

SITE INVESTIGATION FACTUAL REPORT

Report No: 263958
Client: Crawford Claims Management
Site: 10-10a Sherriff Road, London

Client Ref: SU1503174-
Date of Visit: 02/09/2015

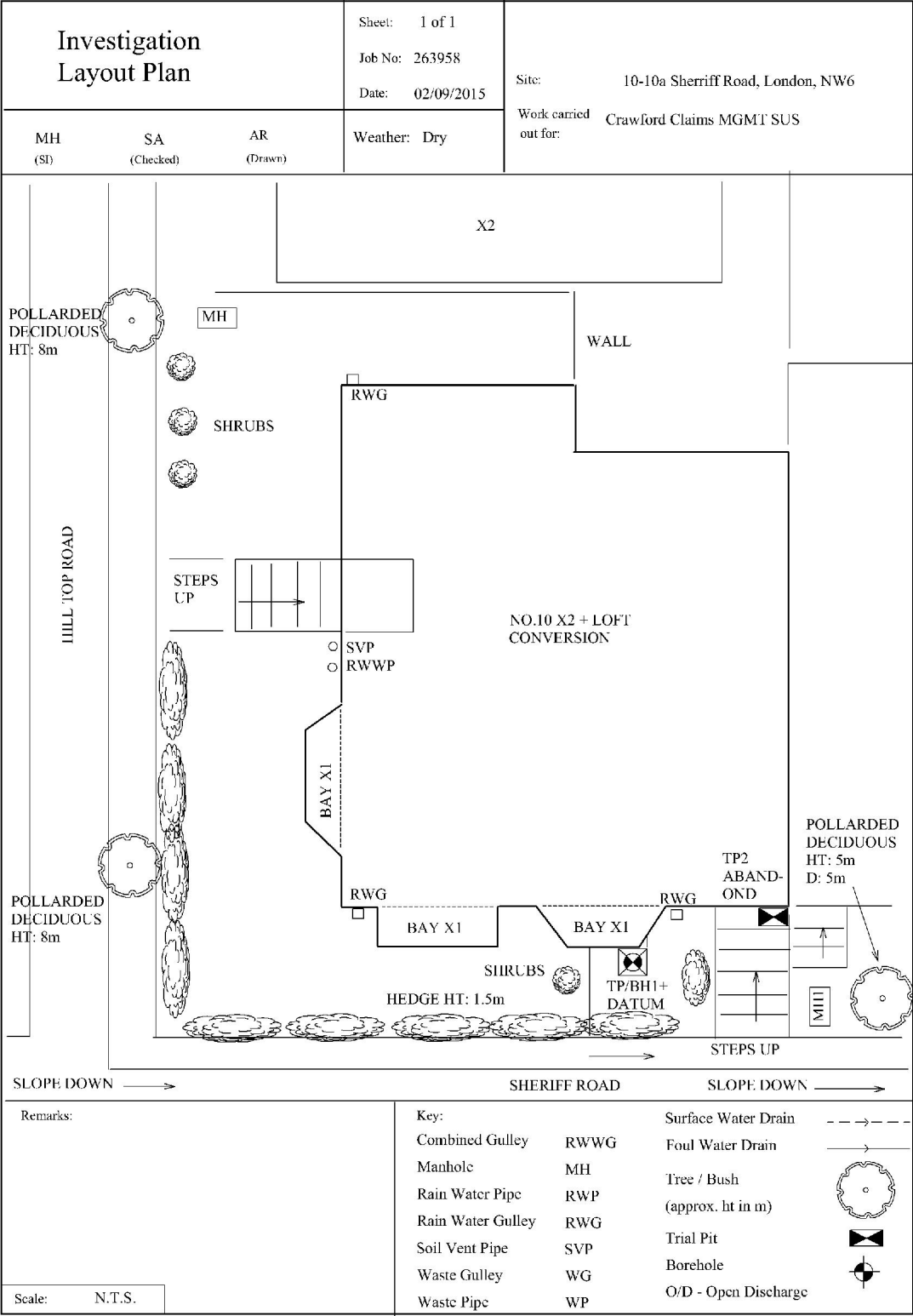


Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys

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CET is the trading name of CET Structures Ltd
Registered in England No. 02527130



Trial Pit No: 1		Sheet: 1 of 1 Job No: 263958 Date: 02/09/2015		Site: 10-10a Sherriff Road, London	
		Excavation Method: Hand tools Weather: Dry		Drawn by: AR Ground Level mOD:	

BRICK

RENDER PLINTH

450 X 350

110

BRICK

CONCRETE FOUNDATION

800

D V 86

92

250

75

75

400

315

50

35

200

FOR STRATA BELOW 1000mm SEE BH LOG 1

Ground Level

PAVING SLABS

CONCRETE

MADE GROUND: Medium compact, mid to dark brown sandy very silty clay with occasional gravel and brick, concrete and clinker fragments.

Roots of live appearance to 20mm Ø.

Firm mid brown, grey veined silty CLAY with partings of orange and brown silt and fine sand.

Roots of live appearance to 13mm Ø.

Stiff as above with occasional claystone nodules.

Roots of live appearance to 2mm Ø with dead and decomposing roots fragments.

Remarks: All measurements in millimetres.		Key: <table style="display: inline-table; vertical-align: top;"> <tr> <td>D</td><td>Small disturbed sample</td> <td>J</td><td>Jar sample</td> </tr> <tr> <td>B</td><td>Bulk disturbed sample</td> <td>V</td><td>Pilcon Vane (kPa)</td> </tr> <tr> <td>W</td><td>Water sample</td> <td>M</td><td>Mackintosh probe</td> </tr> <tr> <td colspan="4">TDTD Too dense to drive</td> </tr> </table>				D	Small disturbed sample	J	Jar sample	B	Bulk disturbed sample	V	Pilcon Vane (kPa)	W	Water sample	M	Mackintosh probe	TDTD Too dense to drive			
		D	Small disturbed sample	J	Jar sample																
B	Bulk disturbed sample	V	Pilcon Vane (kPa)																		
W	Water sample	M	Mackintosh probe																		
TDTD Too dense to drive																					
Logged: MH	Checked: SA	Approved:	Scale: N.T.S.																		

Borehole No: 1 & Datum			Sheet: 1 of 1		Site: 10 - 10A, Sherriff Road, London, NW6				
Boring Method: CFA			Job No: 263958						
Diameter: 100mm Coordinates:			Date: 02/09/2015						
			Ground Level mOD:		Work Carried out for: Crawford Claims MGMT SUS				
Depth (m)	Description of Strata	Thick-ness (m)	Legend	Sample	Test Type	Result	Depth (m)	Field Records/Comments	Depth to water (m)
1.00	As trial pit 1	1.00							
				D	V	120 128	1.00	Roots of live appearance to 1mm diameter to 2.7m	
	Stiff, mid brown, grey veined silty CLAY with partings of orange and brown silt and fine sand and occasional claystone nodules	1.00	— .x — — — — — — .x	D			1.50		
2.00	Stiff as above with crystals		— .x	D	V	122 122	2.00		
2.20		0.20	— .x						
	Very stiff, mid brown, grey veined silty CLAY with partings of orange and brown silt and fine sand with occasional claystone nodules and crystals	0.80	— .x — — — — — — x — — —	D			2.50		
3.00	Borehole sampling ends			D	V	140+ 140+	3.00	Dead and decomposing root fragments to 5.1m	
		2.10							
5.10	Borehole ends at 5.1m Claystone obstruction - too dense for drill to penetrate								
Remarks: Borehole dry and open on completion Datum installed at 5.1m. No soil samples or insitu strength testing below 3.0m					Key: T.D.T.D. Too Dense to Drive D Small disturbed sample J Jar sample B Bulk disturbed sample V Picon Vane (kPa) W Water sample M Mackintosh Probe				
Logged: MH	Checked: SA	Drawn by: Jo F	Scale: NTS			Weather: Dry			

Trial Pit No: 2		Sheet: 1 of 1 Job No: 263958 Date: 02/09/2015	Site: 10-10a Sherriff Road, London Work carried out for: Crawford Claims MGMT SUS
		Excavation Method: Hand tools Weather: Dry	

BRICK

600

565

RENDER PLINTH

PAVING SLABS

GRAVEL

BRICK CHAMBER

WATER MAIN

OLD CONCRETE STEPS

PAVING SLABS

35

20

160

35

180

35

TRIAL PIT 2 ABANDONED AT 465mm.
Water main and old concrete steps obstruct

U/S of foundation not found

Remarks: All measurements in millimetres.		Key: D Small disturbed sample J Jar sample B Bulk disturbed sample V Pilcon Vane (kPa) W Water sample M Mackintosh probe TDTD Too dense to drive	
Logged: MH	Checked: SA	Approved:	Scale: N.T.S.

Laboratory Summary Results

Our Ref: 263958

Date Sampled: 02/09/2015

Location: 10-10a, Sherriff Road, London, NW6

Date Received: 03/09/2015

Work carried out for: Crawford Claims Management

Date Tested: 03/09/2015

Date of Report: 10/09/2015

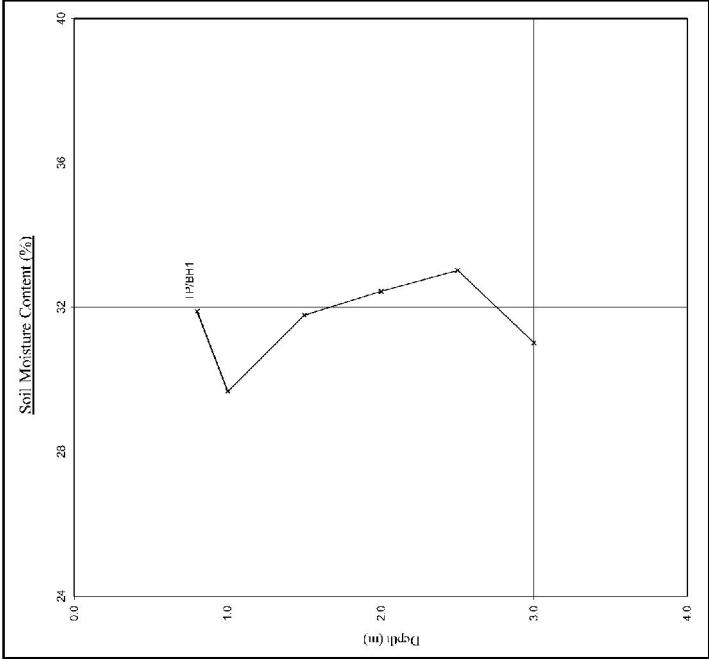
TP/BH No	Sample Ref ¹		Type	Moisture Content (%) ^[11]	Soil Fraction > 0.425mm (%) ^[12]	Liquid Limit (%) ^[13]	Plastic Limit (%) ^[14]	Plasticity Index (%) ^[15]	Liquidity ¹⁸ Index ^[16]	Modified ¹⁸ Plasticity Index (%) ^[16]	Soil ¹⁸ Class ^[17]	Filter Paper Contact Time (h.) ^[18]	Soil Sample Suction (kPa) ^[8]	Oedometer Strain ^[19]	Estimated Heave Potential (Dd) (mm) ^[10]	In situ ²⁰ Shear Vane Strength (kPa) ^[11]	Organic ²¹ Content (%) ^[12]	pH ²² Value ^[13]	Sulphate Content ²³ 4 g/l ^[14]			Class ^[16]	
	Depth (m)																		SO ₃ ^[14]	SO ₄ ^[15]			
1	U/S 0.80		D	32	<5	77	26	51	0.12	51	CV	168	199			89							
	1.0		D	30	<5							168	246			124							
	1.5		D	32	<5							168	260										
	2.0		D	32	<5	77	26	51	0.13	51	CV	168	241			122							
	2.5		D	33	<5							168	247										
	3.0		D	31	<5	76	24	52	0.14	52	CV	168	271			> 140							
Test Methods / Notes																							Key
^[1] BS 1377: Part 2: 1990, Test No 3.2																							D
^[2] Estimated if <5%, otherwise measured																							D
^[3] BS 1377: Part 2: 1990, Test No 4.4																							B
^[4] BS 1377: Part 2: 1990, Test No 5.3																							U
^[5] BS 1377: Part 2: 1990, Test No 5.4																							W
^[6] BRE Digest 240: 1993																							ENP
^[7] BS 5930: 1981, Figure 31 - Plasticity Chart for the Classification of fine soils																							US
^[8] In-house method SNr. clipped from BRE IP 493																							
^[9] In-house Test Procedure S17a: One Dimensional Swell/Shrink Test																							
^[10] Estimated Heave Potential (Dd)																							
^[11] Values of shear strength were determined in situ by CET using a Phoenix hand-vane or Geomir vane (GV)																							
^[12] BS 1377: Part 3: 1990, Test No 4																							
^[13] BS 1377: Part 2: 1990, Test No 9																							
^[14] BS 1377: Part 3: 1990, Test No 5.6																							
^[15] SO ₃ = 1.2 x SO ₄																							
^[16] These tests are not UKAS accredited																							
Full reports can be provided upon request																							
Version: 5BH V1.4 - 11/05/15																							



8618

Moisture Content Profiles

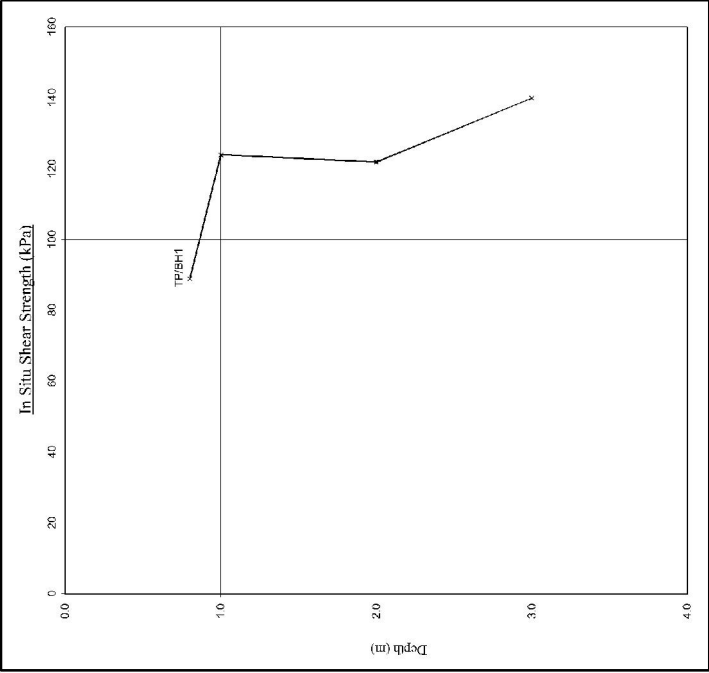
Our Ref: 263958
Location: 10-10a, Sherriff Road, London, NW6
Work carried out for: Crawford Claims Management



- Notes**
1. If plotted, 0.4 LL and PI—2 (after Driscoll, 1987) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
 2. Unless specifically noted the profiles have not been related to a site datum.

Shear Strength Profiles

Date Sampled: 02/09/2015
Date Received: 03/09/2015
Date Tested: 03/09/2015
Date of Report: 10/09/2015



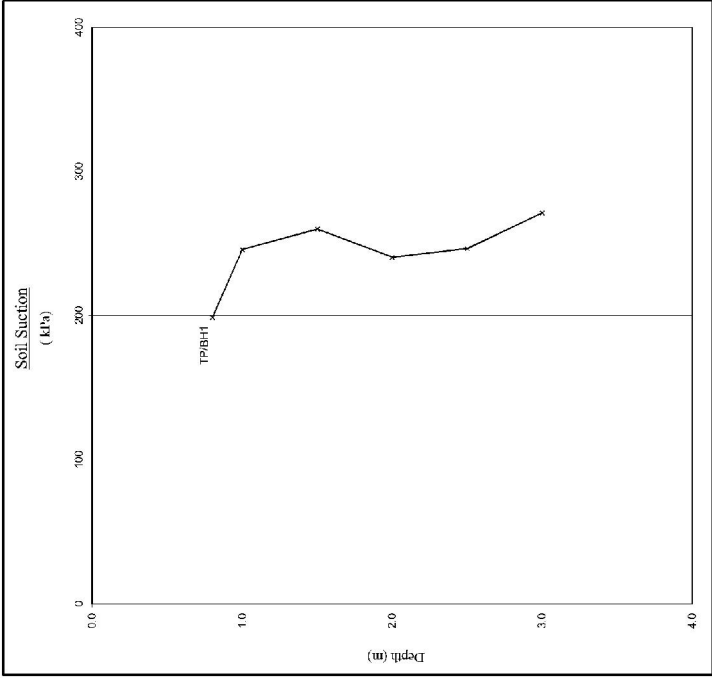
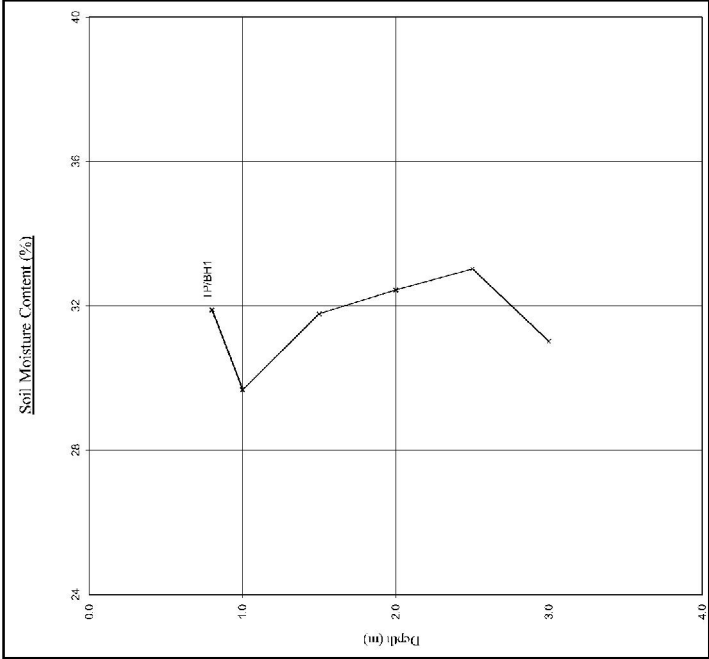
- Note**
1. Unless otherwise stated, values of Shear Strength were determined in situ by CPT using a Picon Hard Vane the calibration of which is limited to a maximum reading of 140 kPa.
 2. Unless specifically noted the profiles have not been related to a site datum.

Moisture Content Profiles

Our Ref: 263958
Location: 10-10a, Sherriff Road, London, NW6
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Soil Suction Profiles

Date Sampled: 02/09/2015
Date Received: 03/09/2015
Date Tested: 03/09/2015
Date of Report: 10/09/2015



Notes

1. If plotted, 0.4 D₁ and P₁-2 (after Driscoll, 1987) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.
2. Unless specifically noted the profiles have not been related to a site datum.

Note

When shown the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

EPSL European Plant Science Laboratory	Sheet: 1 of 1	Site: 10-10A Sheriff Road, London,	
	Job No: 263958 Date: 04/09/2015 Order No: 738028 EPSL Ref: R12094	Work carried out for: Crawford Claims MGMT SUS	

Certificate of Analysis

The following work was commissioned by CET on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.
 The results were as follows -

<u>Trial pit/ Borehole number</u>	<u>Root diameter (mm)</u>	<u>Tree, shrub or climber from which root originates</u>	<u>Result of starch test</u>
TP1 (USF)	2 mm	Cupressaceae spp.	Negative
TP1 (USF)	2 mm	Tilia spp. 3 roots	Positive
TP1 (USF)	2 mm	Ligustrum spp.	Positive
BH1 (to 2.7m)	<1 mm	Tilia spp. 5 roots	Negative

Cupressaceae spp. include Lawson cypress, western red cedar, Monterey cypress, Leyland cypress and junipers.
 Tilia spp. are limes.
 Ligustrum spp. are privets.

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