

Winterbourne
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44 DOWNSHIRE HILL, NW3 date: 22 MARCH 16.

<u>EXISTING LOADS</u>		SEE DRAWING 53025-1.
ON SPINE WALL. WORST CASE - PER METRE		
A) <u>DEAD LOADS:</u>		
- ROOFS - BOTH SIDES		
- CEILING 2ND FLOOR		
" 1ST FLOOR ENS.		
- FLOORS BOTH SIDES 2ND, 1ST + GRND.		
- WALL SELF LOAD.		
<u>GARDEN FLOOR</u>		
WALL	$1 \times 0.25m \times 2.3m = 0.575m^3 \times 20.8kN/m^3 = 12kN$	
<u>GROUND FLOOR</u>		
FLOOR	HALLSIDE = $1 \times 0.82m = 0.82m^2 \times 0.5kN/m^2 = 0.41kN$	
"	RECCPSIDE = $1 \times 2.77m = 2.77m^2 \times 0.5kN/m^2 = 1.39kN$	
WALL	$1 \times 0.25m \times 3.03m = 0.76m^3 \times 20.8kN/m^3 = 15.8kN$	
<u>FIRST FLOOR</u>		
FLOOR	HALLSIDE = $1 \times 0.82m = 0.82m^2 \times 0.5kN/m^2 = 0.41kN$	
"	BEDSIDE = NO DEAD LOADS - JOISTS FRONT TO BACK	
CEILING	ENSUITE = $1 \times 0.82 = 0.82m^2 \times 0.4kN/m^2 = 0.33kN$	
WALL	$1 \times 0.25m \times 2.91m = 0.73m^3 \times 20.8kN/m^3 = 15.18kN$	
<u>SECOND FLOOR</u>		
FLOOR	BEDSIDE = NO DEAD LOADS - JOISTS FRONT TO BACK	
ROOF	ENS. SIDE = $1 \times 1 = 1m^2 \times 1.4kN/m^2 = 1.4kN$	
WALL	$1 \times 0.25m \times 4.37m = 1.1m^3 \times 20.8kN/m^3 = 22.88kN$	
+ PARAPET.		
CEILING	BED = $1 \times 2.7m = 2.7m^2 \times 0.4kN/m^2 = 1.08kN$	
ROOF	BED = $1 \times 1.55m = 1.55m^2 \times 1.4kN/m^2 = 2.17kN$	
<u>TOTAL DEADLOAD/m RUN</u>		<u>= 73.05kN.</u>

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CONT: EXISTING LOADS ON SPINE WALL		
B)	LIVE LOADS /m RUN.	
	ROOF:	
ROOF	SNOW = $1.0 \times 1.5m = 1.5m^2 \times 0.7kNm^2$	= 1.05 kN
	SECOND FLOOR:	
ROOF	SIDE-SNOW = $1 \times 1m = 1m^2 \times 0.7kNm^2$	= 0.7 kN
FLOOR	$1 \times 2.7 = 2.7m^2 \times 1.5kNm^2$	= 4.05 kN
	FIRST FLOOR:	
FLOOR	HALL/ENS = $1 \times 0.82m = 0.82m^2 \times 1.5kNm^2$	= 1.23 kN
"	BED = $1 \times 2.77m = 2.77m^2 \times 1.5kNm^2$	= 4.16 kN
	GROUND FLOOR:	
FLOOR	HALL = $1 \times 0.82m = 0.82m^2 \times 1.5kNm^2$	= 1.23 kN
"	RECEP = $1 \times 2.77m^2 = 2.77m^2 \times 1.5kNm^2$	= 4.16 kN
	TOTAL LIVE LOADS /m RUN	= 16.58 kN
	B/F DEAD LOADS (INCLUDING FOOTING)	= 73.05 kN
		89.63 kN
	TOTAL LIVE + DEAD LOADS /m RUN ON EXISTING FOOTING 0.5m WIDE	$\Delta 90 kN/m$
		= 180 kN/m ²
C)	1. MAXIMUM SOIL BEARING CAPACITY FROM SAMPLE PITS FEB 16 JUST UNDER EXISTING SHALLOW FOOTINGS AT 300mm DEPTH.	
	2. FOUND TO BE MOIST PLASTIC SURFACE CLAYS, WELL COMPACTED WITH SMALL % GRIT. FROM CIVIL ENGINEER TABLES	
		= 15000 kg/m ² = 150 kN/m ²
	3. \therefore MAXIMUM BEARING CAPACITY OF EXISTING SHALLOW SUB-SURFACE SOILS UNDER EXISTING FOOTING $0.5m \times 1m = 0.5m^2$ FOR UDL $= 0.5m^2 \times 150kN$	
		= 75 kN/m
	CURRENT LOADING 90 kN/m	

\therefore EXISTING FOOTING INADEQUATE.

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D).	<u>ACTUAL EXAMPLE LOADS ON SPIKE WALL SECTIONS.</u>	SEE DRAWING 53025-2
D1.	THESE ARE APPROXIMATE LOADS FROM SITE OBSERVATIONS OF SPIKE WALL CRACKING INTO SECTIONS + ACTING AS INDEPENDENT COLUMNS.	
D2.	ALSO ASSUMES FOOTING NOT UNIFORMLY DISTRIBUTING LOADS.	
D3.	LOCATION OF DOOR OPENINGS IN SPIKE WALL TAKEN INTO ACCOUNT.	
E).	<u>CONSIDER - COLUMN PANEL A</u>	
	FOOTING BEARING AREA = $1.13m \times 0.5m$	
		$= 0.57m^2$.
	LOADING ON PANEL A (WORST CASE)	
	LIVE = $16.58kN \times 1.35m = 22.38kN$.	
	DEAD LOADS (FROM PREVIOUS)	
	WALL 1m RUN = $1 \times 0.25 \times 12.6m = 3.15m^2 \times 20.8kN/m^3 = 65.52kN/m$	
	(TOTAL HEIGHT)	
	GROUND FLOOR	
	FLOOR HALL = 0.41kN	
	" RECP = 1.39kN	
	FIRST FLOOR	
	FLOOR HALL = 0.41kN	
	CEILING ENS = 0.33kN	
	SECOND FLOOR	
	ROOF ENS = 1.4kN	
	CEILING BED = 1.08kN	
	ROOF BED = 2.17kN	
	TOTAL DEAD LOADS = 7.19kN + 65.52kN/m	
		<u>72.71kN/m.</u>
	TOTAL DEAD LOADS ON PANEL A	
	FOOTING = $72.71kN \times 1.35m$	
		<u>98.16kN</u>

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ACTUAL SPINE WALL SECTION LOADS

E-CONT: PANEL A

TOTAL LIVE + DEAD LOADS

$$\text{ON PANEL A} = 22.38 \text{ kN} + 98.16 \text{ kN}$$

$$\text{FOOTING AREA } 0.57 \text{ m}^2$$

$$= 120.54 \text{ kN}$$

$$\therefore \text{LOADING} / \text{m}^2 = 120.54 \text{ kN} / 0.57 \text{ m}^2$$

$$= 211 \text{ kN} / \text{m}^2$$

GIVEN THE MAXIMUM BEARING CAPACITY OF THE SHALLOW SUB-SURFACE SOILS ARE $150 \text{ kN} / \text{m}^2$ THE EXISTING FOOTINGS ARE INADEQUATE

F) CONSIDER COLUMN/PANEL B SEE DRAWING 53025-2

$$\text{FOOTING BEARING AREA} = 1.56 \text{ m} \times 0.5 \text{ m} = 0.78 \text{ m}^2$$

LOADING ON PANEL B (WORST CASE)

$$\text{LIVE} = 16.58 \text{ kN} / \text{m} \times 2.05 \text{ m} = 33.99 \text{ kN}$$

DEAD LOADS FOR FLOORS/CEILING/ROOFS (AS PER PANEL A)

$$= 7.19 \text{ kN} / \text{m}$$

$$\text{WALL TOTAL HEIGHT} / \text{m} / \text{RN (AS PER PANEL A)} = 65.52 \text{ kN} / \text{m}$$

$$\text{TOTAL DEAD LOADS} = 72.71 \text{ kN} / \text{m}$$

TOTAL DEAD LOADS ON PANEL B FOOTING

$$= 72.71 \text{ kN} / \text{m} \times 2.05 \text{ m}$$

$$= 149.06 \text{ kN}$$

TOTAL LIVE + DEAD LOADS ON

$$\text{PANEL B FOOTING} = 149.06 + 33.99 \text{ kN}$$

$$= 183.05 \text{ kN}$$

IGNORING REAR WALL AS WALL WAS DETACHED FROM SPINE WALL

$$\therefore \text{LOADING} / \text{m}^2 = 183.05 / 0.78 \text{ m}^2 = 234 \text{ kN} / \text{m}^2$$

AGAIN LOADS ON SUBSOIL UNDER

PANEL B ARE EXCESSIVE.

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A4 DOWNSHIRE HILL/HW3/INU date: 22 MARCH 16.

g)	SPINE WALL REPLACEMENT FOOTING	
	ASSUME:	SEE DRAWING 530 25-2
G1.	NEW FOOTING HAS LINEAR INTEGRITY ∴ LOADS ARE U.D.L.	
G2.	NEW FOOTING DEPTH 1 METRE BELOW ORIGINAL FOOTING BASE ∴ NEW FOOTING DEPTH APPROX. 1.3M BELOW F.F.L.	
G3.	TEST PIT DUG UNDER EXISTING REAR WALL TO 1.3M AT PIN LOCATION 1A FOUND WELL COMPACTED STIFF CLAYS AT DEPTH. FROM CIVIL ENGINEER TABLES MAX SAFE BEARING CAPACITY MEDIUM CLAY = 25,000 kg/m ² USE SAFE BEARING CAPACITY = 200 kN/m ²	
	SPINE WALL LOADS /m RUN	
	LIVE LOADS DIRECTLY ABOVE = 16.58 kN/m	
	DEAD LOADS DIRECTLY ABOVE = 73.84 kN/m	
ADD	50% TRANSFERRED LIVE + DEAD LOADS } = 8.29 kN/m	
	AROUND HALL DOOR } = 36.42 kN/m	
	SELF LOAD NEW FOOTING 1K x 0.9m = 21.0 kN/m	
	TOTAL LIVE + DEAD LOADS /m RUN = 156.13 kN/m	
	(ADJACENT HALL DOOR)	
	USE SAFE BEARING CAPACITY = 200 kN/m ²	
	∴ MINIMUM FOOTING WIDTH REQUIRED	
	= 156.13 / 200 kN	
	= 780mm	
	∴ MAKE FOOTING WIDTH = 800mm	

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H.) <u>FRONT WALL REPLACEMENT FOOTINGS.</u>	
ASSUME:	SEE DRAWING
H1. ASSUMPTIONS AS PER SPINE WALL.	53025-3
M2. REAR WALL SIMILAR - SLIGHTLY THINNER - ASSUME SAME.	
<u>FRONT WALL LOADS/m RUN</u>	
<u>DEAD LOADS/m RUN</u>	
GARDEN FLOOR	
WALL	$1 \times 0.57m \times 2.39m = 1.36m^3 \times 20.8kN/m^3 = 28.29kN$
GROUND FLOOR	
FLOOR	$1 \times 0.9 = 0.9m^2 \times 0.5kN/m^2 = 0.45kN$
WALL	$1 \times 0.47 \times 2.95m = 1.39m^3 \times 20.8kN/m^3 = 28.9kN$
FIRST FLOOR	
FLOOR	$1 \times 2.14m = 2.14m^2 \times 0.5kN/m^2 = 1.07kN$
WALL	$1 \times 0.43m \times 2.95m = 1.27m^3 \times 20.8kN/m^3 = 26.42kN$
SECOND FLOOR	
FLOOR	$1 \times 2.15m = 2.15m^2 \times 0.5kN/m^2 = 1.08kN$
ROOF	$1 \times 2.15m = 2.15m^2 \times 1.4kN/m^2 = 3.01kN$
WALL	$1 \times 0.37m \times 2.34m = 0.87m^3 \times 20.8kN/m^3 = 18.1kN$
PARAPET	
WALL	$1 \times 0.28 \times 1.3 = 0.36m^3 \times 20.8kN/m^3 = 7.49kN$
<u>TOTAL DEAD LOADS/m. = 114.8kN/m.</u>	
<u>LIVE LOADS/m RUN</u>	
GROUND FLOOR	
FLOOR	$0.19m^2 \times 1.5kN/m^2 = 1.35kN$
FIRST FLOOR	
FLOOR	$2.14m^2 \times 1.5kN/m^2 = 3.21kN$
SECOND FLOOR	
FLOOR	$2.15m^2 \times 1.5kN/m^2 = 3.23kN$
ROOF	
SNOW	$2.15m^2 \times 0.7kN/m^2 = 1.5kN$
<u>TOTAL LIVE LOADS/m = 9.3kN/m.</u>	

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H) Cont: FRONT WALL REPLACEMENT FOOTINGS.	
LOADS/m.	
DEAD LOADS	= 114.8 kN
LIVE LOADS	= 9.3 kN
(1m x 1m x 1m) FOOTING SELF LOAD WIDTH OF FOOTING 1M.	= 24.0 kN.
∴ TOTAL LOADS/m RUN. = 148.1 kN.	
USE SAFE BEARING CAPACITY 200 kN/m ²	
∴ MINIMUM FOOTING WIDTH FRONT + REAR	= 75cm
REPLACEMENT FOOTINGS OK.	= 900 > 1000 ✓

NOTES:

- 1) MATERIALS & LOADINGS.
 - 1.1 R/CONCRETE = 24 kN/m³
 - 1.2 TIMBER JOIST FLOOR + PLASTER CEILING = 0.5 kN/m²
 - 1.3 TIMBER + PLASTER CEILING = 0.4 kN/m²
 - 1.4 SLATE ROOF 30° SLOPE = 1.4 kN/m²
 - 1.5 SNOW LOADS = 0.7 kN/m²
 - 1.6 MASONRY WALL = 20.8 kN/m³
 - 1.7 DOMESTIC FLOOR LIVE LOAD = 1.5 kN/m²
 - 1.8 SELF LOAD NEW FOOTING 1m³/m RUN = 24 kN.

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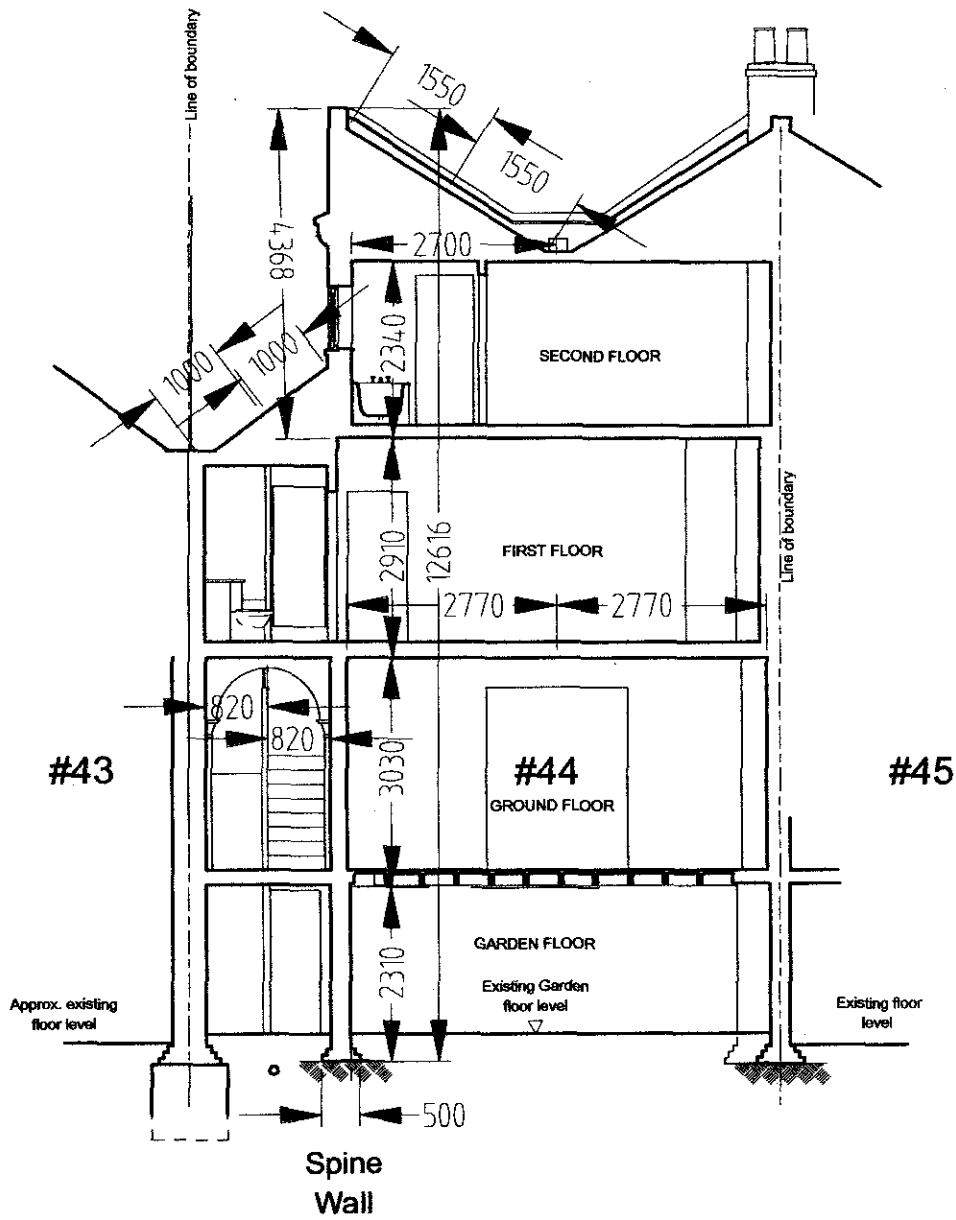
4A DOWNSHIRE HILL, NW3 INU. date: 28 DEC 15

I)	<u>CENTRE CROSS SPINE WALL</u>	
	<u>REPLACEMENT FOOTINGS</u>	SEE DRAWING 53025-3
II.	ASSUME CONDITIONS AS PER SPINE WALL	
	<u>DEAD LOADS/m Run</u>	
	<u>GARDEN FLOOR</u>	
WALL	$1 \times 3.05 \text{ m} \times 2.39 \text{ m} = 0.73 \text{ m}^3 \times 20.8 \text{ kN/m}^3 = 15.18 \text{ kN}$	
	<u>GROUND FLOOR</u>	
FLOOR	$1 \times (0.75 \text{ m} + 0.5 \text{ m}) = 1.225 \text{ m}^2 \times 0.5 \text{ kN/m}^2 = 0.62 \text{ kN}$	
WALL	$1 \times 2.95 \text{ m} = 2.95 \text{ m}^2 \times 1 \text{ kN/m}^2 \text{ (STUD)} = 2.95 \text{ kN}$	
	<u>FIRST FLOOR</u>	
FLOOR	$1 \times (2.08 \text{ m} + 2.05 \text{ m}) = 4.13 \text{ m}^2 \times 0.5 \text{ kN/m}^2 = 3.48 \text{ kN}$	
WALL	$1 \times 2.95 \text{ m} = 2.95 \text{ m}^2 \times 1 \text{ kN/m}^2 \text{ (STUD)} = 2.95 \text{ kN}$	
	<u>SECOND FLOOR</u>	
FLOOR	$1 \times (2.15 + 2.12) = 4.27 \text{ m}^2 \times 0.5 \text{ kN/m}^2 = 2.21 \text{ kN}$	
WALL	$1 \times 2.34 \text{ m} = 2.34 \text{ m}^2 \times 1 \text{ kN/m}^2 = 2.34 \text{ kN}$	
ROOF	$1 \times (2.15 + 2.12) = 4.27 \text{ m}^2 \times 1.4 \text{ kN/m}^2 = 5.98 \text{ kN}$	
	<u>TOTAL DEAD LOADS/m</u>	<u>= 34.71 kN</u>
	<u>LIVE LOADS m/Run.</u>	
	<u>GROUND FLOOR</u>	
FLOOR	$1.225 \text{ m}^2 \times 1.5 \text{ kN/m}^2 = 1.84 \text{ kN}$	
	<u>FIRST FLOOR</u>	
FLOOR	$4.13 \text{ m}^2 \times 1.5 \text{ kN/m}^2 = 6.2 \text{ kN}$	
	<u>SECOND FLOOR & ROOF</u>	
FLOOR	$4.27 \text{ m}^2 \times 1.5 \text{ kN/m}^2 = 6.4 \text{ kN}$	
SNOW	$4.27 \text{ m}^2 \times 0.7 \text{ kN/m}^2 = 3.0 \text{ kN}$	
	<u>TOTAL LIVE LOADS/m</u>	<u>= 17.44 kN</u>
	<u>TOTAL LOADS:</u>	
	DIRECT DEAD LOADS = 34.71 kN	
	TRANSFERRED DEAD LOADS OVER ADS ARCH = 34.71 kN	
	DIRECT LIVE LOADS = 17.44 kN	
	TRANSFERRED LIVE LOAD FROM ADJ ARCH = 17.44 kN	
	SELF LOAD NEW FOOTING = 19.2 kN	
	FOR 800mm WIDE ∴ TOTAL LOADS/m RUN = 123.5 kN	
	FOOTING LOADS/m ² = $71.35 / 0.8 = 89.2 \text{ kN/m}^2$	

USE SAFE BEARING CAPACITY 200 kN/m²

∴ FOOTING OK. = 800mm WIDE.

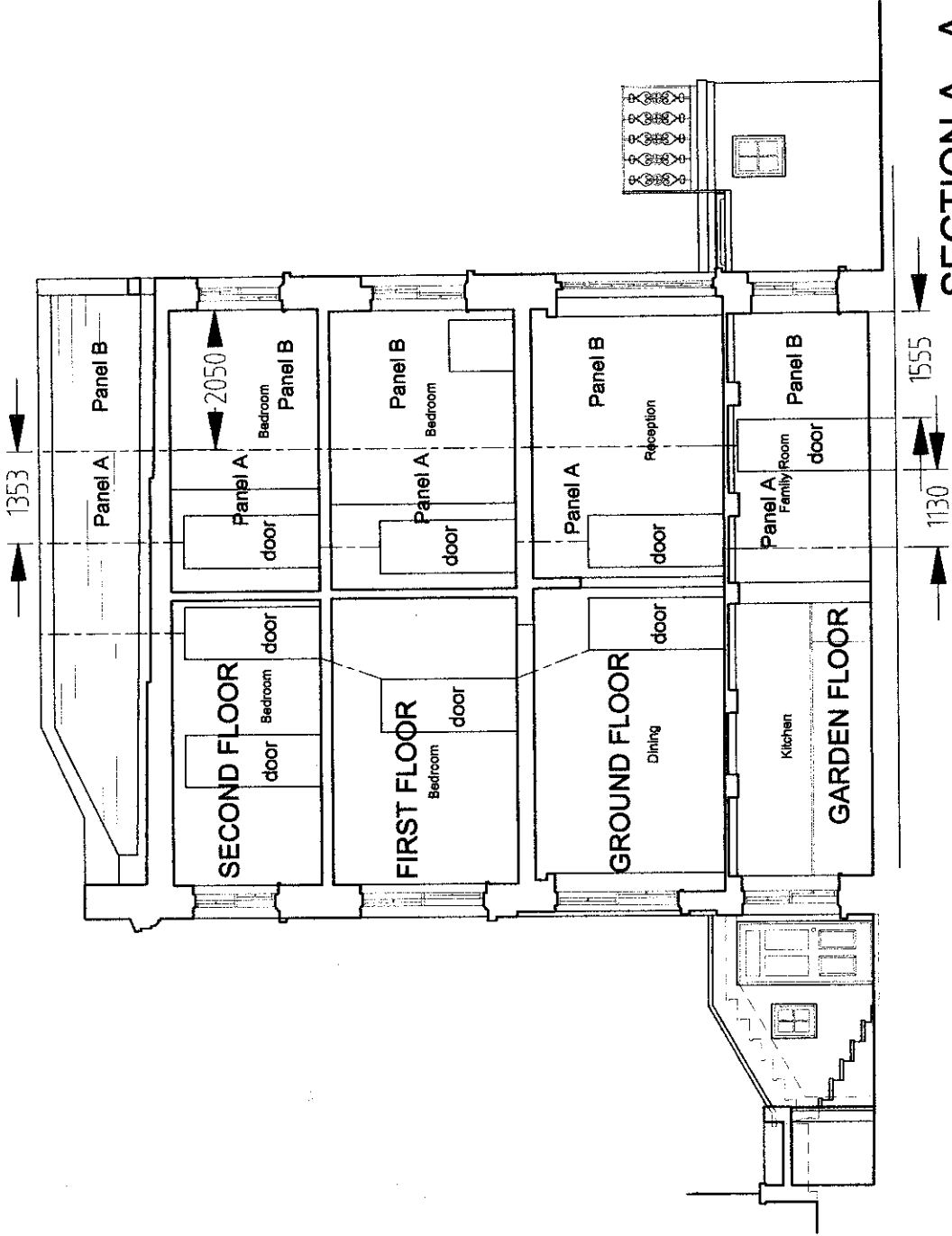
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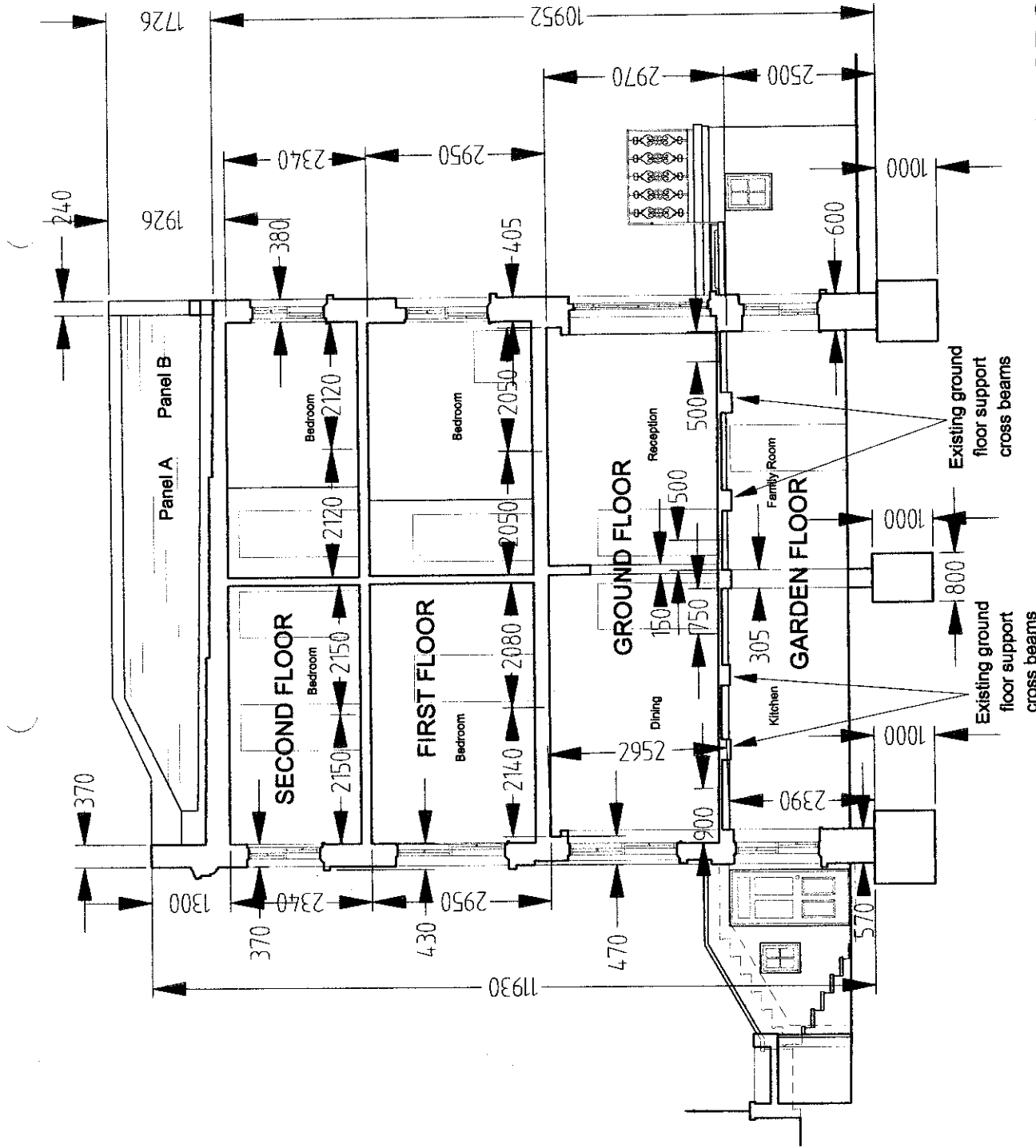
**SECTION C - C - SPINE WALL
DIMENSIONS FOR LOADS**

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1:100 2 Jan16 53025-1

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SECTION A - A - SPINE WALL
FRACTURED WALL PANELS
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SECTION A-A - DIMENSIONS FOR CROSS WALL LOADS
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