

SITE INVESTIGATION FACTUAL REPORT

Report No: 399437
Client: Cunningham Lindsey - Maidstone
Site: Flat 1 & 2, 108 Greencroft Gardens
Client Ref: 6332780-108 [REDACTED]
Date of Visit: 21/02/17



Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys

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CET is the trading name of CET Structures Ltd
Registered in England No. 02527130

Investigation Layout Plan

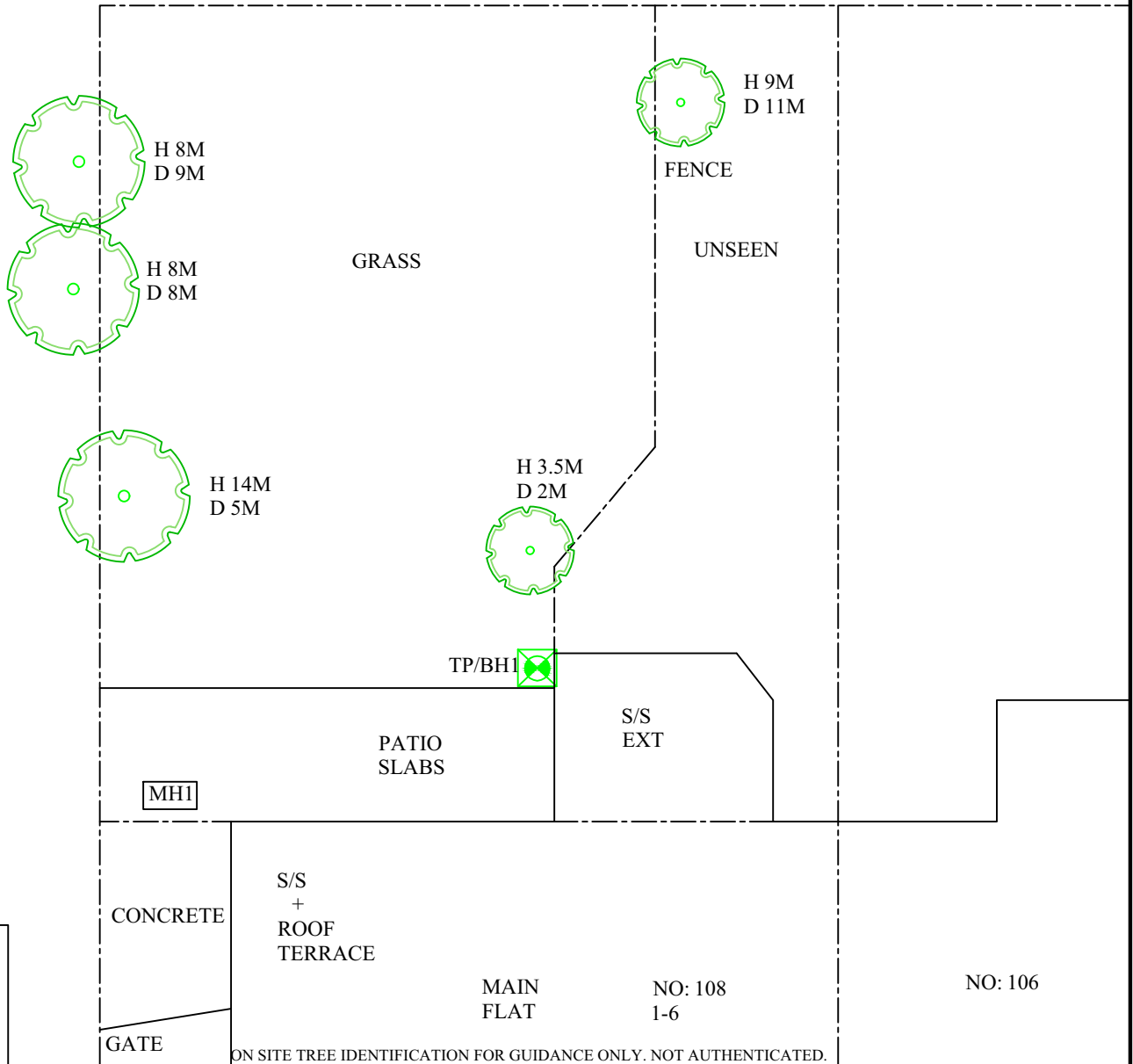
Sheet: 1 of 1
Job No: 399437
Date: 21/02/2017

Site: Flate 1 + 2 108 Greencroft Gardens

MR (SI) SA (Checked) PS (Drawn)

Weather: DRY

Work carried out for: Cunningham Lindsey



Remarks:

Key:

Combined Gulley RWWG
Manhole MH
Rain Water Pipe RWP
Rain Water Gulley RWG
Soil Vent Pipe SVP
Waste Gulley WG
Waste Pipe WP

Surface Water Drain

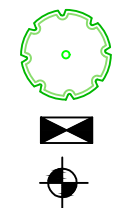
Foul Water Drain

Tree / Bush
(approx. ht in m)

Trial Pit

Borehole

O/D - Open Discharge



Scale: N.T.S.

Trial Pit No: 1

Sheet: 1 of 1

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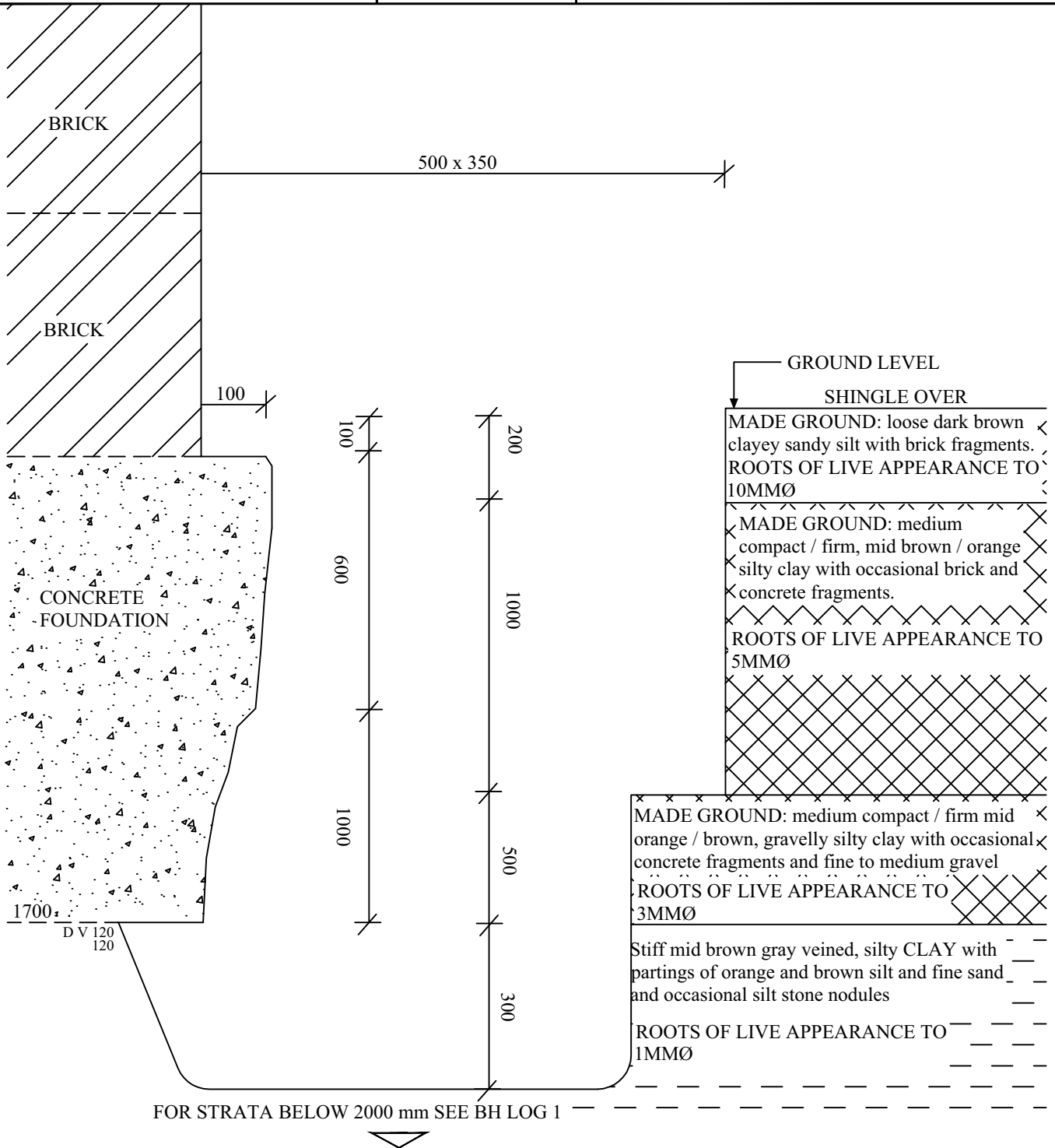
Hand Tools

Drawn by: RM

Work carried out for: Cunningham Lindsey

Weather: DRY

Ground Level
mOD:



Remarks: All measurements in millimetres. TP excavated to 1200MM. Then extended to 2000MM with the aid of a hand auger. curved steel pin driven 100MM under foundation at 1700MM below ground level.

Key:

D	Small disturbed sample	J	Jar sample
B	Bulk disturbed sample	V	Pilcon Vane (kPa)
W	Water sample	M	Mackintosh probe
TDTD	Too dense to drive		

Logged: MR

Checked: SA

Approved:

Scale: N.T.S.

Borehole		1		Sheet: 1 of 1		Site: Flat 1 & 2				
Boring Method: Hand Auger		Weather: dry		Job No: 399437		Date: 21/02/2017				
Diameter (mm): 75		Ground Level:		Client: Cunningham Lindsey - Maidstone						
Depth (m)	Soil Description					Thickness	Legend	Depth	Type	Result
0.00	See Trial Pit					2.00				
2.00	Stiff brown silty CLAY with partings of orange silt and fine sand with occasional siltstone and crystals					3.00	x — x	2.00	DV	120+
							x — x			120+
							x — x			
							x — x			
							x — x			
							x — x	2.50	DV	120+
							x — x			120+
							x — x			
							x — x			
							x — x			
							x — x	3.00	DV	120+
							x — x			120+
							x — x			
							x — x			
							x — x			
							x — x	3.50	DV	120+
							x — x			120+
							x — x			
							x — x			
							x — x			
							x — x	4.00	DV	120+
							x — x			120+
							x — x			
							x — x			
							x — x			
							x — x	4.50	DV	120+
							x — x			120+
5.00	End of BH							5.00	DV	120+
										120+
Remarks: BH ends at 5.0m.BH dry and open on completion.Hair and fibrous roots to 3.1m.Dead and decomposing roots to 4.6m. No roots observed below 4.6m.						Key:		To	Max	
						D - Disturbed Sample		Depth	Dia	
						B - Bulk Sample		(m)	(mm)	
						W - Water Sample		2.90	1	
						Roots				
						J - Jar Sample				
						Roots				
						V - Pilcon Shear Vane (kPa)				
						Roots				
						M - Mackintosh Probe				
						Depth to Water (m)				
						TDTD - Too Dense To Drive				
Logged:	MR	SA	Checked:	Approved:	Version	V1.0 28/01/16		N.T.S.		

Laboratory Summary Results

Our Ref: 399437 Date Sampled: 21/02/17
 Location: Flat 1 & 2, Greencroft Gardens, NW6 Date Received: 22/02/17
 Client: Cunningham Lindsey - Maidstone Date Tested: 23/02/17
 Address: 4 North Court, South Park Business Village, Armstrong Road, ME15 6JZ Date of Report: 07/03/17

TP/BH No	Sample Ref Depth (m)	Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity Index [5]	Modified * Plasticity Index (%) [6]	Soil * Class [7]	Filter Paper Contact Time (h) [7]	Soil Suction (kPa) [8]	Oedometer Strain [9]	Estimated Heave Potential (Dd) (mm) [10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH * Value [13]	Sulphate Content * (g/l)		* Class [16]
																		SO ₃ [14]	SO ₄ [15]	
1	U/S 1.70	D	26	<5	70	23	47	0.07	47	CV	168	677			> 120					
	2.0	D	28	<5											> 120					
	2.5	D	32	<5	72	29	43	0.07	43	CV	168	333			> 120					
	3.0	D	30	<5											> 120					
	3.5	D	30	<5	69	29	40	0.04	40	CH	168	422			> 120					
	4.0	D	29	<5											> 120					
	4.5	D	31	<5							168	481			> 120					
	5.0	D	31	<5							168	521			> 120					

Test Methods / Notes

[1] BS 1377: Part 2: 1990, Test No 3.2
 [2] In-house Test Procedure S17a: One Dimensional Swell/Strain Test
 [3] Estimated if <5%, otherwise measured
 [4] BS 1377: Part 2: 1990, Test No 4.4
 [5] BS 1377: Part 2: 1990, Test No 5.3
 [6] BS 1377: Part 2: 1990, Test No 5.4
 [7] BRE Digest 240: 1993
 [8] BS 5930: 1981: Figure 31 - Plasticity Chart for the classification of fine soils
 [9] In-house method S9a adapted from BRE IP 4/93
 [10] In-house Test Procedure S17a: One Dimensional Swell/Strain Test
 [11] Estimated Heave Potential (Dd)
 [12] Values of shear strength were determined in situ by CET using a Picon hand vane or Geonor vane (GV).
 [13] BS 1377: Part 3: 1990, Test No 4
 [14] BS 1377: Part 3: 1990, Test No 9
 [15] BS 1377: Part 3: 1990, Test No 5.6
 [16] SO₄ = 1.2 x SO₃
 [16] BRE Special Digest One (Concrete in Aggressive Ground) August 2005
 Note that if the SO₄ content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.
 * These tests are not UKAS accredited
 Full reports can be provided upon request

Key

D Disturbed sample (small)
 B Disturbed sample (bulk)
 U Undisturbed sample
 W Groundwater sample
 ENP Essentially Non-Plastic by inspection
 US Underside of Foundation



Moisture Content Profiles

Shear Strength Profiles

Our Ref: 399437

Location: Flat 1 & 2, Greenerof Gardens, NW6

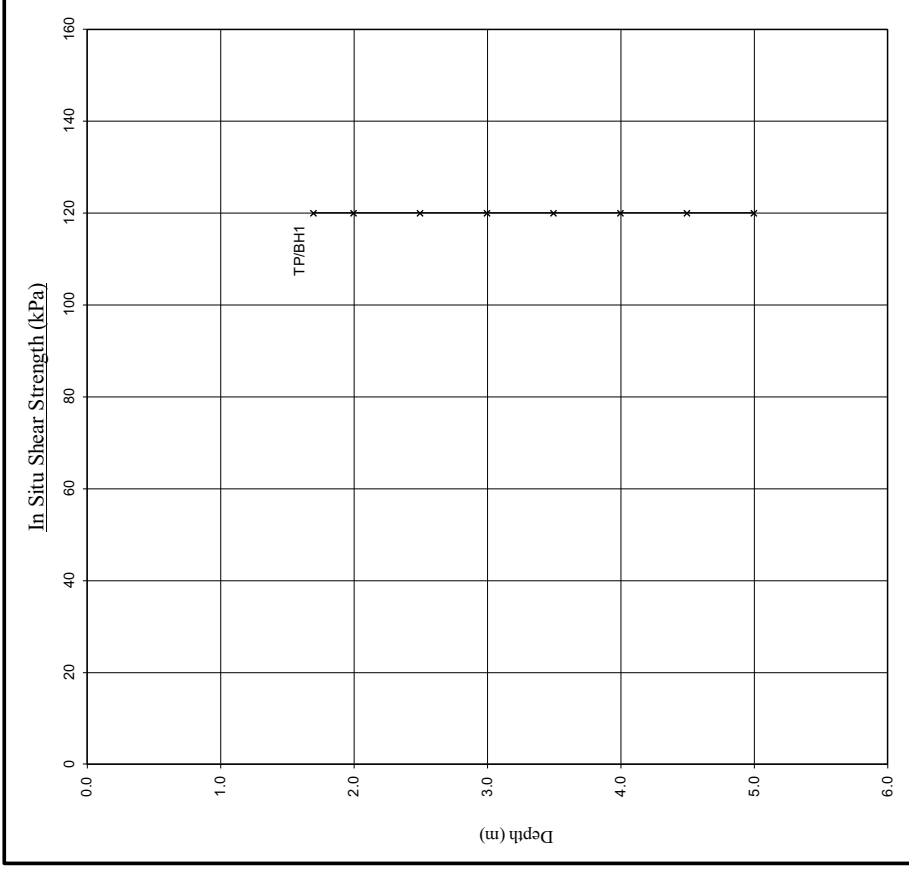
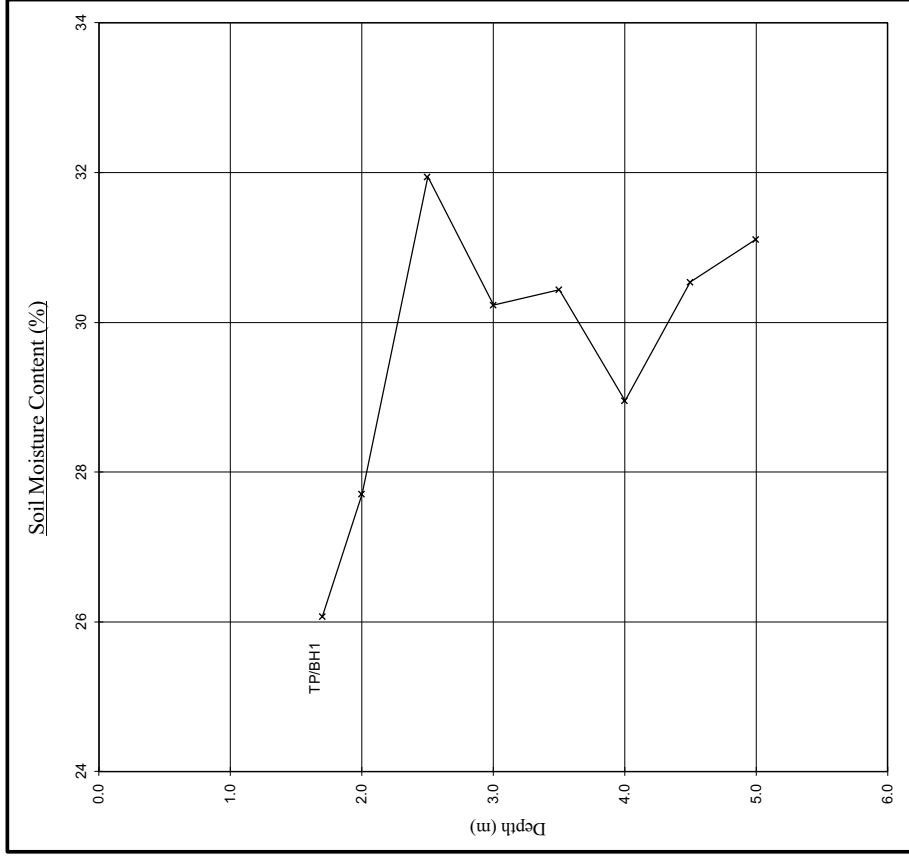
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Notes

1. If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

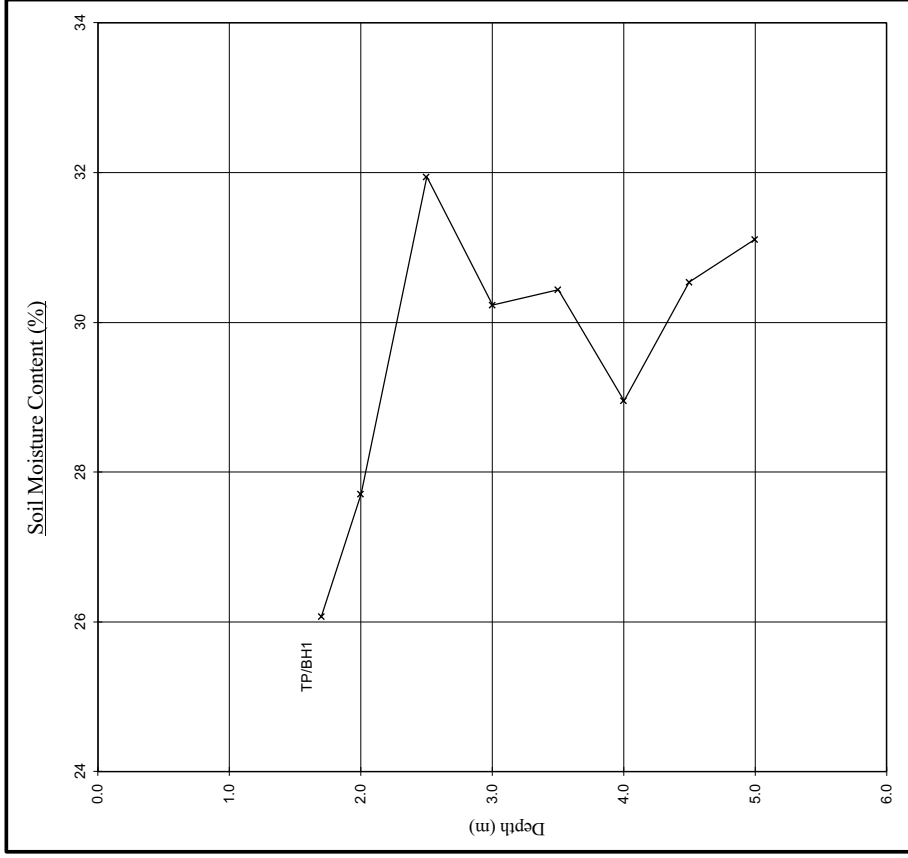
Note

1. Unless otherwise stated, values of Shear Strength were determined in situ by CET using a Picon Hand Vane the calibration of which is limited to a maximum reading of 120 kPa.
2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

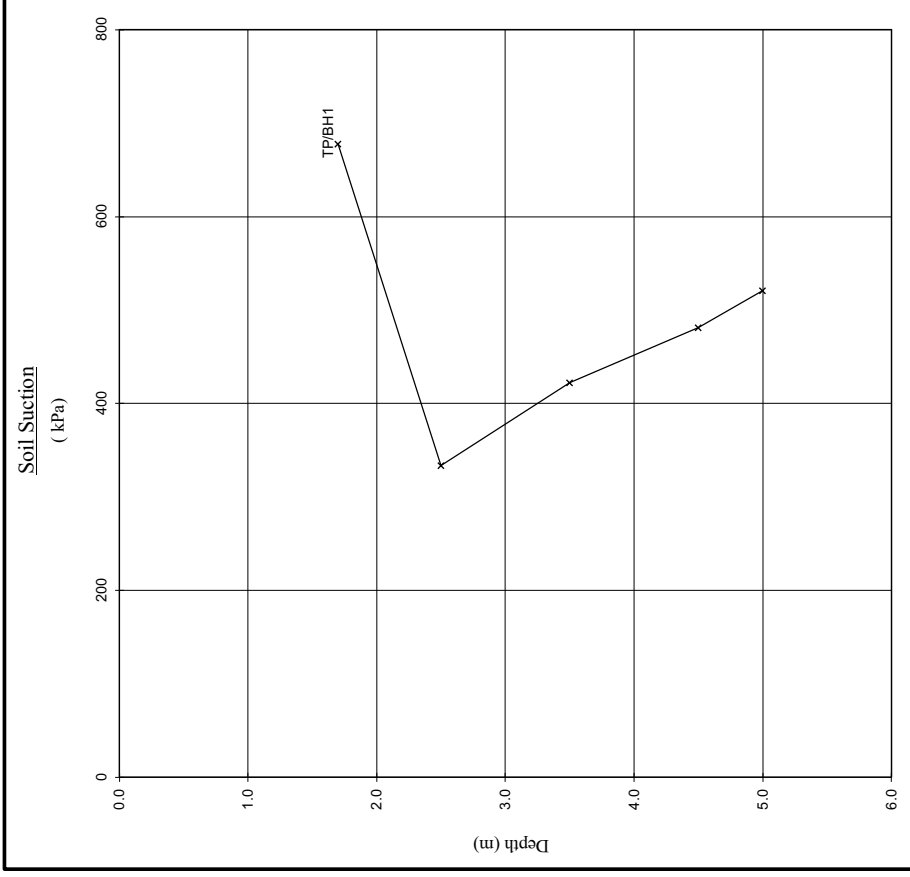
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Soil Suction Profiles



Note
When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.


Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.

The results were as follows -

Trial pit/ Borehole number	Root diameter (mm)	Tree, shrub or climber from which root originates	Result of starch test
TP1 (USF)	1 mm	Quercus spp. 4 roots	Positive
BH1 (2.9-4.6m)	1 mm	Quercus spp. 4 roots	Positive

Quercus spp. are oaks (both deciduous and evergreen).



MDM

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Plant Anatomist : Dr G S Turner B.Sc. (Hons), M.Sc., Ph.D

Plant Anatomist : Dr D P Aebischer B.Sc. (Hons), M.Sc., Ph.D

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