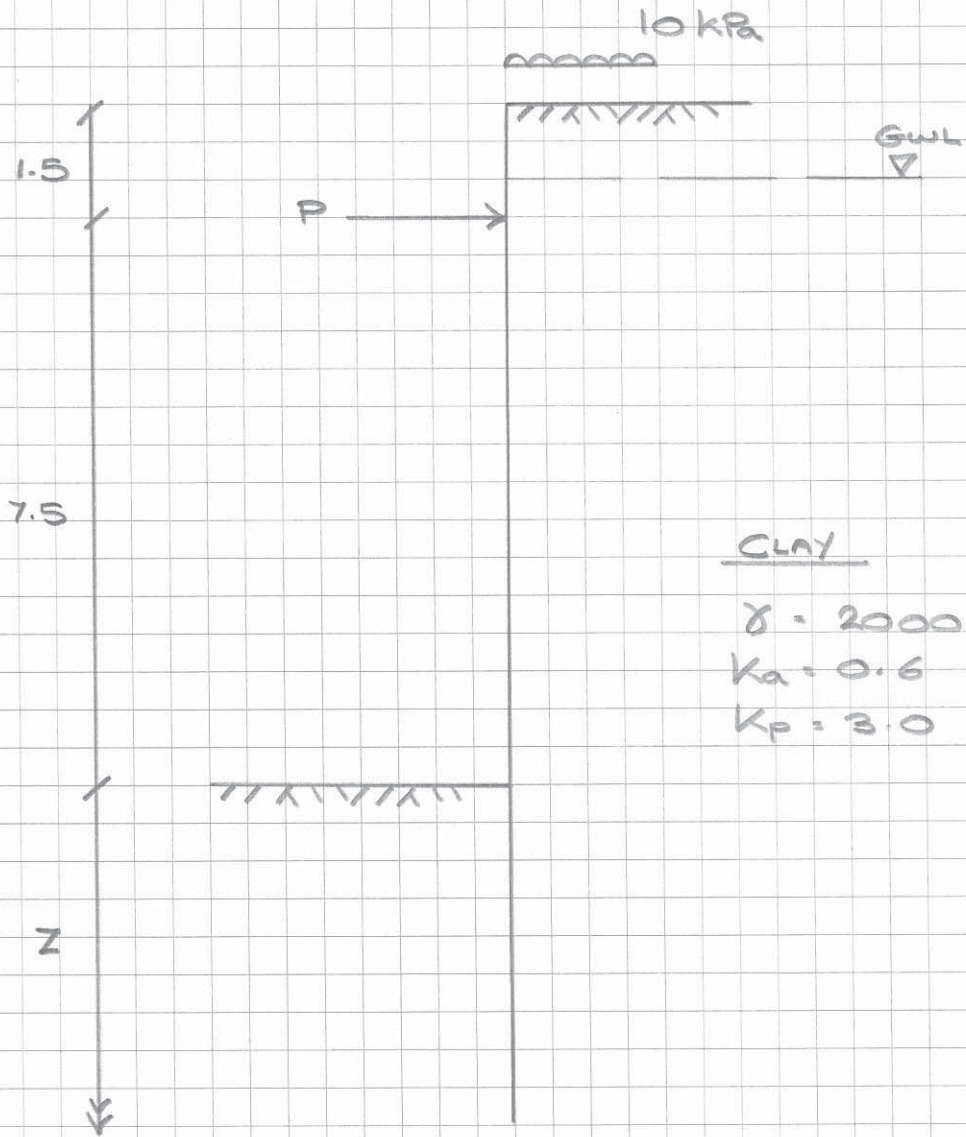


APPENDIX B – STRUCTURAL CALCULATIONS FOR PLANNING

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TEMPORARY PROPPING TO REAR WALL



FROM COMPUTER OUTPUT  $P = 690 \text{ kN} \text{ (SLS)}$

Project 26 Netherhall Gardens			Job no. 8240		
Calcs for Retaining Wall Temporary Case			Start page no./Revision 1		
Calcs by TJM	Calcs date 25/06/2014	Checked by	Checked date	Approved by	Approved date

TEDDS calculation version 1.0.02

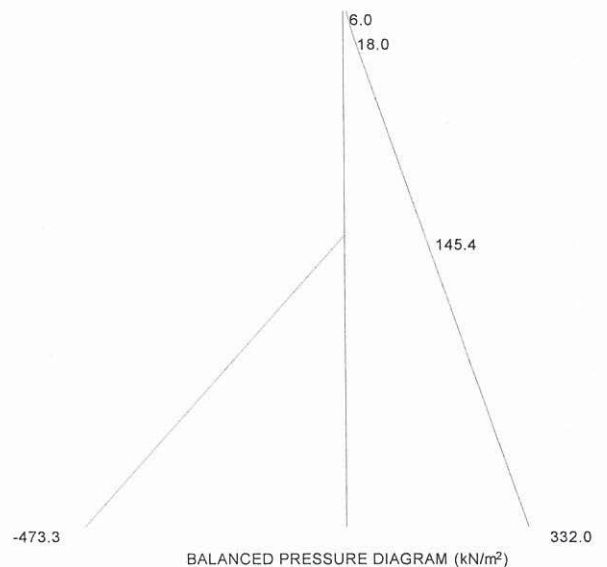
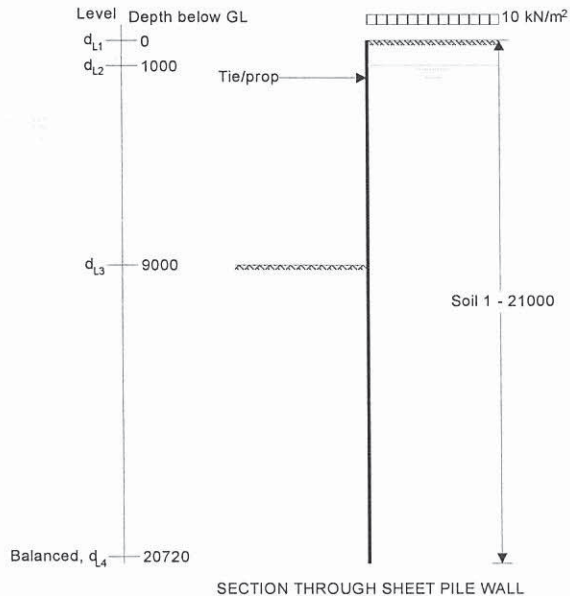
### Tied wall with free earth support

#### Geometry

Length of pile for equi	H = 20720 mm	Length of pile provided	H <sub>pile</sub> = 21000 mm
No. of different types of soil	N <sub>s</sub> = 1	Retained height	d <sub>ret</sub> = 9000 mm
Depth of unplanned excavation	mm	d <sub>ex</sub> = 0 mm	Retained height d <sub>s</sub> = 9000
Angle of retained slope	β = 0.0 deg		
Depth of water retained side	d <sub>w</sub> = 1000 mm	Depth of water retaining side	d <sub>wp</sub> = 9000 mm

#### Soil layer 1

Moist density of soil	γ <sub>m1</sub> = 20.0 kN/m <sup>3</sup>	Dry density of soil	γ <sub>d1</sub> = 10.2 kN/m <sup>3</sup>
Active pressure coefficient	k <sub>a1</sub> = 0.600	Passive pressure coefficient	k <sub>p1</sub> = 3.000



#### Active and passive pressures

Active pressure at 0m	p <sub>a11</sub> = 6.0 kN/m <sup>2</sup>	Passive pressure at 9.0 m	p <sub>p31</sub> = 0.0 kN/m <sup>2</sup>
Active pressure at 1.0 m	p <sub>a21</sub> = 18.0 kN/m <sup>2</sup>	Passive pressure at 20.7 m	p <sub>p41</sub> = 473.3 kN/m <sup>2</sup>
Active pressure at 9.0 m	p <sub>a31</sub> = 145.4 kN/m <sup>2</sup>		
Active pressure at 20.7 m	p <sub>a41</sub> = 332.0 kN/m <sup>2</sup>		

#### Required pile length

Length req'd to balance mnts H<sub>total</sub> = 20720 mm

**Pass - Provided length of sheet pile greater than minimum required length of pile**

#### Required section modulus

Maximum moment in pile	M <sub>pile</sub> = 3140.5 kNm	Permissible stress of pile	σ <sub>pile</sub> = 270 N/mm <sup>2</sup>
Material factor	γ <sub>ms</sub> = 1.2	Min req'd section modulus / m	Z = 13958 cm <sup>3</sup>

#### Load in tie/strut

Tie/strut load **T = 689.9kN/m**

STRUTS

STRUT CRS  $\approx$  3.0 m

Free strut = 690 x 3 = 2070 kN

ADOPT SNORCO 250T STRUTS

Project **26 Netherhall Gardens**  
 Project No. **8240**

Drg No. - Sheet No.  
 By **Tom Musson** Date **May 2014**

**PLANNING STAGE GROUND MOVEMENT ASSESSMENT IN ACCORDANCE WITH CIRIA C580**

**Design Information**

Wall type: Secant bored piles  
 Pile depth: 10 m  
 Support stiffness: High (temporary props installed before permanent props at high level)  
 Excavation depth: 7 m

**Ground Surface Movements due to Wall Installation**

Horizontal Movement		Vertical Movement	
Surface movement at wall (mm)	Distance behind wall to negligible movement (m)	Surface movement at wall (mm)	Distance behind wall to negligible movement (m)
8	15	5	20

**Ground Surface Movements due to Excavation in front of Wall**

Horizontal Movement		Vertical Movement	
Surface movement at wall (mm)	Distance behind wall to negligible movement (m)	Surface movement at wall (mm)	Distance behind wall to negligible movement (m)
10.5	28	7	24.5

Project 26 Netherhall Gardens  
 Project No. 8240

Drg No. - Sheet No.  
 By TJM Date May 14

## Estimated building damage assessment

### Building Data:

Building No. 28 Netherhall Gardens  
 Building length 16.5 m  
 Building height 11 m  
 Length / height 1.50

Vertical differential settlement 7 mm  
 Horizontal differential settlement 10 mm

Vertical strain 0.042 %  
 Horizontal strain 0.061 %

