

## Chelmer Site Investigations

Unit 15, East Hanningfield Industrial Estate  
Old Church Road, East Hanningfield, Essex CM3 8AB  
Telephone: 01245 400 930 Fax: 01245 400 933  
Email: [info@siteinvestigations.co.uk](mailto:info@siteinvestigations.co.uk) Website: [www.siteinvestigations.co.uk](http://www.siteinvestigations.co.uk)

# Factual Report

Client:	William Carter Ltd
Site:	35 Buckland Crescent London NW3
CSI Ref:	FACT/4253
Dated:	31 <sup>ST</sup> January 2014

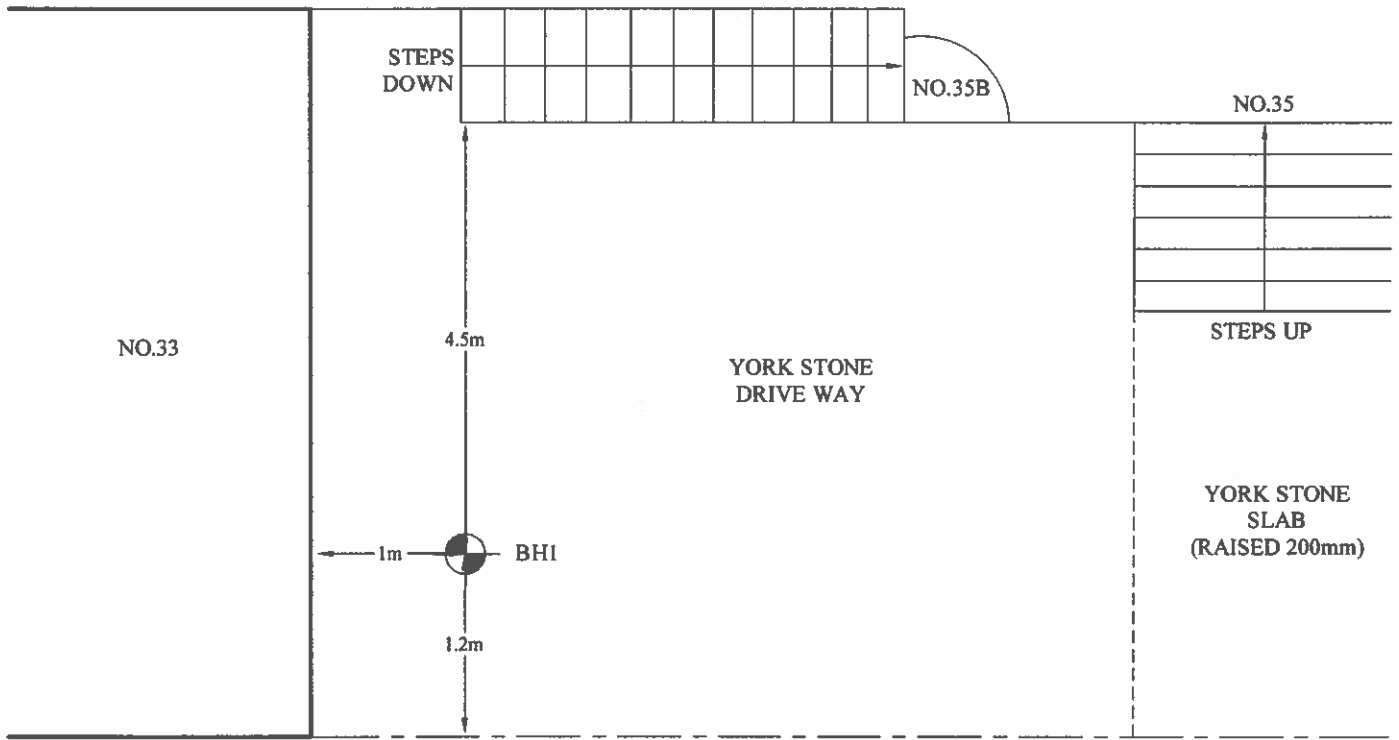
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<b>Client:</b> William Carter Ltd	<b>Scale:</b> N.T.S.	<b>Sheet:</b> 1 of 1	<b>Date:</b> 31.1.14	
<b>Location:</b> 35 Buckland Crescent, London NW3	<b>Job No:</b> 4253	<b>Weather:</b> Rain	<b>Drawn by:</b> JP	<b>Checked by:</b> JH



FOOTPATH

BUCKLAND CRESCENT

## Notes:

*On site tree identification for guidance only. Not authenticated.*

## Key:



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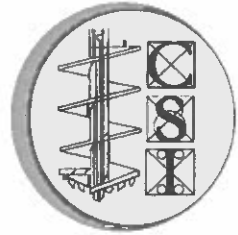
Client: William Carter Ltd		Scale: N.T.S.		Sheet No: 1 of 1		Weather: Rain		Date: 31.1.14	
Site: 35 Buckland Crescent, London NW3		Job No: 4253		Borehole No: 1		Boring method: Secondman (100mm Ø) C.F.A.			
Depth Mtrs.	Description of Strata	Thick-ness	Legend	Sample	Test Type	Result	Root Information	Depth to Water	Depth Mtrs
G.L.	SLAB	0.06					Roots of live appearance to 2mmØ to 2.5m.		
0.06	SAND	0.04	•••••						
0.1	CONCRETE	0.1	■						
0.2	MADE GROUND: medium compact, organic, dark brown, silty clay, with brick and concrete fragments.	1.3	■	D			↓ No roots observed below 2.5m.		0.5
1.5				D	M	08 08 08 08		1.0	
	Stiff, orange-brown, grey veined, silty CLAY, with partings of orange and brown, silt and fine sand.	3.5	■	D				1.5	
				D	V	90 96		2.0	
				D				2.5	
				D	V	100 110		3.0	
				D				3.5	
				D	V	140+ 140+		4.0	
				D				4.5	
5.0				D	V	140+ 140+		5.0	
Borehole ends at 5.0m									
Drawn by: JP      Approved by: JH Remarks: Borehole dry and open on completion.		Key: T.D.T.D. Too Dense to Drive D Small Disturbed Sample      J Jar Sample B Bulk Disturbed Sample      V Pilcon Vane (kPa) U Undisturbed Sample (U100)      M Mackintosh Probe W Water Sample      N Standard Penetration Test Blow Count							

## **Chelmer Site Investigations**

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## **REPORT NOTES**

### **Equipment Used**

Hand tools, Mechanical Concrete Breaker and Spade, Hand Augers, 100mm/150mm diameter Mechanical Flight Auger Rig, GEO205 Flight Auger Rig, Window Sampling Rig, and Large or Limited Access Shell & Auger Rig upon request and/or access permitting.

### **On Site Tests**

By Pilcon Shear-Vane Tester ( $\text{Kn/m}^2$ ) in clay soils, and/or Mackintosh Probe in granular soils or made ground and/or upon request Continuous Dynamic Probe Testing and Standard Penetration Testing.

### **Note:**

Details reported in trial-pits and boreholes relate to positions investigated only as instructed by the client or engineer on the date shown.

We are therefore unable to accept any responsibility for changes in soil conditions not investigated i.e. variations due to climate, season, vegetation and varying ground water levels.

Full terms and conditions are available upon request.



CONSULTING CIVIL AND  
STRUCTURAL ENGINEERS  
PROJECT MANAGERS  
CDM REGULATIONS  
PARTY WALL SURVEYORS  
SUSTAINABLE PROJECT SPECIALISTS

Job No: 14008

Planning stage structural appraisal for proposed new basement construction

at 35 Buckland Crescent, London, NW3 5DJ

### 1.0 Existing property

The existing building is a four storey property constructed in a traditional manner incorporating solid external brick walls with timber upper floors set below a timber pitched roof. We understand the property was constructed around the latter half of the 19<sup>th</sup> century as a single family house. The property has been converted at some time in the past into three self contained flats. The property was originally constructed as semi-detached with the property on the left, number 37, but was later linked to the property on the right, number 33 by small infill connecting extensions.

A planning application is to be made for the construction of a new basement below the entire footprint of the lower ground floor flat; this covers the entire footprint of the property with the exception of the front left hand corner. The internal layout of the lower ground floor is also to be reconfigured in order to provide additional accommodation.

A modest rear extension is also to be constructed, as indicated on William Carter Ltd drawings in order to provide additional accommodation at lower ground and basement levels, including a basement level light well.

The new basement floor level is to be set approximately 2.5 meters below external ground level, as shown on the Architects drawings.

We understand that neither of the adjoining property currently has a basement below lower ground floor level.

TANKERTON WORKS  
12 ARGYLE WALK  
LONDON  
WC1H 8HA

David Rose Associates Ltd. Co. No 6248184  
Reg. Office. Hansford Brown, 1a Green Close, Brookmans Park, Hertfordshire,  
AL9 7ST

Director. Mr D.C Rose, BSc (Hons) C Eng. M.I.C.E. M.I Struct E.  
Associate Director: Mr R Andrews, I Eng. A.M.I. Struct E.

Tel: 020 7833 0666  
Fax: 020 7833 0669

The Institution  
of Structural  
Engineers



## **2.0 Consulting Engineers**

Rose Associates are chartered civil and structural engineers; MICE and MIStructE and ISO 9001 accredited. We have been appointed as Consulting Structural Engineers for the proposed basement and refurbishment works. This will include the design and specification of the basement structure and associated underpinning works.

The underpinning works will be designed and detailed so that the works will not pose any undue risk to the host or neighbouring properties.

## **3.0 New Basement**

It is recommended that the basement structure is undertaken using traditional underpinning techniques, where short sections of existing walls are undercut and new foundations cast at a lower level to suit the new basement depth. The underpinning shall be performed to a set designed sequence on a "hit and miss" basis, following good building practice.

As the underpinning works proceed the new internal basement walls can also be constructed. The underpin sections shall be anchored sufficiently at their base or the concrete underpins suitably back propped to prevent slippage in the temporary condition when the bulk of the earth is removed from the basement. These works are all prior to the new basement slab being cast.

It is our considered opinion that the works will not pose any undue risk to the stability of the existing or surrounding buildings.

Pre-start survey work should also include an appraisal of the existing ground floor construction as the floor may be ground bearing or suspended, supported on traditional brick sleeper walls. The floor may need to be replaced or additionally supported using new beams at basement ceiling level as works proceed.

An excavation sequence together with supplementary sketches has also been attached to this report to further clarify a safe working method.

Reference to geological maps for this area as well as local knowledge shows that the building over lies London Clay. Clay does not allow the free flow of ground water so we do not expect there to be any hydrological issues to deal with when executing the basement works. However any ground water encountered during the excavation works, which would be via seepage through sand lenses sometimes found in London Clay, then this water will be dealt with by a system of sumps and pumps. In the unlikely event a perched water table is encountered a strategy shall be developed to evaluate where the collected water is to be pumped. If this is to the existing underground drainage system appropriate filtration systems must be agreed with the relevant authorities.

The basement construction will involve the excavation of around 3.0 metre depth of material below the existing level ground floor.

When the basement area is excavated there will be a degree of recovery in the remaining clay due to the weight of material removed however, as the ground conditions are clay this recovery will be relatively slow.

If recovery does occur during the construction process it will take the form of a small "pudding" in the middle of the excavation which will be trimmed down for the basement slab construction.

Much of the load removed will be replaced by the new concrete retaining wall and basement floor slab structures.

It is our considered opinion that any additional long term heave recovery of the clay that may occur will not cause damage to neighbouring properties in excess of "Burland Category Slight".

This report has been prepared following a review of the Basement Impact Assessment.

**4.0 Summary**

We believe the proposals shown forming this planning application have a viable structural solution as set out in this report. Our recommendations are broadly as shown on the Architects drawings and described in their Design and Access statement.

In summary we believe they can be safely executed without undue risk to the existing building or the adjoining properties.

Signed

A handwritten signature in black ink, consisting of several fluid, overlapping strokes that form a stylized, somewhat abstract shape.

For and on behalf of Rose Associates Ltd

Dated

6-2-14



**35 Buckland Crescent**  
**Job ref: 14008**

**Excavation sequence**

This sequence is to be read in conjunction with drawings 14008/P01 and P05.  
Underpinning to be excavated and cast using a traditional 1, 3, 5, 2, 4 sequence.

**Stage 1**

As existing. Remove existing floor, as necessary, to provide access to the underpinning works.

**Stage 2**

Bulk excavate in the centre of the building leaving 45° battered earth surfaces to protect the existing foundations.

**Stage 3**

Excavate locally below existing walls for the first sequence in the underpinning and provide planking and strutting on the central earth earth berm.

**Stage 4**

Place reinforcement including dowel bars to adjoining sections and cast base slab for sequence 1.

**Stage 5**

Remove planking and strutting and place reinforcement for wall including dowel bars to adjoining sections in the sequence to be cast later and erect wall framework propped off the central earth berm. Cast concrete wall sequence sections 1.

**Stage 6**

Dry pack between the top of the concrete wall and the underside of the existing foundation. When concrete has gained sufficient strength remove shutter, trim existing protecting foundation and re prop wall into the central earth berm.

**Stage 7**

Continue to excavate and cast floor slab and wall underpin sections 3, 5, 2, 4 in sequence using stages 3 to 6 as above.

**Stage 8**

Commence excavation of the central berm down to 500mm above the top of the basement slab level. Install peri multi-props or similar between opposite underpin sections across the width of the site including perimeter waling beams and bracing as necessary.

**Stage 9**

Complete excavation down to slab formation level.

**Stage 10**

Place reinforcement and cast central section of basement slab.

**Stage 11**

Once slab has cured, remove props and waling beams.

# BGS Borehole Index





**British Geological Survey**

NATURAL ENVIRONMENT RESEARCH COUNCIL

BGS ID: 15020820 : BGS Reference: TQ28SE1769

British National Grid (27700) : 526800,184300

Report an issue with this borehole

<<

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Next >

>>

THAMES CA

AC NO 45355

256

TQ28/209

Project No: 400

Borehole Number: 1

Project: Swiss Cillage

Engineer: Gifford & Potts NGR: TQ 268 843

Client: Camden BC

Elevation: 56m OAD

Kinley Hill Farm,  
Hawthorn, Seaham,  
County Durham, SR7 8SW.  
Tel: 0191 527 3970 (Northern)  
Tel: 01473 236611 (Southern)

TQ28SE/1769

SUBSURFACE PROFILE				INSTALLATION DETAILS		Remarks
Depth	Legend	Description	Elevation	Well Completion Details		
-2		Ground Surface	56.00			
0		Bricks and rubble				Cable Percussion boring at 10" to 9.0m BGL.
1		Dark brown sandy soil				Rotary mud flush drilling 9.0-15.7m BGL
2		Pale brown clay				Drill at 1.5/8" diameter: 9.0-117.0m BGL
3		Pale brown mottled clay	50.80			Permanent mild steel casing 6" diameter GL-117m BGL
4		Brown clay. Laminated from 5.2-9m				Drill at 5.5/8" diameter: 117.0-157m BGL
5						113/103mm uPVC liner installed full depth of hole.
6						Borehole acidized using 2t of 28% HCl.
7						Constant rate pump test carried out for 3 days and 1 day recovery
8						
9						
10						
11						
12						
13						
14						
15						
16						
17			38.50			
18		Grey clay				
19						
20						
21						
22						
23						

Drilled By: N Snowball/L Berry

Date acidized: 2 Nov 04

Logged by: D Miller

Date geophysically logged: n/a

Date step tested: 9-13 Nov 04

Sheet: 1 of 7

ACC NO 46102.

7028/209


**From:** sjenkins@btconnect.com  
**To:** <hydroenq@bgs.ac.uk>  
**Date:** Tue, Aug 15, 2006 5:34 pm  
**Subject:** Swiss Cottage Pump Test - Rosemary Fry

Rosemary  
Please find attached the pump test data for Swiss Cottage. Not quite a dry hole but close.  
Regards

Steve Jenkins  
Contracts Manager  
Drilcorp Ltd  
T 0191 5273970  
F 0191 5273115  
M 07743 806302

yield less than 1 m<sup>3</sup>/hour during pump test.  
Recovery slow.  
Balance in use but will need modifications.




CASING DEPTH (m)	WATER (m)	STRATA DESCRIPTION	DEPTH (m)	TEST RESULTS		SUB-SAMPLING				
				TYPE AND DEPTH	RESULT	SPCM (m)	TO (m)	TYPE		
		Dunse grey reinforced concrete with 6mm twisted bar at 100mm depth. <b>(MADE GROUND)</b>	0.0 0.15							
		Hard dark brown slightly gravelly CLAY with occasional roots. Gravel consists of brick and ash. <b>(MADE GROUND)</b>	0.5							
		Very stiff brown slightly gravelly CLAY with occasional roots. Gravel consists of flint, brick and ash. <b>(MADE GROUND)</b>	1.0	P 1.0m	2.75, 2.75					
				P 1.25m	3.75, 3.75					
				P 1.5m	3.75, 3.75					
				P 1.75m	4.25, 4.0					
				P 2.0m	3.75, 3.75					
				P 2.25m	3.75, 4.0					
			2.5	P 2.5m	2.75, 2.5					
		Stiff brown occasionally mottled light grey CLAY. <b>(LONDON CLAY)</b>		P 2.75m	2.5, 2.5					
				P 3.0m	2.5, 2.25					
				P 3.25m	2.25, 2.0					
DRILLING		GROUNDWATER								
TYPE (DIAMETER):	FROM	TO	DEPTH STRUCK	BEHAVIOUR	DEPTH SEALED	DATE	DEPTH OF CASING			
101mm	0.0m	5.0m	Dry							
REFER TO KEY AT BEGINNING OF THIS APPENDIX FOR EXPLANATION OF SYMBOLS										
DRIVEN TUBE SAMPLER BOREHOLE SHEET 1 OF 2										
 <b>SOILTECHNICS</b> GEOTECHNICAL ENGINEERS, ENVIRONMENTAL CONSULTANTS <small>Cedar Barn, Vinteo Lodge, Weybridge, Surrey TW20 2EX            Tel: (01824) 751277 Fax: (01824) 751007 E-mail: info@soiltechnics.co.uk</small>				GROUND LEVEL		OO-COORDINATES				
				LOCATION PLAN ON DRAWING No		DATE OF EXCAVATION				
				STD0953U-02		06.03.07				
				PROJECT						
No's 3, 5 & 7, Fitzjohn's Avenue, London Proposed Residential Development										
PROJECT REF					BOREHOLE No					
STD0953U					DTS01					

CASING DEPTH (m)	WATER PH	STRATA DESCRIPTION	LOGG	DEPTH (m)	TEST RESULTS		SBS SAMPLING		
					TYPE AND DEPTH	RESULT	FROM (m)	TO (m)	TYPE
					P 3.5m	2.5, 3.0			
					P 3.75m	3.0, 3.75			
		Very stiff brown occasionally mottled light grey CLAY. (LONDON CLAY)	[Pattern]	4.0	P 4.0m	3.5, 3.25			
				P 4.25m	3.5, 4.0				
				P 4.5m	3.25, 3.75				
				P 4.75m	3.25, 3.75				
		BOREHOLE TERMINATED at 5.0m	[Pattern]	5.0	P 5.0m	3.5, 3.5			

DRILLING			GROUNDWATER				
TYPE (DIAMETER)	FROM	TO	DEPTH STRUCK	BEHAVIOUR	DEPTH SEALED	DATE	DEPTH OF CASING
101mm	0.0m	5.0m	Dry				

REFER TO KEY AT BEGINNING OF THIS APPENDIX FOR EXPLANATION OF SYMBOLS  
 DRIVEN TUBE SAMPLER BOREHOLE SHEET 2 OF 2

 <b>SOILTECHNICS</b> GEOTECHNICAL ENGINEERS, ENVIRONMENTAL CONSULTANTS Co-Ce: Barn, White Lodge, Welgrave, Northampton, NN8 8PY Tel: (01804) 781877 Fax: (01804) 781807 E-mail: info@soiltechnics.net	GROUND LEVEL	COORDINATES
	LOCATION PLAN ON DRAWING No	DATE OF EXCAVATION
	PROJECT	
	PROJECT REF.	BOREHOLE No
	STD0953U	DTS01