

Job 15168 - 18 GROVE TERRACE

Date SEPT '16 Page

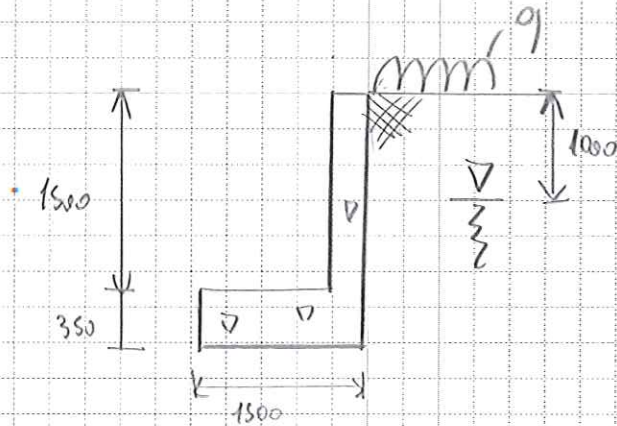
Title BASEMENT WALL

By FDI^P

Chkd IS

GEOMETRY

$q = 1.5 \text{ MN/m}^2$



ALLOW FOR WATER TABLE @ 1m FROM TOP OF WALL
THIS IS CONSERVATIVE, COMPARED TO SITE INVESTIGATION RESULTS.

REAR GARDEN WALLS WILL NOT SURCHARGE THE NEW WALLS. REFER TO SECTION C-C WITHIN APPENDIX B.

ALLOWABLE BEARING PRESSURE FROM SITE INVESTIGATION = $125 \frac{\text{kN}}{\text{m}^2}$

FROM TENDS, FOS = 4.4

Project 18 Grove Terrace				Job no. 15168	
Calcs for Rear Retaining Wall				Start page no./Revision 1	
Calcs by FDP	Calcs date 30/09/2016	Checked by IJ	Checked date	Approved by	Approved date

RETAINING WALL ANALYSIS

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.01

Retaining wall details

Stem type	Cantilever
Stem height	$h_{\text{stem}} = 1500 \text{ mm}$
Stem thickness	$t_{\text{stem}} = 250 \text{ mm}$
Angle to rear face of stem	$\alpha = 90 \text{ deg}$
Stem density	$\gamma_{\text{stem}} = 25 \text{ kN/m}^3$
Toe length	$l_{\text{toe}} = 1250 \text{ mm}$
Base thickness	$t_{\text{base}} = 350 \text{ mm}$
Base density	$\gamma_{\text{base}} = 25 \text{ kN/m}^3$
Height of retained soil	$h_{\text{ret}} = 1500 \text{ mm}$
Angle of soil surface	$\beta = 0 \text{ deg}$
Depth of cover	$d_{\text{cover}} = 0 \text{ mm}$
Height of water	$h_{\text{water}} = 500 \text{ mm}$
Water density	$\gamma_w = 9.8 \text{ kN/m}^3$

Retained soil properties

Soil type	Firm clay
Moist density	$\gamma_{\text{mr}} = 18 \text{ kN/m}^3$
Saturated density	$\gamma_{\text{sr}} = 18 \text{ kN/m}^3$
Characteristic effective shear resistance angle	$\phi'_{r,k} = 18 \text{ deg}$
Characteristic wall friction angle	$\delta_{r,k} = 9 \text{ deg}$

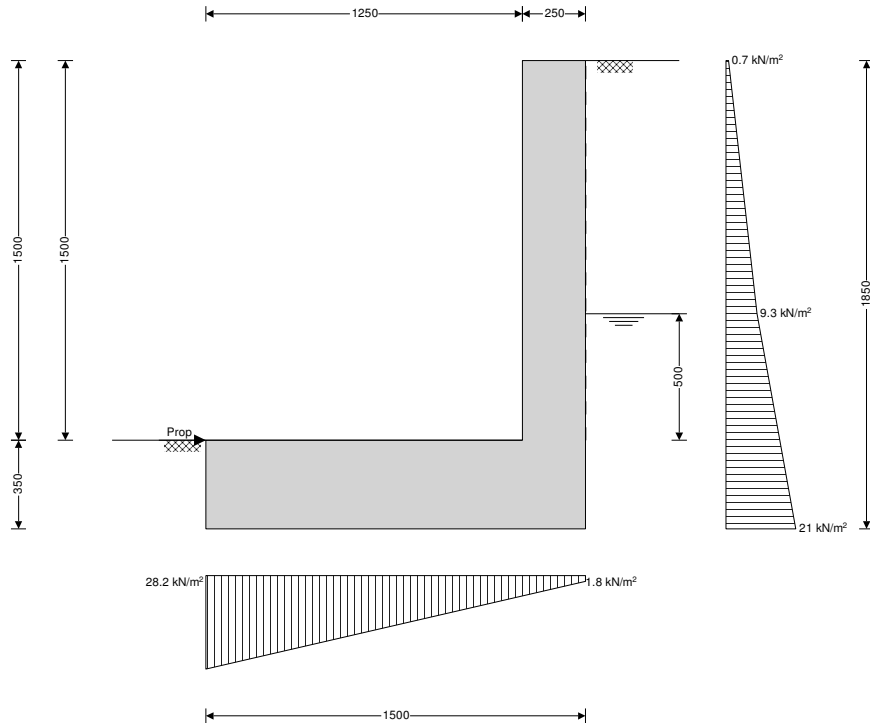
Base soil properties

Soil type	Organic clay
Soil density	$\gamma_b = 18 \text{ kN/m}^3$
Characteristic effective shear resistance angle	$\phi'_{b,k} = 18 \text{ deg}$
Characteristic wall friction angle	$\delta_{b,k} = 9 \text{ deg}$
Characteristic base friction angle	$\delta_{bb,k} = 12 \text{ deg}$
Presumed bearing capacity	$P_{\text{bearing}} = 125 \text{ kN/m}^2$

Loading details

Variable surcharge load	Surcharge _Q = 1.5 kN/m ²
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Project 18 Grove Terrace				Job no. 15168	
Calcs for Rear Retaining Wall				Start page no./Revision 2	
Calcs by FDP	Calcs date 30/09/2016	Checked by IJ	Checked date	Approved by	Approved date



General arrangement

Calculate retaining wall geometry

- Base length $l_{base} = l_{toe} + t_{stem} = \mathbf{1500 \text{ mm}}$
- Saturated soil height $h_{sat} = h_{water} + d_{cover} = \mathbf{500 \text{ mm}}$
- Moist soil height $h_{moist} = h_{ret} - h_{water} = \mathbf{1000 \text{ mm}}$
- Length of surcharge load $l_{sur} = l_{heel} = \mathbf{0 \text{ mm}}$
- Distance to vertical component $x_{sur_v} = l_{base} - l_{heel} / 2 = \mathbf{1500 \text{ mm}}$
- Effective height of wall $h_{eff} = h_{base} + d_{cover} + h_{ret} = \mathbf{1850 \text{ mm}}$
- Distance to horizontal component $x_{sur_h} = h_{eff} / 2 = \mathbf{925 \text{ mm}}$
- Area of wall stem $A_{stem} = h_{stem} \times t_{stem} = \mathbf{0.375 \text{ m}^2}$
- Distance to vertical component $x_{stem} = l_{toe} + t_{stem} / 2 = \mathbf{1375 \text{ mm}}$
- Area of wall base $A_{base} = l_{base} \times t_{base} = \mathbf{0.525 \text{ m}^2}$
- Distance to vertical component $x_{base} = l_{base} / 2 = \mathbf{750 \text{ mm}}$

Using Coulomb theory

- Active pressure coefficient $K_A = \frac{\sin(\alpha + \phi'_{r,k})^2}{(\sin(\alpha)^2 \times \sin(\alpha - \delta_{r,k}) \times [1 + \sqrt{(\sin(\phi'_{r,k} + \delta_{r,k}) \times \sin(\phi'_{r,k} - \beta) / (\sin(\alpha - \delta_{r,k}) \times \sin(\alpha + \beta))}]^2)} = \mathbf{0.483}$
- Passive pressure coefficient $K_P = \frac{\sin(90 - \phi'_{b,k})^2}{(\sin(90 + \delta_{b,k}) \times [1 - \sqrt{(\sin(\phi'_{b,k} + \delta_{b,k}) \times \sin(\phi'_{b,k}) / (\sin(90 + \delta_{b,k}))}]^2)} = \mathbf{2.359}$

Bearing pressure check

Vertical forces on wall

- Wall stem $F_{stem} = A_{stem} \times \gamma_{stem} = \mathbf{9.4 \text{ kN/m}}$
- Wall base $F_{base} = A_{base} \times \gamma_{base} = \mathbf{13.1 \text{ kN/m}}$

Project				Job no.	
18 Grove Terrace				15168	
Calcs for				Start page no./Revision	
Rear Retaining Wall				3	
Calcs by	Calcs date	Checked by	Checked date	Approved by	Approved date
FDP	30/09/2016	IJ			

Total	$F_{total_v} = F_{stem} + F_{base} + F_{water_v} = 22.5 \text{ kN/m}$
Horizontal forces on wall	
Surcharge load	$F_{sur_h} = K_A \times \cos(\delta_{r,d}) \times \text{Surcharge}_Q \times h_{eff} = 1.3 \text{ kN/m}$
Saturated retained soil	$F_{sat_h} = K_A \times \cos(\delta_{r,d}) \times (\gamma_{sr}' - \gamma_w') \times (h_{sat} + h_{base})^2 / 2 = 1.4 \text{ kN/m}$
Water	$F_{water_h} = \gamma_w' \times (h_{water} + d_{cover} + h_{base})^2 / 2 = 3.5 \text{ kN/m}$
Moist retained soil	$F_{moist_h} = K_A \times \cos(\delta_{r,d}) \times \gamma_{mr}' \times ((h_{eff} - h_{sat} - h_{base})^2 / 2 + (h_{eff} - h_{sat} - h_{base}) \times (h_{sat} + h_{base})) = 11.6 \text{ kN/m}$
Base soil	$F_{pass_h} = -K_P \times \cos(\delta_{b,d}) \times \gamma_b' \times (d_{cover} + h_{base})^2 / 2 = -2.6 \text{ kN/m}$
Total	$F_{total_h} = F_{sat_h} + F_{moist_h} + F_{pass_h} + F_{water_h} + F_{sur_h} = 15.3 \text{ kN/m}$
Moments on wall	
Wall stem	$M_{stem} = F_{stem} \times X_{stem} = 12.9 \text{ kNm/m}$
Wall base	$M_{base} = F_{base} \times X_{base} = 9.8 \text{ kNm/m}$
Surcharge load	$M_{sur} = -F_{sur_h} \times X_{sur_h} = -1.2 \text{ kNm/m}$
Saturated retained soil	$M_{sat} = -F_{sat_h} \times X_{sat_h} = -0.4 \text{ kNm/m}$
Water	$M_{water} = -F_{water_h} \times X_{water_h} = -1 \text{ kNm/m}$
Moist retained soil	$M_{moist} = -F_{moist_h} \times X_{moist_h} = -8.2 \text{ kNm/m}$
Total	$M_{total} = M_{stem} + M_{base} + M_{sat} + M_{moist} + M_{water} + M_{sur} = 11.9 \text{ kNm/m}$
Check bearing pressure	
Propping force	$F_{prop_base} = F_{total_h} = 15.3 \text{ kN/m}$
Distance to reaction	$\bar{x} = M_{total} / F_{total_v} = 530 \text{ mm}$
Eccentricity of reaction	$e = \bar{x} - l_{base} / 2 = -220 \text{ mm}$
Loaded length of base	$l_{load} = l_{base} = 1500 \text{ mm}$
Bearing pressure at toe	$q_{toe} = F_{total_v} / l_{base} \times (1 - 6 \times e / l_{base}) = 28.2 \text{ kN/m}^2$
Bearing pressure at heel	$q_{heel} = F_{total_v} / l_{base} \times (1 + 6 \times e / l_{base}) = 1.8 \text{ kN/m}^2$
Factor of safety	$FoS_{bp} = P_{bearing} / \max(q_{toe}, q_{heel}) = 4.431$

PASS - Allowable bearing pressure exceeds maximum applied bearing pressure