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Non-technical Summary – Ground Investigation Report

CLIENT	Kapital Basements Ltd				
SITE ADDRESS	158 Iverson Road, West Hampstead, London NW6 2HH				
REPORT REFERENCE	GWPR1605				
ENGINEER	Philip Allvey/Megan James, Ground and Water Limited				
INVESTIGATION	Please see Figure 1 Attached. Site works were undertaken on the 23 rd March 2016. 19mm diameter				
LOCATIONS	combined bio-gas and groundwater monitoring well was installed in WS1 to 5.00m bgl.				
GROUND CONDITIONS	Summary of Strata Encountered (WS1 and WS2)				
ENCOUNTERED			Denth	Denth	
		Strata	Encountered	Thickness (m)	
		Strata	(m bgl)	interrets (in)	
		MADE GROUND (WS1 and WS2): Concrete and sub	(~8.7		
		hase (WS2). Concrete and Sub-	GL	0.08 - 0.30	
		Dase (WS2)			
		MADE GROUND (WS1 dilu WS2): Ddrk			
		silty alow Sand is fine to source grained. Cravel is rare			
		to occasional fine to coarse grained. Graver is rare	0.08 – 0.30	0.92 - 1.50	
		rounded flint, hrick, compart and carbonaceous			
		matorial			
		grou ciltu clay with accordinal orange ciltu pockets	1 00 1 90	>2.20 >4.00	
		grey silly cidy with occasional orange silly pockets.	1.00 - 1.80	>5.20 - >4.00	
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INSITU-GEOTECHNICAL	. LOI	NDON CLAY FORMATION: (encountered between 1.00 – 1	0.00 bgl) Very low	/low to high undraine	ed
TESTING (DPH)	sne	ear strength (20 – 180kPa – In accordance with (Based on	Shioi-Fukui, High P	lasticity Solls – 1982)	•
GROUNDWATER	No groundwater was encountered during the drilling of the boreholes. A standing water level of			of	
	2.0	0m bgl was noted on the 15 th April 2016 within t	he 5.00m deep o	combined bio-gas a	nd
	gro	undwater monitoring well installed in WS1.			
ROOTS	Roots were noted to a depth of 0.80m in WS1 and to a depth of 1.80m in WS2.				
ANTICIPATED VOLUME	LOI	NDON CLAY FORMATION: High volume change potentia	al in accordance v	vith BRE240 and NH	BC
CHANGE POTENTIAL	Sta	ndards Chapter 4.2.			
	Consistency Index calculations indicated the soils of the London Clay Formation to be stiff to very stiff.			iff.	
	Liquidity Index testing revealed the soils to be heavily overconsolidated. No root exacerbated			ed	
	mo	isture deficits were noted.			
FOUNDATION	At	the time of reporting, May 2016, it is understood that the	proposed develop	oment will comprise t	he
RECOMMENDATIONS	cor	nstruction of a basement beneath the footprint of the st	ructure. The base	ment will be formed	at
	~3.	00 – 3.50m bgl.			
	lt is	s considered likely the proposed basement will be constru	icted with load bea	aring concrete retaini	ng
	walls with semi-ground bearing concrete floors.				
	Foundations constructed on the London Clay Formation at 3.00 – 3.50m bgl can be designed based on				
	a presumed allowable bearing capacity of 90 - 110kN/m ² . This is based on a 5m long by 0.75 – 1.00m				
	wic	le foundation and a maximum settlement of 25mm. The	analysis has been	based on the results	of
FOUNDATION RECOMMENDATIONS	 Liquidity Index talculations indicated the solis of the London Clay Formation to be stift to very stift. Liquidity Index testing revealed the soils to be heavily overconsolidated. No root exacerbated moisture deficits were noted. At the time of reporting, May 2016, it is understood that the proposed development will comprise the construction of a basement beneath the footprint of the structure. The basement will be formed at ~3.00 – 3.50m bgl. It is considered likely the proposed basement will be constructed with load bearing concrete retaining walls with semi-ground bearing concrete floors. Foundations constructed on the London Clay Formation at 3.00 – 3.50m bgl can be designed based on a presumed allowable bearing capacity of 90 - 110kN/m². This is based on a 5m long by 0.75 – 1.00m wide foundation and a maximum settlement of 25mm. The analysis has been based on the results of 			iff. ed he at ing on Om of	

	 geotechnical laboratory testing and detailed settlement analysis. and referral to BS 8004:1986, <i>Code of Practice for Foundations</i>, and based on a 5m long by 0.75 – 1.00m wide foundation and a maximum settlement of 25mm. It must be noted that a bearing capacity of less than 46N/m² at 3.00m bgl and less than 54kN/m² at 3.50m bgl may result in heave of the underlying soils. A swelling pressure of 50kpa was noted in a sample of the London Clay Formation at 3.00m bgl. The sample was disturbed and this must be taken into account in final design. Figure 18 of the Camden Geological, Hydrogeological and Hydrological Study indicated that the Jubilee Underground Line was located close to the site. Further investigation should be undertaken. The West Hampstead Overground Line was noted ~50m south of the site.
GROUND MOVEMENT ASSESSMENT	In terms of building damage assessment and with reference to Table 2.5 of C580 (after Burland et al, 1977), the 'Description of typical damage' given the calculated movements it is likely that the damage assessment will fall into Category 0, 'Negligible'.
RETAINING WALL DESIGN (Design Angle of Shearing Resistance (Ø))	Made Ground: 15° London Clay Formation: 20°
DEWATERING	Groundwater was not encountered during the site works. A standing water level of 2.00m bgl was recorded in the standpipe installed in WS1 during a return visit to site. This is likely to be derived from perched water within the silty layers of the London Clay Formation or the influx of surface water into the borehole.
CONCRETE CLASSIFICATION	AC-4

This preliminary information may be subject to amendment in the final report and no liability can be accepted for any actions based on this preliminary information.