

4 Frogna Rise,
London NW3 6RD
Basement Impact Assessment
Audit

For
London Borough of Camden

Project Number: 12336-32

Rev: F1

January 2017

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1.0 NON-TECHNICAL SUMMARY

- 1.1. CampbellReith was instructed by London Borough of Camden (LBC) to carry out an audit on the Basement Impact Assessment submitted as part of the Planning Submission documentation for 4 Frogna! Rise (planning reference 2015/3525/P). The basement is considered to fall within Category C as defined by the Terms of Reference.
- 1.2. The Audit reviewed the Basement Impact Assessment for potential impact on land stability and local ground and surface water conditions arising from basement development in accordance with LBC's policies and technical procedures.
- 1.3. CampbellReith was obtained the latest revision of submitted documentation from LBC's website and received information directly from the BIA author and reviewed it against an agreed audit check list. Information received directly from the author is presented in Appendix 3.
- 1.4. The BIA, SER and GMA have been carried out by well-known firms of engineering consultants using individuals who possess suitable qualifications.
- 1.5. The BIA has confirmed that an existing detached garage will be demolished, the existing two storey semi-detached house will be extended as its replacement and also extended backwards into the rear garden. A basement will be created below the new ground floor footprint which will retain 3 metres of soil at the front of the property and 6 metres of soil at the rear, due to the sloping nature of the site.
- 1.6. The BIA has shown that the proposed basement will be founded within the Bagshot Formation and will encounter perched groundwater. Further groundwater monitoring is requested to validate groundwater assumptions at the front of the property.
- 1.7. Although the development site has a slope marginally in excess of 7°, it is accepted that mitigation measures reduce the risk of slope failure. However, the construction sequence must clearly demonstrate that stability will be maintained at all times.
- 1.8. It is proposed to construct the basement using a combination of piled retaining walls at the rear of the property and conventional "hit and miss" underpinning elsewhere. Supplementary information has confirmed that none of the nearby properties have existing basements which extend below the proposed basement. On that basis, and the adoption of a contiguous piled wall, it is accepted that there are no cumulative impacts on the wider hydrogeology of the area. Should the retaining wall be altered to a secant piled wall, this will require review.
- 1.9. The Party Wall Approval process should be informed by the following additional information which is requested:

- Additional trial pits to confirm the depth of foundations to No. 2 Frognal Rise
 - Details to ensure fine materials not removed during pumping operations
 - Potential for subsoils to soften during the protracted underpinning process of forming reinforced concrete ell-shaped pins.
- 1.10. A Ground Movement Analysis has been undertaken. A number of queries were raised in the initial audits which have now largely been addressed. It is considered that the propping sequence is not realistic and it is therefore recommended that the final ground movement and building damage assessment is provided in a Basement Construction Plan.
- 1.11. It is accepted that the increased area of hardstanding and roof areas potentially discharging to the existing surface water drainage system have been mitigated by storage attenuation and the introduction of porous surfacing materials.
- 1.12. It is accepted that the site is not located within the Hampstead Heath pond chain catchment area, has no anticipated risk of groundwater or fluvial flooding and has no past history of flooding.
- 1.13. Queries and requests for further information are described in Section 4 and summarised in Appendix 2. It is accepted that, subject to the satisfactory completion of a BCP, the BIA and supporting documents identify the potential impacts arising out of the basement proposals and describe suitable mitigation.

2.0 INTRODUCTION

- 2.1. CampbellReith was instructed by London Borough of Camden (LBC) on 24 February 2016 to carry out a Category C Audit on the Basement Impact Assessment (BIA) submitted as part of the Planning Submission documentation for 4 Frogna! Rise, Planning Reference 2015/3525/P. The audit was delayed until a ground movement assessment was made available on 1 June 2016.
- 2.2. The Audit was carried out in accordance with the Terms of Reference set by LBC, which can be viewed on Camden's website. It reviewed the Basement Impact Assessment for potential impact on land stability and local ground and surface water conditions arising from basement development.
- 2.3. A BIA is required for all planning applications with basements in Camden in general accordance with policies and technical procedures contained within
- Guidance for Subterranean Development (GSD). Issue 01. November 2010. Ove Arup & Partners.
 - Camden Planning Guidance (CPG) 4: Basements and Lightwells.
 - Camden Development Policy (DP) 27: Basements and Lightwells.
 - Camden Development Policy (DP) 23: Water.
- 2.4. The BIA should demonstrate that schemes:
- a) maintain the structural stability of the building and neighbouring properties;
 - b) avoid adversely affecting drainage and run off or causing other damage to the water environment; and,
 - c) avoid cumulative impacts upon structural stability or the water environment in the local area
- and evaluate the impacts of the proposed basement considering the issues of hydrology, hydrogeology and land stability via the process described by the GSD and to make recommendations for the detailed design.
- 2.5. LBC's Audit Instruction described the planning proposal as the “*Erection of two storey side extension, part two storey, part single storey, part first floor rear extension, following a demolition of the existing single storey garage building and first floor rear extension, excavation of a new basement level and front lightwell, relocation of existing vehicular access and gate along Frogna! Rise including building up of a new section of front boundary wall, landscaping of*

the front garden area to create one off-street parking space including new bicycle and bin store all in connect with the existing single family dwellinghouse .”

The Audit Instruction also confirmed that the basement proposals did not involve a listed building nor the site neighboured listed buildings.

2.6. CampbellReith accessed LBC's website on 16 June 2016 and downloaded the following relevant documents for audit purposes:

- Site Investigation & Basement Impact Assessment Report (BIA) dated June 2015 by GEA
- Structural Engineering Report & Subterranean Construction Method Statement (SER) dated June 2015 by Elliot Wood
- Ground Investigation Report (GIR) dated March 2015 by KF Geotechnical
- Ground Movement Report (GMA) dated May 2016 by GEA
- Architectural plans and sections, existing and proposed dated May and October 2015 by Stephen Brandes Architects

2.7. Further to the issue of the initial audit reports, responses to the queries raised were provided by GEA on 11 August and 07 December 2016. The later response is presented in Appendix 3 and considered in this final audit report.

3.0 BASEMENT IMPACT ASSESSMENT AUDIT CHECK LIST

Item	Yes / No / NA	Comment
Are BIA Author(s) credentials satisfactory?	Yes	
Is data required by Cl.233 of the GSD presented?	Yes	BIA and SER.
Does the description of the proposed development include all aspects of temporary and permanent works which might impact upon geology, hydrogeology and hydrology?	Yes	SER Section 5.
Are suitable plan/maps included?	Yes	BIA Section 2.
Do the plans/maps show the whole of the relevant area of study and do they show it in sufficient detail?	Yes	
Land Stability Screening: Have appropriate data sources been consulted? Is justification provided for 'No' answers?	Yes	BIA Section 3.3.
Hydrogeology Screening: Have appropriate data sources been consulted? Is justification provided for 'No' answers?	Yes	BIA Section 3.2.
Hydrology Screening: Have appropriate data sources been consulted? Is justification provided for 'No' answers?	Yes	BIA Section 3.4.
Is a conceptual model presented?	Yes	BIA Section 7.
Land Stability Scoping Provided? Is scoping consistent with screening outcome?	Yes	BIA Section 4.1.

Item	Yes / No / NA	Comment
Hydrogeology Scoping Provided? Is scoping consistent with screening outcome?	Yes	BIA Section 4.1.
Hydrology Scoping Provided? Is scoping consistent with screening outcome?	Yes	BIA Section 4.1.
Is factual ground investigation data provided?	Yes	GIR.
Is monitoring data presented?	Yes	GIR and BIA Section 8.
Is the ground investigation informed by a desk study?	Yes	GIR.
Has a site walkover been undertaken?	Yes	GIR.
Is the presence/absence of adjacent or nearby basements confirmed?	Yes	Supplementary information describes the nature and extent of basements in surrounding properties.
Is a geotechnical interpretation presented?	Yes	BIA Section 8.
Does the geotechnical interpretation include information on retaining wall design?	Yes	BIA Section 8.1.2.
Are reports on other investigations required by screening and scoping presented?	Yes	Ground Movement Assessment Aboricultural Impact Assessment, Archaeological Assessment.
Are baseline conditions described, based on the GSD?	Yes	
Do the base line conditions consider adjacent or nearby basements?	Yes	Addressed in supplementary information (Appendix 3).
Is an Impact Assessment provided?	Yes	BIA Section 9.

Item	Yes / No / NA	Comment
Are estimates of ground movement and structural impact presented?	Yes	GMA Section 7.
Is the Impact Assessment appropriate to the matters identified by screen and scoping?	Yes	
Has the need for mitigation been considered and are appropriate mitigation methods incorporated in the scheme?	Yes	However, GMA to be updated.
Has the need for monitoring during construction been considered?	Yes	GMA Section 7.2.
Have the residual (after mitigation) impacts been clearly identified?	Yes	Although final details to be provided in a BCP.
Has the scheme demonstrated that the structural stability of the building and neighbouring properties and infrastructure will be maintained?	Yes	Although final details to be provided in a BCP.
Has the scheme avoided adversely affecting drainage and run-off or causing other damage to the water environment?	Yes	
Has the scheme avoided cumulative impacts upon structural stability or the water environment in the local area?	Yes	Although final details to be provided in a BCP.
Does report state that damage to surrounding buildings will be no worse than Burland Category 2?	Yes	Revised GMA estimates damage to be Burland Category 0 or 1, with the exception of a small section of boundary wall along Windmill Hill where Category 2 damage is predicted.
Are non-technical summaries provided?	No	However, the report is well written and easily followed.

4.0 DISCUSSION

- 4.1. The BIA has been carried out by a well-known firm of consultants, GEA, and the individuals concerned in its production have suitable qualifications. The SER has similarly been carried out by a well-known firm of engineering consultants, Elliott Wood, and their report has been reviewed by a suitably qualified and experienced individual. The GMA and building damage assessment has been prepared by GEA and was authored and reviewed by suitably qualified persons.
- 4.2. The site is located to the west of Hampstead Heath and lies in an area that slopes to the west/south west. No. 4 Frogna! Rise is a two storey semi-detached property adjoining No. 2. The front elevation of No. 4 faces southwest and is approximately 4.0 metres lower than its rear garden. A detached garage and rear ground floor wall retain the rear garden, which abuts the highway of Windmill Hill behind the property.
- 4.3. It is proposed to demolish the existing garage, push back the rear ground floor retaining wall, and extend the ground floor to the north western boundary line. A basement will be introduced below the reconfigured ground floor footprint. At the front of the property, the external forecourt will step back up to existing levels via a series of terraces. The reconfigured new rear house perimeter will encompass existing garden retaining walls. The new basement walls will retain approximately 3.0 metres of soil at the front of the property increasing to approximately 6.0 metres at the rear.
- 4.4. The BIA has identified that the proposed basement is expected to encounter a significant thickness of Made Ground, overlying the sands and clay bands of the Bagshot Formation and Claygate Member, which approximately follow the slope of the topography. The soils investigation consisted of two boreholes in the front and rear of the property and three trial pits to identify anticipated existing foundation depths. Standpipes were incorporated into the boreholes and monitored on two occasions. The BIA identifies that localised groundwater inflows may be expected within the basement excavation in the north (rear) of the site. Although groundwater monitoring of the south (front) standpipe anticipated no inflow on this face of the excavation, the BIA recognised that the level in the standpipe had not reached equilibrium level (March and April 2015) and monitoring should be continued.
- 4.5. Supplementary information provided in August 2016 states that a further monitoring visit was undertaken towards the end of July. This encountered groundwater at 6.77m beneath the centre of the site. A 5m deep standpipe at the rear of the site was dry. A southerly direction of groundwater flow is assumed in the BIA and confirmation was requested. The groundwater monitoring should be related to Ordnance Datum to demonstrate whether or not this assumption has been verified. The monitoring data should also be reported relative to proposed

excavation levels (in m OD) to demonstrate whether the water table at the rear of the site has been established in sufficient detail.

- 4.6. Trial pits have been undertaken which identify that foundations to existing boundary walls on each side of the property are founded just into the Bagshot Formation as was the existing foundation to No. 4. It has been assumed that the adjoining foundations for No. 2 will be at a similar depth and that no basement is present next door. Further trial pits should be undertaken, prior to construction commencement, to confirm these assumptions and inform the Party Wall process.
- 4.7. The BIA notes that the general slope around the development site is approximately 7°/8°. Whilst it is accepted that the presence of a number of retaining walls exist to break up the slope, the construction works must be sequenced in such a way that slope stability is not compromised.
- 4.8. It is known that a number of basements are currently being constructed in the vicinity of the development site or have recently received planning permission. Supplementary information has noted that surrounding basements and lower ground floor levels are at or above the current ground level of No 4 Frogna! Rise. On the basis of the proposed contiguous wall and the absence of other nearby basements, it is accepted that this additional basement will not have significant cumulative effects on the hydrogeology of the area. Should a review of groundwater levels require a secant piled wall, this will require to be reassessed.
- 4.9. The BIA notes that perched groundwater will be encountered during basement excavation although groundwater flow should be controllable by sump pumping. Care should be taken to ensure that fine materials are not washed out during pumping operations and further details should be provided by the contractor during the Party Wall Process.
- 4.10. Specific care should be taken during the underpinning process which is envisaged under all walls other than the northeastern (rear) retaining wall, which will be piled from existing ground level, to ensure that the subsoils do not soften during the hit and miss underpinning process. This is likely to be potentially significant since the proposed methodology of reinforced concrete 'L'-shaped pins will mean an increased time duration for this phase of the work. This should be considered further during the Party Wall process.
- 4.11. The BIA has identified the potential for heave of the underlying clay soils to occur and suitable mitigation is proposed.
- 4.12. A Ground Movement Assessment has been carried out by GEA to determine the effect of underpinning, piling and excavation on the adjoining/adjacent properties and the highway in Windmill Hill. The GMA predicts damage no worse than Burland Category 1 (very slight) for all

surrounding structures with the exception of a small length of the boundary wall along Windmill Hill where Category 2 damage. The following observations are made

- The BIA refers to a 'top-down' methodology, and the construction sequence adopted in the Wallap is bottom up, however, this is conservative and is accepted.
 - Whilst the temporary works assumed in the analyses seem unrealistic, it has been confirmed that significant propping is required to limit potential damage to surrounding structures and that it will form the basis of the tenders issued to the contractors. It is recommended that the final design of the basement and the temporary works are subject to a further assessment with respect to ground movements and building damage as part of a Basement Construction Plan.
 - Whilst the input data appears to be at variance with the accompanying text, the simplification of the excavation in XDisp is reasonable in light of the reduced significance of ground movements adjacent to the footpath of Windmill Hill.
 - There is discrepancy between the body of the GMA and the XDisp input where the text refers to 60% of CIRIA C580 movement to be anticipated, whilst the XDisp input adopts 50%. It was also requested that consideration was given to importing the movements predicted in the Wallap analyses for the damage assessment since the maximum movements occur at depth. Whilst GEA have confirmed that the XDisp outputs were halved in order to be consistent with the Wallap data, they have not incorporated the Wallap data into their analyses. However, this can be addressed in the BCP once other queries have been resolved in the BIA.
 - The effects of underpinning and the remainder of the excavation have now been assessed using XDisp. Whilst there are significant limitations to this approach due to the method of construction and the ground conditions, it is accepted that it is conservative.
 - The height of the neighbouring building, No. 2 Frogna! Rise is taken as 8m in the damage assessment. Supplementary information confirms this is reasonable and that nos. 5 & 6 Windmill Hill are included.
- 4.13. The BIA and GMA identify the need for lateral stability of the excavations to be maintained by the introduction of temporary works. No details are provided in either document nor in the SER. Additional details were requested to ensure adequate stiffness of temporary propping to comply with the requirements of the GMA. These have not been received, and as recommended above, should be provided in a BCP.
- 4.14. It is accepted that the introduction of porous surfacing to new external terrace areas and the rainwater storage cistern to be installed in the area of the old demolished garage will mitigate the increased area of hardstanding and roof areas discharging to the existing surface water drainage system.

- 4.15. It is accepted that the site is not located within the catchment area of the Hampstead Heath pond chain.
- 4.16. It is accepted that the development site has no anticipated risk of groundwater or fluvial flooding and has no past history of flooding.

5.0 CONCLUSIONS

- 5.1. The BIA, SER and GMA have been carried out by well-known firms of engineering consultants using individuals who possess suitable qualifications.
- 5.2. The BIA has confirmed that an existing detached garage will be demolished, the existing two storey semi-detached house will be extended as its replacement and also extended backwards into the rear garden. A basement will be created below the new ground floor footprint which will retain 3 metres of soil at the front of the property and 6 metres of soil at the rear, due to the sloping nature of the site.
- 5.3. The BIA has shown that the proposed basement will be founded within the Bagshot Formation and will encounter perched groundwater. Further groundwater monitoring is requested to validate groundwater assumptions at the front of the property. This information should be presented in a Basement Construction Plan.
- 5.4. Although the development site has a slope marginally in excess of 7°, it is accepted that mitigation measures reduce the risk of slope failure. However, the construction sequence must clearly demonstrate that stability will be maintained at all times. This information may be presented in the BCP.
- 5.5. It is proposed to construct the basement using a combination of piled retaining walls at the rear of the property and conventional “hit and miss” underpinning elsewhere. Supplementary information has confirmed that none of the nearby properties have existing basements which extend below the proposed basement. On that basis, and the adoption of a contiguous piled wall, it is accepted that there are no cumulative impacts on the wider hydrogeology of the area. Should the retaining wall be altered to a secant piled wall, this will require review.
- 5.6. The Party Wall Approval process should be informed by the following additional information which is requested:
- Additional trial pits to confirm the depth of foundations to No. 2 Frognal Rise
 - Details to ensure fine material not removed during pumping operations
 - Potential for subsoils to soften during the protracted underpinning process of forming reinforced concrete L-shaped pins.
- 5.7. A Ground Movement Analysis has been undertaken. A number of queries were raised in the initial audit which have largely been addressed. It is considered that the propping sequence is not realistic and it is therefore recommended that the final ground movement and building damage assessment is provided in the BCP.

- 5.8. Additional details are requested to ensure adequate stiffness of temporary propping to comply with the requirements of the GMA, and confirm that the assumptions made in the GMA with regard to temporary works are reasonable. These should be provided in the BCP.
- 5.9. It is accepted that the increased area of hardstanding and roof areas potentially discharging to the existing surface water drainage system have been mitigated by storage attenuation and the introduction of porous surfacing materials.
- 5.10. It is accepted that the site is not located within the Hampstead Heath pond chain catchment area, has no anticipated risk of groundwater or fluvial flooding and has no past history of flooding.

Appendix 1: Residents' Consultation Comments

Residents' Consultation Comments

Surname	Address	Date	Issue raised	Response
Heath & Hampstead Society	N/A	21/11/14	Lack of GMA and damage assessment.	Predates the issue of a GMA, see 4.12.
Levy	2 Frognal Rise	November 2015	Underpinning, piling and groundwater affecting stability of adjoining property.	See 4.5 to 4.13.
Fossum	6 Windmill Hill		Damage to water main in Windmill Hill.	See 4.12.
Rustall	5 Windmill Hill		Damage to house	See 4.12.
Rubin	22 Windmill Hill	25.11.15	Piling vibration and flooding due to basement obstruction.	See 4.5.

Appendix 2: Audit Query Tracker

Audit Query Tracker

Query No	Subject	Query	Status	Date closed out
1	Hydrogeology	Further groundwater monitoring requested to determine flow rate, flow direction and pile type	Closed – to be further addressed in BCP.	06.01.17
2	Hydrogeology	Evidence of basements in vicinity of development site	Closed – will require review if retaining wall construction is revised.	30.9.16
3	Stability	Indicative temporary works proposal requested	Closed – to be provided in BCP.	06.01.17
4	Stability	Ground Movement and building damage assessment to be reviewed	Closed – final version to be presented in BCP.	06.01.17
5	Stability	Detailed proposals to ensure softening of clay materials do not occur	To be agreed for Party Wall Award	N/A
6	Stability	Detailed proposals to ensure fine material not removed during sump pumping	To be agreed for Party Wall Award	N/A
7	Stability	Additional trial pit(s) to verify depth of party wall and presence of adjoining basement	To be agreed for Party Wall Award	N/A

Appendix 3: Supplementary Supporting Documents

7 December 2016

Our ref J14373/MC/2

Unit 1, Church Farm
Gotham Road, Kingston on Soar
Notts NG11 0DE

tel 01509 674888
fax 01509 674950
email mail@gea-ltd.co.uk
web www.gea-ltd.co.uk

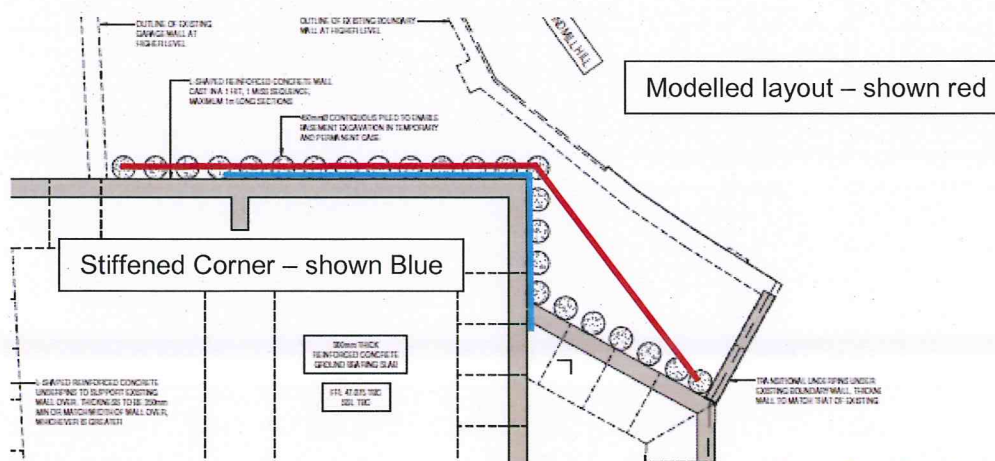
Stephen Brandes
Stephen Brandes Architects Limited
5 Spedan Close
London
NW3 7XF

Dear Stephen

**Re: FURTHER RESPONSE TO THE AUDITS OF THE BASEMENT IMPACT
ASSESSMENT FOR 4 FROGNAL RISE, LONDON NW3 6RD (2014/6736/P)**

Our Basement Impact Assessment (BIA) and Ground Movement Analysis (GMA) has been reviewed by Campbell Reith (CR) and we have discussed the outstanding items with Graham Kite of CR. The outstanding queries have been distilled to a number of points of detail and these are set out in italics along with our responses, where applicable, as follows.

- 1. In the round, we want the GMA to be able to demonstrate that using 'reasonably conservative' parameters and methodology, stability of adjacent structures / environment will be maintained.* Response: Accepted
- 2. Comments were raised about the effects of corner stiffening. Our comments remain that we're concerned the modelled stiffening effects in XDisp are not realistic and would direct you to the Oasys XDisp manual 19.3, section 2.2.1.* Response: The re-entrant corner in the bored pile wall is unable to be modelled within the X-Disp program. We have, therefore modelled this as two walls with an enclosed angle of roughly 127° and shown in red below. We have previously modelled this piled wall both with and without stiffening effects with satisfactory results in both cases. We have nonetheless reconsidered this approach in the light of CR comments. It is our opinion that the central 90° corner of the piled wall, shown blue below, may reasonably be considered as a fully stiffened corner even though the model reflects a wider angle. However the two ends of the bored pile wall have now been modelled without such effects. This is shown on the plan extract below.



Offices in Hertfordshire (tel 01727 824666) and Nottinghamshire (tel 01509 674888)

Steve Branch BSc MSc CGeol FGS FRGS MIEnvSc
Mike Plimmer BSc MSc CGeol FGS MIEnvSc
Martin Cooper BEng CEng MICE
Juliet Fuller BSc MSc DIC FGS

3. *The structural proposal include 3 levels of props and you have already confirmed that this is in-line with EWP's instructions. However, its not considered 'reasonably conservative' to adopt a propping scheme that the Contractor is very likely to change due to buildability issues.* Response: The retaining wall has been remodelled using a 600 mm diameter piles and the stiffer piles have enabled a different propping arrangement to be adopted. Temporary steel props will be used at two levels during construction, 51.8 m OD and 49.3 m OD, reflecting one prop close to each floor level. The permanent propping remains as before with concrete slabs with centrelines at 46.72 m OD, 49.85 m OD and 52.7 m OD. Diagonal bracing will be installed at the 90° corner in the piled wall and the remaining propping forces will be transferred into the existing building at the 51.8 m (OD) level with temporary waling beams spreading the load across the rear of the building. Similarly the props at the 49.3 m OD level will bear against the existing building and be transferred longitudinally to the front of the structure. The excavation below 49.3 m OD will be in maximum 1.0 m width underpins which will be propped against the central earth bund until cured and adequate concrete strength has been gained.

4. *In section 4.12 of the D2 Audit we comment that 'it is not clear whether the effects of underpinning and the remainder of the excavation' have been included within the GMA. I understand from our discussions that you have included this - for clarity please highlight where its included.* Response: We considered that the previous GMA included the effects of underpinning and the main excavation in the P-Disp analysis. However we have now also included the horizontal movements from a fuller excavation within the X-Disp analysis. Whilst not a perfect model due to the reduction in underpin height from rear to the front of the site, the model is considered to be a conservative representation of the key elements that comprise installation, excavation, settlement under load and heave due to unloading.

5. *The damage impact assessment currently indicates the adjacent house (No 2 Frognal Rise) to be category 0 whilst the retaining wall is category 2. Bearing in mind my comments 2 and 3 above, are these realistic?* Response: The results of the changes above indicate that the damage category for No 2 Frognal Rise is between Cat 0 – Negligible and Cat 1 – Very Slight. The retaining wall to Windmill Hill, behind the house, has been split into three sections and the combined segments for Wall 2, as well as Section Nos 1 and 3 all indicate Category 1 - Very slight damage.

6. *The GMA proposes that damage impact restrictions imposed by LBC should be relaxed for the retaining wall, as it is not a dwelling. Bearing in mind comments above regarding propping arrangements, discussion on what level of movement / damage would be acceptable for the retaining structure is required. Please also consider that mitigation measures will need to be proposed to control movements / damage, as well as monitoring mechanisms.* Response: The results of the changes above indicate that the maximum damage category is Cat 1 – very slight. The retaining wall structure is wholly within the 4 Frognal Rise site and as part of the development will be propped in part by the new rear extension and in consequence its long term stability is ensured. However it is proposed that the retaining wall, as well as No 2 Frognal Rise will be subject to a regime of monitoring and review through the weeks in advance of commencement and throughout the construction period. The detail of the monitoring regime remains to be finalised but will be agreed under the party wall process. It is considered that an inclinometer should be cast within one of the retaining wall piles and that survey targets should be affixed to No 2 Frognal Rise and the rear retaining walls and that survey point such as Hilti nails should be cast into the piled wall capping beam.

We trust that the foregoing comments are sufficient to address the Campbell Reith comments in respect of the BIA and GMA, but please do not hesitate to contact us if further clarification is required.

Yours sincerely
GEOTECHNICAL & ENVIRONMENTAL ASSOCIATES

A handwritten signature in black ink, appearing to read 'M Cooper', with a long horizontal stroke extending to the right.

Martin Cooper
BEng CEng MICE FGS

A handwritten signature in black ink, appearing to read 'S Branch', with a large circular loop at the end.

Steve Branch
BSc MSc CGeol FGS FRGS

Units: kN,m

INPUT DATA**SOIL PROFILE**

Stratum no.	Elevation of top of stratum	Soil types
		Active side Passive side
1	52.80	1 Made Ground 1 Made Ground
2	50.00	2 Bagshot (Upper Sand) 2 Bagshot (Upper Sand)
3	49.00	3 Bagshot (Si Sa Clay) 3 Bagshot (Si Sa Clay)
4	47.90	4 Bagshot (Lower Sand) 4 Bagshot (Lower Sand)
5	41.00	5 London Clay 5 London Clay

SOIL PROPERTIES

-- Soil type --	Bulk density	Young's Modulus	At rest coeff.	Consol state.	Active limit	Passive limit	Cohesion
No. Description (Datum elev.)	kN/m3	Eh,kN/m2 (dEh/dy)	Ko (dKo/dy)	NC/OC (Nu)	Ka (Kac)	Kp (Kpc)	kN/m2 (dc/dy)
1 Made Ground	17.00	10000	0.500	OC (0.200)	0.490 (0.000)	2.040 (0.000)	
2 Bagshot (U-pper Sand)	18.50	36000	0.500	OC (0.200)	0.295 (0.000)	3.392 (0.000)	
3 Bagshot (Si Sa Clay)	18.50	50000	1.000	OC (0.490)	1.000 (2.000)	1.000 (2.000)	100.0u
4 Bagshot (.. (47.90)	18.50	40000 (1739)	1.000	OC (0.200)	0.295 (0.000)	3.392 (0.000)	
5 London Clay (41.00)	19.50	57500 (3750)	1.000	OC (0.490)	1.000 (2.000)	1.000 (2.000)	115.0u (7.500)

Additional soil parameters associated with Ka and Kp

--- parameters for Ka ---				--- parameters for Kp ---			
Soil	Wall	Back-		Soil	Wall	Back-	
friction	adhesion	fill		friction	adhesion	fill	
angle	coeff.	angle		angle	coeff.	angle	
1 Made Ground	20.00	0.000	0.00	20.00	0.000	0.00	
2 Bagshot (Upper Sand)	33.00	0.000	0.00	33.00	0.000	0.00	
3 Bagshot (Si Sa Clay)	0.00	0.000	0.00	0.00	0.000	0.00	
4 Bagshot (Lower Sand)	33.00	0.000	0.00	33.00	0.000	0.00	
5 London Clay	0.00	0.000	0.00	0.00	0.000	0.00	

GROUND WATER CONDITIONS

Density of water = 10.00 kN/m3

Initial water table elevation Active side Passive side
 50.00 50.00

Automatic water pressure balancing at toe of wall : No

Active side				Passive side			
Water press.	Point no.	Elev.	Piezo elev.	Point no.	Elev.	Piezo elev.	Water press.
m		m	m		m	m	kN/m2
1	1	50.00	50.00	1	49.10	49.10	0.0 MC+WC
2	1	50.00	50.00	1	46.50	46.50	0.0 MC
3	1	50.00	50.00	1	46.47	46.47	0.0 MC

WALL PROPERTIES

Type of structure = Fully Embedded Wall
 Elevation of toe of wall = 41.00
 Maximum finite element length = 0.60 m
 Youngs modulus of wall E = 2.8000E+07 kN/m2
 Moment of inertia of wall I = 8.4820E-03 m4/m run
 E.I = 237496 kN.m2/m run
 Yield Moment of wall = Not defined

STRUTS and ANCHORS

Strut/ anchor no.	Elev.	Strut spacing m	X-section area of strut sq.m	Youngs modulus kN/m2	Free length m	Inclin -ation (degs)	Pre- stress /strut kN	Tension allowed
1	51.80	3.00	0.100000	2.000E+08	3.00	0.00	0	No
2	49.30	3.00	0.100000	2.000E+08	3.00	0.00	0	No
3	46.72	1.00	0.400000	3.000E+07	3.00	0.00	0	No
4	49.85	1.00	0.300000	3.000E+07	3.00	0.00	0	No
5	52.70	1.00	0.250000	3.000E+07	3.00	0.00	0	No

SURCHARGE LOADS

Surch -arge no.	Elev.	Distance from wall	Length parallel to wall	Width perpend. to wall	Surcharge ----- Near edge	Surcharge ----- Far edge	Equiv. soil type	Partial factor/ Category
1	52.80	0.00(A)	20.00	30.00	14.40	=	N/A	1.00 -
2	46.47	-0.00(P)	20.00	30.00	35.00	=	N/A	1.00 -

Note: A = Active side, P = Passive side

Limit State Categories P/U = Permanent Unfavourable
 P/F = Permanent Favourable
 Var = Variable (unfavourable)

CONSTRUCTION STAGES

Construction stage no.	Stage description
1	Change EI of wall to 1.0000E-05 kN.m2/m run Yield moment not defined No adjustments to wall displacements
2	Apply surcharge no.1 at elevation 52.80
3	Change EI of wall to 237496 kN.m2/m run Yield moment not defined Reset wall displacements to zero at this stage
4	Excavate to elevation 51.50 on PASSIVE side
5	Install strut or anchor no.1 at elevation 51.80
6	Apply water pressure profile no.1 (Mod. Conserv.)
7	Excavate to elevation 49.10 on PASSIVE side
8	Install strut or anchor no.2 at elevation 49.30
9	Apply water pressure profile no.2 (Mod. Conserv.)
10	Apply water pressure profile no.3 (Mod. Conserv.)
11	Excavate to elevation 46.47 on PASSIVE side
12	Install strut or anchor no.3 at elevation 46.72
13	Apply surcharge no.2 at elevation 46.47
14	Install strut or anchor no.4 at elevation 49.85
15	Remove strut or anchor no.2 at elevation 49.30
16	Install strut or anchor no.5 at elevation 52.70
17	Remove strut or anchor no.1 at elevation 51.80

FACTORS OF SAFETY and ANALYSIS OPTIONS

Limit State options: Serviceability Limit State
All loads and soil strengths are unfactored

Stability analysis:

Method of analysis - Strength Factor method
Factor on soil strength for calculating wall depth = 1.00

Parameters for undrained strata:

Minimum equivalent fluid density = 5.00 kN/m3
Maximum depth of water filled tension crack = 0.00 m

Bending moment and displacement calculation:

Method - Subgrade reaction model using Influence Coefficients
Open Tension Crack analysis? - No
Non-linear Modulus Parameter (L) = 0 m

Boundary conditions:

Length of wall (normal to plane of analysis) = 1000.00 m

Width of excavation on active side of wall = 20.00 m

Width of excavation on passive side of wall = 20.00 m

Distance to rigid boundary on active side = 20.00 m

Distance to rigid boundary on passive side = 20.00 m

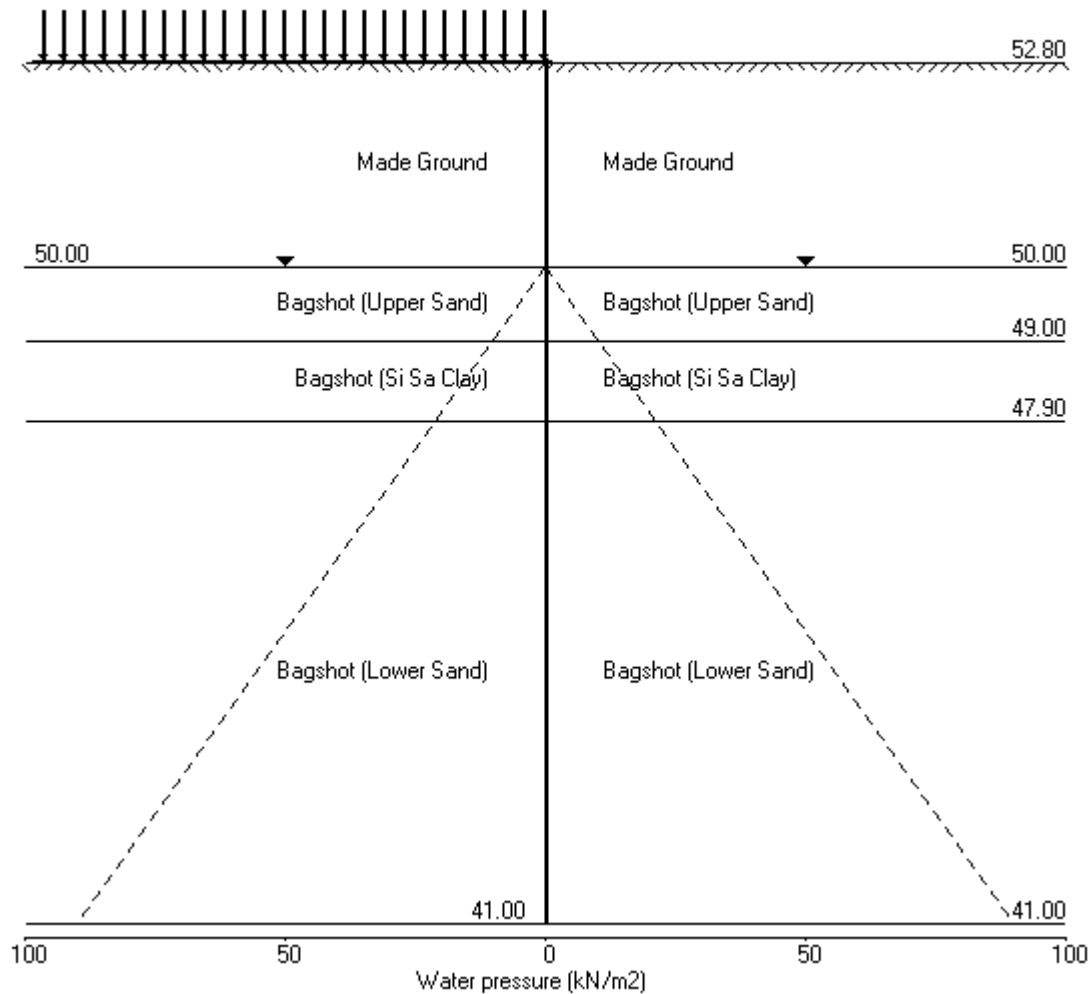
OUTPUT OPTIONS

Stage no.	Stage description	Displacement Bending mom. Shear force	Active, Passive pressures	Graph. output
1	Change EI of wall to 1.0000E-05kN.m2/m	No	No	No
2	Apply surcharge no.1 at elev. 52.80	Yes	No	Yes
3	Change EI of wall to 237496kN.m2/m run	No	Yes	Yes
4	Excav. to elev. 51.50 on PASSIVE side	No	No	No
5	Install strut no.1 at elev. 51.80	No	No	No
6	Apply water pressure profile no.1	No	No	No
7	Excav. to elev. 49.10 on PASSIVE side	No	No	No
8	Install strut no.2 at elev. 49.30	No	No	No
9	Apply water pressure profile no.2	No	No	No
10	Apply water pressure profile no.3	No	No	No
11	Excav. to elev. 46.47 on PASSIVE side	Yes	No	Yes
12	Install strut no.3 at elev. 46.72	No	No	No
13	Apply surcharge no.2 at elev. 46.47	No	No	No
14	Install strut no.4 at elev. 49.85	No	No	No
15	Remove strut no.2 at elev. 49.30	No	No	No
16	Install strut no.5 at elev. 52.70	No	No	No
17	Remove strut no.1 at elev. 51.80	No	No	No
*	Summary output	Yes	-	Yes

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Units: kN, m

Stage No.2 Apply surcharge no.1 at elev. 52.80



Units: kN,m

Stage No. 2 Apply surcharge no.1 at elevation 52.80

STABILITY ANALYSIS of Fully Embedded Wall according to Strength Factor method

Factor of safety on soil strength

				FoS for toe elev. = 41.00		Toe elev. for FoS = 1.000	
				-----		-----	
Stage	--- G.L. ---		Strut	Factor	Moment	Toe	Wall
No.	Act.	Pass.	Elev.	of	equilib.	elev.	Penetr
				Safety	at elev.		-ation
2	52.80	52.80	Cant.	11.063	42.24	52.09	0.71

BENDING MOMENT and DISPLACEMENT ANALYSIS of Fully Embedded Wall**Analysis options**

Length of wall perpendicular to section = 1000.00m

Subgrade reaction model - Boussinesq Influence coefficients

Soil deformations are elastic until the active or passive limit is reached

Open Tension Crack analysis - No

Rigid boundaries: Active side 20.00 from wall

Passive side 20.00 from wall

Limit State: Serviceability Limit State

Calculated Bending Moments and Strut Forces are to be multiplied by a factor of 1.35 to obtain values for structural design. See summary for factored values.

*** Convergence failure - Out of balance shear force at elevation 41.00
is 64.749% of the maximum shear force. see Note.

Node no.	Y coord	Nett pressure kN/m2	Wall disp. m	Wall rotation rad.	Shear force kN/m	Bending moment kN.m/m	Strut forces kN/m	EI of wall kN.m2/m
1	52.80	7.06	0.004	1.57E-04	0.0	0.0		0
2	52.70	4.29	0.004	1.30E-04	0.6	0.0		0
3	52.25	0.00	0.004	4.06E-05	1.6	-0.0		0
4	51.80	-0.01	0.003	1.23E-04	1.6	-0.0		0
5	51.50	0.00	0.003	4.75E-05	1.6	0.0		0
6	51.13	0.00	0.003	3.10E-04	1.6	-0.0		0
7	50.75	0.00	0.003	-7.34E-04	1.6	0.0		0
8	50.38	0.00	0.003	3.23E-03	1.6	-0.0		0
9	50.00	4.48	0.001	6.50E-03	2.4	0.0		0
		-11.26	0.001	6.50E-03	2.4	0.0		
10	49.85	0.01	0.000	3.64E-03	1.6	0.0		0
11	49.30	-0.00	0.000	3.16E-05	1.6	-0.0		0
12	49.10	-0.00	0.000	-1.39E-03	1.6	0.0		0
13	49.00	-3.01	0.000	-2.13E-03	1.4	-0.0		0
		0.59	0.000	-2.13E-03	1.4	-0.0		
14	48.45	0.00	0.000	6.10E-04	1.6	0.0		0
15	47.90	2.65	0.000	1.39E-04	2.3	-0.0		0
		-2.51	0.000	1.39E-04	2.3	-0.0		
16	47.31	0.00	0.000	1.93E-04	1.6	0.0		0
17	46.72	-0.01	0.000	-2.65E-05	1.6	-0.0		0
18	46.50	-0.01	0.000	5.89E-05	1.6	-0.0		0
19	46.47	0.01	0.000	5.93E-05	1.6	0.0		0
20	46.04	0.00	0.000	3.39E-07	1.6	-0.0		0
21	45.60	0.01	0.000	1.79E-05	1.6	0.0		0
22	45.00	0.00	0.000	1.14E-05	1.6	-0.0		0
23	44.40	0.00	0.000	1.31E-05	1.6	0.0		0
24	43.80	0.00	0.000	1.21E-05	1.6	0.0		0

Run ID. J14373 Wallap 1_SLS Revised Nov 2016
4 Frogna! Rise
Rear Retaining Wall

| Sheet No.
| Date:28-11-2016
| Checked :

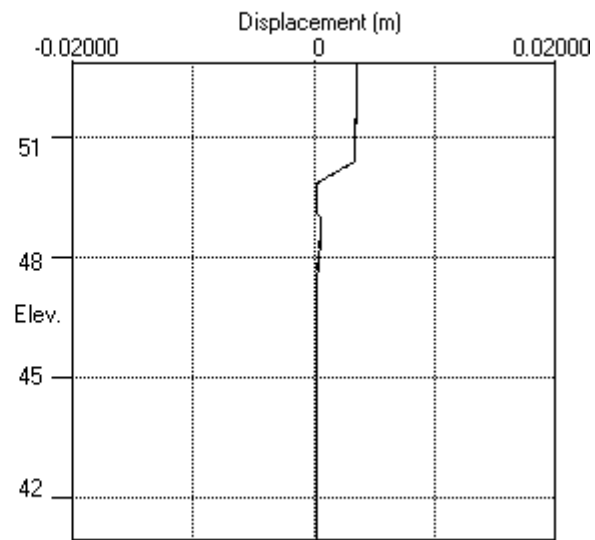
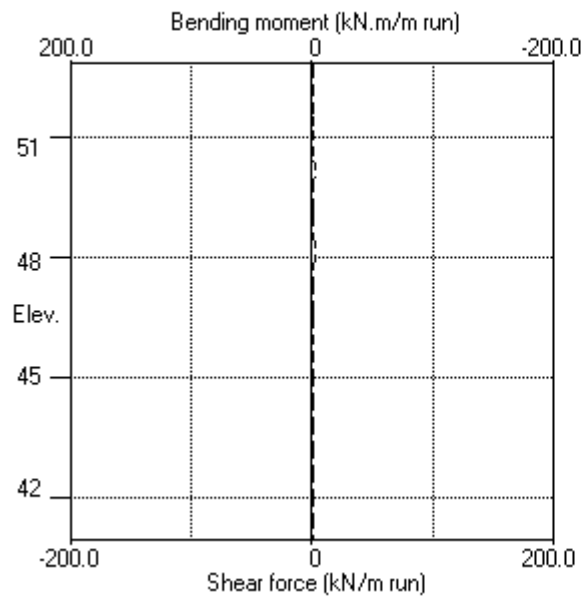
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Stage No.2 Apply surcharge no.1 at elevation 52.80

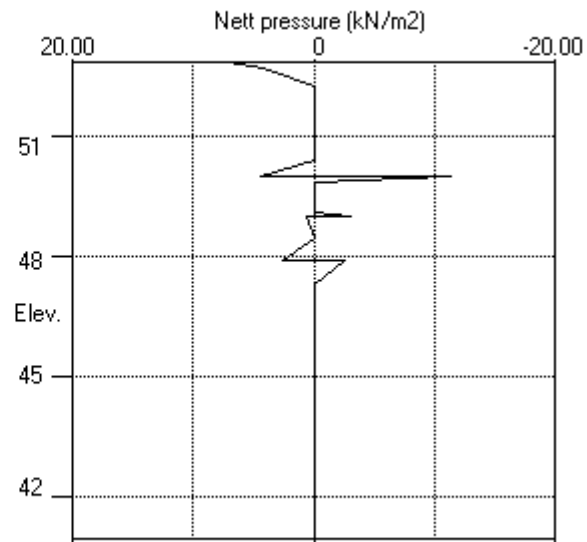
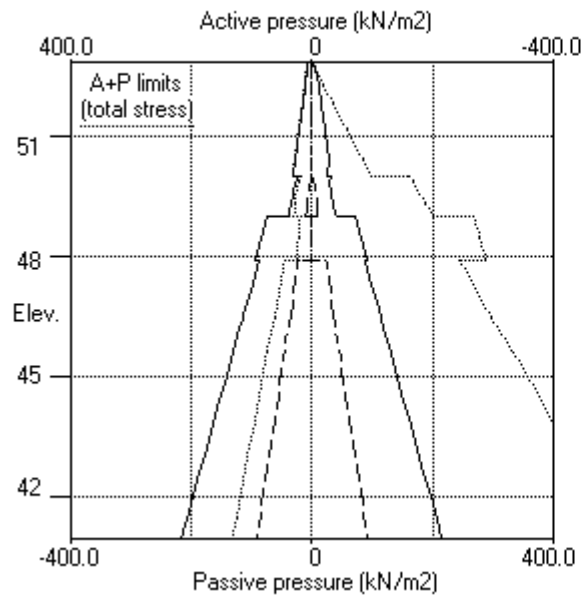
Node no.	Y coord	Nett pressure kN/m2	Wall disp. m	Wall rotation rad.	Shear force kN/m	Bending moment kN.m/m	Strut forces kN/m	EI of wall kN.m2/m
25	43.20	0.00	0.000	1.19E-05	1.6	0.0		0
26	42.60	0.00	0.000	1.09E-05	1.6	0.0		0
27	42.00	-0.00	0.000	1.12E-05	1.6	-0.0		0
28	41.50	-0.00	0.000	9.88E-06	1.6	0.0		0
29	41.00	-0.02	0.000	7.64E-06	1.6	0.0		---

Units: kN,m

Stage No.2 Apply surcharge no.1 at elev. 52.80

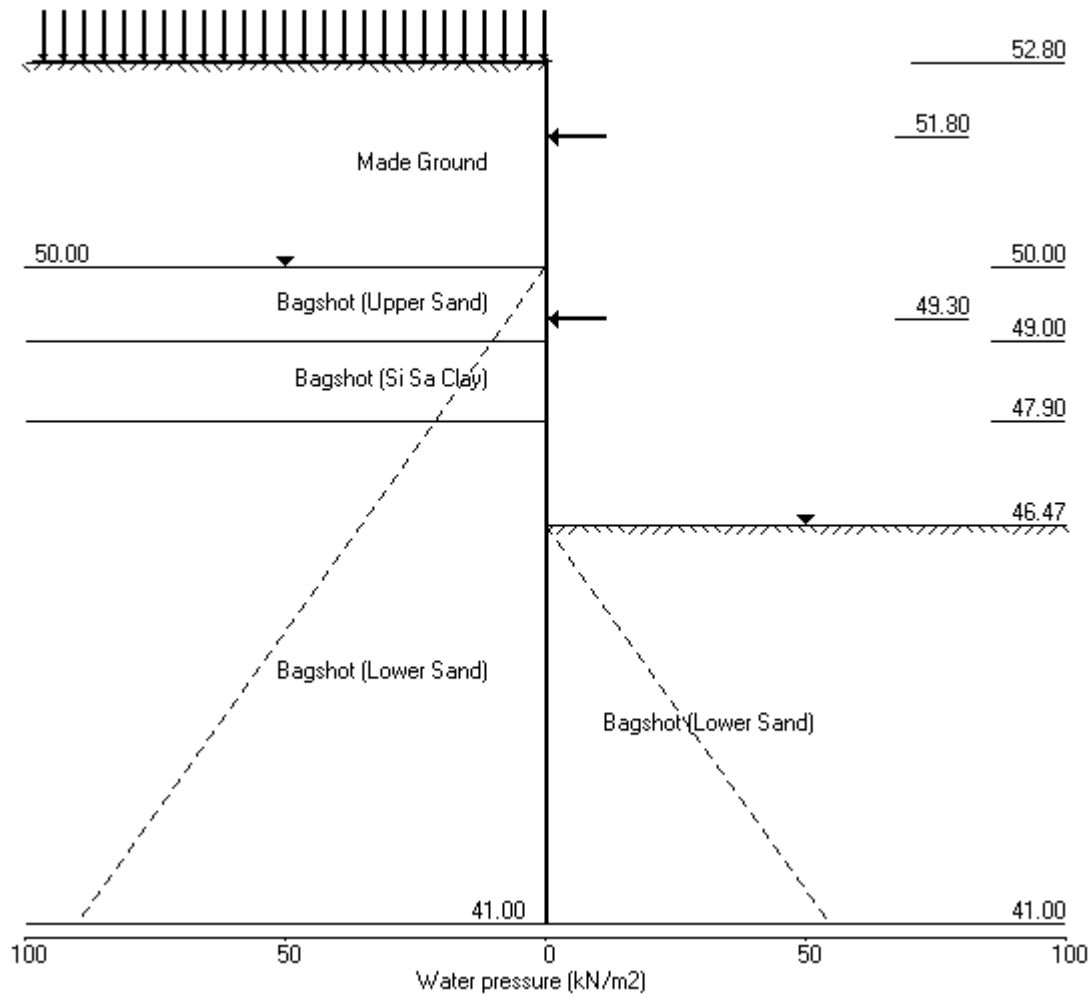


Stage No.2 Apply surcharge no.1 at elev. 52.80



Units: kN, m

Stage No.11 Excav. to elev. 46.47 on PASSIVE side



Units: kN,m

Stage No. 11 Excavate to elevation 46.47 on PASSIVE side

STABILITY ANALYSIS of Fully Embedded Wall according to Strength Factor method

Factor of safety on soil strength

			FoS for toe elev. = 41.00		Toe elev. for FoS = 1.000		

Stage	--- G.L. ---		Strut	Factor	Moment	Toe	Wall
No.	Act.	Pass.	Elev.	of	equilibr.	elev.	Penetr
				Safety	at elev.		-ation
11	52.80	46.47		More than one strut			

BENDING MOMENT and DISPLACEMENT ANALYSIS of Fully Embedded Wall

Analysis options

Length of wall perpendicular to section = 1000.00m

Subgrade reaction model - Boussinesq Influence coefficients

Soil deformations are elastic until the active or passive limit is reached

Open Tension Crack analysis - No

Rigid boundaries: Active side 20.00 from wall

Passive side 20.00 from wall

Limit State: Serviceability Limit State

Calculated Bending Moments and Strut Forces are to be multiplied by a factor of 1.35 to obtain values for structural design. See summary for factored values.

*** Wall displacements reset to zero at stage 3

Node no.	Y coord	Nett pressure kN/m2	Wall disp. m	Wall rotation rad.	Shear force kN/m	Bending moment kN.m/m	Strut forces kN/m	EI of wall kN.m2/m
1	52.80	23.88	-0.000	-1.07E-03	0.0	0.0		237496
2	52.70	23.70	-0.000	-1.07E-03	2.5	0.1		237496
3	52.25	22.94	0.000	-1.07E-03	12.9	3.2		237496
4	51.80	22.32	0.001	-1.08E-03	23.1	10.7	-0.0	237496
5	51.50	24.19	0.001	-1.10E-03	30.0	18.2		237496
6	51.13	26.50	0.002	-1.14E-03	39.5	30.6		237496
7	50.75	28.72	0.002	-1.20E-03	49.9	47.0		237496
8	50.38	30.83	0.002	-1.29E-03	61.1	67.2		237496
9	50.00	32.78	0.003	-1.42E-03	73.0	91.6		237496
		27.02	0.003	-1.42E-03	73.0	91.6		
10	49.85	27.03	0.003	-1.48E-03	77.0	102.6		237496
11	49.30	26.96	0.004	-1.77E-03	91.9	148.3	257.1	237496
		26.96	0.004	-1.77E-03	-165.3	148.3		
12	49.10	29.45	0.004	-1.88E-03	-159.6	115.5		237496
13	49.00	30.70	0.005	-1.92E-03	-156.6	99.6		237496
		26.77	0.005	-1.92E-03	-156.6	99.6		
14	48.45	29.18	0.006	-2.06E-03	-141.2	17.0		237496
15	47.90	32.53	0.007	-2.01E-03	-124.3	-56.7		237496
		53.83	0.007	-2.01E-03	-124.3	-56.7		
16	47.31	60.58	0.008	-1.79E-03	-90.6	-120.8		237496
17	46.72	66.39	0.009	-1.44E-03	-53.3	-163.9		237496
18	46.50	68.80	0.009	-1.28E-03	-38.1	-174.6		237496
19	46.47	69.08	0.009	-1.26E-03	-36.4	-175.5		237496
20	46.04	57.15	0.010	-9.35E-04	-8.8	-185.4		237496
21	45.60	45.79	0.010	-5.94E-04	13.7	-184.8		237496
22	45.00	31.18	0.010	-1.46E-04	36.8	-170.0		237496
23	44.40	17.70	0.010	2.50E-04	51.5	-143.9		237496
24	43.80	5.22	0.010	5.73E-04	58.3	-111.6		237496

Run ID. J14373 Wallap 1_SLS Revised Nov 2016
 4 Frogna! Rise
 Rear Retaining Wall

| Sheet No.
 | Date:28-11-2016
 | Checked :

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Stage No.11 Excavate to elevation 46.47 on PASSIVE side

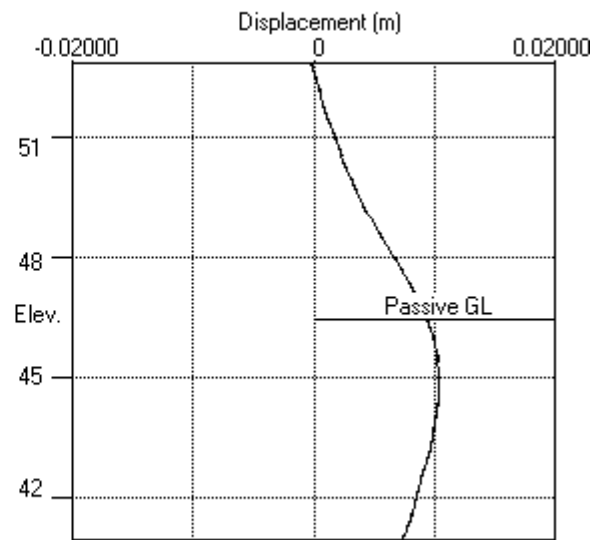
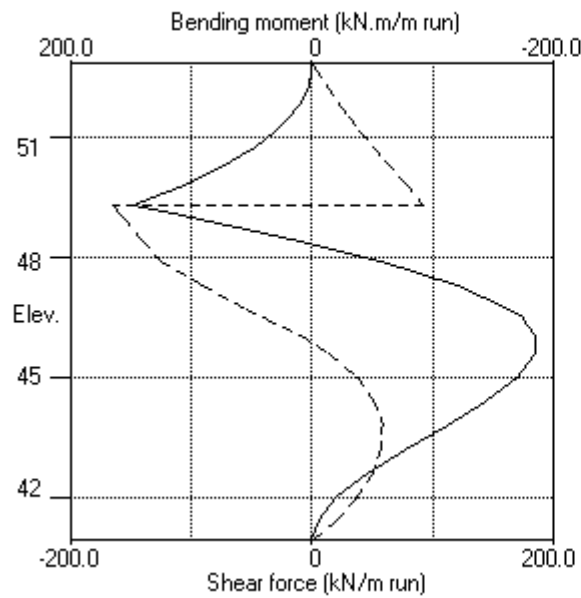
Node no.	Y coord	Nett pressure kN/m2	Wall disp. m	Wall rotation rad.	Shear force kN/m	Bending moment kN.m/m	Strut forces kN/m	EI of wall kN.m2/m
25	43.20	-6.43	0.010	8.11E-04	58.0	-77.3		237496
26	42.60	-17.48	0.009	9.66E-04	50.8	-45.4		237496
27	42.00	-28.17	0.008	1.04E-03	37.1	-19.7		237496
28	41.50	-36.94	0.008	1.07E-03	20.8	-5.9		237496
29	41.00	-40.21	0.007	1.08E-03	1.6	0.0		---

At elev. 51.80 The strut is slack

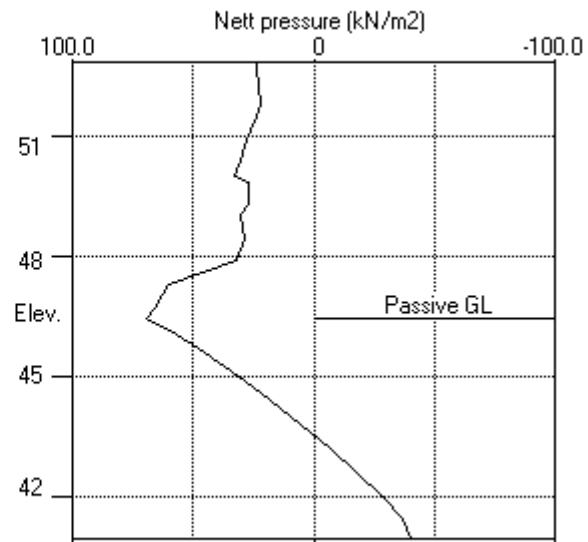
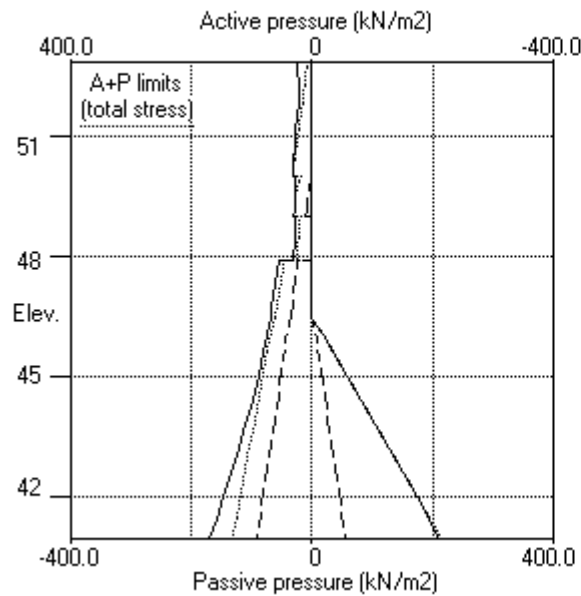
At elev. 49.30 Strut force = 771.4 kN/strut = 257.1 kN/m run

Units: kN, m

Stage No.11 Excav. to elev. 46.47 on PASSIVE side



Stage No.11 Excav. to elev. 46.47 on PASSIVE side



			FoS for toe elev. = 41.00		Toe elev. for FoS = 1.000		
Stage	--- G.L. ---		Strut	Factor	Moment	Toe	Wall
No.	Act.	Pass.	Elev.	of	equilib.	elev.	Penetr
				Safety	at elev.		-ation
1	52.80	52.80	Cant.	Conditions not suitable for FoS calc.			
2	52.80	52.80	Cant.	11.063	42.24	52.09	0.71
3	52.80	52.80		No analysis at this stage			
4	52.80	51.50	Cant.	4.074	42.00	48.56	2.94
5	52.80	51.50		No analysis at this stage			
6	52.80	51.50	51.80	Conditions not suitable for FoS calc.			
7	52.80	49.10	51.80	2.532	n/a	48.71	0.39
8	52.80	49.10		No analysis at this stage			
All remaining stages have more than one strut - FoS calculation n/a							

Units: kN,m

Summary of results**BENDING MOMENT and DISPLACEMENT ANALYSIS of Fully Embedded Wall****Analysis options**

Length of wall perpendicular to section = 1000.00m

Subgrade reaction model - Boussinesq Influence coefficients

Soil deformations are elastic until the active or passive limit is reached

Open Tension Crack analysis - No

Rigid boundaries: Active side 20.00 from wall

Passive side 20.00 from wall

Limit State: Serviceability Limit State

Calculated Bending Moments and Strut Forces have been multiplied by a factor of 1.35 to obtain values for structural design.

Bending moment, shear force and displacement envelopes

Node no.	Y coord	Displacement		Bending moment				Shear force			
				Calculated		Factored		Calculated		Factored	
		max. m	min. m	max. kN.m/m	min. kN.m/m	max. kN.m/m	min. kN.m/m	max. kN/m	min. kN/m	max. kN/m	min. kN/m
1	52.80	0.005	-0.001	0	0	0	0	0	0	0	0
2	52.70	0.004	-0.001	0	0	0	0	3	0	4	0
3	52.25	0.004	-0.000	4	-0	5	-0	14	0	19	0
4	51.80	0.004	0.000	12	-0	16	-0	25	-44	34	-59
5	51.50	0.004	0.000	20	-6	27	-8	33	-39	44	-52
6	51.13	0.004	0.000	34	-20	46	-27	43	-31	58	-42
7	50.75	0.004	0.000	51	-31	69	-41	53	-23	72	-31
8	50.38	0.004	0.000	73	-38	99	-51	65	-13	87	-18
9	50.00	0.004	0.000	99	-42	133	-56	77	-2	104	-3
10	49.85	0.004	0.000	110	-42	149	-57	81	-132	109	-179
11	49.30	0.004	0.000	148	-40	200	-53	92	-165	124	-223
12	49.10	0.005	0.000	116	-37	156	-50	19	-160	26	-215
13	49.00	0.005	0.000	100	-35	134	-47	22	-157	29	-211
14	48.45	0.006	0.000	48	-51	65	-68	11	-141	14	-191
15	47.90	0.007	0.000	45	-100	60	-136	3	-124	4	-168
16	47.31	0.008	0.000	40	-140	53	-189	13	-91	18	-122
17	46.72	0.009	0.000	33	-164	45	-221	17	-53	23	-72
18	46.50	0.009	0.000	31	-175	41	-236	17	-38	22	-51
19	46.47	0.009	0.000	30	-176	41	-237	17	-36	22	-49
20	46.04	0.010	0.000	25	-185	34	-250	14	-10	18	-13
21	45.60	0.010	0.000	21	-185	28	-249	17	-9	23	-12
22	45.00	0.010	0.000	15	-170	20	-229	37	-7	50	-10
23	44.40	0.010	0.000	10	-144	13	-194	51	-6	69	-8
24	43.80	0.010	0.000	6	-112	8	-151	58	-4	79	-5
25	43.20	0.010	0.000	5	-77	7	-104	58	-2	78	-3
26	42.60	0.009	0.000	3	-45	4	-61	51	-1	69	-2
27	42.00	0.008	0.000	2	-20	2	-27	37	-1	50	-1
28	41.50	0.008	0.000	0	-6	1	-8	21	0	28	0
29	41.00	0.007	0.000	0	-0	0	-0	2	0	2	0

Summary of results (continued)

Calculated Bending Moments and Strut Forces have been multiplied by a factor of 1.35 to obtain values for structural design.

Maximum and minimum bending moment and shear force at each stage

Stage no.	Bending moment						Shear force					
	Calculated			Factored			Calculated			Factored		
	max.	elev.	min.	max.	min.		max.	elev.	min.	max.	min.	
	kN.m/m		kN.m/m	kN.m/m			kN/m		kN/m	kN/m		kN/m
1	0	52.80	0	52.80	0	0	0	52.80	0	52.80	0	0
2	0	49.10	-0	50.38	0	-0	2	50.00	0	52.80	3	0
3	No calculation at this stage											
4	49	49.00	0	52.80	66	0	23	50.00	-10	46.47	31	-13
5	No calculation at this stage											
6	48	49.00	-0	41.50	65	-0	22	50.00	-11	47.31	30	-15
7	7	51.80	-42	49.85	9	-57	22	49.00	-44	51.80	29	-59
8	No calculation at this stage											
9	6	51.80	-20	46.72	9	-27	30	49.30	-28	51.80	40	-37
10	6	51.80	-20	46.72	9	-27	30	49.30	-28	51.80	40	-37
11	148	49.30	-185	46.04	200	-250	92	49.30	-165	49.30	124	-223
12	No calculation at this stage											
13	144	49.30	-179	46.04	194	-242	90	49.30	-163	49.30	121	-221
14	No calculation at this stage											
15	110	49.85	-173	46.04	149	-234	81	49.85	-132	49.85	109	-179
16	No calculation at this stage											
17	110	49.85	-173	46.04	149	-234	81	49.85	-132	49.85	109	-179

Maximum and minimum displacement at each stage

Stage no.	Displacement				Stage description
	maximum	elev.	minimum	elev.	
	m		m		
1	0.000	52.80	0.000	52.80	Change EI of wall to 1.0000E-05kN.m2/m run
2	0.004	52.80	0.000	52.80	Apply surcharge no.1 at elev. 52.80
3	Wall displacements reset to zero				Change EI of wall to 237496kN.m2/m run
4	0.005	52.80	0.000	52.80	Excav. to elev. 51.50 on PASSIVE side
5	No calculation at this stage				Install strut no.1 at elev. 51.80
6	0.004	52.80	0.000	52.80	Apply water pressure profile no.1
7	0.004	49.85	0.000	52.80	Excav. to elev. 49.10 on PASSIVE side
8	No calculation at this stage				Install strut no.2 at elev. 49.30
9	0.004	46.72	0.000	52.80	Apply water pressure profile no.2
10	0.004	46.72	0.000	52.80	Apply water pressure profile no.3
11	0.010	45.00	-0.000	52.80	Excav. to elev. 46.47 on PASSIVE side
12	No calculation at this stage				Install strut no.3 at elev. 46.72
13	0.010	45.00	0.000	52.80	Apply surcharge no.2 at elev. 46.47
14	No calculation at this stage				Install strut no.4 at elev. 49.85
15	0.010	45.00	-0.001	52.80	Remove strut no.2 at elev. 49.30
16	No calculation at this stage				Install strut no.5 at elev. 52.70
17	0.010	45.00	-0.001	52.80	Remove strut no.1 at elev. 51.80

Summary of results (continued)

Calculated Bending Moments and Strut Forces have been multiplied by a factor of 1.35 to obtain values for structural design.

Strut forces at each stage (horizontal components)

Stage no.	----- Strut no. 1 ----- at elev. 51.80			----- Strut no. 2 ----- at elev. 49.30			----- Strut no. 3 ----- at elev. 46.72		
	--Calculated-- kN per m run	Factored kN per strut	Factored kN per strut	--Calculated-- kN per m run	Factored kN per strut	Factored kN per strut	--Calculated-- kN per m run	Factored kN per strut	Factored kN per strut
6	1	3	4	---	---	---	---	---	---
7	59	177	238	---	---	---	---	---	---
9	43	128	173	34	103	139	---	---	---
10	43	128	172	34	103	139	---	---	---
11	slack	slack	slack	257	771	1041	---	---	---
13	slack	slack	slack	253	760	1026	slack	slack	slack
15	slack	slack	slack	---	---	---	28	28	37
17	---	---	---	---	---	---	28	28	37

Stage no.	----- Strut no. 4 ----- at elev. 49.85			----- Strut no. 5 ----- at elev. 52.70		
	--Calculated-- kN per m run	Factored kN per strut	Factored kN per strut	--Calculated-- kN per m run	Factored kN per strut	Factored kN per strut
15	213	213	287	---	---	---
17	213	213	287	slack	slack	slack

* Indicates that the total force shown is the sum of the force in the strut plus a force applied at the same elevation which may represent temperature load or other forces which are part of the strut load. Force components are listed in the detailed results for individual stages.

*** Convergence errors have occurred in at least one Construction Stage. The errors are cumulative, and the results of all stages must be inspected for significant out of balance moment or shear at the toe of the wall.

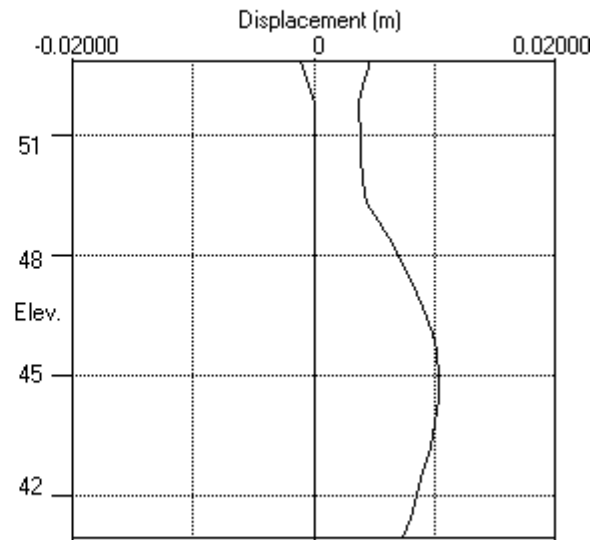
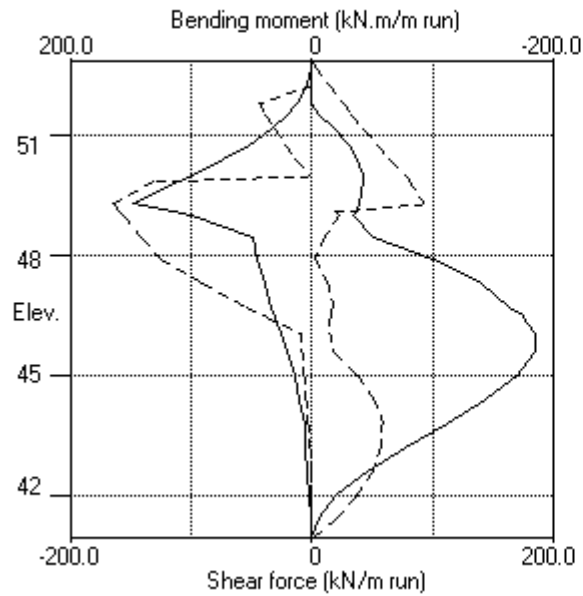
Failure of the iterative procedure to converge to an equilibrium solution may be due to a very high ratio of soil stiffness to wall stiffness. The data should be reviewed to see if realistic values have been specified

Out of balance shear forces.

Percentage Error	Interpretation
-----	-----
< 2%	Generally acceptable
2% to 4%	Use with caution
> 4%	Should not be used

Units: kN,m

Bending moment, shear force, displacement envelopes



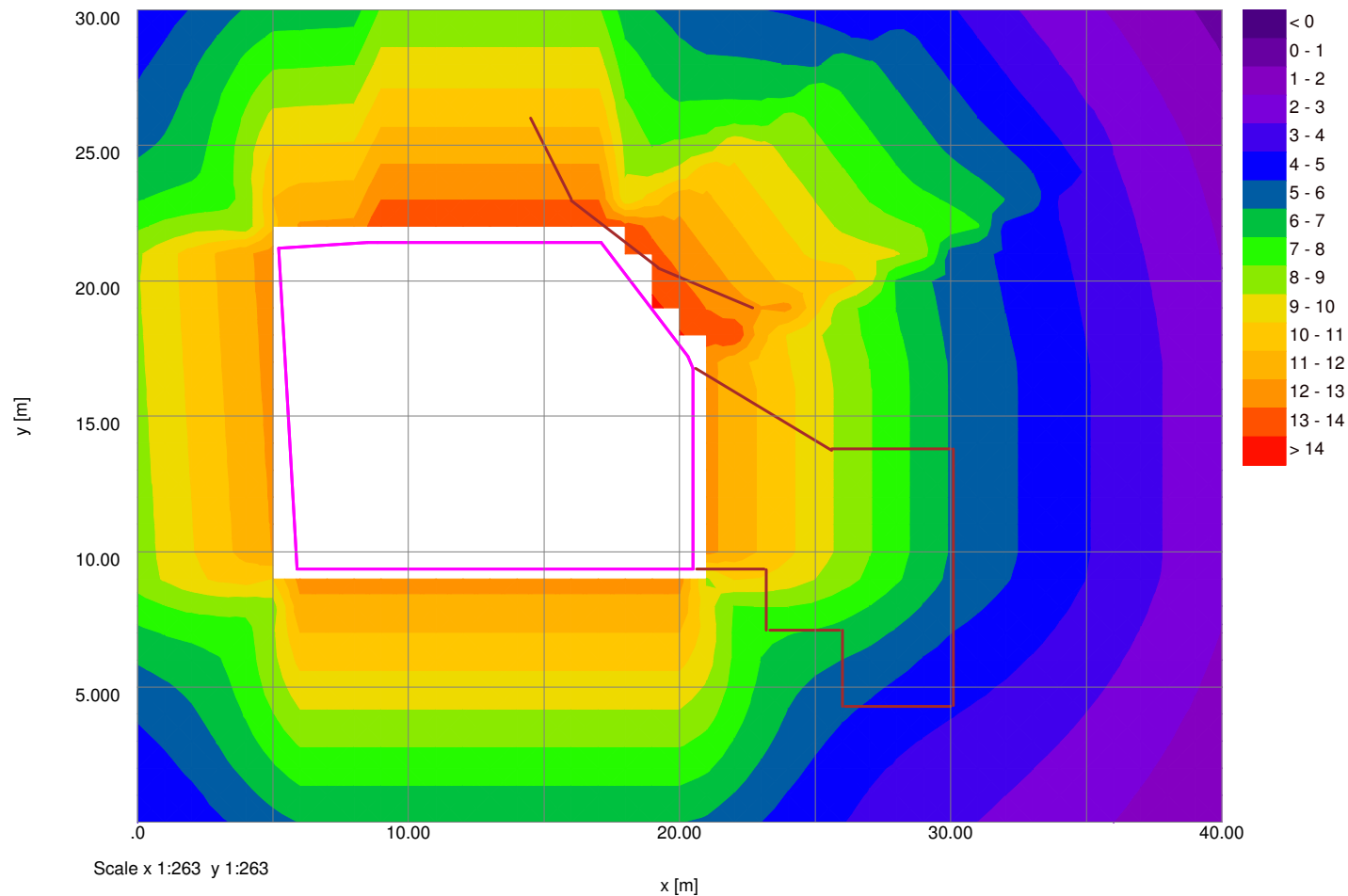
4 Frognal Rise

Underpinned and Piled Retaining Walls - Imported PDisp

Pile and Underpin Installation and Excavation Final

Job No.	Sheet No.	Rev.
J14373		
Drg. Ref.		
Made by	Date	Checked
MC	28-Nov-2016	

Horizontal Displacement Contours: Grid 2 (level 49.000m) Interval 1mm



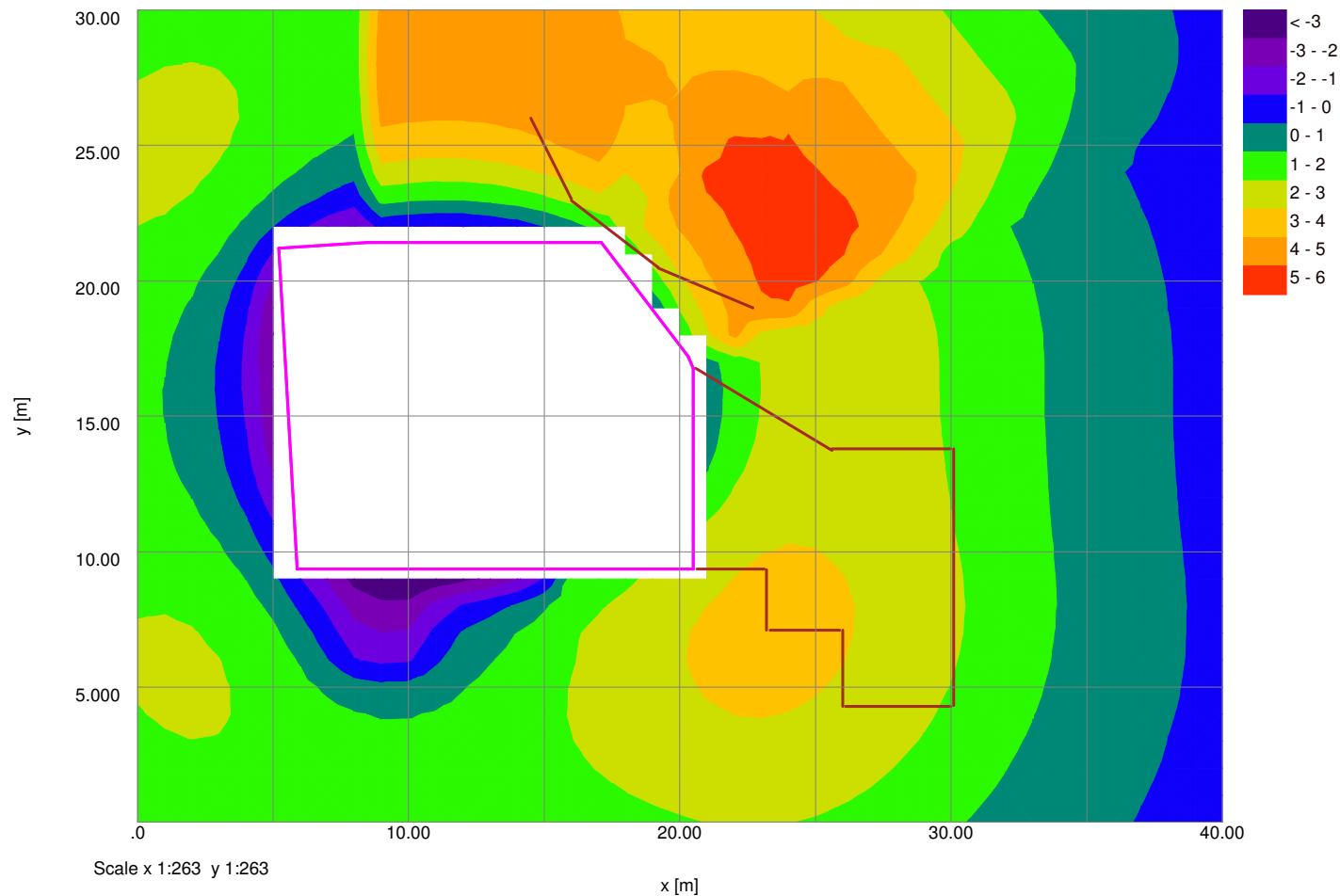
4 Frognal Rise

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Drg. Ref.		
Made by	Date	Checked
MC	28-Nov-2016	

Vertical Settlement Contours: Grid 2 (level 49.000m) (Interval 1mm)



4 Frogнал Rise

Underpinned and Piled Retaining Walls - Imported PDisp
Pile and Underpin Installation and Excavation Final

Problem Type

Problem Type : Tunnelling and Embedded Wall Excavations

Displacement Data

Type	Name	Direction of extrusion	Point/Line/Line for extrusion						No. of intervals across extrusion/line	Extrusion depth	No. of intervals along extrusion	Calculate	Surface type for tunnels
			First point			Second point							
			X	Y	Z(level)	X	Y	Z(level)					
			[m]	[m]	[m]	[m]	[m]	[m]					
Grid	Basement Grid	Global X	0.00000	0.00000	46.47500	-	30.00000	46.47500	30	40.00000	40	Yes	Surface
Grid	Foundation Level	Global X	0.00000	0.00000	49.00000	-	30.00000	49.00000	30	40.00000	40	Yes	Surface
Line	2 FR Rear 1	-	20.60000	16.75000	49.00000	25.60000	13.75000	49.00000	12	-	-	Yes	Surface
Line	2 FR Rear 2	-	25.60000	13.80000	49.00000	30.00000	13.80000	49.00000	9	-	-	Yes	Surface
Line	2 FR Side	-	30.10000	13.80000	49.00000	30.10000	4.30000	49.00000	18	-	-	Yes	Surface
Line	2 FR Front 1	-	30.00000	4.30000	49.00000	26.10000	4.30000	49.00000	8	-	-	Yes	Surface
Line	2 FR Front 2	-	26.00000	4.30000	49.00000	26.00000	7.10000	49.00000	6	-	-	Yes	Surface
Line	2 FR Front 3	-	25.90000	7.10000	49.00000	23.30000	7.10000	49.00000	6	-	-	Yes	Surface
Line	2 FR Front 4	-	23.20000	7.10000	49.00000	23.20000	9.35000	49.00000	5	-	-	Yes	Surface
Line	2 FR Front 5	-	23.10000	9.35000	49.00000	20.60000	9.35000	49.00000	6	-	-	Yes	Surface
Line	Windmill Hill 1	-	14.50000	26.00000	52.80000	16.00000	23.00000	52.80000	5	-	-	Yes	Surface
Line	Windmill Hill 2	-	16.00000	22.95000	52.80000	19.20000	20.50000	52.80000	5	-	-	Yes	Surface
Line	Windmill Hill 3	-	19.20000	20.45000	52.80000	22.70000	19.00000	52.80000	5	-	-	Yes	Surface

Vertical Ground Movement Curves

Curve Name: Installation of contiguous bored pile wall in stiff clay (CIRIA 580 Fig. 2.8(b))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Settlement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.040] [2.000,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 1

y Order: 0

Polynomial: z = -2.0E-2x + 4.0E-2

Coeff. of Determination: 1.0

Curve Name: Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Settlement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.050] [0.050,0.000,0.047] [0.100,0.000,0.043] [0.150,0.000,0.040] [0.200,0.000,0.037] [0.250,0.000,0.034] [0.300,0.000,0.031] [0.350,0.000,0.028] [0.400,0.000,0.025] [0.450,0.000,0.022] [0.500,0.000,0.020] [0.550,0.000,0.018] [0.600,0.000,0.016] [0.650,0.000,0.014] [0.700,0.000,0.012] [0.750,0.000,0.010] [0.800,0.000,0.008] [0.850,0.000,0.007] [0.900,0.000,0.006] [0.950,0.000,0.005] [1.000,0.000,0.004] [1.050,0.000,0.003] [1.100,0.000,0.003] [1.150,0.000,0.002] [1.200,0.000,0.002] [1.250,0.000,0.001] [1.300,0.000,0.001] [1.350,0.000,0.001] [1.400,0.000,0.001] [1.450,0.000,0.000] [1.500,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 4

y Order: 0

Polynomial: z = -1.2355E-2x⁴ + 3.4814E-2x³ - 2.8885E-3x² - 6.5618E-2x + 4.9987E-2

Coeff. of Determination: 1.0000

Curve Name: Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Settlement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.039] [0.100,0.000,0.049] [0.200,0.000,0.056] [0.300,0.000,0.062] [0.400,0.000,0.067] [0.500,0.000,0.070] [0.600,0.000,0.072] [0.700,0.000,0.073] [0.800,0.000,0.073] [0.900,0.000,0.072] [1.000,0.000,0.070] [1.100,0.000,0.068] [1.200,0.000,0.065] [1.300,0.000,0.061] [1.400,0.000,0.058] [1.500,0.000,0.054] [1.600,0.000,0.050] [1.700,0.000,0.046] [1.800,0.000,0.042] [1.900,0.000,0.038] [2.000,0.000,0.034] [2.100,0.000,0.029] [2.200,0.000,0.027] [2.300,0.000,0.025] [2.400,0.000,0.020] [2.500,0.000,0.017] [2.600,0.000,0.014] [2.700,0.000,0.012] [2.800,0.000,0.010] [2.900,0.000,0.008] [3.000,0.000,0.007] [3.100,0.000,0.005] [3.200,0.000,0.004] [3.300,0.000,0.004] [3.400,0.000,0.003] [3.500,0.000,0.002] [3.600,0.000,0.002] [3.700,0.000,0.002] [3.800,0.000,0.001] [3.900,0.000,0.001] [4.000,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 4

y Order: 0

Polynomial: z = -2.6455E-3x⁴ + 2.8495E-2x³ - 1.0051E-1x² + 1.0569E-1x + 3.8990E-2

Coeff. of Determination: 9.9991E-1

Curve Name: Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Settlement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.041] [0.050,0.000,0.039] [0.100,0.000,0.036] [0.150,0.000,0.034] [0.200,0.000,0.032] [0.250,0.000,0.030] [0.300,0.000,0.029] [0.350,0.000,0.027] [0.400,0.000,0.025] [0.450,0.000,0.023] [0.500,0.000,0.022] [0.550,0.000,0.020] [0.600,0.000,0.019] [0.650,0.000,0.018] [0.700,0.000,0.016] [0.750,0.000,0.015] [0.800,0.000,0.014] [0.850,0.000,0.013] [0.900,0.000,0.012] [0.950,0.000,0.010] [1.000,0.000,0.009] [1.050,0.000,0.008] [1.100,0.000,0.007] [1.150,0.000,0.006] [1.200,0.000,0.005] [1.250,0.000,0.004] [1.300,0.000,0.004] [1.350,0.000,0.003] [1.400,0.000,0.002] [1.450,0.000,0.001] [1.500,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 3

y Order: 0

Polynomial: z = -4.2486E-3x³ + 1.9096E-2x² - 4.6221E-2x + 4.0729E-2

Coeff. of Determination: 1.0000

Curve Name: Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Horizontal movement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.050] [1.500,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 1

y Order: 0

Polynomial: z = -3.33E-2x + 5.00E-2

Coeff. of Determination: 1.00

Curve Name: Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))

Coordinates: [Distance from wall / wall depth or max. excavation depth (x), Depth / wall depth or max. excavation depth (y), Horizontal movement / wall depth or max. excavation depth (z) (%)]
[0.000,0.000,0.150] [4.000,0.000,0.000]

Curve Fitting Method: Polynomial

x Order: 1

y Order: 0

Polynomial: z = -3.75E-2x + 1.50E-1

Coeff. of Determination: 1.00

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Made by MC	Date 28-Nov-2016	Checked

4 Frogнал Rise

Underpinned and Piled Retaining Walls - Imported PDisp
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Type	Name	Direction of extrusion	Point/Line/Line for extrusion	No. of intervals across extrusion/line	Extrusion depth	No. of intervals along extrusion	Calculate Surface type for tunnels		
Determination:									
Polygonal Excavations									
Excavation Name:			Pile Installation						
Surface level [m]:			52.800						
Contribution:			Positive						
Enabled:			Yes						
Surface movement curves which are selected are applied between surface and [m]:			41.000						
Corner	x	y	Base Level	Stiffened	Previous Side	Next Side			
	[m]	[m]	[m]		d p1 p2*	d p1 p2*			
					[m] [%] [%]	[m] [%] [%]			
1	8.8500	21.400	41.000	No	- - -	- - -			
2	17.100	21.400	41.000	No	- - -	- - -			
3	20.300	17.200	41.000	No	- - -	- - -			
4	20.500	16.750	46.475	No	- - -	- - -			
5	20.500	9.3500	46.475	No	- - -	- - -			
6	5.9000	9.3500	46.475	No	- - -	- - -			
7	5.2000	21.200	46.475	No	- - -	- - -			
8	8.5000	21.400	46.475	No	- - -	- - -			
Side	Corner 1		Corner 2		Ground Movement Curve				
	x	y	x	y	Vertical	Horizontal			
	[m]	[m]	[m]	[m]					
1	8.8500	21.400	17.100	21.400	Installation of contiguous bored pile wall in stiff clay (CIRIA 580 Fig. 2.8(b))	Installation of contiguous bored pile wall in stiff clay (CIRIA 580 Fig. 2.8(a))			
2	17.100	21.400	20.300	17.200	Installation of contiguous bored pile wall in stiff clay (CIRIA 580 Fig. 2.8(b))	Installation of contiguous bored pile wall in stiff clay (CIRIA 580 Fig. 2.8(a))			
3	20.300	17.200	20.500	16.750	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
4	20.500	16.750	20.500	9.3500	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
5	20.500	9.3500	5.9000	9.3500	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
6	5.9000	9.3500	5.2000	21.200	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
7	5.2000	21.200	8.5000	21.400	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
8	8.5000	21.400	8.8500	21.400	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(b))	Installation of planar diaphragm wall in stiff clay (CIRIA 580 Fig. 2.9(a))			
Excavation Name:			Excavation						
Surface level [m]:			52.800						
Contribution:			Positive						
Enabled:			Yes						
Surface movement curves which are selected are applied between surface and [m]:			46.475						
Corner	x	y	Base Level	Stiffened	Previous Side	Next Side			
	[m]	[m]	[m]		d p1 p2*	d p1 p2*			
					[m] [%] [%]	[m] [%] [%]			
1	8.8500	21.400	46.475	No	- - -	- - -			
2	17.100	21.400	46.475	Yes	0.0 67.000 25.000	0.0 67.000 25.000			
3	20.300	17.200	46.475	No	- - -	- - -			
4	20.500	16.750	46.475	No	- - -	- - -			
5	20.500	9.3500	46.475	No	- - -	- - -			
6	5.9000	9.3500	46.475	No	- - -	- - -			
7	5.2000	21.200	46.475	No	- - -	- - -			
8	8.5000	21.400	46.475	No	- - -	- - -			
Side	Corner 1		Corner 2		Ground Movement Curve				
	x	y	x	y	Vertical	Horizontal			
	[m]	[m]	[m]	[m]					
1	8.8500	21.400	17.100	21.400	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
2	17.100	21.400	20.300	17.200	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
3	20.300	17.200	20.500	16.750	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
4	20.500	16.750	20.500	9.3500	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
5	20.500	9.3500	5.9000	9.3500	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
6	5.9000	9.3500	5.2000	21.200	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
7	5.2000	21.200	8.5000	21.400	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
8	8.5000	21.400	8.8500	21.400	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(b))	Excavation in front of high stiffness wall in stiff clay (CIRIA 580 Fig. 2.11(a))			
Damage Category Strains									
Name	0 (Negligible) to	1 (Very Slight) to	2 (Slight) to	3 (Moderate) to	4 (Severe)				
Burland Strain Limits	1 (Very Slight)	2 (Slight)	3 (Moderate)	4 (Severe)					
	0.0	500.00E-6	750.00E-6	0.0015000					
Specific Structures - Geometry									
Structure Name	Sub-Structure Name	Displacement Line	Start Distance Along Line	End Distance Along Line	Vertical Offsets from Line for Vertical Movement Calculations	Vertical Displacement Limit Sensitivity	Damage Category Strains	Poisson's Ratio	E/G
			[m]	[m]	[m]	[mm]			
2 Frogнал Rise	2 FR Rear 1	2 FR Rear 1	0.00000	5.82500	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Rear 2	2 FR Rear 2	0.00000	4.39000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Side	2 FR Side	0.00000	9.49000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Front 1	2 FR Front 1	0.00000	3.89000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Front 2	2 FR Front 2	0.00000	2.79000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Front 3	2 FR Front 3	0.00000	2.59000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Front 4	2 FR Front 4	0.00000	2.24000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
2 Frogнал Rise	2 FR Front 5	2 FR Front 5	0.00000	2.49000	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
Windmill Hill	Windmill Hill 1	Windmill Hill 1	0.00000	3.35300	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
Windmill Hill	Windmill Hill 2	Windmill Hill 2	0.00000	4.02900	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
Windmill Hill	Windmill Hill 3	Windmill Hill 3	0.00000	3.78700	0.0	0.10000	Burland Strain Limits	0.20000	2.6000
Specific Structures - Bending Parameters									
Structure Name	Sub-Structure Name	Height	Default Properties	Hogging	Sagging				
				2nd Moment	Distance	Distance	2nd Moment	Distance	



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4 Frognal Rise

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Job No.	Sheet No.	Rev.
Drg. Ref.		
Made by	Date	Checked
MC	28-Nov-2016	

			of Area (per unit width)	of Bending Strain from N.A.	of N.A. from Edge of Beam in Tension	of Area (per unit width)	of Bending Strain from N.A.	of N.A. from Edge of Beam in Tension
2 Frognal Rise	2 FR Rear 1	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Rear 2	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Side	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Front 1	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Front 2	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Front 3	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Front 4	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
2 Frognal Rise	2 FR Front 5	8.0000 Yes	170.67	8.0000	8.0000	42.667	4.0000	4.0000
Windmill Hill	Windmill Hill 1	3.0000 Yes	9.0000	3.0000	3.0000	2.2500	1.5000	1.5000
Windmill Hill	Windmill Hill 2	3.0000 Yes	9.0000	3.0000	3.0000	2.2500	1.5000	1.5000
Windmill Hill	Windmill Hill 3	3.0000 Yes	9.0000	3.0000	3.0000	2.2500	1.5000	1.5000

Building Segment Combinations

Structure Name	Sub-Structure Name	Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Combined Segment
Windmill Hill	Windmill Hill 2	0.0	1	0.0	0.85601	Sagging	1
			2	0.85601	2.0513	Hogging	1
			3	2.9073	0.61610	Hogging	1
			4	3.5234	0.50564	None	1

Utility Strain Calculation Options

Neglect beneficial contribution of axial strains : No

Warnings

- 1 Multiple excavations have been specified. The displacements resulting from these excavations are calculated by summing the displacements resulting from each individual excavation. No account has been taken of the interactions between excavations (e.g. overlapping zones of influence or 'shielding' of one excavation by another).

Errors

None

Displacement and Strain Results

Type/No.	Coordinates			Displacements				Angle of Line		
Name	Dist.	x	y	z	x	y	z	Horizontal displacement along Line	Horizontal displacement perpendicular to line	to x Axis
Basement Grid	Grid 1	[m]	[m]	[m]	[mm]	[mm]	[mm]	[mm]	[mm]	[°]
		0.00000	0.00000	46.47500	1.9855	3.4699	1.3255	-	-	-
		1.00000	0.00000	46.47500	1.7623	3.8677	1.4268	-	-	-
		2.00000	0.00000	46.47500	1.4843	4.2892	1.4945	-	-	-
		3.00000	0.00000	46.47500	1.1551	4.7276	1.5262	-	-	-
		4.00000	0.00000	46.47500	0.78231	5.1721	1.5215	-	-	-
		5.00000	0.00000	46.47500	0.37996	5.6383	1.4882	-	-	-
		6.00000	0.00000	46.47500	0.0	6.0271	1.4194	-	-	-
		7.00000	0.00000	46.47500	0.0	6.0271	1.3454	-	-	-
		8.00000	0.00000	46.47500	0.0	6.0271	1.2895	-	-	-
		9.00000	0.00000	46.47500	0.0	6.0271	1.2550	-	-	-
		10.00000	0.00000	46.47500	0.0	6.0271	1.2437	-	-	-
		11.00000	0.00000	46.47500	0.0	6.0271	1.2553	-	-	-
		12.00000	0.00000	46.47500	0.0	6.0271	1.2877	-	-	-
		13.00000	0.00000	46.47500	0.0	6.0271	1.3378	-	-	-
		14.00000	0.00000	46.47500	0.0	6.0271	1.4020	-	-	-
		15.00000	0.00000	46.47500	0.0	6.0271	1.4769	-	-	-
		16.00000	0.00000	46.47500	0.0	6.0271	1.5596	-	-	-
		17.00000	0.00000	46.47500	0.0	6.0271	1.6473	-	-	-
		18.00000	0.00000	46.47500	0.0	6.0271	1.7380	-	-	-
		19.00000	0.00000	46.47500	0.0	6.0271	1.8299	-	-	-
		20.00000	0.00000	46.47500	0.0	6.0271	1.9219	-	-	-
		21.00000	0.00000	46.47500	-0.20467	5.8130	2.0069	-	-	-
		22.00000	0.00000	46.47500	-0.60178	5.3406	2.0485	-	-	-
		23.00000	0.00000	46.47500	-0.97436	4.8837	2.0512	-	-	-
		24.00000	0.00000	46.47500	-1.3097	4.4339	2.0133	-	-	-
		25.00000	0.00000	46.47500	-1.5981	3.9982	1.9349	-	-	-
		26.00000	0.00000	46.47500	-1.8346	3.5850	1.8196	-	-	-
		27.00000	0.00000	46.47500	-2.0177	3.1995	1.6722	-	-	-
		28.00000	0.00000	46.47500	-2.1487	2.8439	1.4989	-	-	-
		29.00000	0.00000	46.47500	-2.2306	2.5183	1.3070	-	-	-
		30.00000	0.00000	46.47500	-2.2672	2.2217	1.1043	-	-	-
		31.00000	0.00000	46.47500	-2.2629	1.9523	0.89873	-	-	-
		32.00000	0.00000	46.47500	-2.2218	1.7077	0.69794	-	-	-
		33.00000	0.00000	46.47500	-2.1481	1.4857	0.50893	-	-	-
		34.00000	0.00000	46.47500	-2.0455	1.2839	0.33770	-	-	-
		35.00000	0.00000	46.47500	-1.9173	1.1002	0.18893	-	-	-
		36.00000	0.00000	46.47500	-1.7666	0.93276	0.065770	-	-	-
		37.00000	0.00000	46.47500	-1.5959	0.77970	-0.030330	-	-	-
		38.00000	0.00000	46.47500	-1.4076	0.63948	-0.099799	-	-	-
		39.00000	0.00000	46.47500	-1.2036	0.51071	-0.14506	-	-	-
		40.00000	0.00000	46.47500	-0.98567	0.39218	-0.17061	-	-	-
		0.00000	1.00000	46.47500	2.2963	3.4888	1.5262	-	-	-
		1.00000	1.00000	46.47500	2.0523	3.9222	1.6224	-	-	-
		2.00000	1.00000	46.47500	1.7664	4.4570	1.6946	-	-	-
		3.00000	1.00000	46.47500	1.4102	5.0595	1.7275	-	-	-
		4.00000	1.00000	46.47500	0.97273	5.6673	1.7141	-	-	-
		5.00000	1.00000	46.47500	0.47510	6.2533	1.6585	-	-	-
		6.00000	1.00000	46.47500	0.0	6.7354	1.5657	-	-	-
		7.00000	1.00000	46.47500	0.0	6.7354	1.4701	-	-	-
		8.00000	1.00000	46.47500	0.0	6.7354	1.3979	-	-	-
		9.00000	1.00000	46.47500	0.0	6.7354	1.3545	-	-	-
		10.00000	1.00000	46.47500	0.0	6.7354	1.3426	-	-	-
		11.00000	1.00000	46.47500	0.0	6.7354	1.3613	-	-	-
		12.00000	1.00000	46.47500	0.0	6.7354	1.4070	-	-	-
		13.00000	1.00000	46.47500	0.0	6.7354	1.4742	-	-	-
		14.00000	1.00000	46.47500	0.0	6.7354	1.5576	-	-	-
		15.00000	1.00000	46.47500	0.0	6.7354	1.6524	-	-	-
		16.00000	1.00000	46.47500	0.0	6.7354	1.7548	-	-	-
		17.00000	1.00000	46.47500	0.0	6.7354	1.8616	-	-	-
		18.00000	1.00000	46.47500	0.0	6.7354	1.9704	-	-	-
		19.00000	1.00000	46.47500	0.0	6.7354	2.0793	-	-	-
		20.00000	1.00000	46.47500	0.0	6.7354	2.1869	-	-	-
		21.00000	1.00000	46.47500	-0.25605	6.4688	2.2864	-	-	-
		22.00000	1.00000	46.47500	-0.75144	5.8893	2.3413	-	-	-
		23.00000	1.00000	46.47500	-1.1993	5.2767	2.3455	-	-	-
		24.00000	1.00000	46.47500	-1.5760	4.6609	2.2990	-	-	-
		25.00000	1.00000	46.47500	-1.8664	4.0648	2.2005	-	-	-
		26.00000	1.00000	46.47500	-2.1277	3.6103	2.0825	-	-	-
		27.00000	1.00000	46.47500	-2.3242	3.1952	1.9280	-	-	-
		28.00000	1.00000	46.47500	-2.4593	2.8193	1.7428	-	-	-
		29.00000	1.00000	46.47500	-2.5381	2.4812	1.5345	-	-	-
		30.00000	1.00000	46.47500	-2.5666	2.1779	1.3120	-	-	-
		31.00000	1.00000	46.47500	-2.5506	1.9061	1.0839	-	-	-
		32.00000	1.00000	46.47500	-2.4957	1.6625	0.85896	-	-	-
		33.00000	1.00000	46.47500	-2.4068	1.4436	0.64512	-	-	-

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Job No.	Sheet No.	Rev.
Drg. Ref.		
Made by MC	Date 28-Nov-2016	Checked

4 Frogнал Rise

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Pile and Underpin Installation and Excavation Final

Type/No.		Coordinates			Displacements			Angle of Line		
Name	Dist.	x	y	z	x	y	z	Horizontal displacement	Horizontal displacement	to x Axis
		3.00000	12.00000	49.00000	10.691	0.63156	0.50199	-	-	*
		4.00000	12.00000	49.00000	11.397	0.67326	-0.28317	-	-	*
		5.00000	12.00000	49.00000	12.103	0.71496	-1.5776	-	-	*
		6.00000	12.00000	49.00000				Point lies within an excavation.		*
		7.00000	12.00000	49.00000				Point lies within an excavation.		*
		8.00000	12.00000	49.00000				Point lies within an excavation.		*
		9.00000	12.00000	49.00000				Point lies within an excavation.		*
		10.00000	12.00000	49.00000				Point lies within an excavation.		*
		11.00000	12.00000	49.00000				Point lies within an excavation.		*
		12.00000	12.00000	49.00000				Point lies within an excavation.		*
		13.00000	12.00000	49.00000				Point lies within an excavation.		*
		14.00000	12.00000	49.00000				Point lies within an excavation.		*
		15.00000	12.00000	49.00000				Point lies within an excavation.		*
		16.00000	12.00000	49.00000				Point lies within an excavation.		*
		17.00000	12.00000	49.00000				Point lies within an excavation.		*
		18.00000	12.00000	49.00000				Point lies within an excavation.		*
		19.00000	12.00000	49.00000				Point lies within an excavation.		*
		20.00000	12.00000	49.00000				Point lies within an excavation.		*
		21.00000	12.00000	49.00000	-12.296	0.0	1.4732	-	-	*
		22.00000	12.00000	49.00000	-11.587	0.0	2.1324	-	-	*
		23.00000	12.00000	49.00000	-10.879	0.0	2.5192	-	-	*
		24.00000	12.00000	49.00000	-10.171	0.0	2.7037	-	-	*
		25.00000	12.00000	49.00000	-9.4625	0.0	2.7481	-	-	*
		26.00000	12.00000	49.00000	-8.7542	0.0	2.6998	-	-	*
		27.00000	12.00000	49.00000	-8.0458	0.0	2.5906	-	-	*
		28.00000	12.00000	49.00000	-7.3375	0.0	2.4364	-	-	*
		29.00000	12.00000	49.00000	-6.6292	0.0	2.2380	-	-	*
		30.00000	12.00000	49.00000	-5.9250	0.0	1.9808	-	-	*
		31.00000	12.00000	49.00000	-5.5500	0.0	1.7277	-	-	*
		32.00000	12.00000	49.00000	-5.1750	0.0	1.4496	-	-	*
		33.00000	12.00000	49.00000	-4.8000	0.0	1.1640	-	-	*
		34.00000	12.00000	49.00000	-4.4250	0.0	0.88564	-	-	*
		35.00000	12.00000	49.00000	-4.0500	0.0	0.62623	-	-	*
		36.00000	12.00000	49.00000	-3.6750	0.0	0.39484	-	-	*
		37.00000	12.00000	49.00000	-3.3000	0.0	0.19777	-	-	*
		38.00000	12.00000	49.00000	-2.9250	0.0	0.038652	-	-	*
		39.00000	12.00000	49.00000	-2.5500	0.0	-0.081543	-	-	*
		40.00000	12.00000	49.00000	-2.1750	0.0	-0.16447	-	-	*
		0.00000	13.00000	49.00000	8.6155	0.50894	1.3197	-	-	*
		1.00000	13.00000	49.00000	9.3214	0.55063	1.1355	-	-	*
		2.00000	13.00000	49.00000	10.027	0.59233	0.80318	-	-	*
		3.00000	13.00000	49.00000	10.733	0.63403	0.25015	-	-	*
		4.00000	13.00000	49.00000	11.439	0.67572	-0.64526	-	-	*
		5.00000	13.00000	49.00000	12.145	0.71742	-2.1524	-	-	*
		6.00000	13.00000	49.00000				Point lies within an excavation.		*
		7.00000	13.00000	49.00000				Point lies within an excavation.		*
		8.00000	13.00000	49.00000				Point lies within an excavation.		*
		9.00000	13.00000	49.00000				Point lies within an excavation.		*
		10.00000	13.00000	49.00000				Point lies within an excavation.		*
		11.00000	13.00000	49.00000				Point lies within an excavation.		*
		12.00000	13.00000	49.00000				Point lies within an excavation.		*
		13.00000	13.00000	49.00000				Point lies within an excavation.		*
		14.00000	13.00000	49.00000				Point lies within an excavation.		*
		15.00000	13.00000	49.00000				Point lies within an excavation.		*
		16.00000	13.00000	49.00000				Point lies within an excavation.		*
		17.00000	13.00000	49.00000				Point lies within an excavation.		*
		18.00000	13.00000	49.00000				Point lies within an excavation.		*
		19.00000	13.00000	49.00000				Point lies within an excavation.		*
		20.00000	13.00000	49.00000				Point lies within an excavation.		*
		21.00000	13.00000	49.00000	-12.296	0.0	0.94182	-	-	*
		22.00000	13.00000	49.00000	-11.587	0.0	1.8190	-	-	*
		23.00000	13.00000	49.00000	-10.879	0.0	2.3083	-	-	*
		24.00000	13.00000	49.00000	-10.171	0.0	2.5522	-	-	*
		25.00000	13.00000	49.00000	-9.4625	0.0	2.6349	-	-	*
		26.00000	13.00000	49.00000	-8.7542	0.0	2.6130	-	-	*
		27.00000	13.00000	49.00000	-8.0458	0.0	2.5227	-	-	*
		28.00000	13.00000	49.00000	-7.3375	0.0	2.3824	-	-	*
		29.00000	13.00000	49.00000	-6.6292	0.0	2.1945	-	-	*
		30.00000	13.00000	49.00000	-5.9250	0.0	1.9453	-	-	*
		31.00000	13.00000	49.00000	-5.5500	0.0	1.6985	-	-	*
		32.00000	13.00000	49.00000	-5.1750	0.0	1.4254	-	-	*
		33.00000	13.00000	49.00000	-4.8000	0.0	1.1438	-	-	*
		34.00000	13.00000	49.00000	-4.4250	0.0	0.86860	-	-	*
		35.00000	13.00000	49.00000	-4.0500	0.0	0.61181	-	-	*
		36.00000	13.00000	49.00000	-3.6750	0.0	0.38257	-	-	*
		37.00000	13.00000	49.00000	-3.3000	0.0	0.18729	-	-	*
		38.00000	13.00000	49.00000	-2.9250	0.0	0.029658	-	-	*
		39.00000	13.00000	49.00000	-2.5500	0.0	-0.089289	-	-	*
		40.00000	13.00000	49.00000	-2.1750	0.0	-0.17117	-	-	*
		0.00000	14.00000	49.00000	8.6572	0.51140	1.2520	-	-	*
		1.00000	14.00000	49.00000	9.3631	0.55310	1.0394	-	-	*
		2.00000	14.00000	49.00000	10.069	0.59479	0.66725	-	-	*
		3.00000	14.00000	49.00000	10.775	0.63649	0.058349	-	-	*
		4.00000	14.00000	49.00000	11.481	0.67819	-0.91342	-	-	*
		5.00000	14.00000	49.00000	12.187	0.71988	-2.5164	-	-	*
		6.00000	14.00000	49.00000				Point lies within an excavation.		*
		7.00000	14.00000	49.00000				Point lies within an excavation.		*
		8.00000	14.00000	49.00000				Point lies within an excavation.		*
		9.00000	14.00000	49.00000				Point lies within an excavation.		*
		10.00000	14.00000	49.00000				Point lies within an excavation.		*
		11.00000	14.00000	49.00000				Point lies within an excavation.		*
		12.00000	14.00000	49.00000				Point lies within an excavation.		*
		13.00000	14.00000	49.00000				Point lies within an excavation.		*
		14.00000	14.00000	49.00000				Point lies within an excavation.		*
		15.00000	14.00000	49.00000				Point lies within an excavation.		*
		16.00000	14.00000	49.00000				Point lies within an excavation.		*
		17.00000	14.00000	49.00000				Point lies within an excavation.		*
		18.00000	14.00000	49.00000				Point lies within an excavation.		*
		19.00000	14.00000	49.00000				Point lies within an excavation.		*
		20.00000	14.00000	49.00000				Point lies within an excavation.		*
		21.00000	14.00000	49.00000	-12.296	0.0	0.59479	-	-	*
		22.00000	14.00000	49.00000	-11.587	0.0	1.5831	-	-	*
		23.00000	14.00000	49.00000	-10.879	0.0	2.1434	-	-	*
		24.00000	14.00000	49.00000	-10.171	0.0	2.4327	-	-	*
		25.00000	14.00000	49.00000	-9.4625	0.0	2.5458	-	-	*
		26.00000	14.00000	49.00000	-8.7542	0.0	2.5450	-	-	*
		27.00000	14.00000	49.00000	-8.0458	0.0	2.4697	-	-	*
		28.00000	14.00000	49.00000	-7.3375	0.0	2.3406	-	-	*
		29.00000	14.1							

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Xdisp1 Pile Installation and Excavation November 2016 with Imported Movements from PDisp.xdd
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Job No.	Sheet No.	Rev.
Drg. Ref.		
Made by MC	Date 28-Nov-2016	Checked

4 Frognal Rise

Underpinned and Piled Retaining Walls - Imported PDisp
Pile and Underpin Installation and Excavation Final

Type/No.	Coordinates	Displacements	Angle of Line
Name	Dist. x y z x y z	Horizontal displacement	Horizontal displacement to x Axis
2 FR Front Line 7 4	23.20000 7.10000 49.00000	-5.6667 4.4938 3.3578	4.4938 5.6667 90.000 *
	0.45000 23.20000 7.55000 49.00000	-6.4766 3.8749 3.3494	3.8749 6.4766 90.000 *
	0.90000 23.20000 8.00000 49.00000	-7.4090 3.1027 3.3226	3.1027 7.4090 90.000 *
	1.3500 23.20000 8.45000 49.00000	-8.4558 2.1782 3.2774	2.1782 8.4558 90.000 *
	1.8000 23.20000 8.90000 49.00000	-9.5850 1.1261 3.2143	1.1261 9.5850 90.000 *
	2.2500 23.20000 9.35000 49.00000	-10.737 0.0 3.1339	0.0 10.737 90.000 *
2 FR Front Line 8 5	23.10000 9.35000 49.00000	-10.808 0.0 3.1253	10.808 0.0 180.00 *
	0.41667 22.68333 9.35000 49.00000	-11.103 0.0 3.0748	11.103 0.0 180.00 *
	0.83333 22.26667 9.35000 49.00000	-11.399 0.0 2.9991	11.399 0.0 180.00 *
	1.2500 21.85000 9.35000 49.00000	-11.694 0.0 2.8973	11.694 0.0 180.00 *
	1.6667 21.43333 9.35000 49.00000	-11.989 0.0 2.7709	11.989 0.0 180.00 *
	2.0833 21.01667 9.35000 49.00000	-12.284 0.0 2.6285	12.284 0.0 180.00 *
	2.5000 20.60000 9.35000 49.00000	-12.579 0.0 2.5035	12.579 0.0 180.00 *
Windmill Hill 1 Line 9	14.50000 26.00000 52.80000	0.0 -10.755 4.2960	9.6196 -4.8098 296.57 *
	0.67082 14.80000 25.40000 52.80000	0.0 -11.184 4.0699	10.003 -5.0017 296.57 *
	1.3416 15.10000 24.80000 52.80000	0.0 -11.622 3.7609	10.395 -5.1975 296.57 *
	2.0125 15.40000 24.20000 52.80000	0.0 -12.069 3.3607	10.795 -5.3976 296.57 *
	2.6833 15.70000 23.60000 52.80000	0.0 -12.527 2.8604	11.204 -5.6021 296.57 *
	3.3541 16.00000 23.00000 52.80000	0.0 -12.994 2.2499	11.622 -5.8112 296.57 *
Windmill Hill 2 Line 10	16.00000 22.95000 52.80000	0.0 -13.034 2.1816	7.9232 -10.349 322.56 *
	0.80604 16.64000 22.46000 52.80000	0.0 -13.424 1.8195	8.1605 -10.659 322.56 *
	1.6121 17.28000 21.97000 52.80000	-2.9330 -9.9430 0.71327	3.7157 -9.6778 322.56 *
	2.4181 17.92000 21.48000 52.80000	-8.3117 1.6460 -3.6091	-13.231 322.56 *
	3.2242 18.56000 20.99000 52.80000	-10.772 -8.2073 1.9320	-3.5638 -13.065 322.56 *
	4.0302 19.20000 20.50000 52.80000	-10.636 -8.1038 2.2630	-3.5189 -12.900 322.56 *
Windmill Hill 3 Line 11	19.20000 20.45000 52.80000	-10.656 -8.1186 2.2162	-6.7370 -11.579 337.50 *
	0.75769 19.90000 20.16000 52.80000	-10.414 -7.9345 2.8272	-6.5842 -11.316 337.50 *
	1.5154 20.60000 19.87000 52.80000	-10.176 -7.7530 3.3969	-6.4336 -11.057 337.50 *
	2.2731 21.30000 19.58000 52.80000	-9.9410 -7.5741 3.9108	-6.2851 -10.802 337.50 *
	3.0304 22.00000 19.29000 52.80000	-9.7094 -7.3976 4.3642	-6.1387 -10.551 337.50 *
	3.7885 22.70000 19.00000 52.80000	-9.5668 -7.1638 4.7532	-6.0965 -10.280 337.50 *

* Result includes imported displacement(s).

Specific Building Damage Results - Horizontal Displacements

Structure: 2 Frognal Rise | Sub-structure: 2 FR Rear 1

Dist.	Coordinates	Displacements	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line
x y z x y z	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line		
[m] [m] [m] [m] [mm] [mm] [mm] [mm]				
0.0 20.60000 16.75000 49.00000 -12.579 0.0	-10.787	-6.4719 d		
0.48591 21.01667 16.50000 49.00000 -12.284 0.0	-10.533	-6.3201 d		
0.97193 21.43333 16.25000 49.00000 -11.989 0.0	-10.280	-6.1682 d		
1.4577 21.85000 16.00000 49.00000 -11.694 0.0	-10.027	-6.0164 d		
1.9437 22.26667 15.75000 49.00000 -11.399 0.0	-9.7742	-5.8645 d		
2.4296 22.68333 15.50000 49.00000 -11.103 0.0	-9.5211	-5.7127 d		
2.9155 23.10000 15.25000 49.00000 -10.808 0.0	-9.2681	-5.5608 d		
3.4014 23.51667 15.00000 49.00000 -10.513 0.0	-9.0150	-5.4090 d		
3.8873 23.93333 14.75000 49.00000 -10.218 0.0	-8.7619	-5.2571 d		
4.3732 24.35000 14.50000 49.00000 -9.9229 0.0	-8.5088	-5.1053 d		
4.8591 24.76667 14.25000 49.00000 -9.6278 0.0	-8.2558	-4.9535 d		
5.3450 25.18333 14.00000 49.00000 -9.3326 0.0	-8.0027	-4.8016 d		
5.8310 25.60000 13.75000 49.00000 -9.0375 0.0	-7.7496	-4.6498 d		

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Rear 2

Dist.	Coordinates	Displacements	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line
x y z x y z	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line		
[m] [m] [m] [m] [mm] [mm] [mm] [mm]				
0.0 25.60000 13.80000 49.00000 -9.0375 0.0	-9.0375	0.0 d		
0.48889 26.08889 13.80000 49.00000 -8.6912 0.0	-8.6912	0.0 d		
0.97778 26.57778 13.80000 49.00000 -8.3449 0.0	-8.3449	0.0 d		
1.4667 27.06667 13.80000 49.00000 -7.9986 0.0	-7.9986	0.0 d		
1.9556 27.55556 13.80000 49.00000 -7.6523 0.0	-7.6523	0.0 d		
2.4444 28.04444 13.80000 49.00000 -7.3060 0.0	-7.3060	0.0 d		
2.9333 28.53333 13.80000 49.00000 -6.9597 0.0	-6.9597	0.0 d		
3.4222 29.02222 13.80000 49.00000 -6.6134 0.0	-6.6134	0.0 d		
3.9111 29.51111 13.80000 49.00000 -6.2671 0.0	-6.2671	0.0 d		
4.4000 30.00000 13.80000 49.00000 -5.9250 0.0	-5.9250	0.0 d		

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Side

Dist.	Coordinates	Displacements	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line
x y z x y z	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line		
[m] [m] [m] [m] [mm] [mm] [mm] [mm]				
0.0 30.10000 13.80000 49.00000 -5.8875 0.0	0.0	-5.8875 d		
0.52778 30.10000 13.27222 49.00000 -5.8875 0.0	0.0	-5.8875 d		
1.0556 30.10000 12.74444 49.00000 -5.8875 0.0	0.0	-5.8875 d		
1.5833 30.10000 12.21667 49.00000 -5.8875 0.0	0.0	-5.8875 d		
2.1111 30.10000 11.68889 49.00000 -5.8875 0.0	0.0	-5.8875 d		
2.6389 30.10000 11.16111 49.00000 -5.8875 0.0	0.0	-5.8875 d		
3.1667 30.10000 10.63333 49.00000 -5.8875 0.0	0.0	-5.8875 d		
3.6944 30.10000 10.10556 49.00000 -5.8875 0.0	0.0	-5.8875 d		
4.2222 30.10000 9.57778 49.00000 -5.8875 0.0	0.0	-5.8875 d		
4.7500 30.10000 9.05000 49.00000 -5.7687 0.11706	-0.11706	-5.7687 d		
5.2778 30.10000 8.52222 49.00000 -5.5525 0.32166	-0.32166	-5.5525 d		
5.8056 30.10000 7.99444 49.00000 -5.3292 0.52258	-0.52258	-5.3292 d		
6.3333 30.10000 7.46667 49.00000 -5.1013 0.71762	-0.71762	-5.1013 d		
6.8611 30.10000 6.93889 49.00000 -4.8709 0.90477	-0.90477	-4.8709 d		
7.3889 30.10000 6.41111 49.00000 -4.6403 1.0823	-1.0823	-4.6403 d		
7.9167 30.10000 5.88333 49.00000 -4.4113 1.2487	-1.2487	-4.4113 d		
8.4444 30.10000 5.35556 49.00000 -4.1855 1.4028	-1.4028	-4.1855 d		
8.9722 30.10000 4.82778 49.00000 -3.9644 1.5437	-1.5437	-3.9644 d		
9.5000 30.10000 4.30000 49.00000 -3.7489 1.6709	-1.6709	-3.7489 d		

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 1

Dist.	Coordinates	Displacements	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line
x y z x y z	Horizontal displacement along the Line	Horizontal displacement perpendicular to Line		
[m] [m] [m] [m] [mm] [mm] [mm] [mm]				
0.0 30.00000 4.30000 49.00000 -3.7568 1.6961	3.7568	-1.6961 d		
0.48750 29.51250 4.30000 49.00000 -3.7882 1.8252	3.7882	-1.8252 d		
0.97500 29.02500 4.30000 49.00000 -3.8067 1.9651	3.8067	-1.9651 d		
1.4625 28.53750 4.30000 49.00000 -3.8109 2.1170	3.8109	-2.1170 d		
1.9500 28.05000 4.30000 49.00000 -3.8831 2.3329	3.8831	-2.3329 d		
2.4375 27.56250 4.30000 49.00000 -3.9314 2.5687	3.9314	-2.5687 d		
2.9250 27.07500 4.30000 49.00000 -3.9516 2.8260	3.9516	-2.8260 d		
3.4125 26.58750 4.30000 49.00000 -3.9405 3.1070	3.9405	-3.1070 d		
3.9000 26.10000 4.30000 49.00000 -3.8944 3.4143	3.8944	-3.4143 d		

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 2

Underpinned and Piled Retaining Walls - Imported PDisp Pile and Underpin Installation and Excavation Final

Dist.	Coordinates		Displacements		
[m]	x [m]	y [m]	z [m]	z [mm]	
Vertical Offset 1	0.0	20.60000	16.75000	49.00000	-0.15133 d
	0.48591	21.01667	16.50000	49.00000	0.42108 d
	0.97183	21.43333	16.25000	49.00000	0.89237 d
	1.4577	21.85000	16.00000	49.00000	1.2795 d
	1.9436	22.26667	15.75000	49.00000	1.692 d
	2.4296	22.68333	15.50000	49.00000	1.8580 d
	2.9155	23.10000	15.25000	49.00000	2.0666 d
	3.4014	23.51667	15.00000	49.00000	2.2306 d



GEA LIMITED (GEOTECHNICAL & ENV ASSOC) **J14373**

Job No.	Sheet No.	Rev.
Drg. Ref.		
Made by MC	Date 28-Nov-2016	Checked

4 Frognal Rise

Underpinned and Piled Retaining Walls - Imported PDisp
Pile and Underpin Installation and Excavation Final

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
3.8873	23.93333	14.75000	49.00000	2.3561 d
4.3732	24.35000	14.50000	49.00000	2.4485 d
4.8591	24.76667	14.25000	49.00000	2.5126 d
5.3450	25.18333	14.00000	49.00000	2.5528 d
5.8310	25.60000	13.75000	49.00000	2.5727 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Rear 2

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	30.10000	13.80000	49.00000	2.5692 d
0.48889	26.08889	13.80000	49.00000	2.5525 d
0.97778	26.57778	13.80000	49.00000	2.5191 d
1.4667	27.06667	13.80000	49.00000	2.4718 d
1.9556	27.55556	13.80000	49.00000	2.4121 d
2.4444	28.04444	13.80000	49.00000	2.3409 d
2.9333	28.53333	13.80000	49.00000	2.2580 d
3.4222	29.02222	13.80000	49.00000	2.1621 d
3.9111	29.51111	13.80000	49.00000	2.0514 d
4.4000	30.00000	13.80000	49.00000	1.9229 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Side

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	30.10000	13.80000	49.00000	1.9004 d
0.52778	30.10000	13.27222	49.00000	1.9143 d
1.0556	30.10000	12.74444	49.00000	1.9306 d
1.5833	30.10000	12.21667	49.00000	1.9490 d
2.1111	30.10000	11.68889	49.00000	1.9695 d
2.6389	30.10000	11.16111	49.00000	1.9918 d
3.1667	30.10000	10.63333	49.00000	2.0157 d
3.6944	30.10000	10.10556	49.00000	2.0411 d
4.2222	30.10000	9.57778	49.00000	2.0677 d
4.7500	30.10000	9.05000	49.00000	2.0935 d
5.2778	30.10000	8.52222	49.00000	2.1099 d
5.8056	30.10000	7.99444	49.00000	2.1155 d
6.3333	30.10000	7.46667	49.00000	2.1104 d
6.8611	30.10000	6.93889	49.00000	2.0946 d
7.3889	30.10000	6.41111	49.00000	2.0682 d
7.9167	30.10000	5.88333	49.00000	2.0314 d
8.4444	30.10000	5.35556	49.00000	1.9844 d
8.9722	30.10000	4.82778	49.00000	1.9277 d
9.5000	30.10000	4.30000	49.00000	1.8619 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 1

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	30.00000	4.30000	49.00000	1.8891 d
0.48750	29.51250	4.30000	49.00000	2.0191 d
0.97500	29.02500	4.30000	49.00000	2.1433 d
1.4625	28.53750	4.30000	49.00000	2.2600 d
1.9500	28.05000	4.30000	49.00000	2.3904 d
2.4375	27.56250	4.30000	49.00000	2.5065 d
2.9250	27.07500	4.30000	49.00000	2.6107 d
3.4125	26.58750	4.30000	49.00000	2.7044 d
3.9000	26.10000	4.30000	49.00000	2.7884 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 2

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	26.00000	4.30000	49.00000	2.8044 d
0.46667	26.00000	4.76667	49.00000	2.8735 d
0.93333	26.00000	5.23333	49.00000	2.9340 d
1.4000	26.00000	5.70000	49.00000	2.9851 d
1.8667	26.00000	6.16667	49.00000	3.0262 d
2.3333	26.00000	6.63333	49.00000	3.0566 d
2.8000	26.00000	7.10000	49.00000	3.0753 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 3

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	25.90000	7.10000	49.00000	3.0923 d
0.43333	25.46667	7.10000	49.00000	3.1616 d
0.86667	25.03333	7.10000	49.00000	3.2231 d
1.3000	24.60000	7.10000	49.00000	3.2751 d
1.7333	24.16667	7.10000	49.00000	3.3160 d
2.1667	23.73333	7.10000	49.00000	3.3440 d
2.6000	23.30000	7.10000	49.00000	3.3570 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 4

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	23.20000	7.10000	49.00000	3.3578 d
0.45000	23.20000	7.55000	49.00000	3.3494 d
0.90000	23.20000	8.00000	49.00000	3.3226 d
1.3500	23.20000	8.45000	49.00000	3.2774 d
1.8000	23.20000	8.90000	49.00000	3.2143 d
2.2500	23.20000	9.35000	49.00000	3.1339 d

d - Displacements include imported displacements.

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 5

Dist.	Coordinates		Displacements	
	x	y	z	z
[m]	[m]	[m]	[m]	[mm]
Vertical Offset 1				
0.0	23.10000	9.35000	49.00000	3.1253 d
0.41667	22.68333	9.35000	49.00000	3.0748 d
0.83333	22.26667	9.35000	49.00000	3.0991 d
1.2500	21.85000	9.35000	49.00000	2.8973 d
1.6667	21.43333	9.35000	49.00000	2.7709 d
2.0833	21.01667	9.35000	49.00000	2.6285 d

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4 Frognal Rise

Underpinned and Piled Retaining Walls - Imported PDisp
Pile and Underpin Installation and Excavation Final

Dist.	Coordinates		Displacements	
[m]	x [m]	y [m]	z [m]	z [mm]
2.5000	20.60000	9.35000	49.00000	2.5035 d
d - Displacements include imported displacements.				

Structure: Windmill Hill | Sub-structure: Windmill Hill 1

Dist.	Coordinates		Displacements	
[m]	x [m]	y [m]	z [m]	z [mm]
Vertical Offset 1				
0.0	14.50000	26.00000	52.80000	4.2960 d
0.67082	14.80000	25.40000	52.80000	4.0699 d
1.3416	15.10000	24.80000	52.80000	3.7609 d
2.0125	15.40000	24.20000	52.80000	3.3607 d
2.6833	15.70000	23.60000	52.80000	2.9604 d
3.3541	16.00000	23.00000	52.80000	2.2499 d
d - Displacements include imported displacements.				

Structure: Windmill Hill | Sub-structure: Windmill Hill 2

Dist.	Coordinates		Displacements	
[m]	x [m]	y [m]	z [m]	z [mm]
Vertical Offset 1				
0.0	16.00000	22.95000	52.80000	2.1816 d
0.80604	16.64000	22.46000	52.80000	1.8195 d
1.6121	17.28000	21.97000	52.80000	0.71327 d
2.4181	17.92000	21.48000	52.80000	1.6460 d
3.2242	18.56000	20.99000	52.80000	1.9320 d
4.0302	19.20000	20.50000	52.80000	2.2630 d
d - Displacements include imported displacements.				

Structure: Windmill Hill | Sub-structure: Windmill Hill 3

Dist.	Coordinates		Displacements	
[m]	x [m]	y [m]	z [m]	z [mm]
Vertical Offset 1				
0.0	19.20000	20.45000	52.80000	2.2162 d
0.75769	19.90000	20.16000	52.80000	2.8272 d
1.5154	20.60000	19.87000	52.80000	3.3969 d
2.2731	21.30000	19.58000	52.80000	3.9108 d
3.0308	22.00000	19.29000	52.80000	4.3642 d
3.7885	22.70000	19.00000	52.80000	4.7532 d
d - Displacements include imported displacements.				

Specific Building Damage Results - All Segments

Structure: 2 Frognal Rise | Sub-structure: 2 FR Rear 1

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	5.8250	Sagging	0.014964	0.052083	0.066840	-520.56E-6	-0.0011774	2243.5	1 (Very Slight)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

Structure: 2 Frognal Rise | Sub-structure: 2 FR Rear 2

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	4.3900	Sagging	0.0029622	0.070740	0.073062	-707.83E-6	262.65E-6	12820.1	1 (Very Slight)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

Structure: 2 Frognal Rise | Sub-structure: 2 FR Side

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	3.8224	Hogging	322.64E-6	0.0	317.97E-6	0.0	-50.440E-6	116310.	0 (Negligible)
	2	3.8224	5.6676	Sagging	0.0025764	-0.029439	0.0060346	387.82E-6	124.85E-6	25971.	0 (Negligible)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 1

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	1.2491	Sagging	415.95E-6	0.0041828	0.0042821	-64.402E-6	-266.70E-6	38098.	0 (Negligible)
	2	1.2491	0.29669	Hogging	561.26E-6	0.0047723	0.0048268	-148.17E-6	-267.35E-6	172680.	0 (Negligible)
	3	1.5458	2.3442	Sagging	0.0013784	0.0030764	0.0036845	-148.17E-6	-267.35E-6	19631.	0 (Negligible)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 2

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	2.7900	Sagging	0.0016079	-0.049708	0.0099822	726.88E-6	-148.19E-6	18129.	0 (Negligible)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

Structure: 2 Frognal Rise | Sub-structure: 2 FR Front 3

Vertical Offset from Line for Vertical Movement Calculations	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement Curve	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
[m]		[m]	[m]		[%]	[%]	[%]			[m]	
0.0	1	0.0	2.5900	Sagging	0.0019295	-0.023585	0.0048412	676.46E-6	-159.93E-6	12078.	0 (Negligible)
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.											

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4 Frognaal Rise

Underpinned and Piled Retaining Walls - Imported PDisp
Pile and Underpin Installation and Excavation Final

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
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Structure: 2 Frognaal Rise | Sub-structure: 2 FR Front 4

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
Calculations											
[m] 0.0	1	[m] 0.0	[m] 2.2400	Sagging	[%] 0.0024219	[%] -0.19950	[%] 0.039923	0.0025088	179.22E-6	[m] 10957.0	0 (Negligible)

Tensile horizontal strains are +ve, compressive horizontal strains are -ve.

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Front 5

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
Calculations											
[m] 0.0	1	[m] 0.0	[m] 1.8791	Sagging	[%] 0.0033612	[%] 0.070833	[%] 0.072031	-707.83E-6	341.46E-6	[m] 6808.7	1 (Very Slight)
	2	[m] 1.8791	[m] 0.61085	Hogging	[%] 926.64E-6	[%] 0.070833	[%] 0.070888	-707.83E-6	341.46E-6	[m] 6709.4	1 (Very Slight)

Tensile horizontal strains are +ve, compressive horizontal strains are -ve.

Structure: Windmill Hill | Sub-structure: Windmill Hill 1

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
Calculations											
[m] 0.0	1	[m] 0.0	[m] 3.3530	Sagging	[%] 0.0087004	[%] 0.059708	[%] 0.071039	-622.85E-6	909.40E-6	[m] 3997.6	1 (Very Slight)

Tensile horizontal strains are +ve, compressive horizontal strains are -ve.

Structure: Windmill Hill | Sub-structure: Windmill Hill 2

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
Calculations											
[m] 0.0	1	[m] 0.0	[m] 0.85601	Sagging	[%] 0.0050777	[%] -0.0044728	[%] 0.0038626	0.0055450	0.0013801	[m] 445.07	0 (Negligible)
	2	[m] 0.85601	[m] 2.0513	Hogging	[%] 0.051531	[%] -0.55900	[%] 0.11551	0.0091707	0.0013801	[m] 961.83	2 (Slight)
	3	[m] 2.9073	[m] 0.61610	Hogging	[%] 0.0013933	[%] 0.0055992	[%] 0.0058753	-56.208E-6	-410.66E-6	[m] 4981.5	0 (Negligible)
	4	[m] 3.5234	[m] 0.50564	None	[%] 0.0	[%] 0.0055760	[%] 0.0055760	-55.757E-6	-410.66E-6	[m] 2942.9	0 (Negligible)

Tensile horizontal strains are +ve, compressive horizontal strains are -ve.

Structure: Windmill Hill | Sub-structure: Windmill Hill 3

Vertical Offset from Line for Vertical Movement	Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature	Damage Category
Calculations											
[m] 0.0	1	[m] 0.0	[m] 3.7870	Sagging	[%] 0.0045431	[%] 0.016911	[%] 0.023174	-201.57E-6	-806.34E-6	[m] 8781.6	0 (Negligible)

Tensile horizontal strains are +ve, compressive horizontal strains are -ve.

Specific Building Damage Results - Critical Values for All Segments within Each Sub-Structure

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Rear 1

Vertical Offset from Line for Vertical Movement	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
Calculations										
[m] 0.0	[%] 0.014964	[%] 0.052083	[mm] -0.0011774	[mm] 2.5724	[%] 0.066840	[mm] -520.56E-6	[mm] -0.0011774	[m] -	[m] 2243.5	1 (Very Slight)

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Rear 2

Vertical Offset from Line for Vertical Movement	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
Calculations										
[m] 0.0	[%] 0.0029622	[%] 0.070740	[mm] 262.65E-6	[mm] 2.5692	[%] 0.073062	[mm] -707.83E-6	[mm] 262.65E-6	[m] -	[m] 12820.1	1 (Very Slight)

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Side

Vertical Offset from Line for Vertical Movement	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
Calculations										
[m] 0.0	[%] 0.0025764	[%] -0.029439	[mm] 124.85E-6	[mm] 2.1153	[%] 0.0060346	[mm] 387.82E-6	[mm] 124.85E-6	[m] 116310.	[m] 25971.0	0 (Negligible)

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Front 1

Vertical Offset from Line for Vertical Movement	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
Calculations										
[m] 0.0	[%] 0.0013784	[%] 0.0047723	[mm] -267.35E-6	[mm] 2.7866	[%] 0.0048268	[mm] -148.17E-6	[mm] -267.35E-6	[m] 172680.	[m] 19631.0	0 (Negligible)

Structure: 2 Frognaal Rise | Sub-structure: 2 FR Front 2

Vertical Offset from Line for Vertical Movement	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
Calculations										
[m] 0.0	[%] 0.0016079	[%] -0.049708	[mm] -148.19E-6	[mm] 3.0749	[%] 0.0099822	[mm] 726.88E-6	[mm] -148.19E-6	[m] -	[m] 18129.0	0 (Negligible)

4 Frognal Rise

Underpinned and Piled Retaining Walls - Imported PDisp
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Vertical Offset from Line for Vertical Movement Calculations	Deflection Ratio	Average Horizontal Strain	Max Slope	Max Settlement	Max Tensile Strain	Max Gradient of Horizontal Displacement	Max Gradient of Vertical Displacement Curve	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category		
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 3												
[m] 0.0	[%] 0.0019295	[%] -0.023585	-159.93E-6	[mm] 3.3567	[%] 0.0048412	676.46E-6	-159.93E-6	[m] -	[m] 12078.0	0 (Negligible)		
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 4												
[m] 0.0	[%] 0.0024219	[%] -0.19950	179.22E-6	[mm] 3.3578	[%] 0.039923	0.0025088	179.22E-6	[m] -	[m] 10957.0	0 (Negligible)		
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 5												
[m] 0.0	[%] 0.0033612	[%] 0.070833	341.46E-6	[mm] 3.1253	[%] 0.072031	-707.83E-6	341.46E-6	[m] 6709.4	[m] 6808.7	1 (Very Slight)		
Structure: Windmill Hill Sub-structure: Windmill Hill 1												
[m] 0.0	[%] 0.0087004	[%] 0.059708	909.40E-6	[mm] 4.2960	[%] 0.071039	-622.85E-6	909.40E-6	[m] -	[m] 3997.6	1 (Very Slight)		
Structure: Windmill Hill Sub-structure: Windmill Hill 2												
[m] 0.0	[%] 0.051531	[%] -0.55900	0.0013801	[mm] 2.2625	[%] 0.11551	0.0091707	0.0013801	[m] 961.83	[m] 445.07	2 (Slight)		
Structure: Windmill Hill Sub-structure: Windmill Hill 3												
[m] 0.0	[%] 0.0045431	[%] 0.016911	-806.34E-6	[mm] 4.7524	[%] 0.023174	-201.57E-6	-806.34E-6	[m] -	[m] 8781.6	0 (Negligible)		
Specific Building Damage Results - Critical Segments within Each Structure												
Structure Name	Parameter	Critical Sub-Structure	Critical Segment	Start	End	Curvature	Max Slope	Max Settlement	Max Tensile Strain	Min Radius of Curvature (Hogging)	Min Radius of Curvature (Sagging)	Damage Category
2 Frognal Rise	Max Slope	2 FR Rear 1	1	0.0	5.8250	Sagging	0.0011774	2.5724	0.066840	-	2243.5	1 (Very Slight)
	Max Settlement	2 FR Front 4	1	0.0	2.2400	Sagging	179.22E-6	3.3578	0.039923	-	10957.0	0 (Negligible)
	Max Tensile Strain	2 FR Rear 2	1	0.0	4.3900	Sagging	262.65E-6	2.5692	0.073062	-	12820.1	1 (Very Slight)
	Min Radius of Curvature (Hogging)	2 FR Front 5	2	1.8791	2.4900	Hogging	341.46E-6	2.6983	0.070888	6709.4	-1	1 (Very Slight)
	Min Radius of Curvature (Sagging)	2 FR Rear 1	1	0.0	5.8250	Sagging	0.0011774	2.5724	0.066840	-	2243.5	1 (Very Slight)
Windmill Hill	Max Slope	Windmill Hill 2	1	0.0	0.85601	Sagging	0.0013801	2.1816	0.0038626	-	445.07	0 (Negligible)
	Max Settlement	Windmill Hill 3	1	0.0	3.7870	Sagging	806.34E-6	4.7524	0.023174	-	8781.6	0 (Negligible)
	Max Tensile Strain	Windmill Hill 2	2	0.85601	2.9073	Hogging	0.0013801	1.8195	0.11551	961.83	-2	2 (Slight)
	Min Radius of Curvature (Hogging)	Windmill Hill 2	2	0.85601	2.9073	Hogging	0.0013801	1.8195	0.11551	961.83	-2	2 (Slight)
	Min Radius of Curvature (Sagging)	Windmill Hill 2	1	0.0	0.85601	Sagging	0.0013801	2.1816	0.0038626	-	445.07	0 (Negligible)
Specific Building Damage Results - All Combined Segments												
Structure: 2 Frognal Rise Sub-structure: 2 FR Rear 1												
Vertical Offset from Line for Vertical Movement Calculations	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage Category				
[m] No structures have segments combined.	[m]	[m]			[%]		[%]					
Structure: 2 Frognal Rise Sub-structure: 2 FR Rear 2												
Vertical Offset from Line for Vertical Movement Calculations	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage Category				
[m] No structures have segments combined.	[m]	[m]			[%]		[%]					
Structure: 2 Frognal Rise Sub-structure: 2 FR Side												
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage Category				

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Underpinned and Piled Retaining Walls - Imported PDisp
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Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 1									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 2									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 3									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 4									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: 2 Frognal Rise Sub-structure: 2 FR Front 5									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: Windmill Hill Sub-structure: Windmill Hill 1									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									
Structure: Windmill Hill Sub-structure: Windmill Hill 2									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	0.0	1	[m]	[m]	[%]	[%]	[%]		
Tensile horizontal strains are +ve, compressive horizontal strains are -ve.									
Structure: Windmill Hill Sub-structure: Windmill Hill 3									
Vertical Offset from Line for Vertical Movement	Combined Segment	Start	Length	Curvature	Deflection Ratio	Average Horizontal Strain	Max Tensile Strain	Damage	Category
Calculations									
[m]	[m]	[m]			[%]	[%]	[%]		
No structures have segments combined.									

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