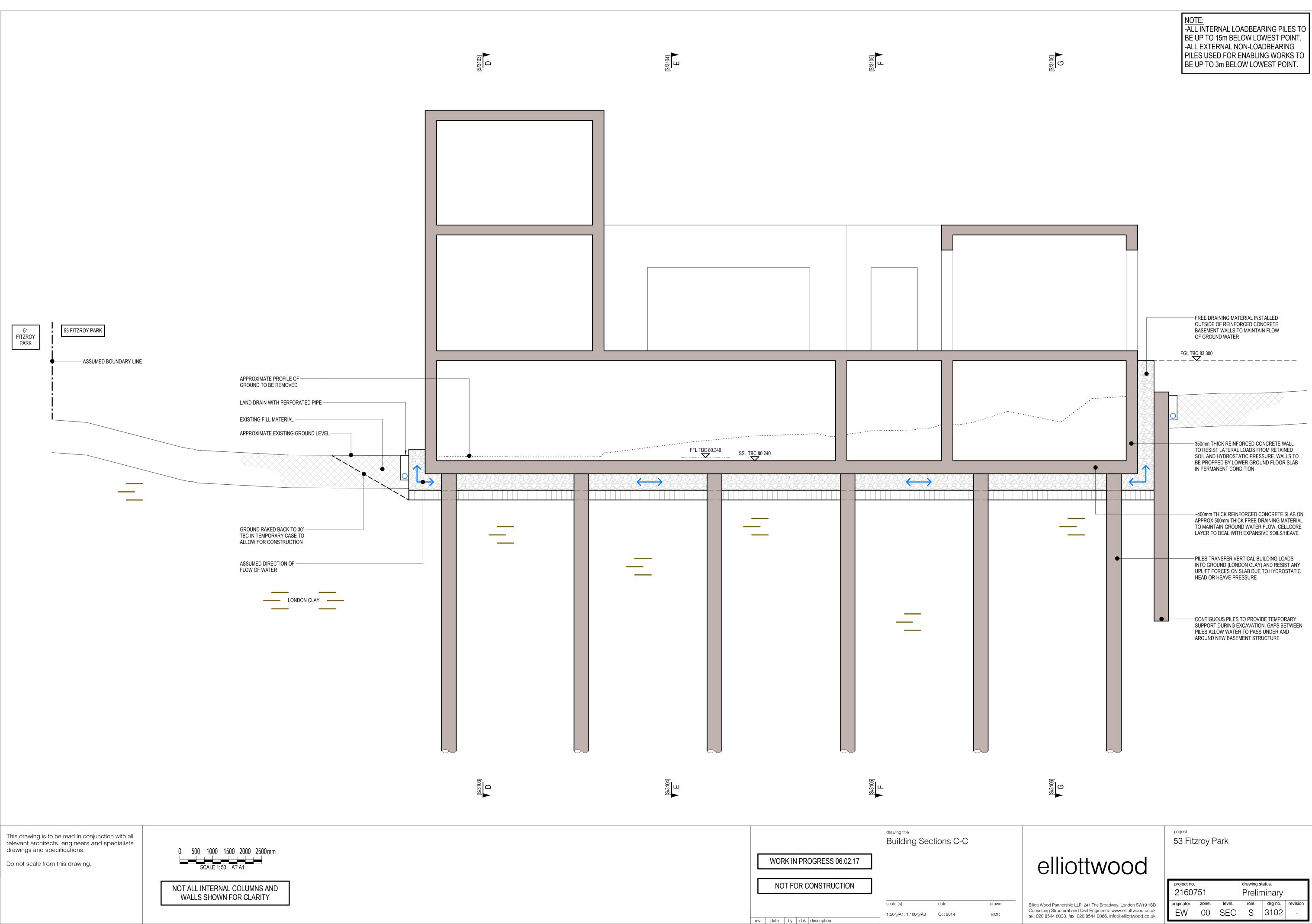
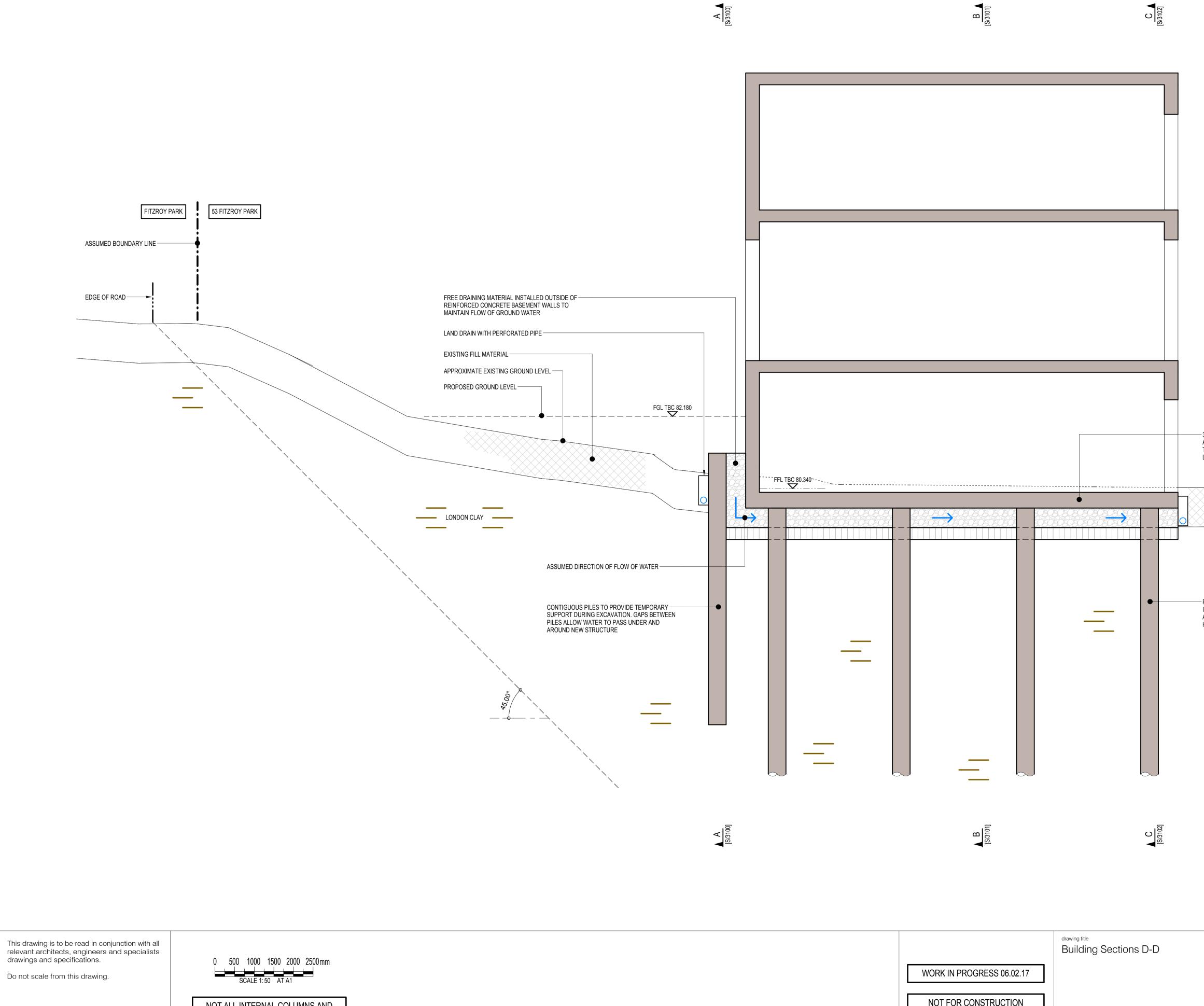


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NOT ALL INTERNAL COLUMNS AND WALLS SHOWN FOR CLARITY

		drawing title Building Sec	ctions D-D	
	WORK IN PROGRESS 06.02.17			
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300mm THICK REINFORCED CONCRETE SLAB ON APPROX 500mm THICK FREE DRAINING MATERIAL TO MAINTAIN GROUND WATER FLOW. CELLCORE LAYER TO DEAL WITH EXPANSIVE SOILS/HEAVE

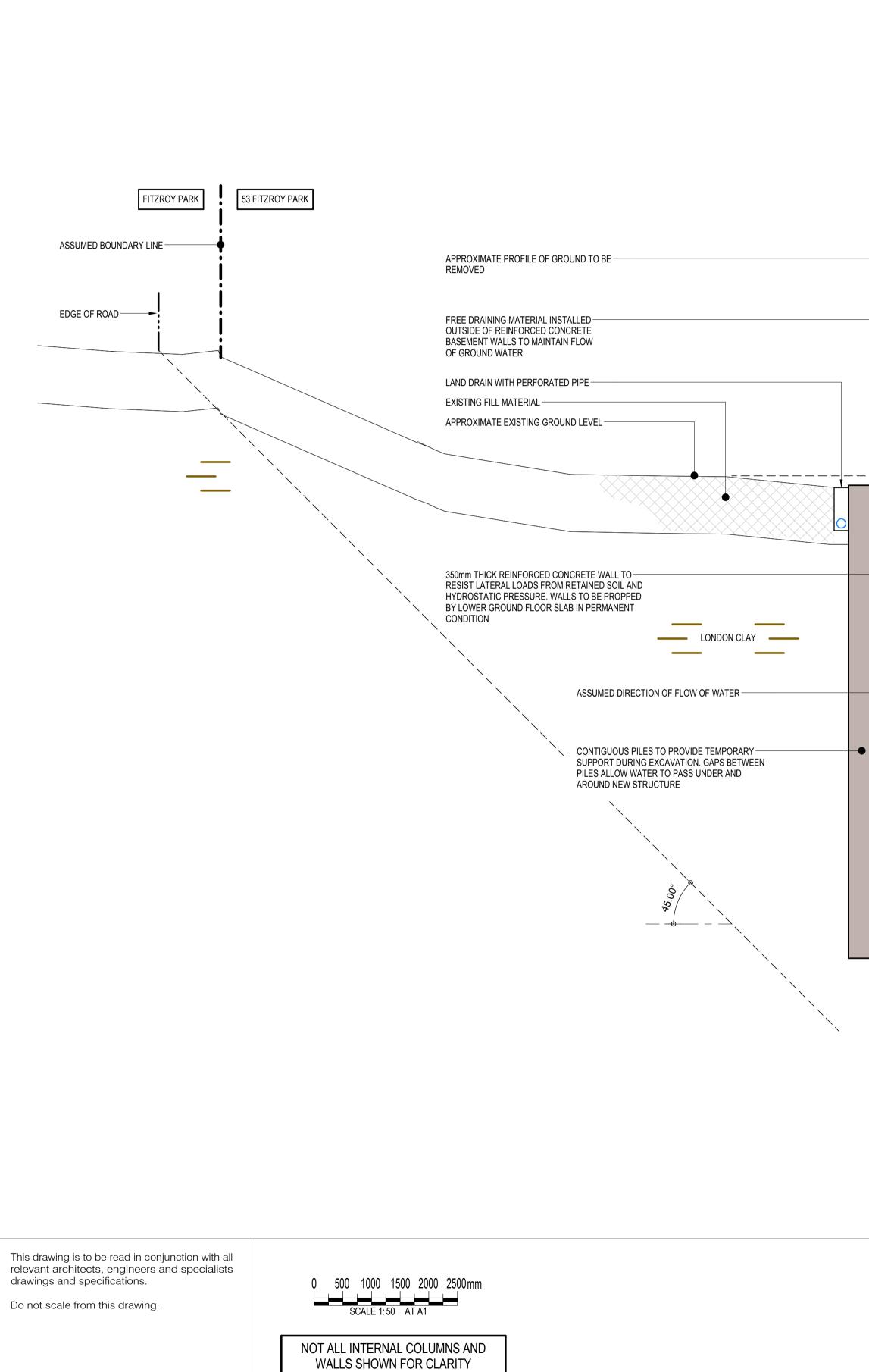
— PILES TRANSFER VERTICAL BUILDING LOADS INTO GROUND (LONDON CLAY) AND RESIST ANY UPLIFT FORCES ON SLAB DUE TO HYDROSTATIC HEAD OR HEAVE PRESSURE

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project 53 Fitzroy Park

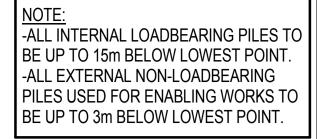
Elliott Wood Partnership LLP, 241 The Broadway, London SW19 1SD Consulting Structural and Civil Engineers. www.elliottwood.co.uk tel: 020 8544 0033. fax: 020 8544 0066. info@elliottwood.co.uk

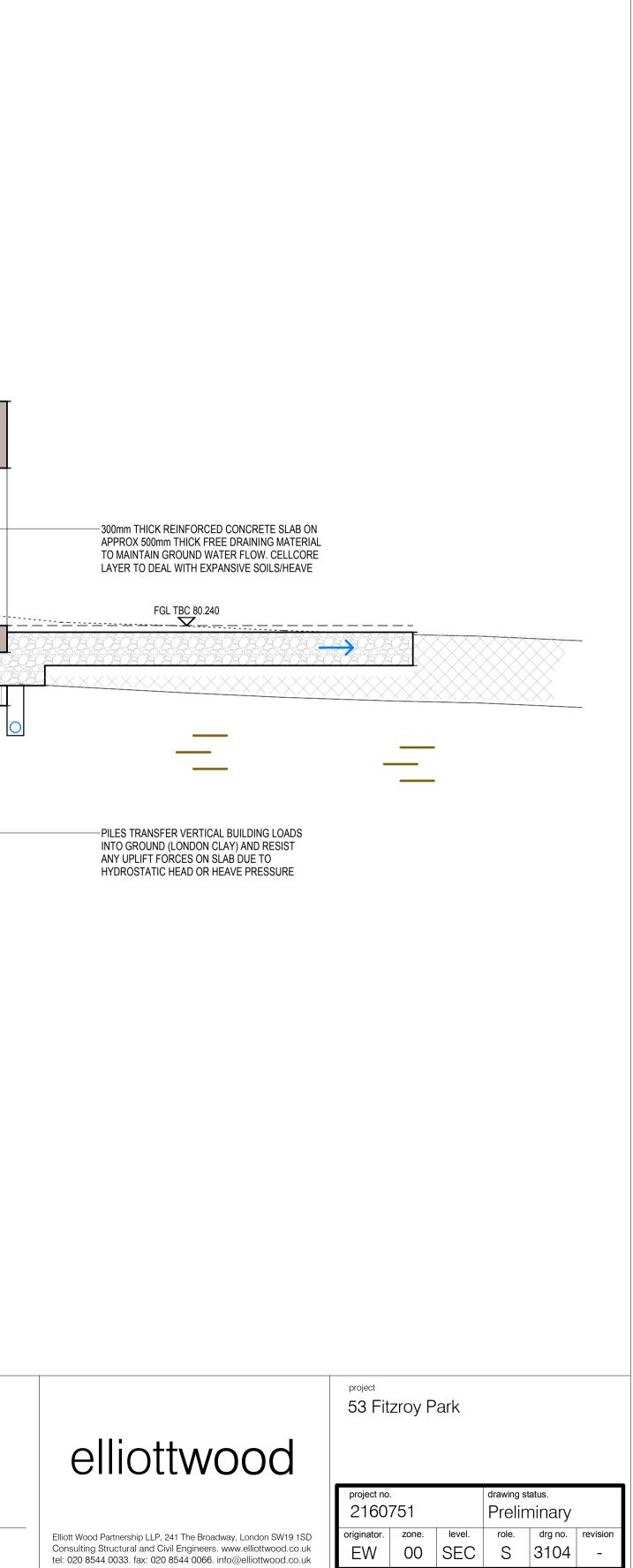
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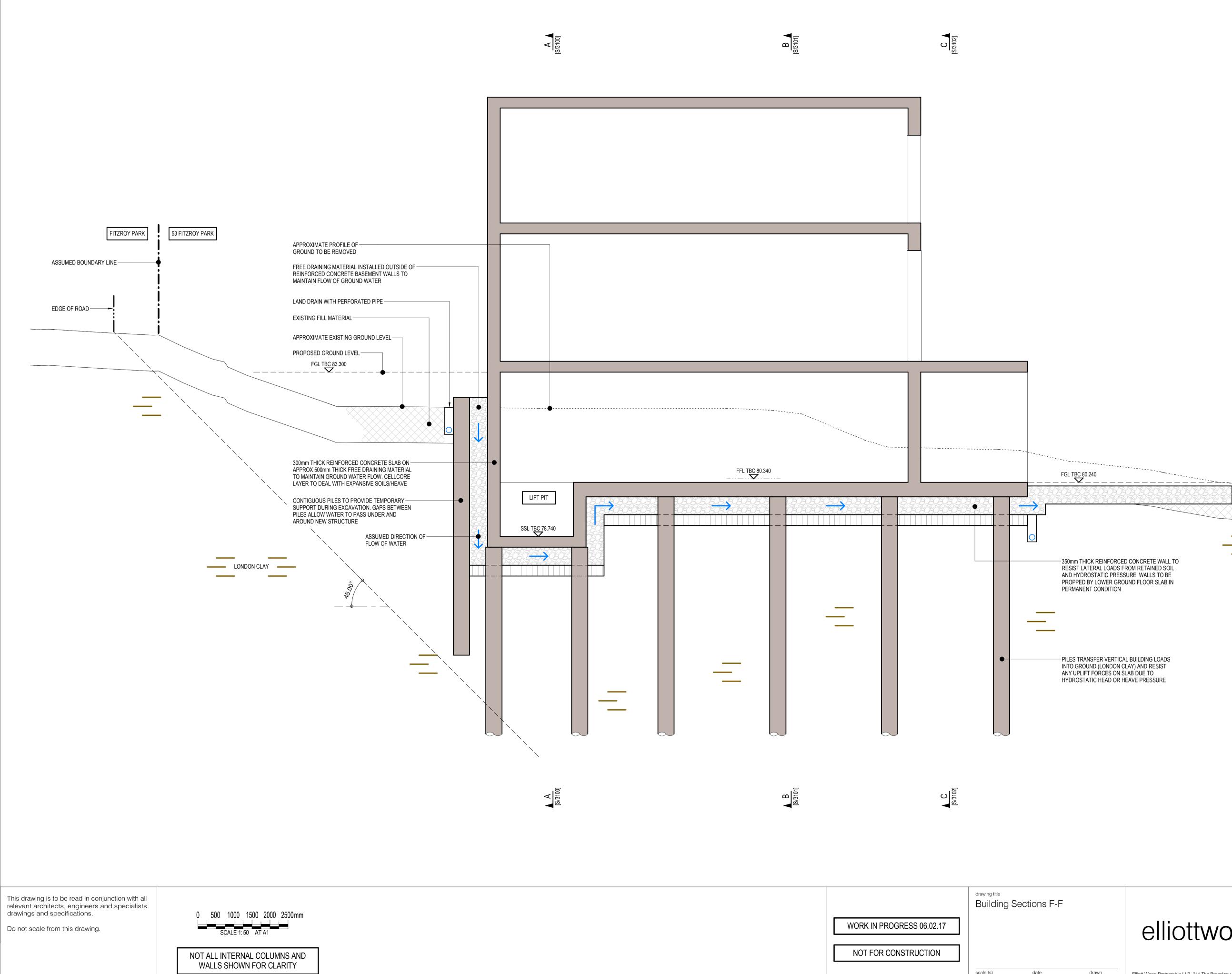


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- 350mm THICK REINFORCED CONCRETE WALL TO RESIST LATERAL LOADS FROM RETAINED SOIL AND HYDROSTATIC PRESSURE. WALLS TO BE PROPPED BY LOWER GROUND FLOOR SLAB IN PERMANENT CONDITION

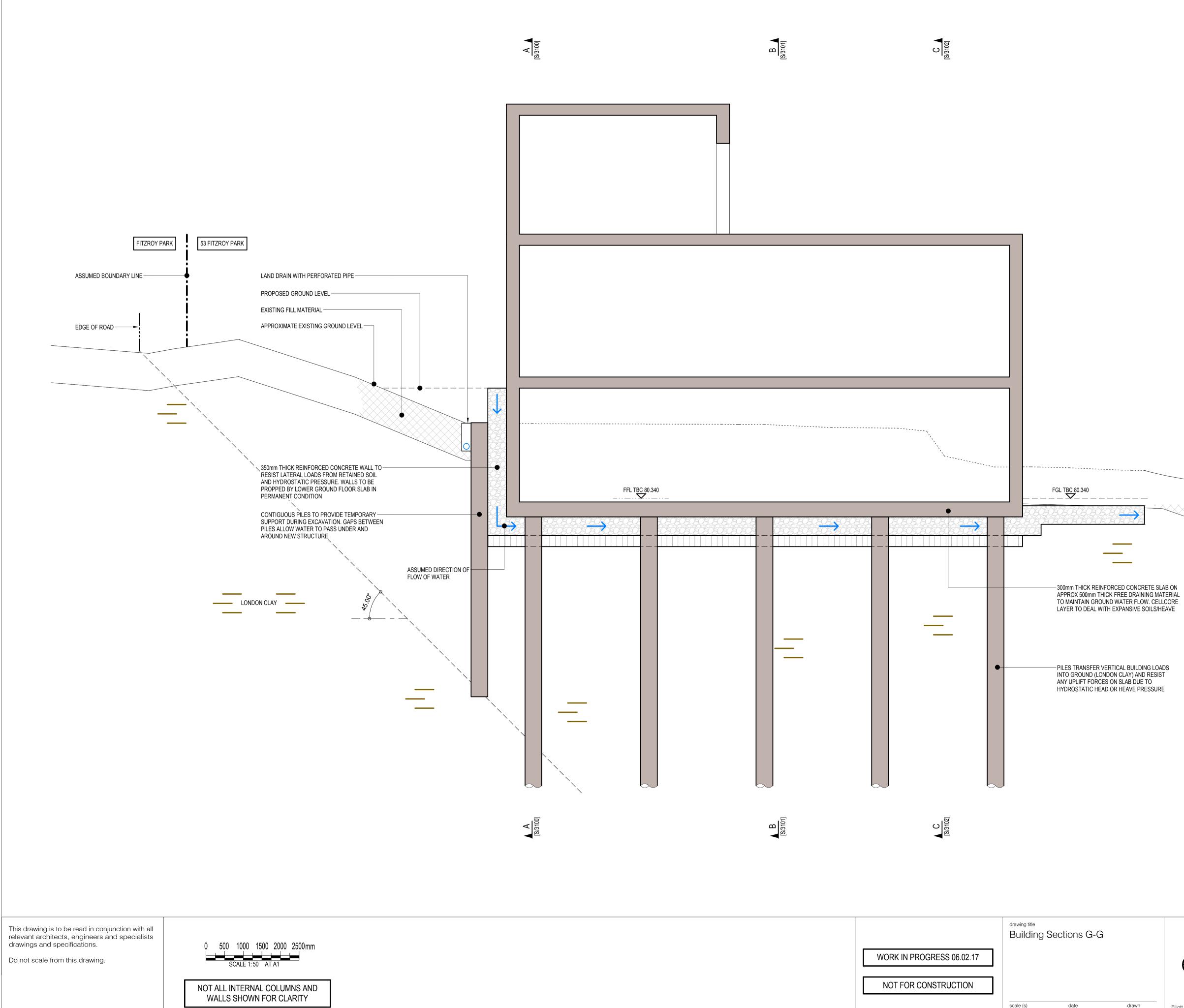
- PILES TRANSFER VERTICAL BUILDING LOADS INTO GROUND (LONDON CLAY) AND RESIST HYDROSTATIC HEAD OR HEAVE PRESSURE

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project

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originator.	zone.	level.	role.	drg no.	revision	
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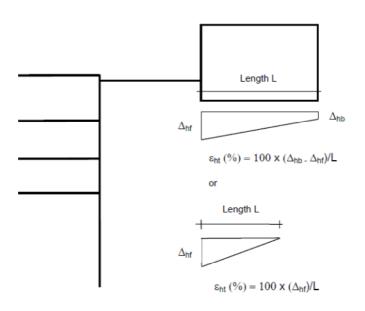


APPENDIX B DETERMINATION OF HORIZONTAL TENSILE STRAINS AND DEFLECTION RATIOS



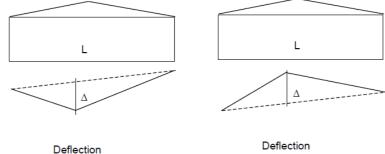
Horizontal Tensile Strain Eht

To determine the resulting horizontal tensile strain that will develop in the adjacent properties, the lateral displacement to the rear of the property (Δ_{hb}) is subtracted from the lateral displacement at the front of the property (Δ_{hf}). The resulting differential lateral displacement is then divided by the length of the property perpendicular to the basement wall to determine the horizontal tensile strain. Where the lateral displacement is zero at the rear of the property the lateral displacement at the front of the property is divided by the distance from the front of the property to the point of zero lateral displacement to determine the lateral strain. This approach is illustrated below and has been adopted for both the lateral displacements caused by wall installation and excavation.



Deflection Ratio A/L

The deflection ratio is defined as the off linear vertical displacement across a structure as illustrated below.



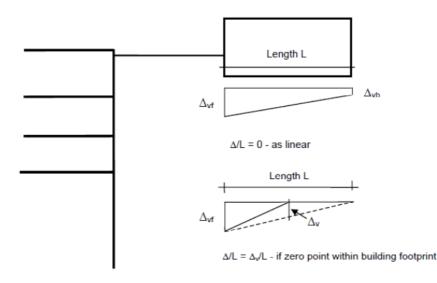


Ratio = $-\Delta/L$



A negative deflection ratio will result in greater damage to a property, as tension will be developed in the superstructure. Conversely with a positive deflection ratio compression will develop within the superstructure, which can be more easily resisted.

In the case of properties adjacent to a basement excavation the deflection ratio will depend on the distribution of vertical ground movement across its footprint. Reference to Figure 2.8 of CIRIA 580 in fact indicates a linear distribution of settlement due to wall installation with distance from the wall. As such the Deflection Ratio will be zero unless the point at which settlement ceases is within the footprint of the building as illustrated below.



Reference to Figure 2.11 of CIRIA 580 also indicates that the pattern of ground settlement due to basement excavation is relatively linear. As such the resulting Deflection Ratio is likely to be small. Within one excavation depth of the basement wall some non linearity is indicated however this would tend to result in a positive Deflection Ratio and as such be less likely to cause damage.

From the above discussion it is apparent that based on the empirical approach the vertical movements resulting from basement construction will only have a small influence on the damage to adjacent property compared to the lateral movements.



APPENDIX C DEFINITION OF DAMAGE CATEGORIES



Table 2.5 Classification of visible damage to walls (after Burland et al, 1977, Boscardin and Cording, 1989; and Burland, 2001)						
	ntegory of mage	Description of typical damage (ease of repair is underlined)	Approximate crack width (mm)	Limiting tensile strain ɛ _{lim} (per cent)		
0	Negligible	Hairline cracks of less than about 0.1 mm are classed as negligible.	< 0.1	0.0-0.05		
1	Very slight	Fine cracks that can easily be treated during normal decoration. Perhaps isolated slight fracture in building. Cracks in external brickwork visible on inspection.	< 1	0.05–0.075		
2	Slight	<u>Cracks easily filled. Redecoration probably</u> <u>required</u> . Several slight fractures showing inside of building. Cracks are visible externally and <u>some repointing may be required externally</u> to ensure weathertightness. Doors and windows may stick slightly.	< 5	0.075–0.15		
3	Moderate	The cracks require some opening up and can be patched by a mason. Recurrent cracks can be masked by suitable linings. Repointing of external brickwork and possibly a small amount of brickwork to be replaced. Doors and windows sticking. Service pipes may fracture. Weathertightness often impaired.	5–15 or a number of cracks > 3	0.15-0.3		
4	Severe	Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Windows and frames distorted, floor sloping noticeably. Walls leaning or bulging noticeably, some loss of bearing in beams. Service pipes disrupted.	15–25 but also depends on number of cracks	> 0.3		
5	Very severe	This requires a major repair involving partial or complete rebuilding. Beams lose bearings, walls lean badly and require shoring. Windows broken with distortion. Danger of instability.	but depends on number of			
 Notes In assessing the degree of damage, account must be taken of its location in the building or structure. Crack width is only one aspect of damage and should not be used on its own as a direct measure of it. 						