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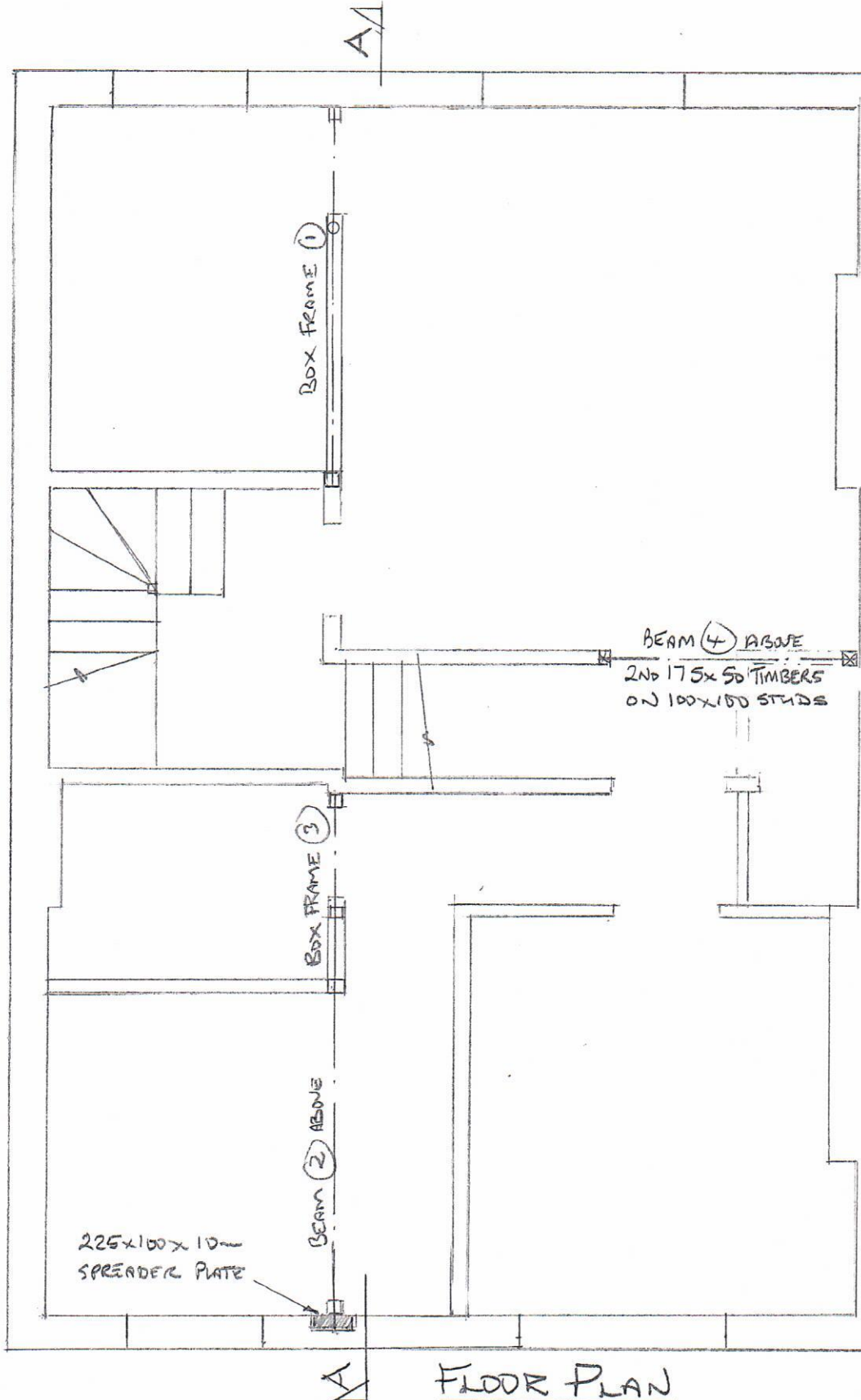
Date 3/17

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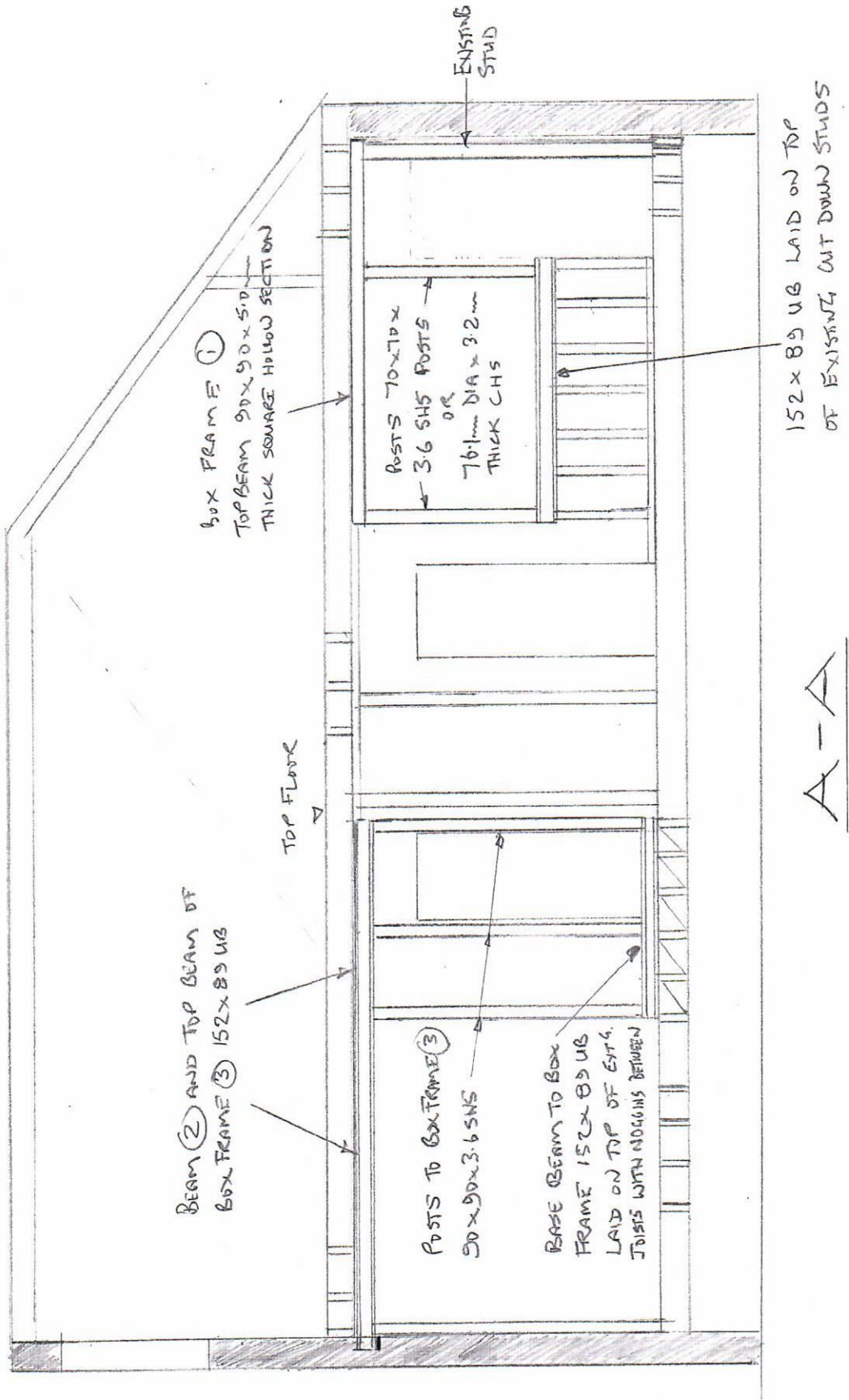
Eng. *MG*

7139/SKI

Job. FLAT 4
17 PRINCE ALBERT ROAD
LONDON NW1



FLOOR PLAN



STRUCTURAL DESIGN FOR PROPOSED ALTERATIONS

TO BE READ IN CONJUNCTION WITH DRAWINGS NOS 7139/SK1 - SK2

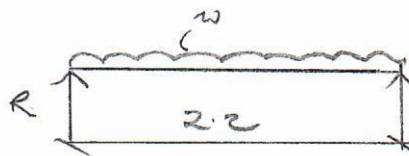
UNIT LOADS

ROOF	DL+IL	= 1.6 kN/m ² PLAN
TIMBER FLOOR	DL+IL	= 2.0 kN/m ²
STUD PARTITION		= 0.5 kN/m ² ELEV.

BOX FRAME ①

WE WILL USE A BOX FRAME TO SPREADER THE LOAD FROM ABOVE BACK TO THE EXISTING STUDS

TOP BEAM LOADING



W. ATTIC FLOOR ALLOW $2.0 \times 6.5 \times 0.5 = 6.5 \text{ kNm}$
 PARTN ALLOW $0.5 \times 2.4 = 1.2$
 7.7 kNm

$R = 8.5 \text{ kN}$

BENDING $M = 7.7 \times \frac{2.2^2}{8} = 4.7 \text{ kNm}$

FOR $90 \times 90 \times 5.0$ SHS, $f_{ax} = \frac{4.7 \times 10^6}{45.0 \times 10^3} = 104 \text{ N/mm}^2$ $< p_{bx}$

DEFLECTION. TOTAL $\delta = \frac{5}{384} \times \frac{7.7 \times 2.2^4 \times 10^6}{21 \times 10^4 \times 202 \times 10^4} = 5.5 \text{ mm}$
 $\approx \frac{1}{400}$

BEAM $90 \times 90 \times 5.0$ SHS

POSTS $R = 8.5 \text{ kN}$. For $70 \times 70 \times 3.6 \text{ SHS}$, $f_c = \frac{8.5 \times 10^3}{95 \times 10^4} = 9 \text{ N/mm}^2 < p_c$
OR $76.1 \times 3.2 \text{ CHS}$, $f_c = 12 \text{ N/mm}^2 < p_c$

POSTS $70 \times 70 \times 3.6 \text{ SHS}$ OR $76.1 \times 3.2 \text{ THICK CHS}$

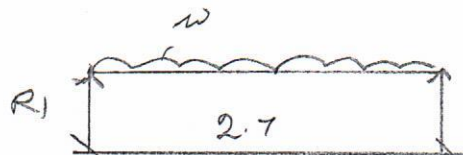
BOTTOM BEAM BY INSPECTION $152 \times 89 \text{ UB}$

BOTTOM BEAM $152 \times 89 \text{ UB}$

NOTE: IF SOME ROOF LOAD IS TRANSFERRED TO THE BOX FRAME, THE ELEMENTS HAVE PLENTY OF ADDITIONAL CAPACITY

BEAM (2)

LOADING



w . ALLOW AS BOX FRAME (1) $= 7.7 \text{ kNm}$
 $R_1 = 10.4 \text{ kN}$.

BENDING $M = 7.7 \times \frac{2.7^2}{8} = 7.0 \text{ kNm}$

FOR $152 \times 89 \text{ UB}$, $f_{bc} = \frac{7.0 \times 10^6}{110 \times 10^3} = 64 \text{ N/mm}^2$

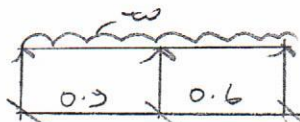
DEFLECTION, TOTAL $\delta = 3.1 \text{ mm}$

$152 \times 89 \text{ UB}$

BOX FRAME (3)

WE WILL USE A BOX FRAME, WITHIN THE BATHROOM PARTITION, TO SPREAD THE LOADS ONTO THE PARTITION BELOW, ASSUMING THAT THE FRAME IS STIFF ENOUGH TO SPREAD THE LOAD FROM BEAM (2) OVER ITS LENGTH

LOADING



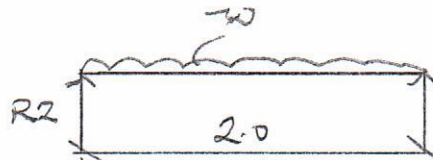
W. AS BEAM (2) = 7.7 kNm
BEAM (2) LOAD = $\frac{10.4}{1.5}$ = 6.9
14.6 kNm

BY INSPECTION TOP + BOTTOM BEAMS 152 x 89 UBS
POSTS 90 x 90 x 3.6 SHS,

BEAMS 152 x
89 UBS AND
POSTS 90 x 90
x 3.6 SHS

BEAM (4)

LOADING



W PARTN 0.5 x 1.0 = 0.5 kNm
ROOF ALLOW 1.6 x 3.4 x 0.5 = 2.7
3.2 kNm

R2 = 3.2 kN

BENDING $M = 3.2 \times \frac{2.0^2}{8} = 1.6 \text{ kNm}$

FOR 2 NO 175 x 50 TIMBERS, $\sigma = \frac{1.6 \times 10^6}{2 \times 50 \times \frac{170^3}{6}} = 3.3 \text{ N/mm}^2$

DEFLECTION TOTAL $\delta = \frac{5}{384} \times \frac{3.2 \times 2.0^4 \times 10^4}{5800 \times 1.14 \times \frac{2 \times 50 \times 170^3}{12}} = 2.3 \text{ mm}$



2 NO 175 x
50 TIMBERS
ON 100 x 100
TIMBER STUDS