

APPENDIX D
Ground and Projects Consultants Limited Report dated September 2016



28 Marefield Gardens, London, NW3 5SX
Basement Impact Assessment Land Stability
September 2016



Client:

**Ground and Water Ltd.,
2 The Long Barn
Norton Farm, Selborne Road
Alton
Hampshire GU34 3NB**

Ground and Project Consultants Ltd

28 Maresfield Gardens, London NW3 5SX: BIA: Land Stability Report

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Non-Technical Summary,

It is proposed to construct a 3.5m deep basement at 28 Maresfield Gardens, London, NW3 5SX. The basement will be built under the full footprint of the existing house.

The screening exercise identified a number of issues for further consideration as follows:

- The property is relatively close to an existing railway tunnel
- London Clay is the shallowest natural geological strata
- There is a tree close to the front of the house and there are bushes in the rear garden
- A 'lost river' runs relatively close to the site
- Groundwater may be encountered during construction works
- The basement will be deeper than neighbouring properties

The published geology suggests London Clay at site with the possibility of Head Deposits, which are softer and weaker.

A ground investigation was carried out by Ground & Water consisting of two boreholes to 5.0m and 10.45m bgl. These encountered a thin cover of Made Ground (i.e. ground placed by human activity) overlying London Clay as a grey silty clay and of high shrinkage potential. Groundwater was not encountered during the investigation.

The scoping and assessment of the BIA concluded that:

- Groundwater inflow, if encountered, should be properly managed and controlled such that there is no significant wash out of fine material.
- The retaining wall should be appropriately designed.
- The construction of the basement is carried out by competent and experienced contractors and precautions are taken to maintain the stability of the excavations.
- Care should be taken to minimise the disturbance and damage to bushes and their roots. Should bushes be removed then an assessment of the potential for swelling of the London Clay soils should be carried out.
- Concrete should be designed accounting for the sulphate conditions anticipated.
- Monitoring of the structures should be carried out before and during construction. The exact nature of this monitoring should be determined by the structural engineer.

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1 Introduction

Ground and Project Consultants Ltd have been instructed by Ground and Water Ltd (G&W) to undertake the land stability element of a Basement Impact Assessment, for 28 Maresfield Gardens, London NW3 5SX. The property is located in the London Borough of Camden, London in the Frognal and Fitzjohns ward, its location is indicated on Figure 1.

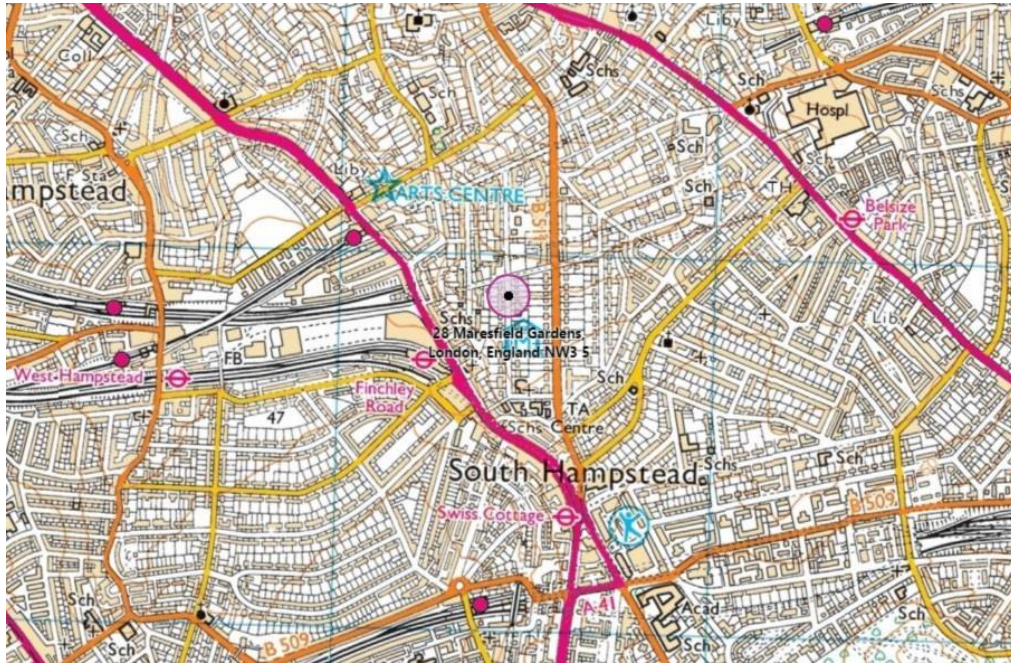


Figure 1: Site Location

Ordnance Survey Data © Crown copyright and database right 2014

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2 Scope and Objective

The scope of this report and approach is as follows:

- A review of the existing data supplied by the client has been carried out, including the proposal drawings produced to date, Ground Investigation data, photos of the building and the background data available through London Borough of Camden's website and other freely available data such as BGS geological information and purchased environmental data.
- In line with the London Borough of Camden guidance, CPG4, latest revision:
- In line with the CPG4 guidance:
 - A detailed assessment of the published and encountered geology
 - Development of a ground model including an assessment of geotechnical properties
 - An engineering interpretation including an assessment of slope stability and commentary and assessment regarding ground movements.
- Recommendations for additional work/ monitoring and observation have been provided.

Assessment of Ground Movements due to the proposed basement construction have not been carried out at this stage.

The report has not considered contaminated land aspects of the site.

This report and the work to support it has been carried out by Jon Smithson who is a Director of Ground and Project Consultants Ltd and is a Chartered Geologist (CGeol) with over 30 years' experience.

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3 BIA Screening for Slope/Land Stability

A screening exercise has been carried out as per the guidance in Camden's Guidance for Basements, CPG4 as follows:

Question	Answer	Action/ Comment
Question 1: Does the existing site include slopes, natural or manmade, greater than 7 degrees? (approximately 1 in 8)	No. The front garden is approximately 1m higher than the rear.	None
Question 2: Will the proposed re-profiling of landscaping at site change slopes at the property boundary to more than 7deg? (approximately 1 in 8)	No. There are no significant changes in surface profile planned.	None
Question 3: Does the development neighbour land, including railway cuttings and the like, with a slope greater than 7deg? (approximately 1 in 8)	No. There are no railway cuttings in the immediate vicinity.	None
Question 4: Is the site within a wider hillside setting in which the general slope is greater than 7degrees? (approximately 1 in 8)	The slope in the area is around 1 in 30 (<2°) based on Ordnance Survey data. The site is relatively close to Hampstead Heath and steeper ground	None
Question 5: Is the London Clay the shallowest strata at the site?	Yes: London Clay is indicated as the shallowest strata on the BGS maps. Head deposits may also be expected	The presence of London Clay close to surface is further discussed in the Impact Assessment
Question 6: Will any tree/s be felled as part of the proposed development and/or are any works proposed within any tree protection zones where trees are to be retained? (Note that consent is required from LB Camden to undertake work to any tree/s protected by a Tree Protection Order or to tree/s in a Conservation Area if the tree is over certain dimensions).	It is understood that there will not be a need to fell trees. Trees (plane) are present close to site on the street in front of the property and in the rear garden.	Further discussed in the Impact Assessment.
Question 7: Is there a history of seasonal shrink-swell subsidence in the local area, and/or evidence of such effects at the site?	None known. However London Clay/Head Deposits is close to surface.	Further discussed in the Impact Assessment
Question 8: Is the site within 100m of a watercourse or a potential spring line?	Figure 11 of the Arup report indicates a 'Lost River' around 200m distance to the west of the property.	None
Question 9: Is the site within an area of previously worked ground?	None known or suspected.	None

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<p>Question 10: Is the site within an aquifer? If so, will the proposed basement extend beneath the water table such that dewatering may be required during construction?</p>	<p>No. The London Clay is classified by the Environment Agency as unproductive strata (rock layers with low permeability and negligible significance for water supply or river base flow). The site is not within a source protection zone of a public water supply. However the basement may extend into the water table.</p>	<p>Groundwater management is discussed in the Impact Assessment.</p>
<p>Question 11: Is the site within 50m of the Hampstead Heath ponds?</p>	<p>No</p>	<p>None</p>
<p>Question 12: Is the site within 5m of a highway or pedestrian right of way?</p>	<p>No</p>	<p>None</p>
<p>Question 13: Will the proposed basement significantly increase the differential depth of foundations relative to neighbouring properties?</p>	<p>It is understood that no basements are present in neighbouring properties, although they do appear to have lower ground floors.</p>	<p>This is further discussed in the Impact Assessment.</p>
<p>Question 14: Is the site over (or within the exclusion zone of) any tunnels, e.g. railway lines?</p>	<p>The Main Line Railway is understood about 20m to the north of the site beneath Nutley Terrace.</p>	<p>This is further discussed in the Impact Assessment.</p>

4 Site Information

Existing Property and Basement Proposals

The property at 28 Maresfield Gardens is located on the east side of the road, about 20m from its junction with Nutley Terrace. The property is a brick built 5 storey terraced property (including lower ground floor and attic rooms, probably constructed around 1890.

The property is around 1.8km North Northwest of Regents Park and around 1.5km south west of Hampstead Heath. The Main Railway line passes in tunnel around 20m north of the site, beneath Nutley Terrace.

There are plane trees on the road side and smaller trees and bushes in the rear and adjacent rear gardens.

The basement proposals comprise a single storey beneath the footprint of the rear of the property and below part of the back garden. The basement depth will be around 4.7m. The basement footprint will be approximately 92m². The descriptions and dimensions above have been estimated from drawings provided by G&W.

The National Grid reference for the property is TQ 25228 84702. The location of the property is provided in Figure 1 above.

Topography

The OS map indicates the property is around 72m AOD. The ground surface rises generally towards the North at around 1 in 30 (approx. 1.5°). The slope becomes steeper to the north as the land rises towards Hampstead Heath. There is no significant change in elevation at the property although the front garden level is around 1m higher than the back. .

Geology

The available geological mapping (Ref 1.) indicates that the site lies on London Clay which typically comprises a stiff grey fissured clay, weathering to brown near surface. Concretions of argillaceous limestone in nodular form (Claystones) occur throughout the formation. The geological map (North London 256) indicates that the property is within an area of 'propensity' for Head Deposits (stippled on the map), associated with the higher ground of Hampstead Heath. Typically these deposits are thin (<2m) and consist of soft, ochreous brown silty clay with blue-grey mottling in places and angular, frost-shattered fragments of flint occur sporadically throughout. The base of the London Clay is likely to occur significant depth below the property. The Claygate Member (darker brown on map), which immediately overlies the London Clay, and overlying Bagshot Formation (yellow on map) are indicated to the north. The boundary of the London Clay and Claygate Member is around 100-150m to the north. See figure 2 below.

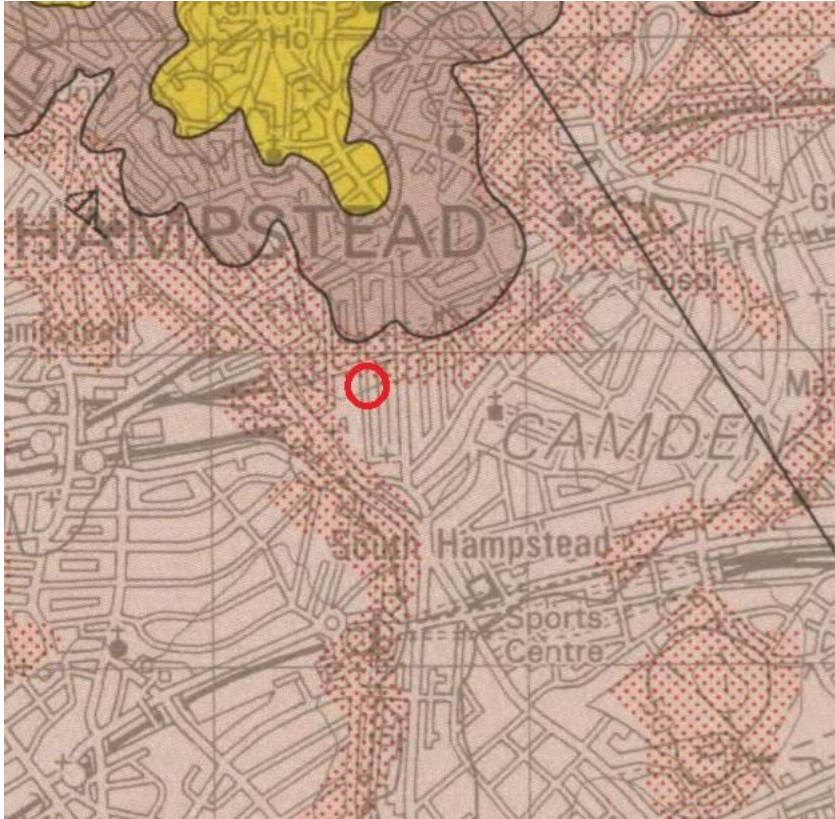


Figure 2: Geology

BGS copyright and database right 2015

Hydrology and Hydrogeology

The OS Map indicates that there are no surface water bodies in the vicinity of the site. The Hampstead Ponds are approximately 1.5km to the NE. There are no springs shown on OS mapping. There is a 'lost river' (a tributary of the Tyburn) indicated approximately 50-100m to the east.

The London Clay is classified by the Environment Agency as unproductive strata (rock layers with low permeability and negligible significance for water supply or river base flow). The site is not within a source protection zone of a public water supply. There are no groundwater abstraction licenses within 2 km of the site and no source protection zones within 500 m of the site. (Ref 5. Groundsure Report). Some springs and/or groundwater flow may be expected at or close to the boundary of the Claygate Member and the London Clay.

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Other Environmental Data

A Groundsure report for a nearby property gives a wealth of background data on local environmental issues and hazards. (See Appendix A). The key issues are summarised in the table below:

Drift Deposits	None are indicated on BGS mapping
Made Ground	None are indicated on BGS mapping
Groundwater Abstraction	7 No. groundwater abstractions are recorded between 500m and 2000m of the site.
Shrink/ Swell	There is a moderate Hazard of shrink and swell from the Claygate Member/London Clay soils
Landslide	Very Low Risk: Slope instability problems are unlikely to be present. No special actions required to avoid problems due to landslides. No special ground investigation required, and increased construction costs or increased financial risks are unlikely due to potential problems with landslides.
Soluble Rocks	Negligible Risk
Compressible Ground	Negligible Risk
Collapsible Ground	Very Low Risk
Running Sand	Very Low Risk
Mining	None recorded

5 Ground Investigation

A ground investigation (GI) has been carried out at the site by Ground and Water Ltd and results of these have been made available by G&W. The GI was carried out in August 2016.

The work comprised two boreholes, BH1 and WS2, drilled to 10.45m bgl and 5.0m bgl respectively. BH1 was drilled using a windowless sampler close to the southeast corner of the property. WS12 drilled using a handheld window sampler was located in the rear garden to the east of the house. Two hand dug trial pits were also excavated to expose existing foundations. A standpipe piezometer was installed in BH1 to allow longer term monitoring of groundwater.

Below is a summary derived from the Ground Investigation report. The boreholes encountered a cover of Made Ground 0.60 to 0.90m thick. This can be summarised as a dark brown slightly gravelly silty clay with brick, flint cement and carbonaceous fragments. Below this the boreholes encountered a clay deposit described as brown, becoming dark brown and grey with depth, silty clay with occasional silt pockets and rare fine selenite crystals throughout. The clay is likely to be London Clay.

Groundwater was not encountered during drilling, but was encountered in the standpipe piezometer at 2.70m bgl on 21 September 2016. Rootlets were noted above 1.5m and 1.0m depth respectively and decaying roots were noted at 3.50m bgl in both boreholes, presumably related to past trees.

TP1 was excavated to the rear and TP2 to the rear right in a corner. TP1 encountered made ground to 0.3m bgl overlying London Clay. Adjacent to this the pit revealed a stepped brick wall and underlying concrete foundation with a founding depth of 0.8mbgl (i.e. on London Clay). TP2 encountered a Made Ground to 0.80m bgl overlying London Clay. Again the wall overlies a stepped brick foundation with the concrete foundation base being at 1.20m below ground level, again onto London Clay.

Standard Penetration Tests (SPTs) were carried out in BH1. These gave results (N values) of between 4 and 8 from 1.0m to 3.0m bgl and from 14 increasing steadily to 32 from 4.0m bgl to 10.0m bgl. These indicate a soft to firm clay at the shallower depth range, and a firm to stiff becoming stiff clay at depth.

Laboratory tests were carried out on the samples collected from the boreholes. Testing consisted of the following:

- 4 No. Atterberg Limit test including moisture content determination
- 8 Moisture content determination
- 2 No. Soluble Sulphate, pH and related tests for Concrete Classification on soil samples

All of the Atterberg tests were conducted in the London Clay. These show general consistency with a slight reduction in water content with depth.

- Moisture content: 21 to 38%, generally in the range 31 to 38%
- Plastic Limit: 26 to 30%
- Liquid Limit: 75 to 86%
- Plasticity Index: 49 to 56%
- Liquidity Index 0.08 to 0.14

The low variation in liquid, plastic limits and plasticity index is consistent with undisturbed London Clay. There is limited correlation to the dynamic probe tests. The low liquidity index values are

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suggestive of a firm and firm to stiff consistency. The London Clay here is of medium to high plasticity and has high volume change potential.

6 Conceptual Ground Model

From the above a conceptual Ground model has been developed and is presented in tabular form below:

Strata	Typical Description	Depth at Property encountered in GI	Geotechnical Properties – Tentative Characteristic Values*	Other
Made Ground	Dark brown, very silty clay, with grave	Ground level to between 0.30 and 0.90m, deeper in foundation areas	N/A	Made Ground should not be relied upon as a bearing strata. Made Ground is likely to be encountered to greater depths at the front of the property.
London Clay	Brown and grey silty clay. Probably firm to stiff becoming stiff with depth.	From between 0.30 and 0.90m (base not proven).	$C' = 0$ $\phi' = 20^\circ$ $C_u = 20$ increasing to 70kN/m^2 at formation and to 160kN/m^2 at 10m. **	The undrained shear strength of the London Clay should be confirmed prior to construction
Groundwater	2.70m bgl	Dipped in sandpipe at 21/9/16. Not encountered during drilling		May significantly vary seasonally or after prolonged wet or dry periods.

Table 3: Summary of Strata Characteristics

*The determination of parameters is tentative due to the lack of test data.

**Strength should be verified by hand held shear vane/ inspection during ground excavation.

7 Impact Assessment

There are no apparent major issues which should seriously affect the viability of the construction of the new basement. However the assessment of the geological environment of 28 Maresfield Gardens and the screening exercise indicate some areas for further discussion in this report with suggested mitigation where appropriate.

Nearby Railway Tunnel

The mainline is understood to be located in tunnel some 20m to the south of the property. The exact location, depth and details of the tunnel are not known and should be ascertained prior to construction. The tunnel appears to pass beneath Nutley Terrace. Network Rail will need to be consulted regarding the basement construction if appropriate. Network Rail make various stipulations in terms of activities and approvals and impose liabilities on the 'proposer'. These must be understood and adhered to as appropriate. It is understood that piling is not required or planned for the basement construction.

London Clay/Shrink and Swell:

The basement will be founded in London Clay. These soils at this site are of high plasticity and high volume change potential. The basement will be founded at around 3.5m bgl, therefore below any seasonal shrink and swell. The London Clay soils are known for their high levels of soluble sulphate. The concrete mix design should take appropriate account of sulphate levels in accordance with BRE Special Digest 1. The basement structure should be designed to account for swelling pressures. It will be important to account for the shallow nature of the existing foundations at the property and its neighbours. Any change in drainage or significant interruption/change to groundwater levels and flow patterns will need to be assessed for its implication on soil water content and consequential effect on soil volume change.

Trees and Bushes

No trees are located in the garden although there are some bushes and small trees in the rear garden and a plane tree outside on the pavement to the front. Roots have been noted in the ground investigation. Care should be taken to minimise root damage during construction works. Should bushes be removed there is potential for the soils to swell as a result which may affect this and neighbouring properties and this should be accounted for in design and further assessed as appropriate.

Groundwater/Aquifer:

Groundwater was not encountered during the drilling but was found in the standpipe piezometer on 21 September at 2.7m bgl. It is recommended that a design level of ground surface is used, this accounts for seasonal variations and leaks from water supply, etc. Groundwater may be encountered during the works, particularly as seepages through sandy silty layers within London Clay or at the base of the Made Ground. These should be managed carefully to prevent ground loss particularly through loss of fines. Softening of

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formation due to water ingress is a risk and softened soils should be excavated and replaced where practicable. Consideration should be given to limiting the size and time of face exposures during construction should significant flows be encountered during construction. Baseline and ongoing regular monitoring of the building and its immediate neighbours for settlement and movement/distress is highly recommended during building works and for a short period after completion. It is recommended that ongoing monitoring of groundwater levels is carried out during and up to the end of construction of the basement structure.

Basement Depth:

It is proposed to be construct the basement to a level of approximately 4.7m below the existing ground floor. The property adjoins neighbouring houses either side. The proposals to construct the basement is to be via underpinning at the party and rear and front walls. Underpinning proposals are understood to involve a 'hit and miss' approach in stages so each 'panel' is separated by 3-5 others from the next open one. It will be important that the building contractor is closely supervised and is experienced in this type of construction. It will be critical to prevent exposed faces from collapse or significant ground loss into the new excavation and temporary face support should be maintained where practicable. It is understood the there are no deep basements in adjoin/adjacent properties. Most ground movement should occur during wall installation, excavation of the basement and construction so the adequacy of temporary support will be critical in limiting ground movements. Heave movements will occur due to removal of soils. The presence of softer weathered London Clay may lead to slightly larger ground movements.

A number of factors will assist in limiting ground movements:

- The speed of propping and support
- Good workmanship
- Ensuring that adequate propping is in place at all times during construction
- Installation of the first (stiff) support quickly and early in the construction sequence.
- Avoidance of ground loss through the gaps between the piles.
- Avoid leaving ground unsupported.
- Minimise deterioration of the central soil mass by the use of blinding/covering with a waterproof membrane.
- Avoid overbreak
- Control dewatering to minimise fines removal and drawdown.

8 Conclusions

The methodology and approach of CPG4 has been followed in developing this BIA with respect to Land stability. It is concluded that with the construction of the new basement at 28 Maresfield Gardens should not have significant impacts on land stability provided that:

- Groundwater inflow, if encountered, is reduced to a minimum and properly controlled such that there is no significant wash out of fine material. Groundwater levels should be monitored before and during construction.
- The presence and location of the railway beneath Nutley Terrace should be further assessed and where appropriate action and consultation undertaken.
- The retaining wall should be appropriately designed.
- The construction of the basement is carried out by competent and experienced contractors and precautions are taken to maintain the stability of the excavations.
- Care should be taken to minimise the disturbance and damage to bushes and their roots. Should bushes be removed then an assessment of the potential for swelling of the London Clay soils should be carried out.
- Concrete should be designed in accordance with BRE Special Digest 1 accounting for the sulphate conditions anticipated.
- Monitoring of the structures is carried out before and during construction. The exact nature of this monitoring should be determined by the structural engineer.

9 References

1. BGS Geological Map Sheet 256.
2. Ordnance Survey Map, Explorer 173, London North
3. Arup: Camden Geological, Hydrogeological and Hydrological Study.
4. Design Drawings supplied by G&W
5. G&W Ground Investigation Data available at 15/9/16

APPENDIX E
Fieldwork Logs

Project Name
28 Marefield Gardens

Project No.
GWPR1761

Co-ords: -

Hole Type
WLS

Location: South Hampstead, London NW3 5SX

Level: -

Scale
1:50

Client: Mr and Mrs Freedman

Dates: 05/08/2016

Logged By
RF

Well	Water Strikes	Samples & In Situ Testing			Depth (m)	Level (m AOD)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.07		MADE GROUND: Tarmac	
					0.20			MADE GROUND: Brick and sand sub-base.
			0.50	D				MADE GROUND: Dark brown slightly gravelly silty clay. Gravel is occasional, fine to medium, sub-angular to sub-rounded flint and gravel and rare fine sub-angular to sub-rounded carbonaceous material (ash/clinker). LONDON CLAY FORMATION: Brown silty CLAY with rare fine selenite crystals.
			0.80	D				
			1.00	SPT	N=4			
			1.00	D	(1,1/ 1,1,1,1)			
			1.80	D				
			2.00	SPT	N=8			
			2.00	D	(1,1/ 1,2,2,3)			
			2.50	D				
			3.00	SPT	N=7			
			3.00	D	(2,3/ 2,1,2,2)			
			3.50	D				
			4.00	SPT	N=14			
			4.00	D	(2,2/ 3,3,4,4)			
			4.50	D				
			5.00	SPT	N=15			
			5.00	D	(3,4/ 4,4,3,4)			
			5.50	D				
			6.00	SPT	N=17			
		6.00	D	(4,4/ 3,4,5,5)				
		6.50	D					
		7.00	SPT	N=21				
		7.00	D	(4,5/ 5,5,6,5)				
		7.50	D					
		8.00	SPT	N=21				
		8.00	D	(5,5/ 5,6,5,5)				
		8.50	D					
		9.00	SPT	N=25				
		9.00	D	(5,5/ 6,6,6,7)				
		9.50	D					

Continued next sheet

Remarks: No groundwater encountered.
Roots noted to 1.50m bgl.
Decaying roots noted at 3.50m bgl.



Project Name
28 Marefield Gardens

Project No.
GWPR1761

Co-ords: -

Hole Type
WS

Location: South Hampstead, London NW3 5SX

Level: -

Scale
1:50

Client: Mr and Mrs Freedman

Dates: 05/08/2016

Logged By
RF

Well	Water Strikes	Samples & In Situ Testing			Depth (m)	Level (m AOD)	Legend	Stratum Description	
		Depth (m)	Type	Results					
		0.20	D		0.90		MADE GROUND: Dark brown slightly gravelly silty clay. Gravel is occasional, fine to medium, sub-angular to sub-rounded brick and rare to occasional fine sub-angular to sub-rounded carbonaceous material (ash/clinker).		
		0.50	D						
		0.80	D						
		1.00	D						
		1.50	D		1.90		LONDON CLAY FORMATION: Brown silty CLAY with very rare fine selenite crystals.	1	
		2.00	D						
		2.50	D		2.20		LONDON CLAY FORMATION: Brown silty CLAY with fine selenite crystals and sub-rounded flint gravel	2	
		3.00	D						
		3.50	D		5.00		LONDON CLAY FORMATION: Brown silty CLAY with veins of grey silt, pockets of orange silt and rare fine selenite crystals.	3	
		4.00	D						
		4.50	D						
		5.00	D						
End of Borehole at 5.00 m								5	
								6	
								7	
								8	
								9	

Remarks: No groundwater encountered.
Fine roots to 1.00m bgl.
Decayed root noted at 3.50m bgl.



APPENDIX F
Geotechnical Laboratory Test Results



Summary of Natural Moisture Content, Liquid Limit and Plastic Limit Results

Job No. 21471	Project Name 28 Maresfield Gardens, South Hampstead, London NW3	Programme	
		Samples received	24/08/2016
Project No. GWPR1761	Client Ground and Water Ltd	Schedule received	23/08/2016
		Project started	24/08/2016
		Testing Started	09/09/2016

Hole No.	Sample				Soil Description	NMC %	Passing 425µm %	LL %	PL %	PI %	Remarks
	Ref	Top	Base	Type							
BH1	S	1.00	-	D	Brown and occasional orangish brown silty CLAY with rare medium angular gravel	34					
BH1	S	1.50	-	D	Brown silty CLAY with rare roots	33	100	75	26	49	
BH1	S	2.00	-	D	Brown and rare grey silty CLAY with rare siltstone nodules and medium sub-rounded gravel	30					
BH1	S	2.50	-	D	Brown slightly mottled grey silty CLAY with occasional orange fine sand pockets	38					
BH1	S	3.00	-	D	Brown and occasional grey silty CLAY with rare fine selenite crystals	38	100	86	30	56	
BH1	S	3.50	-	D	Brown slightly mottled grey silty CLAY with rare carbonaceous deposits and occasional selenite crystals	36					
WS2	S	1.00	-	D	Brown and rare orangish brown silty CLAY with traces of carbonaceous deposits and rare fmc sub-angular gravel	33	99	79	26	53	
WS2	S	1.50	-	D	Brown silty CLAY	31					
WS2	S	2.00	-	D	Brown gravelly silty CLAY (gravel is fmc and sub-angular to sub-rounded)	21					
WS2	S	2.50	-	D	Brown and rare orangish brown silty CLAY with rare fine mudstone fragments and rare fine gravel	32	99	80	28	52	
WS2	S	3.00	-	D	Brown slightly mottled grey silty CLAY with rare orange fine sand pockets	36					
WS2	S	3.50	-	D	Brown slightly mottled grey silty CLAY	34					

Test Methods: BS1377: Part 2: 1990: Natural Moisture Content : clause 3.2 Atterberg Limits: clause 4.3 and 5.0	Test Report by K4 SOILS LABORATORY Unit 8 Olds Close Olds Approach Watford Herts WD18 9RU Tel: 01923 711 288 Email: James@k4soils.com	Checked and Approved Initials J.P Date: 13/09/2016
2519	Approved Signatories: K.Phaure (Tech.Mgr) J.Phaure (Lab.Mgr)	MSF-5-R1(b)



Francis Williams
Ground & Water Ltd
2 The Long Barn
Norton Farm
Selborne Road
Alton
Hampshire
GU34 3NB



QTS Environmental Ltd
Unit 1
Rose Lane Industrial Estate
Rose Lane
Lenham Heath
Kent
ME17 2JN
t: 01622 850410
russell.jarvis@qtsenvironmental.com

QTS Environmental Report No: 16-48409

Site Reference: 28 Maresfield Gardens, South Hampstead, London NW3

Project / Job Ref: GWPR1761

Order No: None Supplied

Sample Receipt Date: 24/08/2016

Sample Scheduled Date: 24/08/2016

Report Issue Number: 1

Reporting Date: 31/08/2016

Authorised by:

A handwritten signature in black ink, appearing to read 'Kevin Old'.

Kevin Old
Associate Director of Laboratory

Authorised by:

A handwritten signature in black ink, appearing to read 'Russell Jarvis'.

Russell Jarvis
Associate Director of Client Services



QTS Environmental Ltd
 Unit 1, Rose Lane Industrial Estate
 Rose Lane
 Lenham Heath
 Maidstone
 Kent ME17 2JN
 Tel : 01622 850410



Soil Analysis Certificate					
QTS Environmental Report No: 16-48409	Date Sampled	05/08/16	05/08/16		
Ground & Water Ltd	Time Sampled	None Supplied	None Supplied		
Site Reference: 28 Maresfield Gardens, South Hampstead, London NW3	TP / BH No	BH1	WS2		
Project / Job Ref: GWPR1761	Additional Refs	None Supplied	None Supplied		
Order No: None Supplied	Depth (m)	2.50	1.50		
Reporting Date: 31/08/2016	QTSE Sample No	224463	224464		

Determinand	Unit	RL	Accreditation				
pH	pH Units	N/a	MCERTS	7.1	7.2		
Total Sulphate as SO ₄	mg/kg	< 200	NONE	1449	359		
Total Sulphate as SO ₄	%	< 0.02	NONE	0.14	0.04		
W/S Sulphate as SO ₄ (2:1)	mg/l	< 10	MCERTS	412	411		
W/S Sulphate as SO ₄ (2:1)	g/l	< 0.01	MCERTS	0.41	0.41		
Total Sulphur	%	< 0.02	NONE	0.05	< 0.02		
Ammonium as NH ₄	mg/kg	< 0.5	NONE	4.9	5.5		
Ammonium as NH ₄	mg/l	< 0.05	NONE	0.49	0.55		
W/S Chloride (2:1)	mg/kg	< 1	MCERTS	15	16		
W/S Chloride (2:1)	mg/l	< 0.5	MCERTS	7.5	7.9		
Water Soluble Nitrate (2:1) as NO ₃	mg/kg	< 3	MCERTS	< 3	< 3		
Water Soluble Nitrate (2:1) as NO ₃	mg/l	< 1.5	MCERTS	< 1.5	< 1.5		
W/S Magnesium	mg/l	< 0.1	NONE	16	2.9		

Analytical results are expressed on a dry weight basis where samples are assisted-dried at less than 30°C
 Analysis carried out on the dried sample is corrected for the stone content
 Subcontracted analysis ^(S)



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 Lenham Heath
 Maidstone
 Kent ME17 2JN
 Tel : 01622 850410



Soil Analysis Certificate - Sample Descriptions	
QTS Environmental Report No: 16-48409	
Ground & Water Ltd	
Site Reference: 28 Maresfield Gardens, South Hampstead, London NW3	
Project / Job Ref: GWPR1761	
Order No: None Supplied	
Reporting Date: 31/08/2016	

QTSE Sample No	TP / BH No	Additional Refs	Depth (m)	Moisture Content (%)	Sample Matrix Description
\$ 224463	BH1	None Supplied	2.50	23.2	Light brown clay
\$ 224464	WS2	None Supplied	1.50	19.4	Light brown clay

Moisture content is part of procedure E003 & is not an accredited test

Insufficient Sample ^{I/S}

Unsuitable Sample ^{U/S}

\$ samples exceeded recommended holding times

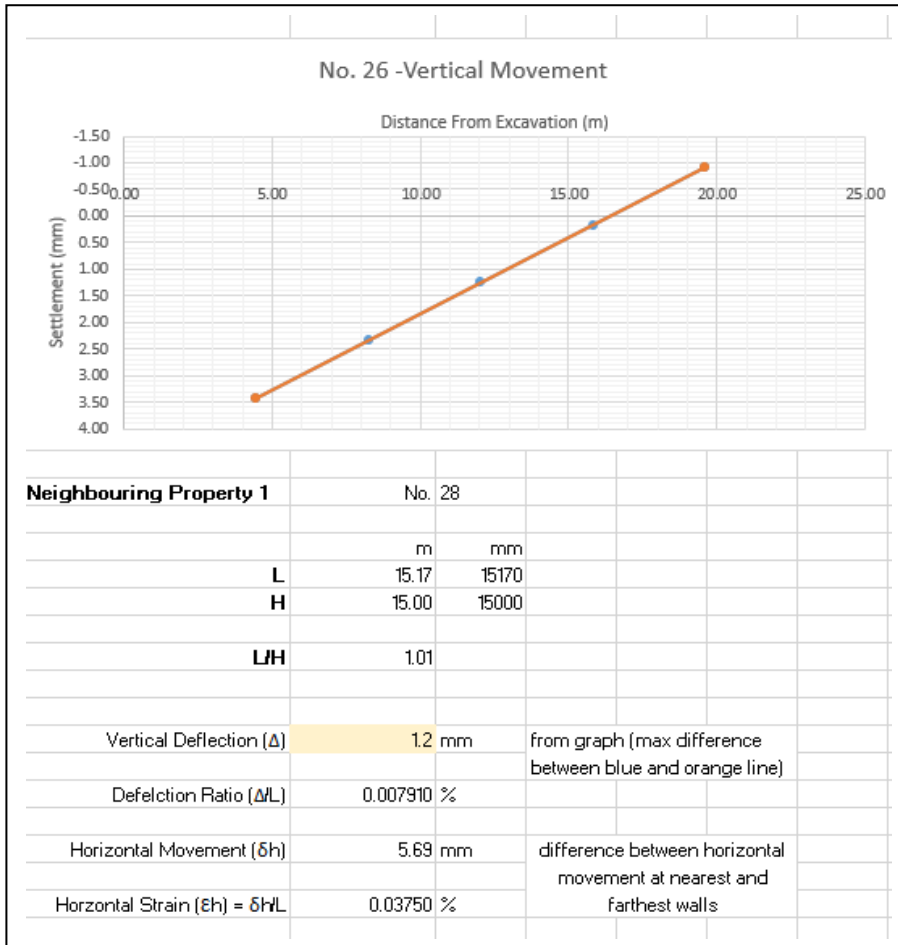
Soil Analysis Certificate - Methodology & Miscellaneous Information
QTS Environmental Report No: 16-48409
Ground & Water Ltd
Site Reference: 28 Maresfield Gardens, South Hampstead, London NW3
Project / Job Ref: GWPR1761
Order No: None Supplied
Reporting Date: 31/08/2016

Matrix	Analysed On	Determinand	Brief Method Description	Method No
Soil	D	Boron - Water Soluble	Determination of water soluble boron in soil by 2:1 hot water extract followed by ICP-OES	E012
Soil	AR	BTEX	Determination of BTEX by headspace GC-MS	E001
Soil	D	Cations	Determination of cations in soil by aqua-regia digestion followed by ICP-OES	E002
Soil	D	Chloride - Water Soluble (2:1)	Determination of chloride by extraction with water & analysed by ion chromatography	E009
Soil	AR	Chromium - Hexavalent	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry	E016
Soil	AR	Cyanide - Complex	Determination of complex cyanide by distillation followed by colorimetry	E015
Soil	AR	Cyanide - Free	Determination of free cyanide by distillation followed by colorimetry	E015
Soil	AR	Cyanide - Total	Determination of total cyanide by distillation followed by colorimetry	E015
Soil	D	Cyclohexane Extractable Matter (CEM)	Gravimetrically determined through extraction with cyclohexane	E011
Soil	AR	Diesel Range Organics (C10 - C24)	Determination of hexane/acetone extractable hydrocarbons by GC-FID	E004
Soil	AR	Electrical Conductivity	Determination of electrical conductivity by addition of saturated calcium sulphate followed by electrometric measurement	E022
Soil	AR	Electrical Conductivity	Determination of electrical conductivity by addition of water followed by electrometric measurement	E023
Soil	D	Elemental Sulphur	Determination of elemental sulphur by solvent extraction followed by GC-MS	E020
Soil	AR	EPH (C10 - C40)	Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soil	AR	EPH Product ID	Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soil	AR	EPH TEXAS (C6-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C40)	Determination of acetone/hexane extractable hydrocarbons by GC-FID for C8 to C40. C6 to C8 by headspace GC-MS	E004
Soil	D	Fluoride - Water Soluble	Determination of Fluoride by extraction with water & analysed by ion chromatography	E009
Soil	D	FOC (Fraction Organic Carbon)	Determination of fraction of organic carbon by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	D	Loss on Ignition @ 450oC	Determination of loss on ignition in soil by gravimetrically with the sample being ignited in a muffle furnace	E019
Soil	D	Magnesium - Water Soluble	Determination of water soluble magnesium by extraction with water followed by ICP-OES	E025
Soil	D	Metals	Determination of metals by aqua-regia digestion followed by ICP-OES	E002
Soil	AR	Mineral Oil (C10 - C40)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge	E004
Soil	AR	Moisture Content	Moisture content: determined gravimetrically	E003
Soil	D	Nitrate - Water Soluble (2:1)	Determination of nitrate by extraction with water & analysed by ion chromatography	E009
Soil	D	Organic Matter	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	AR	PAH - Speciated (EPA 16)	Determination of PAH compounds by extraction in acetone and hexane followed by GC-MS with the use of surrogate and internal standards	E005
Soil	AR	PCB - 7 Congeners	Determination of PCB by extraction with acetone and hexane followed by GC-MS	E008
Soil	D	Petroleum Ether Extract (PEE)	Gravimetrically determined through extraction with petroleum ether	E011
Soil	AR	pH	Determination of pH by addition of water followed by electrometric measurement	E007
Soil	AR	Phenols - Total (monohydric)	Determination of phenols by distillation followed by colorimetry	E021
Soil	D	Phosphate - Water Soluble (2:1)	Determination of phosphate by extraction with water & analysed by ion chromatography	E009
Soil	D	Sulphate (as SO4) - Total	Determination of total sulphate by extraction with 10% HCl followed by ICP-OES	E013
Soil	D	Sulphate (as SO4) - Water Soluble (2:1)	Determination of sulphate by extraction with water & analysed by ion chromatography	E009
Soil	D	Sulphate (as SO4) - Water Soluble (2:1)	Determination of water soluble sulphate by extraction with water followed by ICP-OES	E014
Soil	AR	Sulphide	Determination of sulphide by distillation followed by colorimetry	E018
Soil	D	Sulphur - Total	Determination of total sulphur by extraction with aqua-regia followed by ICP-OES	E024
Soil	AR	SVOC	Determination of semi-volatile organic compounds by extraction in acetone and hexane followed by GC-MS	E006
Soil	AR	Thiocyanate (as SCN)	Determination of thiocyanate by extraction in caustic soda followed by acidification followed by addition of ferric nitrate followed by colorimetry	E017
Soil	D	Toluene Extractable Matter (TEM)	Gravimetrically determined through extraction with toluene	E011
Soil	D	Total Organic Carbon (TOC)	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	AR	TPH CWG (ali: C5- C6, C6-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C34, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C35)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C35. C5 to C8 by headspace GC-MS	E004
Soil	AR	TPH LQM (ali: C5-C6, C6-C8, C8-C10, C10-C12, C12-C16, C16-C35, C35-C44, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C35, C35-C44)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C44. C5 to C8 by headspace GC-MS	E004
Soil	AR	VOCS	Determination of volatile organic compounds by headspace GC-MS	E001
Soil	AR	VPH (C6-C8 & C8-C10)	Determination of hydrocarbons C6-C8 by headspace GC-MS & C8-C10 by GC-FID	E001

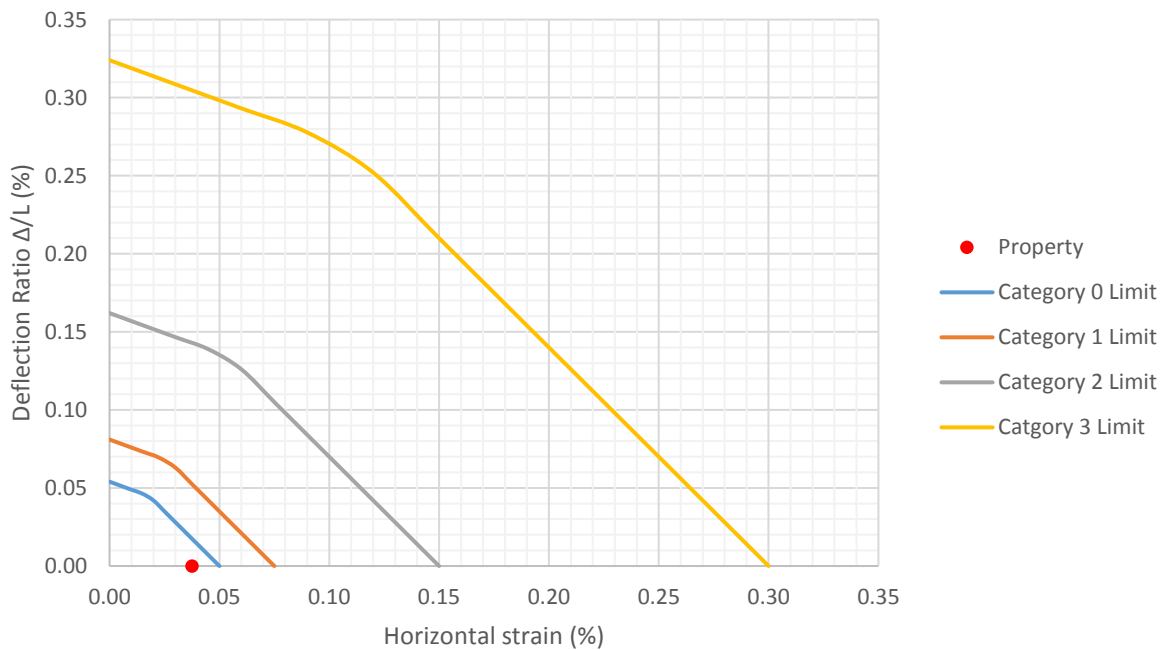
D Dried
 AR As Received

APPENDIX G
Ground Movement Assessment Calculations

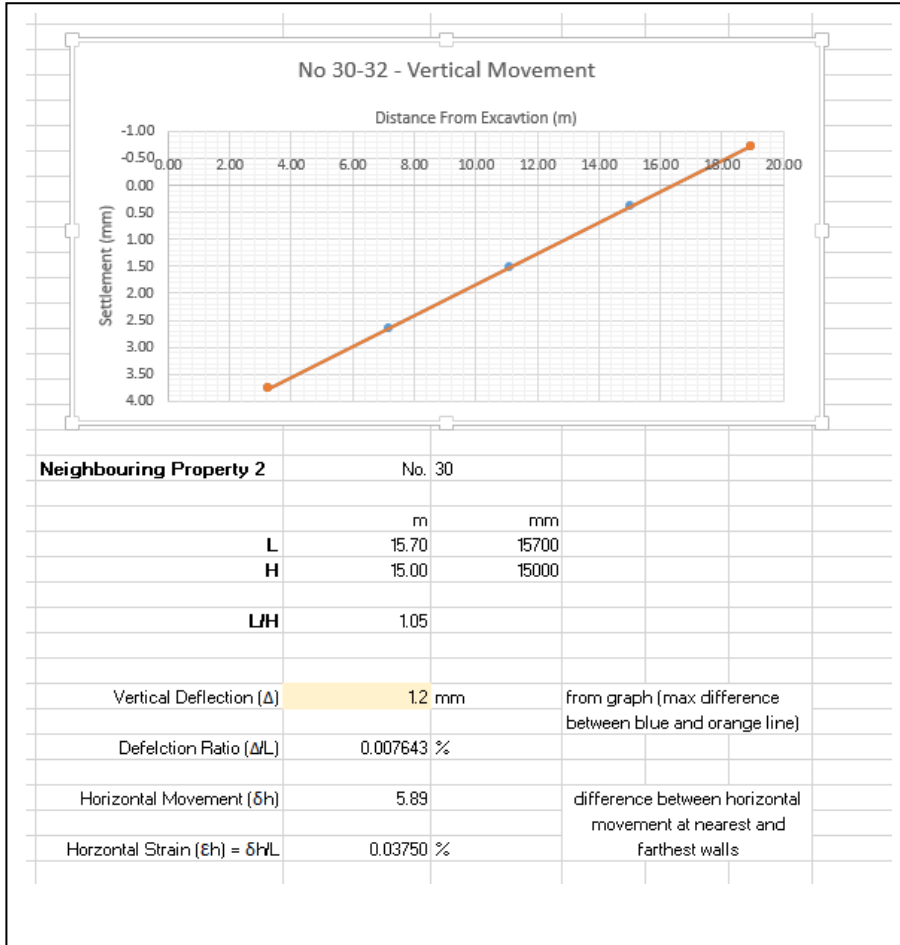
No.26 Maresfield Gardens – Potential Damage Calculations in Stiff Clay



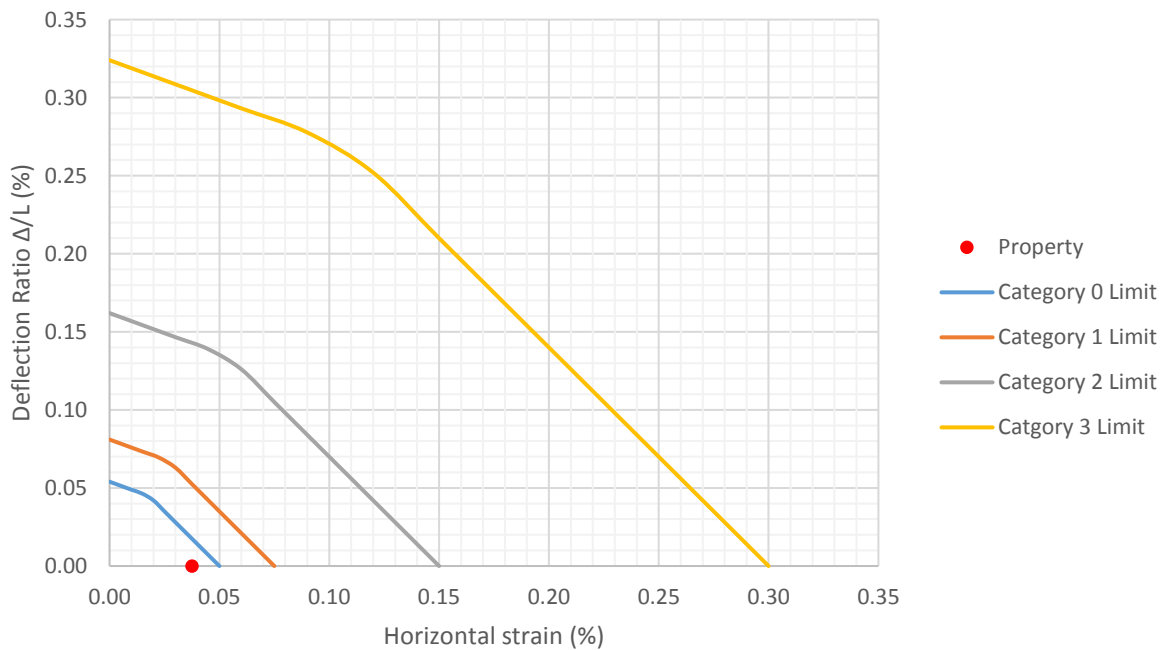
Relationship between damage category and deflection ratio and horizontal strain for L/H = 1.0



No.30-32 Maresfield Gardens – Potential Damage Calculations in Stiff Clay



Relationship between damage category and deflection ratio and horizontal strain for L/H = 1.0



Appendix H
Network Rail Enquiry



Francis Williams <francis.williams@groundandwater.co.uk>

Fwd: WB20941-EN13771 - London 28 Maresfield Gardens NW3 5SX - Excavation of basement near Belsize Tunnel

Trevor Vincent <trevor-vincent@sky.com>
To: Francis Williams <francis.williams@groundandwater.co.uk>

30 January 2017 at 16:18

Regards,

Trevor Vincent

Begin forwarded message:

From: Ako Jesse <Jesse.Ako@networkrail.co.uk>
Date: 27 January 2017 at 15:36:28 GMT
To: "trevor-vincent@sky.com" <trevor-vincent@sky.com>
Cc: Mohammad Jakeer <Jakeer.Mohammad@networkrail.co.uk>, Keegan Benedicta <Benedicta.Keegan@networkrail.co.uk>
Subject: FW: WB20941-EN13771 - London 28 Maresfield Gardens NW3 5SX - Excavation of basement near Belsize Tunnel

Trevor,

On review of the further detail given by yourself below, the structures team have no further objections to your proposals. I have attached a long section of the tunnel for your reference which you can use if needed, however please note that it is an approximate plan based upon archive information and may not be 100% accurate

Kind Regards

Jesse Ako | Project Management Assistant | Asset Protection Team | LNE - EM Route | Network Rail

Desk 4 – 2nd Floor | Kings Cross Station | West Side Offices | Euston Road | London | N1C 4AP

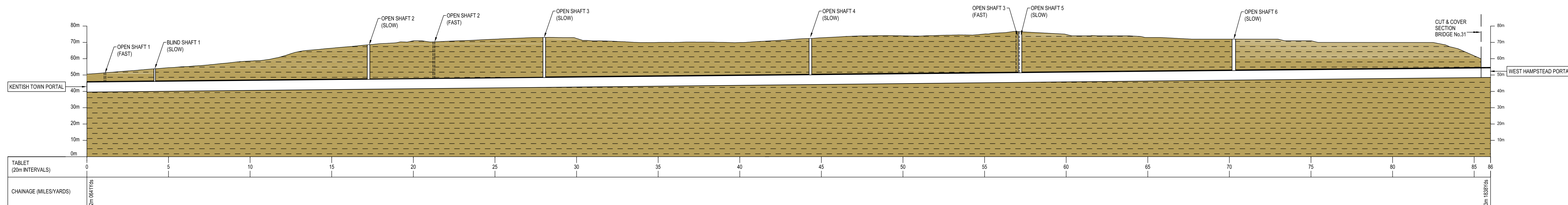
☎ 0207 922 9065 | 002 9065

✉ jesse.ako@networkrail.co.uk | www.networkrail.co.uk

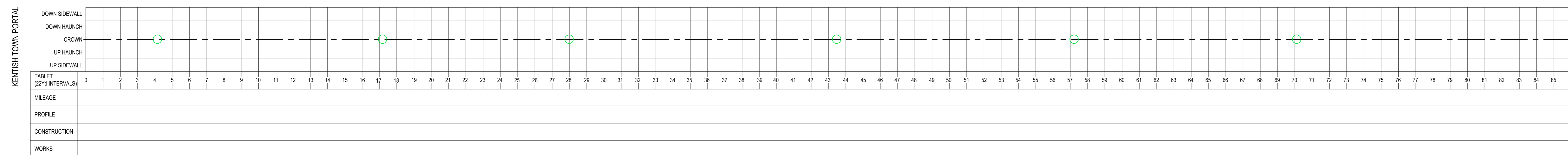
From: TREVOR VINCENT [<mailto:trevor-vincent@sky.com>]
Sent: 13 January 2017 16:43
To: Asset Protection LNE EM; Foster Barry (Structures)
Cc: Mohammad Jakeer; Keegan Benedicta
Subject: Re: WB20941-EN13771 - London 28 Maresfield Gardens NW3 5SX - Excavation of basement near Belsize Tunnel



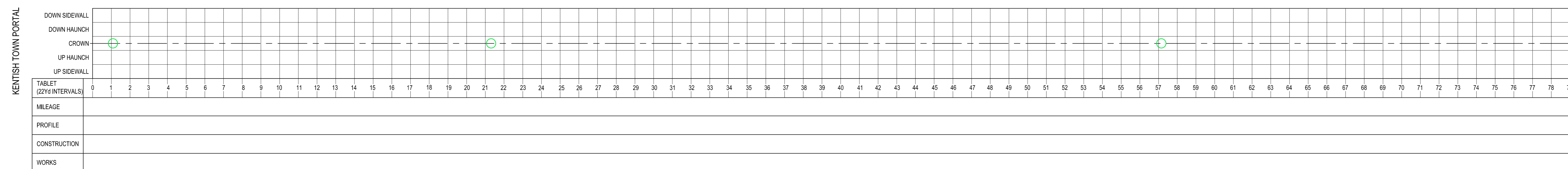
PLAN
SCALE: 1:2000



GEOLOGICAL SECTION
SCALE: HORIZ 1:2000 VERT 1:1000



SLOW (NEW) TUNNEL PLAN
SCALE: HORIZ 1:2000



FAST TUNNEL PLAN
SCALE: HORIZ 1:2000

- NOTES:**
- ALL MEASUREMENTS ARE IN MILES AND YARDS UNLESS OTHERWISE STATED
 - ALL LEVELS ARE IN METRES AND RELATE TO ORDINANCE DATUM UNLESS STATED OTHERWISE
 - ALL LEVELS, TOGETHER WITH THE GROUND PROFILE SHOWN ON THE LONGITUDINAL SECTION ARE APPROXIMATE AND ARE INTENDED FOR ILLUSTRATIVE PURPOSES ONLY
 - TABLETS ARE AT 22yd INTERVALS IN FAST TUNNEL & 20m IN SLOW TUNNEL. TABLET 0 AT KENTISH TOWN PORTAL
 - ZONE OF INFLUENCE = 22m EITHER SIDE OF TUNNEL CENTRELINE
 - THIS DRAWING SHOULD BE READ IN CONJUNCTION WITH THE FOLLOWING DOCUMENTS:
DESK STUDY REPORT J1377/DESK STUDY/BELSIZE TUNNELS

SLOW (NEW) TUNNEL SHAFT LOCATIONS			
SHAFT No.	SHAFT STATUS	LOCATION	OS REF.
1	BLIND (CAPPED)	T04+10	TQ277853
2	OPEN	T17+17	TQ274852
3	OPEN	T28+00	TQ272852
4	OPEN	T43+10	TQ269851
5	OPEN	T57+12	TQ267850
6	OPEN	T70+17	TQ264184

FAST TUNNEL SHAFT LOCATIONS			
SHAFT No.	SHAFT STATUS	LOCATION	OS REF.
1	OPEN	T01+07	TQ277852
2	OPEN	T21+17	TQ272851
3	OPEN	T32+02	TQ267850

KEY

- TABLET
- SHAFT OPEN
- SHAFT HIDDEN CONFIRMED
- SHAFT HIDDEN UNCONFIRMED

GEOLOGICAL KEY

- LONDON CLAY

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Rev.	Revision Description	Drn	Chkd	Appd	Date
0	FIRST ISSUE	SOS	MAR	PH	03/8/15

STATUS:
FIRST ISSUE

DONALDSON ASSOCIATES
Plot House
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Tel: 01332 343600
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Website: www.donaldsonassociates.com

Client:
Network Rail

Project:
LONDON NORTH EASTERN TERRITORY
TUNNEL MANAGEMENT STRATEGY UPGRADE

LINE:	ST PANCRAS TO CHESTERFIELD
ELLID:	SPC1 [STRUCTURE REF No: 29 & 30]
MILEAGE:	02 MILES 041 YARDS TO 03 MILES 071 YARDS
AT:	KENTISH TOWN & WEST HAMPSTEAD
OS GRID REF:	TQ 277 853 TO TQ 262 849
Title:	NETWORK RAIL LAND OWNERSHIP PLAN, GEOLOGICAL SECTION & TUNNEL PLAN BELSIZE TUNNELS (SPC1/29 & 30)

Drawn by:	SOS	Designed by:	PH	Checked:	MAR
Date:	DEC 2014	Original Sheet Size:	A0	Approved:	PH
Scales:	AS SHOWN				

Drawing No: J1377/SPC1/29/30/001 **Revision:** 0