Document Control

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1.0 INTRODUCTION

Geotechnical and Environmental Associates (GEA) has been commissioned by Gurney Consulting Engineers, on behalf of Dexbay Properties Limited, to complete a ground movement assessment for the proposed redevelopment of this site at 42 Caversham Road, London, NW5 2DS.

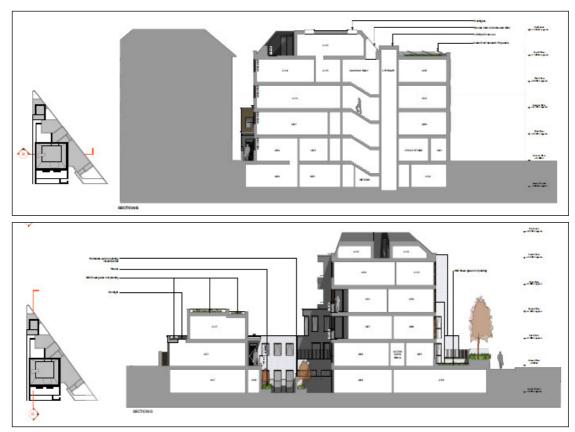
A Desk Study and Basement Impact Assessment (BIA) has also been carried out by GEA (report ref J14004, dated April 2015), the findings of which have been used in the derivation of parameters for use in this assessment. It should, however, be noted that due to the limited nature of the investigation a number of assumptions and a reliance on non-site specific information has been made, such that it would be prudent to carry out additional investigation once access is available to confirm the findings of this assessment.

The purpose of this assessment has been to determine the effects of the proposed basement construction upon the neighbouring structures.

1.1 **Proposed Development**

It is understood that consideration is being given to the demolition of the existing building and construction of a new two-storey to five-storey concrete framed building with single level basement to comprise residential flats. It is understood that the maximum column load at foundation level is anticipated to be 800 kN.

The proposed basement, or lower ground floor level, will extend to a depth of approximately 3.0 m below existing site level. A number of sections showing the proposed basement construction are below.



Ref J14023A Issue No 1 8 May 2015



The levels shown on the drawing are understood to be related to an arbitrary datum, the location of which is not known. These levels have not therefore been adopted for the purpose of this report, with all depths given relative to ground level.

This report is specific to the proposed development and the advice herein should be reviewed if the development proposals are amended.

1.2 Limitations

The conclusions and recommendations made in this report are limited to those that can be made on the basis of the investigation. The results of the work should be viewed in the context of the range of data sources consulted, the number of locations where the ground was sampled and the number of soil, gas or groundwater samples tested; no liability can be accepted for information in other data sources or conditions not revealed by the sampling or testing. Any comments made on the basis of information obtained from the client or other third parties are given in good faith on the assumption that the information is accurate; no independent validation of such information has been made by GEA.

2.0 THE SITE

2.1 Site Description

The site is located in Kentish Town within the London Borough of Camden, approximately 235 m to the west of St Lukes Church, 295 m southeast of Kentish Town London Underground station, and 745 m north of Camden Town railway station.

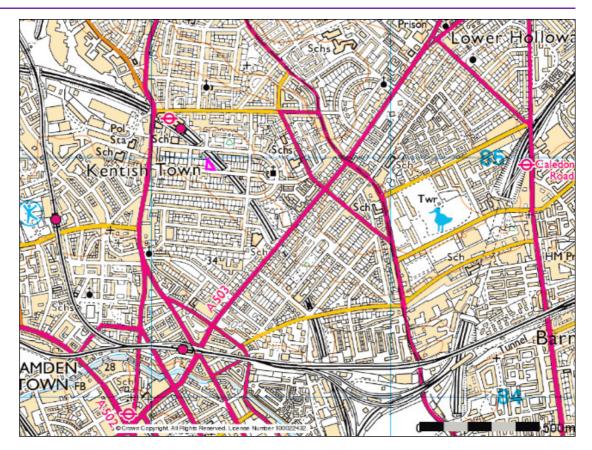
The site forms a triangular shape with maximum dimensions of approximately 35 m east-west by 40 m north-south and is occupied by a mixed use single storey to three-storey building fronting onto Caversham Road to the south. At the time of the site investigation the building was used as a Family Services and Social Work Centre and in view of the continued occupancy of the site the fieldwork was limited to the site frontage only.

The building occupies the majority of the site, with a narrow alleyway along the western boundary, a small garden and areas of hardstanding with delivery bays along the frontage. The garden has maximum dimensions of approximately 7 m by 10 m and is laid to lawn with 3 m tall semi-mature deciduous trees around the perimeter. A ramp leads down to a delivery door approximately 0.50 m below street level.

The site is bordered by three-storey houses with semi-basements to the south and west, a builder's yard to the east and a Network Rail railway line to the north. The site is essentially level and the local topography slopes down towards the west.

The site may be additionally located by National Grid Reference 529249, 184960 and is shown on the map below.





3.0 SUMMARY OF GROUND CONDITIONS

The investigation encountered the expected ground conditions in that, below a moderate thickness of made ground, London Clay was encountered and proved to the maximum depth investigated of 5.45 m. The made ground generally comprised dark brown, grey and brown slightly sandy silty clay with gravel of flint, brick, tile, ash and clinker and occasional roots, with horizons of black ash gravel and orange-brown coarse sand, and extended to a depth of 1.40 m. in Borehole No 2 and the maximum depth investigated in Trial Pit No 1 of 1.50 m.

The London Clay comprised firm fissured grey and brown mottled clay with occasional selenite crystals and decomposing roots, frequent black clay pockets and rare black flint gravel, becoming brown mottled blue-grey with depth, and extended to the maximum depth investigated of 5.45 m.

In the western section of the site frontage, beneath a maximum 300 mm thickness of reinforced concrete, Borehole Nos 1 and 1A encountered made ground comprised of dark brown slightly sandy silty clay with gravel of brick, flint and ash was, which extended to depths of 0.50 m and 0.65 m, wherein a concrete slab was encountered. The concrete slab is approximately 0.85 m below street level and it is unknown what this may represent. It was not possible to extend Borehole Nos 1 and 1A beyond this depth.

Groundwater was not encountered during drilling of the boreholes, but subsequent monitoring of a standpipe installed to a depth of 5.00 m recorded groundwater at a depth of 1.15 m four weeks after installation.



3.1 British Geological Society (BGS) Archives

A search of borehole records held by the British Geological Survey (BGS) has indicated that the London Clay was found beneath a moderate thickness of made ground, at a distance of 100 m to the southwest of the site. Another borehole, drilled 450 m to the south indicated the base of the London Clay to be at a depth of 52 m, at a level of approximately -22 m OD.

4.0 CONSTRUCTION SEQUENCE

The following sequence of operations has been derived to enable analysis of the ground movements around the basement both during and after construction. The exact method of construction is not known at the present stage and it has therefore been assumed that the proposed basement will be formed with a contiguous piled wall, with a minimum embedment equivalent to the retained height of approximately 3.0 m.

Essentially the sequence may be considered as three groups of activities, the first two comprising the short and medium term temporary works whilst the third represents the construction of the permanent works.

The detail of the support provided to adjacent walls is beyond the scope of this report at this stage and the structural engineer will be best placed to agree a methodology with the piling contractor once appointed.

4.1 **Temporary Support to Piled Walls**

Following the installation of the bored pile wall and capping beams at ground floor level, temporary props will be installed and the basement excavation will proceed. The detail of section sizes and spacings will be finalised by the contractor but it is anticipated that the general philosophy adopted will be for diagonal braces to be used across the corners or returns of the basement walls whilst props will be positioned at regular intervals along the long walls of the basement. Where horizontal restraint cannot be provided by other parts of the piled wall the prop forces will be provided by so-called 'flying shores' where the reaction to horizontal forces is provided by pile caps, gravity blocks or basement thickenings in the centre of the excavation.

It is anticipated that steel temporary props will be used with strut forces spread along the wall by steel waling beams fixed to the piles. Although the detail of the propping is to be finalised there is the option to use hydraulic 'active' props where the propping force is applied prior to excavation in order to minimise movement at critical locations.

Excavation will proceed in stages and in broad terms the order of operations will be install capping beam props, excavate to a suitable depth below the next propping level, install props and then repeat the operation until the final excavation level has been reached.

4.2 **Permanent Works**

When the final excavation depths have been reached the permanent works will be formed, which are likely to comprise reinforced concrete walls with a drained cavity lining the inside of the bored pile wall. Reinforced concrete will be used for floor slabs and it is anticipated that heave protection will be installed beneath the lowest slabs.



It is anticipated that the floor slabs will be constructed lowest level first and when each floor has achieved adequate strength, the temporary props will be removed and the subsequent walls and floors cast until the structure is complete.

5.0 GROUND MOVEMENTS

An assessment of ground movements within and surrounding the excavation has been undertaken using the X-Disp and P-Disp computer programs licensed from the OASYS suite of geotechnical modelling software from Arup. These programs are commonly used within the ground engineering industry and are considered to be appropriate tools for this analysis.

The X-Disp program has been used to predict ground movements likely to arise from the construction of the proposed basement. This includes the settlement of the ground (vertical movement) and the lateral movement of soil behind the proposed retaining walls (horizontal movement).

The analysis of potential ground movements within the excavation, as a result of unloading of the underlying soils, has been carried out using the Oasys P-Disp (Version 19.2 – Build 12) software package and is based on the assumption that the soils behave elastically, which provides a reasonable approximation to soil behaviour at small strains.

For the purpose of these analyses, the corners have been defined by x and y coordinates, with the x-direction parallel with the orientation north-south, whilst the y-direction is parallel with the orientation of east-west. Vertical movement is in the z-direction.

The full outputs of all the analyses can be provided on request but samples of the output movement contour plots are included within the appendix. As mentioned previously, due to the limited nature of the investigation, further site work in the form of a number of deep boreholes is likely to be necessary to support the assumptions made with respect to the London Clay beneath the site and confirm the findings of this assessment.

5.1 Ground Movements – Surrounding the Basement

5.1.1 Model Used

For the X-Disp analysis, the soil movement relationships used for the embedded retaining walls are the default values within CIRIA report C580¹, which were derived from a number of historic case studies.

The ground movement curves for 'excavations in front of high stiffness wall in stiff clay' have been adopted as being considered most appropriate for the proposed excavation and its support at this site.

5.1.2 Results

An assessment of ground movements surrounding the excavation has been undertaken by GEA using the X-Disp computer program licensed from the OASYS suite of programmes from Arup. The predicted movements are summarised in the table below. The results are presented in the following tables to the degree of accuracy required to allow predicted variations in ground movements around the structure(s) to be illustrated, but may not reflect the anticipated accuracy of the predictions.



¹ Gaba, A, Simpson, B, Powrie, W and Beadman, D (2003) *Embedded retaining walls – guidance for economic design* .CIRIA Report C580.

Phase of Works	Wall Movement (mm)				
	Vertical Settlement	Horizontal Movement			
Pile Installation	1 to 1.25	1 to 1.25			
Basement Excavation	2 to 2.5	4 to 5			
Combined Movements	3 to 4	5 to 6			

The analysis has indicated that the maximum vertical and horizontal settlements that will result from pile installation are less than 1.5 mm. The maximum vertical settlement that will take place behind the walls as a result of the basement excavation is unlikely to exceed 2.5 mm with up to 5 mm of maximum horizontal movement.

The movements arising from the combined piling and excavation phases are therefore not likely to exceed 4 mm of vertical settlement, whilst the maximum horizontal movements are anticipated to be between 5 mm to 6 mm.

The movements calculated are considered to represent a worst case scenario, particularly as the movements resulting from basement excavation will be minimised due to control of the propping in the temporary works and a regime of monitoring.

5.2 Movements within the Excavation (Heave)

5.2.1 Model Used

At this site unloading of the London Clay will take place as a result of the basement excavation and the reduction in vertical stress will cause heave to take place. Undrained soil parameters have been used to estimate the potential short term movements, which include the "immediate" or elastic movements as a result of the basement excavation. Drained parameters have been used to provide an estimate of the total long-term movement.

The elastic analysis requires values of soil stiffness at various levels to calculate displacements. Values of stiffness for the soils at this site are readily available from published data and we have used a well-established method to provide our estimates. This relates values of E_u and E', the drained and undrained stiffness respectively, to values of undrained cohesion, as described by Padfield and Sharrock² and Butler³ and more recently by O'Brien and Sharp⁴. Relationships of $E_u = 500 C_u$ and E' = 300 C_u for the cohesive soils and 2000 x SPT 'N' for granular soils have been used to obtain values of Young's modulus. More recent published data⁵ indicates stiffness values of 750 x Cu for the London Clay and a ratio of E' to Cu of 0.75, but it is considered that the use of the more conservative values provides a sensible approach for this stage in the design.

The demolition of the existing buildings and proposed construction of the new basement will result in a net unloading of roughly 60 kN/m².



² Padfield CJ and Sharrock MJ (1983) *Settlement of structures on clay soils*. CIRIA Special Publication 27

³ Butler FG (1974) *Heavily overconsolidated clays: a state of the art review.* Proc Conf Settlement of Structures, Cambridge, 531-578, Pentech Press, Lond

⁴ O'Brien AS and Sharp P (2001) Settlement and heave of overconsolidated clays - a simplified non-linear method. Part Two,

Ground Engineering, Nov 2001, 48-53

⁵ Burland JB, Standing, JR, and Jardine, FM (2001) *Building response to tunnelling, case studies from construction of the Jubilee Line Extension.*. CIRIA Special Publication 200

A rigid boundary for the analysis has been set within the London Clay at a depth of about 50 m below existing ground level, where nearby BGS records indicate that the base of this formation is likely to be present. Below this depth the essentially incompressible soils of the Lambeth Group should be present.

5.2.2 Results

The P-Disp analysis indicates that, by the time the basement construction is complete, up to 12 mm of heave is likely to have taken place at the centre of the proposed excavations, reducing to approximately 3 mm to 6 mm at the edges.

In the long term, following completion of the basement construction, a further 10mm of heave is estimated as a result of long term swelling of the underlying London Clay.

The results of the P-Disp analysis also indicate the likely impact of the proposed basement construction beyond the site boundaries. On the basis of the analysis, total vertical heave movements outside the proposed basement are unlikely to exceed 5 mm at a distance of approximately 5 m, reducing to less than 1 mm at distances in excess of between 10 m.

Leasting	Movement (mm)					
Location	Short-term Heave (Excavation Phase)	Long-term Heave (post construction)	Total Heave			
Centre of excavations	12	10	22			
Edge of excavations	3 to 6	3 to 6	6 to 12			
At 5 m from edge of excavations	2 to 3	1 to 2	3 to 5			

The potential movements are summarised in the table below.

In order to mitigate the effects of heave on the new building, the basement could be designed to transmit heave forces into the wall piles or onto tension piles within the basement.

Alternatively, or in any case, a void or layer of compressible material should be incorporated into the design to accommodate these potential long term movements. If a compressible material is used beneath the slab, it will need to be designed to be able to resist the potential uplift forces generated by the ground movements. In this respect potential heave pressures are typically taken to equate to around 50 % to 60 % of the total unloading pressure.

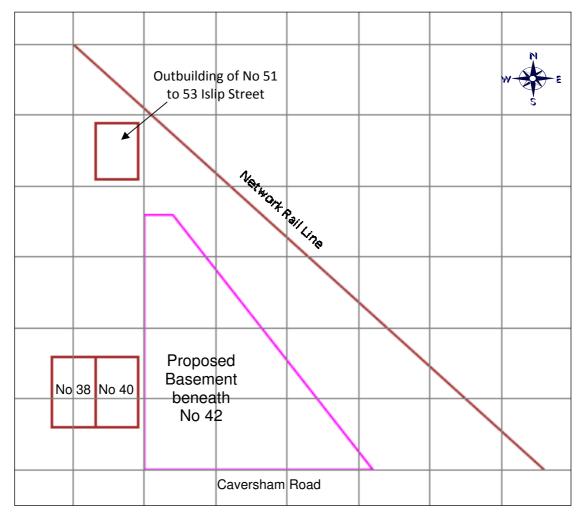
6.0 DAMAGE ASSESSMENT

In addition to the above assessment of the likely movements that will result from the proposed development, some of the neighbouring structures have been considered as sensitive structures, requiring Building Damage Assessments, on the basis of the classification given in Table 2.5 of C580¹. These include:

- The adjoining semi-detached property of 38 to 40 Caversham Road, to the west;
- a single-storey structure, comprising part of the Boma Garden Centre (51 to 53 Islip Street) to the northwest; and
- the network rail line to the northeast of the site.



The sensitive structures outlined above have been modelled as lines in the analysis and are the lines along which the damage assessment has been undertaken. The critical lines for the remaining sensitive structures are shown on the plan below.



Whilst Nos 38 and 40 Caversham Road are known to have lower ground floor levels, it has been assumed that the outbuilding to the northwest does not have an existing basement, such that the average foundation depths can therefore be assumed as being close to existing ground level.

6.1 **Damage to Neighbouring Structures**

The combined movements resulting from both pile installation and basement excavation calculated using the X-Disp modelling software have been used to carry out an assessment of the likely damage to adjacent properties and the results are summarised in the table below.

The potential heave movements predicted by P-Disp have not been included in this assessment, which can therefore be considered as conservative, as these movements are likely to have a mitigating effect on the downward settlement predicted by X-Disp.



	Building Damage Assessment			
Sensitive Structure	Elevation	Category of Damage*		
	Northern	Category 0 (Negligible)		
	Eastern	Category 0 (Negligible)		
No 38 to 40 Caversham Road	Southern	Category 0 (Negligible)		
	Western	Undetectable		
	Party Wall	Category 0 (Negligible)		
	Northern	Undetectable		
Outbuilding of No 51 to 52 Idia Dood	Eastern	Category 0 (Negligible)		
Outbuilding of No 51 to 53 Islip Road	Southern	Category 0 (Negligible)		
	Western	Category 0 (Negligible)		
Network Rail Line	N/A	Category 0 (Negligible)		

*From Table 2.5 of C580¹: Classification of visible damage to walls.

The building damage reports for sensitive structures highlighted in the above table predict that the damage to the adjoining and nearby structures would generally be Category 0 (negligible).

On this basis, the damage that would inevitably occur as a result of such an excavation would fall well within the acceptable limits.

6.2 Monitoring of Ground Movements

The predictions of ground movement based on the ground movement analysis should be checked by monitoring of adjacent properties and structures. The structures to be monitored during the construction stages should include:

- The adjoining semi-detached property of 38 to 40 Caversham Road, to the west;
- □ the single-storey structure, comprising part of the Boma Garden Centre (51 to 53 Islip Street) to the northwest; and
- the network rail line to the northeast of the site.

Condition surveys of the above existing structures should be carried out before and after the proposed works.

The precise monitoring strategy will be developed at a later stage and it will be subject to discussions and agreements with the owners of the adjacent properties and structures. Contingency measures will be implemented if movements of the adjacent structures exceed predefined trigger levels. Both contingency measures and trigger levels will need to be developed within a future monitoring specification for the works.



7.0 CONCLUSIONS

The analysis has concluded that the predicted damage to the neighbouring properties would be 'Negligible'. On this basis, the damage that would inevitably occur as a result of such an excavation would fall well within the acceptable limits.

The separate phases of work, including formation of the lower ground floor extension, piling and subsequent excavation of the proposed basement, will in practice be separated by a number of weeks during which time construction of capping beams and pile curing will take place. This will provide an opportunity for the ground movements during and immediately after piling to be measured and the data acquired can be fed back into the design and compared with the predicted values. Such a comparison will allow the ground model to be reviewed and the predicted wall movements to be reassessed prior to the main excavation taking place so that propping arrangements can be adjusted if required.



APPENDICES

SOIL DISPLACEMENT MODEL RESULTS

X-DISP ANALYSIS

Pile Installation

Contour Plots of Vertical Movements and Horizontal Movements

Basement Excavation

Contour Plots of Vertical and Horizontal Movements

Pile Installation and Basement Excavation

Contour Plots of Combined Vertical Movements and Horizontal Movements

P-DISP ANALYSIS

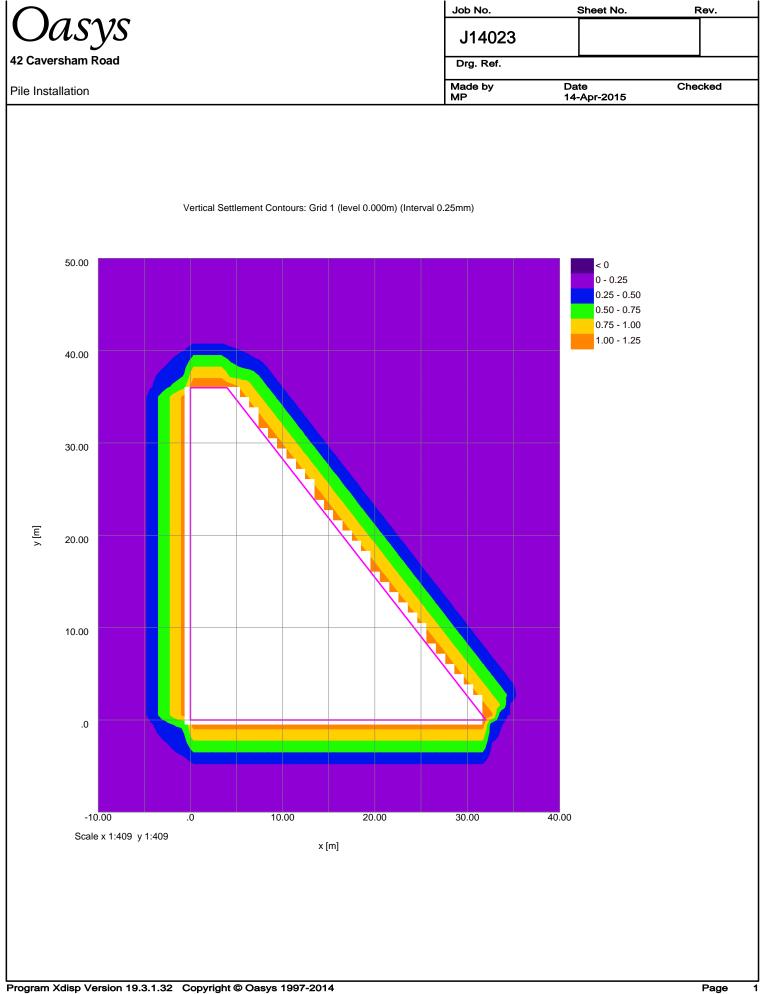
Short Term Movement

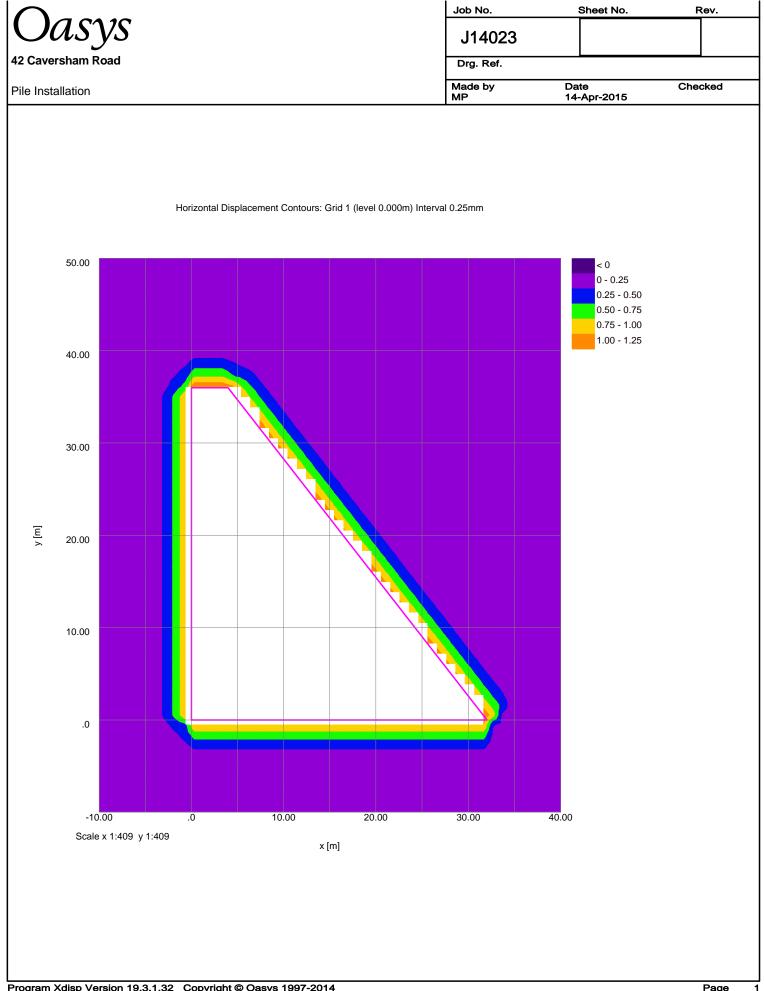
Total Movement

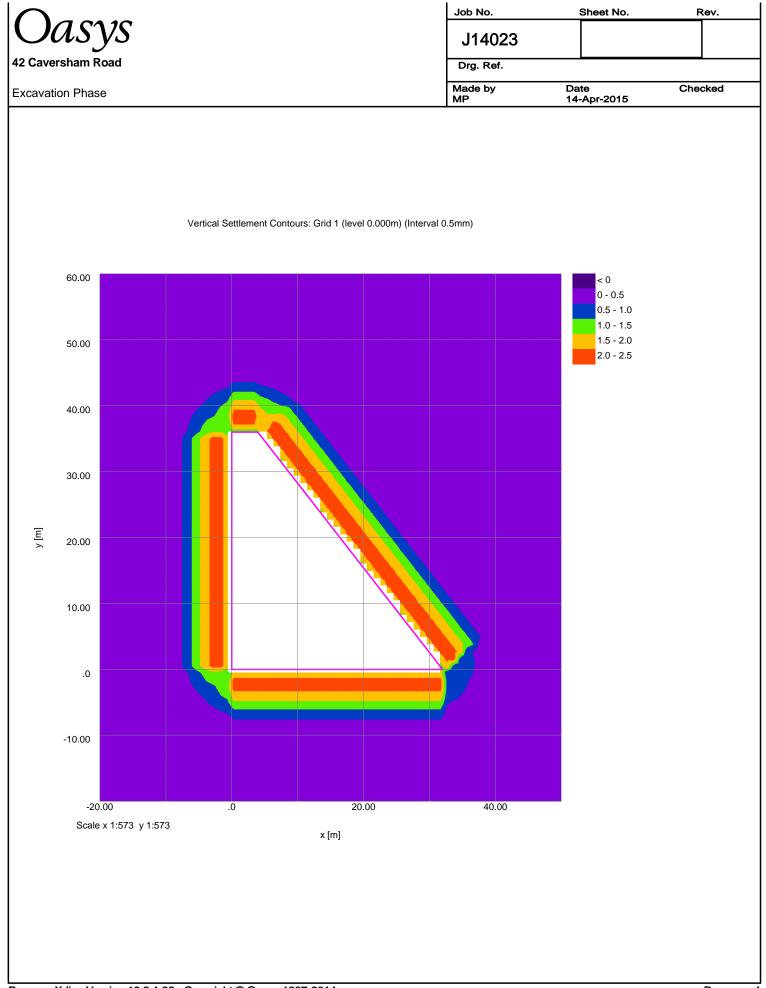
BUILDING DAMAGE ASSESSMENT (X-DISP)

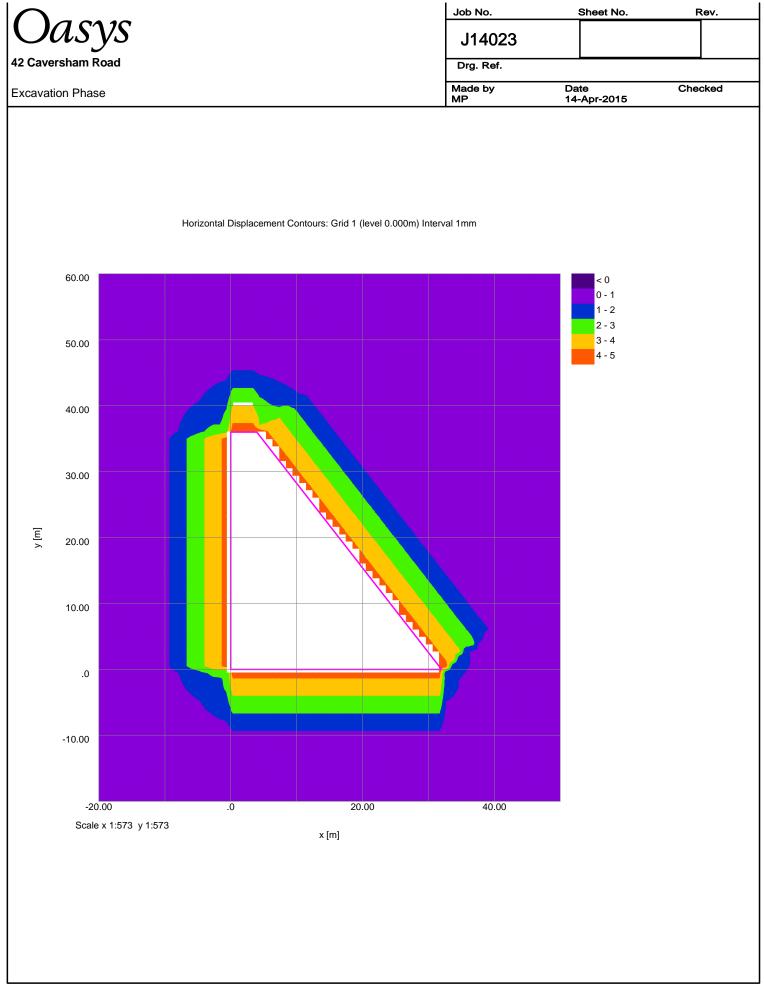
Tabular Output of Results

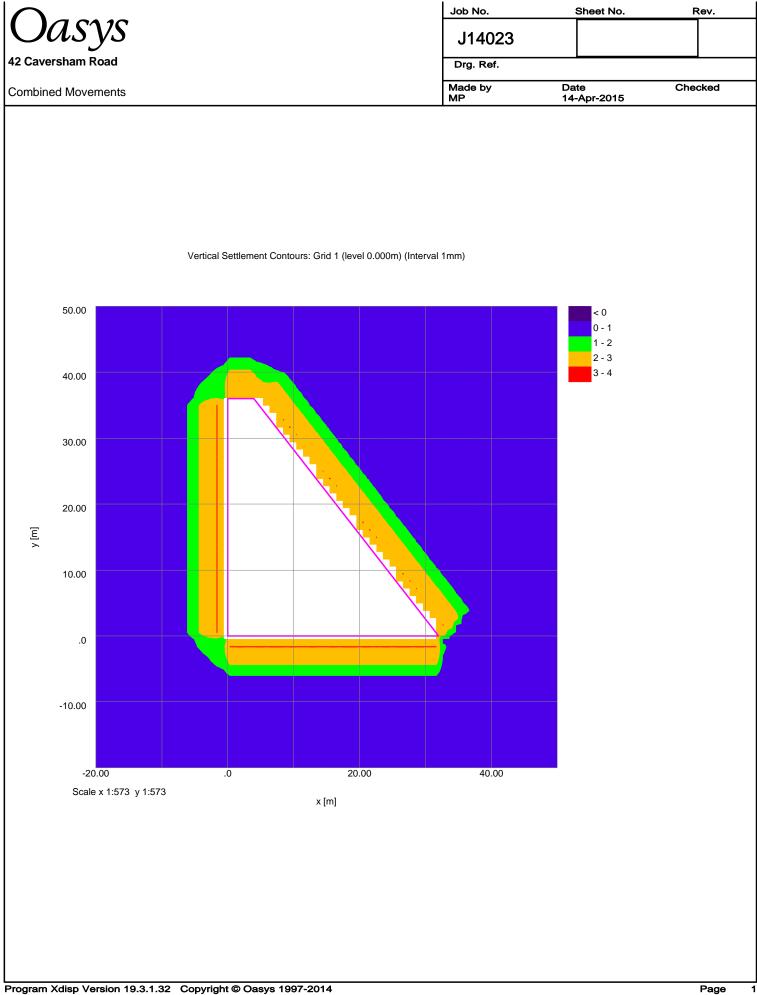


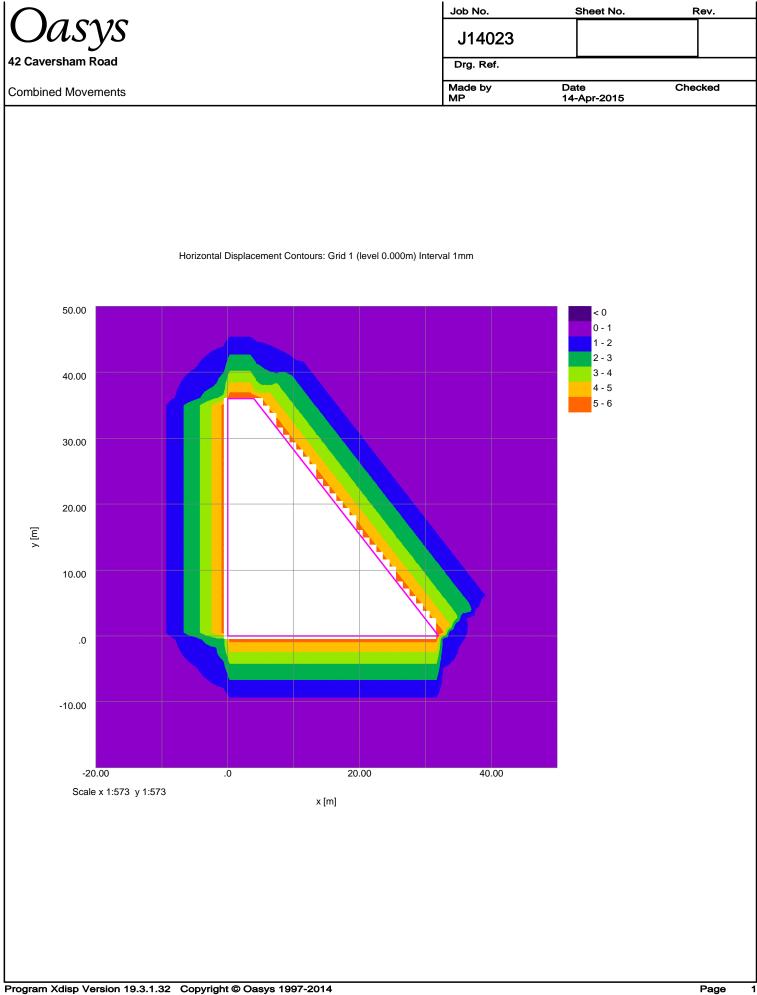


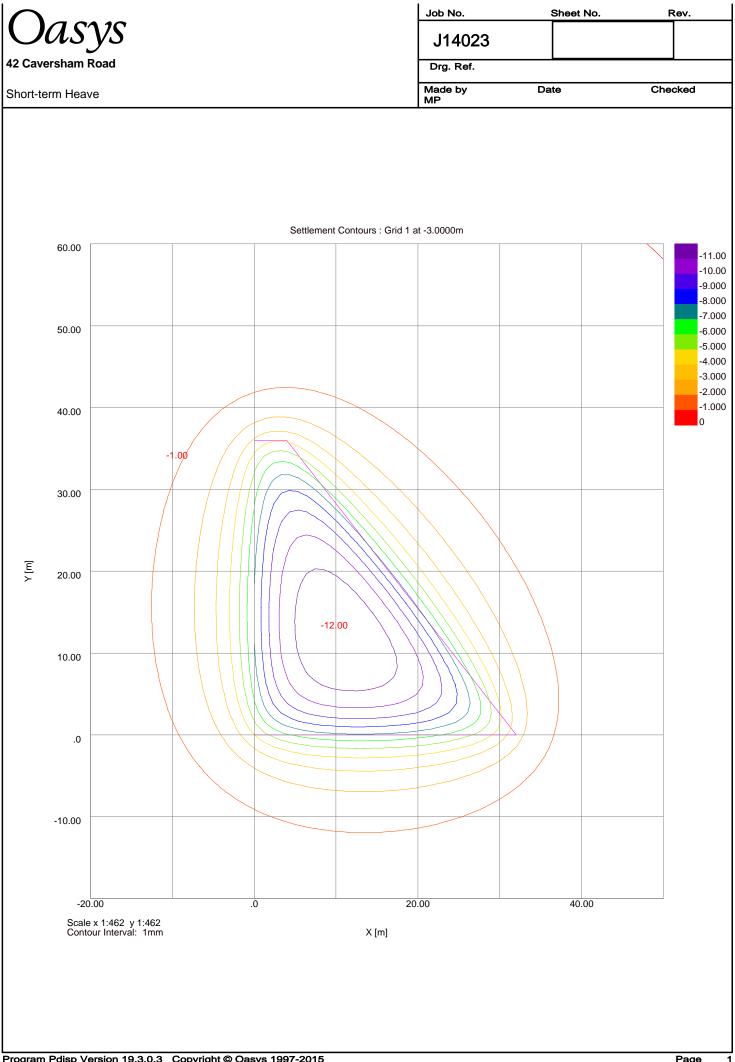


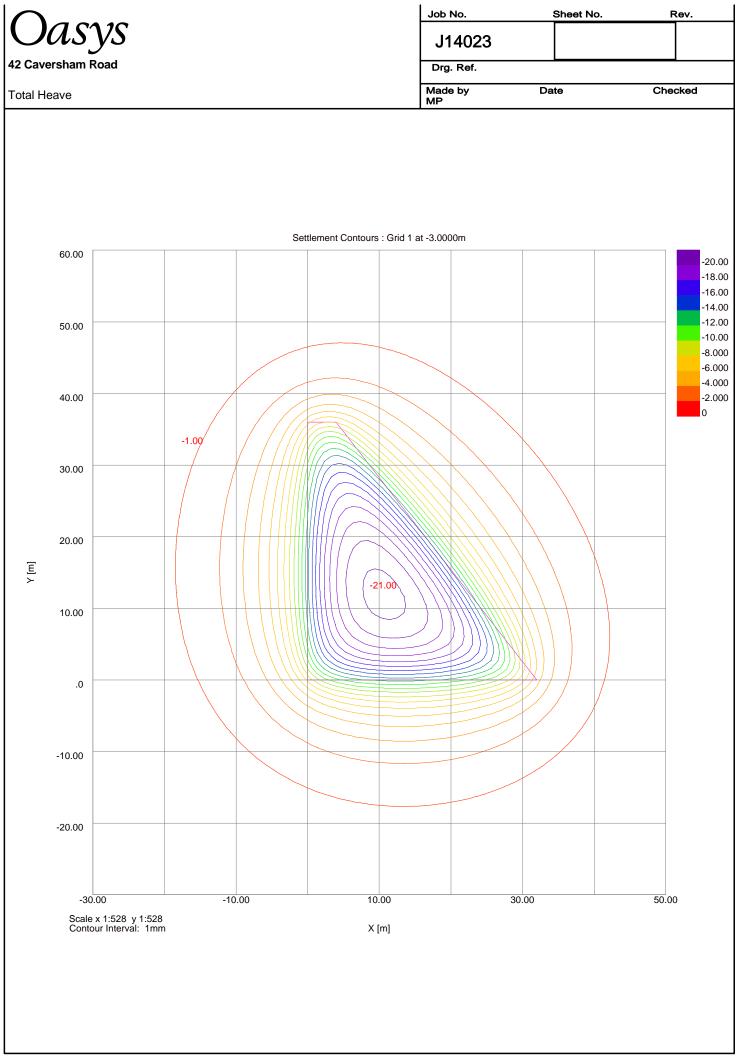












\frown	Job No.	Sheet No.	Rev.
Oasys	J14023		
42 Caversham Road	Drg. Ref.		
Damage Assessment	Made by MP	Date 14-Apr-2015	Checked

Specific Building Damage Results - Horizontal Displacements Structure: No 38 to 40 | Sub-structure: Southern Elevation

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1.0000 2.0000 3.0000	[m] [m] -1.00000 41.00 -2.00000 41.00 -3.00000 41.00 -4.00000 41.00 -5.00000 41.00 -6.00000 41.00 -7.00000 41.00	0000 -0.50000 0000 -0.50000 0000 -0.50000	0.54137 0.65823 0.68697	-1.35	918 -0.318 534 -0.541 970 -0.658 871 -0.686	36 .37 123 97	b Line [mm] 1.5918 1.3534 1.0970 0.85871 0.65349 0.52265 0.39676
	re: Outbuildir		ub-struct				
2.0000 3.0000 4.0000 5.0000 6.0000 7.0000	coordin x y [m] [m] -1.0000 41.00 -1.0000 43.00 -1.0000 45.00 -1.00000 45.00 -1.00000 47.00 -1.00000 48.00 -1.00000 49.00	z [m] 0000 -0.50000 0000 -0.50000 0000 -0.50000 0000 -0.50000 0000 -0.50000 0000 -0.50000	0 0.1671 0 0.1177 0 0.07878 0 0.04718 0 0.02107 0 0.	Y [mr 6 -1.9 7 -1.3 1 -1.3 8 -0.94 0 -0.70 4 -0.4 9 -0.23 0	the I n] [mn 5918 -1.5 3882 -1.3 1697 -1.1 4228 -0.94 0902 -0.70 7184 -0.47 3187 -0.23	g Per ine 918 882 697 228 902 184	Pendicular to Line [mm] -0.31836 -0.23137 -0.16711 -0.11778 -0.078780 -0.047184 -0.021079 0.0 0.0

$\bigcap a c v c$	Job No.	Sheet No.	Rev.
Oasys	J14023		
2 Caversham Road	Drg. Ref.	Date	Checked
Damage Assessment	Made by MP	14-Apr-2015	Checked
x y z x y Along Perpendicular [m] [m] [m] [m] the Line to Line [m] [m] [mm] [mm] [mm] [mm] Structure: Outbuilding to NW Sub-structure: Northern Elevation			
Dist. Coordinates Displacements x y z x y Along Perpendicular the to Line Line			
[m] [m] [m] [mm] [mm] [mm] 0.0 -1.00000 49.00000 -0.0000 0.0 0.0 0.0 0.000 -2.00000 49.00000 -0.50000 0.0 0.0 0.0 2.0000 -3.00000 49.00000 -0.50000 0.0 0.0 0.0 2.0000 -3.00000 -0.50000 0.0 0.0 0.0 0.0 3.0000 -4.00000 -0.50000 0.0 0.0 0.0 0.0			
1.0000 -9.00000 -9.00000 -0.00000 -0.00000 1.0000 -5.00000 -0.50000 0.0 0.0 0.0 5.0000 -6.00000 49.00000 -0.50000 0.0 0.0 0.0 6.0000 -7.00000 49.00000 -0.50000 0.0 0.0 0.0			
Structure: Outbuilding to NW Sub-structure: Western Elevation Dist. Coordinates Displacements x y z x y Along Perpendicular			
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			
6.0000 -7.0000 43.0000 -0.50000 0.27849 -0.27849 0.27849 0.27849 7.0000 -7.0000 42.0000 -0.50000 0.40898 -0.35056 0.35056 0.40898 8.0000 -7.00000 41.00000 -0.50000 0.55546 -0.39676 0.39676 0.55546			
Structure: Network Rail Line Sub-structure: Sub 10 Dist. Coordinates Displacements x y z x y Along the Perpendicular Line to Line			
[m] [m] [m] [mm] [m] [m]			
4.0088 53.03371 2.69663 0.00000 0.0 0.0 0.0 5.010 52.29213 3.37079 0.00000 0.0 0.0 0.0 6.0132 51.55056 4.04494 0.00000 0.0 0.0 0.0 7.0154 50.80899 4.71910 0.00000 0.0 0.0 0.0			
3.0177 50.06742 5.39326 0.00000 0.0 0.0 0.0 0.0 0.0 5.0199 49.32584 6.6742 0.00000 0.0 0.0 0.0 0.0 10.022 48.58427 6.74157 0.00000 0.0 0.0 0.0 0.0 0.0 11.024 47.84270 7.41573 0.00000 0.0 0.0 0.0 0.0 0.0 12.024 47.0112 8.08989 0.00000 0.0 0.0 0.0 0.0			
13.029 46.35955 8.76404 0.00000 0.0 0.0 0.0 0.0 0.0 14.031 45.61798 9.4382 0.00000 0.0 0.0 0.0 0.0 15.033 44.87640 10.11236 0.00000 0.0 0.0 0.0 0.0 0.0 16.035 44.13483 10.78652 0.00000 0.0 0.0 0.0 0.0 0.0 17.038 43.39326 11.4667 0.00000 0.0 0.0 0.0 0.0			
18.040 42.65169 12.13483 0.00000 0.0 0.0 0.0 19.042 41.91011 12.8089 0.00000 0.0 0.0 0.0 20.044 41.16854 13.48315 0.00000 0.0 0.0 0.0 21.046 40.42697 14.15730 0.00000 0.0 0.0 0.0 22.049 39.6833 14.83146 0.00000 0.0 0.0 0.0			
23.051 38.94382 15.05562 0.00000 0.0 0.0 0.0 24.053 38.20225 16.17978 0.00000 0.0 0.0 0.0 25.055 37.46067 16.85393 0.00000 0.0 0.0 0.0 26.057 36.71910 17.52809 0.00000 0.0 0.0 0.0 26.057 36.77953 18.20222 0.00000 0.0 0.0 0.0			
28.062 35.23596 18.9764 0.00000 0.0 0.0 0.0 29.064 34.4943 19.5505 0.00000 0.0 0.0 0.0 30.066 33.75281 20.22472 0.00000 0.0 0.0 0.0 31.068 33.01124 20.8988 0.00000 0.0 0.0 0.0 32.071 32.26966 21.5733 0.00000 0.0 0.0 0.0			
33.073 31.52809 22.24719 0.00000 0.0 0.0 0.0 34.075 30.7652 22.9213 0.00000 0.0 0.0 0.0 35.077 30.04494 23.59551 0.00000 0.0 0.0 0.0 36.079 23.0337 24.26966 0.00000 0.0 0.0 0.0 37.082 28.56180 24.94382 0.00000 0.0 0.0 0.0			
38.084 27.82022 25.61798 0.00000 0.0 0.0 0.0 93.068 27.07665 26.2913 0.00000 0.0 0.0 0.0 40.088 26.33708 26.96629 0.00000 0.0 0.0 0.0 41.090 25.59551 27.64045 0.00000 -0.025400 -0.015507 0.031703 42.093 24.85393 28.31461 0.00000 -0.076155 -0.1055723 0.016507			
42.095 24.11236 28.98876 0.00000 -0.12691 -0.098709 0.027508 0.15841 44.097 23.37079 29.66292 0.00000 -0.17767 -0.13819 0.038510 0.22176 45.099 22.62921 30.33708 0.00000 -0.22842 -0.17766 0.049511 0.28512 46.102 21.88764 31.01124 0.00000 -0.27918 -0.21714 0.060512 0.34847 47.104 21.14607 31.68539 0.00000 -0.3294 -0.25662 0.071514 0.41842			
48.106 20.40449 32.35955 0.00000 -0.38069 -0.29609 0.082515 0.47517 49.108 19.66292 33.03371 0.00000 -0.43145 -0.33557 0.093517 0.53853 50.110 18.92135 33.70787 0.00000 -0.48221 -0.37555 0.10452 0.60188 51.113 18.17978 34.38202 0.00000 -0.53296 -0.41453 0.11552 0.66523			
3.117 16.69663 35.73034 0.00000 -0.63447 -0.49348 0.13752 0.79194 14.19 15.95506 36.4049.000000 -0.68523 -0.53296 0.14852 0.85529 5.121 15.21348 37.07865 0.00000 -0.73599 -0.57243 0.15953 0.91865 6.124 14.47191 37.75281 0.00000 -0.78674 -0.61191 0.17053 0.98200			
57.126 13.73034 38.42697 0.00000 -0.83750 -0.65139 0.18153 1.0454 88.128 12.98876 39.10112 0.00000 -0.88826 -0.19023 1.1087 59.130 12.24719 39.77528 0.00000 -0.93901 -0.73034 0.20353 1.1721 50.132 11.5562 40.44944 0.00000 -0.98977 -0.76982 0.21453 1.2354 1.135 10.76404 41.12360 0.00000 -1.0495 -0.82553 1.2988			
52.137 10.02247 41.79775 0.00000 -0.77307 -0.74423 0.071406 1.0707 31.39 9.28090 42.4719 0.00000 -0.65719 -0.80541 -0.055495 1.0380 54.141 8.53933 43.14607 0.00000 -0.52190 -0.82161 -0.16650 0.95901 55.143 7.79775 43.82022 0.00000 -0.35127 -0.7302 -0.24848 0.84585 6.146 7.0518 4.49438 0.00000 -0.27704 -0.31297 0.75612			
57,148 6.31461 45,16854 0.00000 -0.17637 -0.69863 -0.33945 0.63559 81,150 5.57703 45,84270 0.00000 -0.93164 -0.82314 0.43401 59,152 4.83146 46,51685 0.00000 -0.03340 -0.42930 -0.26366 0.34049 70,154 4.08989 47,19101 0.00000 -0.016568 -0.24474 -0.16318 0.18242 71,157 3.34831 47.86517 0.00000 0.0 -0.055626 -0.34012 0.037413			
12:159 2:60674 48:5393 0:00000 0:0 0:0 0:0 73:161 1:86517 49:2134 0:00000 0:0 0:0 0:0 74:163 1:12360 49:88764 0:00000 0:0 0:0 0:0 75:166 0:38202 50:55180 0:00000 0:0 0:0 0:0 75:166 0:38205 51:23595 0:00000 0:0 0:0 0:0			
77.170 -1.101251.91011 0.00000 0.0 0.0 0.0 0.0 0.0 78.172 -1.84270 52.58427 0.00000 0.0 0.0 0.0 0.0 0.0 79.174 -2.58427 53.25843 0.00000 0.0 0.0 0.0 0.0 0.0 0.177 -3.2584 53.92585 0.00000 0.0 0.0 0.0 0.0			
82.181 -4.60899 55.28090 0.00000 0.0 0.0 0.0 82.183 -5.5505 55.5505 0.00000 0.0 0.0 0.0 84.185 -6.29213 56.62921 0.00000 0.0 0.0 0.0 85.188 -7.03371 57.3337 0.00000 0.0 0.0 0.0			
86.190 -7.77528 57.97753 0.00000 0.0 0.0 0.0 0.0 0.0 87.192 -8.1565 58.6516 0.00000 0.0 0.0 0.0 0.0 88.194 -9.25843 59.32584 0.00000 0.0 0.0 0.0 0.0 0.0 89.196 -10.00000 60.00000 0.00000 0.0 0.0 0.0 0.0			
Specific Building Damage Results - Vertical Displacements			
<pre>tructure: No 38 to 40 SuD-structure: Southern Elevation ist. Coordinates Displacements x y z z [m] [m] [m] [m] [mm]</pre>			

	Job No.	Sheet No.	Rev.
Oasys	J14023		
42 Caversham Road	Drg. Ref.		
Damage Assessment	Made by MP	Date 14-Apr-2015	Checked
Dist. Coordinates Displacements x y z z [m] [m] [m] [mm]			
Vertical Offset 1 0.0 13.0000 6.0000 -2.0000 0.0 1.000 -12.0000 6.0000 -2.0000 0.0 0.1 2.000 -11.0000 6.0000 -2.0000 0.0025761 2.000 -11.0000 6.0000 -2.0000 0.05370 3.000 -10.0000 6.0000 -2.0000 0.19370 5.000 -8.0000 6.0000 -2.0000 0.38086 6.000 -7.0000 6.0000 -2.0000 1.0194 8.000 -6.00000 2.00000 1.4240 9.0000 -6.00000 -2.0000 1.4240 9.0000 -0.0000 2.0000 1.4240 9.0000 -2.00000 1.4240 9.0000 -2.00000 2.1018 11.000 -2.00000 2.0000 2.1811 12.000 -1.00000 6.00000 -2.0000 2.1811			
Structure: No 38 to 40 Sub-structure: Eastern Elevation Dist. Coordinates x y z			
[m] [m] [m] [mm] vertical Offset 1 0.0 0.0 0.0 0.0 0.0 0.000 -1.00000 6.00000 -2.00000 1.9223 0.000 -1.00000 7.00000 -2.0000 1.9223 0.000 -1.00000 9.00000 -2.0000 1.9223 0.000 -1.00000 9.0000 -2.0000 1.9223 0.0000 -1.00000 1.00000 1.9223 6.0000 -1.00000 1.2.0000 1.9223 7.0000 -1.00000 1.2.0000 1.9223 8.0000 -1.00000 1.4.0000 2.0000 1.9223 9.0000 -1.00000 1.4.0000 1.9223 10.0000 1.00000 1.9000 1.9223 10.0000 1.00000 1.9000 1.9223 10.0000 1.00000 1.9000 1.9223 10.0000 1.00000 1.9000 1.9223			
Structure: No 38 to 40 Sub-structure: Northern Elevation Dist. Coordinates Displacements x y z z			
[m] [m] [m] [mm] vertical Offset 1 0.0 1.0000 16.00000 2.00000 1.9223 1.0000 -2.00000 16.00000 -2.00000 3.0000 3.0000 -4.00000 16.00000 -2.00000 1.0108 3.0000 -6.00000 16.00000 -2.00000 1.4240 5.0000 -6.00000 16.00000 -2.00000 0.65933 7.0000 -8.00000 16.00000 -2.00000 0.38086 9.0000 -10.00000 16.00000 -2.00000 0.059704 10.000 -10.00000 16.00000 -2.00000 0.059704 11.0000 15.00000 -2.00000 0.025761 12.000 -13.00000 16.00000 -2.00000 0.025761			
Structure: No 38 to 40 Sub-structure: Western Elevation Dist. Coordinates x y z [m] [m] [m]			
Vertical Offset 1 0.0 -13.00000 15.0000 -2.00000 0.0 1.0000 -13.00000 15.00000 -2.00000 0.0 0.0 2.0000 -13.00000 13.00000 -2.00000 0.0 0.0 3.0000 -13.00000 13.00000 -2.00000 0.0 0.0 5.0000 -13.00000 11.00000 -2.00000 0.0 0.0 5.0000 -13.00000 12.00000 -2.00000 0.0 0.0 7.0000 -13.00000 9.0000 -2.00000 0.0 0.0 9.0000 -13.00000 7.00000 -2.00000 0.0 0.0			
Structure: No 38 to 40 Sub-structure: Party Wall Dist. Coordinates Displacements			
Structure: Outbuilding to NW Sub-structure: Southern Elevation Dist. Coordinates Displacements			
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			
Structure: Outbuilding to NW Sub-structure: Eastern Elevation Dist. Coordinates x y z [m] [m] [m]			
Vertical Offset 1 0.0 0.1 0.0000 1.00000 0.50000 0.86784 1.0000 -1.00000 42.00000 -0.50000 0.86784 2.0000 -1.00000 42.00000 -0.50000 0.46684 3.0000 -1.00000 45.00000 -0.50000 0.23579 4.0000 -1.00000 45.00000 -0.50000 0.12286 5.0000 -1.00000 45.00000 -0.50000 0.052609 6.0000 -1.00000 47.00000 0.033808 -0.0000 8.00000 -0.50000 0.0 8.0000 -1.00000 45.00000 -0.50000 0.0 -0.000 8.0000 -0.50000 0.0			
Structure: Outbuilding to NW Sub-structure: Northern Elevation Dist. Coordinates Displacements [m] [m] [m] [m] [m] [m] [m] [m] Vertical Offset 1 0.0 -0.50000 0.0 0.0			

Job No.	Sheet No.	Rev.		
J14023				
2 Caversham Road Drg. Ref.	Drg. Ref.			
amage Assessment Made by MP	Date 14-Apr-2015	Checked		
ist. Coordinates Displacements x y z z [m] [m] [m] [m]	•			
.0000 -2.0000 49.0000 -0.50000 0.0 .0000 -3.0000 49.0000 -0.50000 0.0 .0000 -4.0000 49.00000 -0.50000 0.0				
1.0000 -5.0000 49.0000 -0.50000 0.0 ;0000 -6.0000 49.00000 -0.50000 0.0 ;0000 -7.00000 49.00000 -0.50000 0.0				
Structure: Outbuilding to NW Sub-structure: Western Elevation				
x y z z Jopferson i i i i i i i i i i i i i i i i i i i				
0.0 - 7.0000 49.0000 - 0.50000 0.0 .0000 - 7.0000 48.0000 - 0.50000 0.0 .0000 - 7.0000 49.0000 - 0.50000 0.0				
4.0000 -7.0000 45.0000 -0.50000 0.03886 5.0000 -7.0000 44.0000 -0.50000 0.038489 6.0000 -7.0000 43.00000 -0.50000 0.053230 7.0000 -7.0000 42.00000 -0.50000 0.087627				
3.0000 -7.00000 41.00000 -0.50000 0.13886 Structure: Network Rail Line Sub-structure: Sub 10				
ist. Coordinates Displacements x y z z [m] [m] [m] [m]				
Terrical Offset 1 0.0 56.0000 0.00000 0.00000 0.0 0.0022 55.2543 0.67416 0.00000 0.0				
2.0044 54.51685 1.34831 0.00000 0.0 3.0066 53.77528 2.02247 0.00000 0.0 4.0088 53.03371 2.69663 0.00000 0.0 5.1010 52.29213 3.377079 0.00000 0.0				
5.0132 51.55056 4.04494 0.00000 0.0 7.0154 50.80899 4.71910 0.00000 0.0 3.0177 50.06742 5.39326 0.00000 0.0 9.019 49.32584 6.06742 0.00000 0.0				
0.022 48.58427 6.74157 0.00000 0.0 1.024 47.84270 7.41573 0.00000 0.0 2.026 47.10112 8.08989 0.00000 0.0 3.029 46.3555 8.76404 0.00000 0.0				
4,031 45.61798 9.43820 0.00000 0.0 5.03 44.97640 10.11236 0.0000 0.0 5.035 44.13483 10.78652 0.00000 0.0 7.038 43.33262 11.46067 0.0000 0.0				
8.040 42.65169 12.13483 0.00000 0.0 9.042 41.91011 12.8089 0.00000 0.0 0.044 41.16854 13.48315 0.00000 0.0 1.046 40.26297 14.1573 0.00000 0.0				
12.049 39.6539 14.83146 0.00000 0.0 13.051 38.94382 15.5052 0.00000 0.0 14.053 38.20225 16.17978 0.00000 0.0 15.055 37.40607 16.833 0.0000 0.0				
66.057 36.71910 17.52809 0.00000 0.0 7.060 35.97753 18.20225 0.00000 0.0 88.062 35.23596 18.87640 0.00000 0.0 9.064 34.49438 19.55056 0.0000 0.0				
0.066 33.75281 20.22472 0.00000 0.0 1.068 33.01124 20.8988 0.00000 0.0 2.071 32.26966 21.57303 0.00000 0.0 3.073 31.52809 22.24719 0.00000 0.0				
14.075 30.78652 22.92135 0.00000 0.0 15.077 30.04494 23.59551 0.00000 0.0 16.079 29.30337 24.28966 0.00000 0.0				
88.084 27.82022 25.61798 0.00000 0.0 93.068 27.07685 26.29213 0.00000 0.0 10.088 26.33708 26.96629 0.00000 0.0 11.090 25.5551 27.64045 0.00000 0.0090126				
12.093 24.65393 28.31461 0.00000 0.020260 13.095 24.11236 28.98976 0.00000 0.029724 14.097 23.37079 29.66292 0.00000 0.029728 15.099 22.62921 30.33708 0.00000 0.044977				
16.102 21.88764 31.01124 0.00000 0.051553 17.104 21.14607 31.68539 0.00000 0.057921 18.106 20.40449 32.35955 0.00000 0.064422 19.108 19.65292 33.0371 0.00000 0.071381				
0.110 18.92135 33.70787 0.00000 0.079099 1.113 18.17978 34.38202 0.00000 0.087858 2.115 17.43820 35.05618 0.00000 0.087919 3.117 16.6566 35.7304 0.00000 0.10952				
4.119 15.95506 36.40449 0.00000 0.12289 5.121 15.21348 37.07865 0.00000 0.13823 6.124 14.47191 37.75281 0.00000 0.15871 7.126 13.73034 38.42697 0.00000 0.15750				
8.128 12,98876 39,10112 0.0000 0.19773 9.130 12,24719 39,77528 0.00000 0.22253 0.132 11,50562 40.44944 0.00000 0.24999 1.135 10.76404 41.1236 0.00000 0.28020				
2.137 10.02247 41.7975 0.0000 0.23897 3.139 9.2000 42.4191 0.00000 0.23206 4.141 8.5393 43.14607 0.00000 0.28236 5.143 7.7975 43.82022 0.00000 0.017333				
.6.146 7.05518 44.49438 0.00000 0.14234 7.148 6.31461 45.1684 0.00000 0.10865 8.150 5.57303 45.84270 0.00000 0.078867 9.152 4.83146 45.1685 0.00000 0.056276				
0.154 4.08989 47.19101 0.00000 0.037474 1.157 3.34831 47.86517 0.00000 0.012436 2.159 2.60674 48.53933 0.00000 0.02436 3.161 1.86517 49.21348 0.00000 0.0				
4.163 1.12360 49.88764 0.00000 0.0 5.166 0.3202 50.56180 0.00000 0.0 6.168 -0.35955 51.23596 0.00000 0.0 7.170 -1.10112 51.90110 0.00000 0.0				
18.172 - 1.84270 52.58427 0.00000 0.0 9.174 -2.58427 53.28643 0.00000 0.0 0.177 -3.32584 53.93258 0.00000 0.0 1.179 -4.06742 54.60674 0.00000 0.0				
2.161 - 4.80899 55.28090 0.00000 0.0 3.183 -5.5505 55.9506 0.00000 0.0 4.185 -6.29213 56.62931 0.00000 0.0 5.188 -7.0371 57.3371 0.00000 0.0				
6.190 -7.7528 57.97753 0.00000 0.0 7.192 -8.5168 58.6168 59.00000 0.0 8.194 -9.25843 59.32584 0.00000 0.0 9.196 -10.0000 60.00000 0.0000 0.0				
Specific Building Damage Results - All Segments				
tructure: No 38 to 40 Sub-structure: Southern Elevation ertical Offset Segment Start Length Curvature Deflection Average Max. Maximum Maximum Min. Damage				
from Line for Ratio Horizontal Tensile Gradient of Gradient of Radius of Category Vertical Strain Horizontal Vertical Curvature Kovement Displacement Displacement				
Calculations Curve Curve Curve [m] [m] [%] [%] [%] [m] [m] [m] [m] [m] [%]				
2 7.6372 4.3628 Sagging 0.010838 0.037500 0.041962 -374.86E-6 -404.46E-6 2691.0 0 (Negligible)				

Oas	s y:)				J14	023		
42 Caversha	_					Drg. I	Ref.		
Damage Asse	essmen	t				Made I MP	ру	Date 14-Apr-2015	Checked
Vertical Offset from Line for Vertical Movement	Segment	Start Length Curvature		Max. Maximum Tensile Gradient of Strain Horizontal Displacement		Min.	Damage Category		
		+ve, compressive horizon		-	-				
Vertical Offset	s 40 Sub-: Segment	structure: Eastern Elevati Start Length Curvature :	Deflection Average	Max. Maximum	Maximum		Damage		
from Line for Vertical Movement Calculations			Strain	Tensile Gradient of (Strain Horizontal Displacement I Curve	Vertical C	irvature	ategory		
[m] 0		[m] [m] 1 0.0 10.000 None	[%] [%] 0.0 0.0	[%] 0.0 0.0	0.0 -	[m] (Ne	0 gligible)		
		+ve, compressive horizon							
Vertical Offset	Segment	structure: Northern Elevat Start Length Curvature	Deflection Average	Max. Maximum	Maximum	Min.	Damage		
from Line for Vertical Movement Calculations		<i>.</i> . <i>.</i> .	Strain	Curve		Curvature	Category		
[m] 0		[m] [m] 1 0.0 4.3628 Sagging		[%] 0.041962 -374.86E-6		[m] 2691.0 (1	0 Negligible)		
Tensile horizontal		2 4.3628 3.6372 Hogging +ve, compressive horizon	0.0036184 0.037500 tal strains are -ve.	0.038131 -374.86E-6	404.46E-6	11210.	0 Negligible)		
		structure: Western Elevati							
Vertical Offset from Line for Vertical	Segment	Start Length Curvature	Deflection Average Ratio Horizontal Strain	Max. Maximum Tensile Gradient of Strain Horizontal	Vertical	Radius of C Curvature	Damage ategory		
Movement Calculations [m]		[m] [m]	[%] [%]	Curve [%]	: Displacement Curve	[m]			
0 A1	settlement strains are	s are less than the Settl +ve, compressive horizon	ement Trough Limit Ser tal strains are -ve.	sitivity.					
		structure: Party Wall							
Vertical Offset from Line for Vertical Movement	Segment	Start Length Curvature	Ratio Horizontal	Max. Maximum Tensile Gradient of O Strain Horizontal Displacement H	Vertical C	adius of C	Damage ategory		
Calculations [m] 0		[m] [m] 1 0.0 10.000 None	[%] [%] 0.0 0.0	Curve [%] 0.0 0.0	Curve 0.0 -	[m]	0		
Tensile horizontal		e +ve, compressive horizon				(Ne	gligible)		
Structure: Outbuil	ding to NW	Sub-structure: Southern	Elevation						
Vertical Offset from Line for Vertical Movement	Segment	Start Length Curvature		Max. Maximum Tensile Gradient of Strain Horizontal Displacement		Curvature	Damage Category		
Calculations [m] 0		[m] [m] 1 0.0 2.2065 Sagging	[%] [%] 501.74E-6 -0.015672	Curve [%]	Curve	[m]	0		
U U		2 2.2065 3.7935 Hogging		0.0039804 -71.716E-		5 38921.	(Negligible) 0 (Negligible)		
Tensile horizontal	strains are	e +ve, compressive horizon	tal strains are -ve.				(negrigibre)		
Structure: Outbuild	ding to NW	Sub-structure: Eastern E Start Length Curvature		Max. Maximum	Maximum	Min.	Damage		
from Line for Vertical Movement Calculations			Ratio Horizontal Strain		Gradient of 1 Vertical		Category		
[m] 0		[m] [m] 1 0.0 4.0000 Hogging	[%] [%] 0.0022023 0.022069	[%]	241.62E-6	[m] 18458.	0 egligible)		
Tensile horizontal	strains are	+ve, compressive horizon	tal strains are -ve.						
Structure: Outbuild	ding to NW	Sub-structure: Northern Start Length Curvature		Max. Maximum	Maximum	Min.	Damage		
from Line for Vertical Movement Calculations	begmenic	Start hength curvature	Ratio Horizontal Strain	Tensile Gradient of Strain Horizontal	Gradient of	Radius of C Curvature			
[m] 0 Al	l settlement strains are	[m] [m] ts are less than the Settl +ve, compressive horizon	[%] [%] ement Trough Limit Ser tal strains are -ve.	[%]		[m]			
		Sub-structure: Western E							
Vertical Offset from Line for	Segment	Start Length Curvature	Deflection Average Ratio Horizontal	Max. Maximum Tensile Gradient of			Damage Category		
Vertical Movement Calculations		for land		Strain Horizontal Displacement Curve		Curvature			
[m] 0		[m] [m] 1 8.0000 0.0 None	[%] [%] 0.0 0.0	[%] 0.0 -46.200E-6	-51.234E-6	[m] 58668. (N	0 egligible)		
		+ve, compressive horizon	tal strains are -ve.						
Structure: Network Vertical Offset	Rail Line Segment	Sub-structure: Sub 10 Start Length Curvature	Deflection Average	Max. Maximum	Maximum	Min.	Damage		
from Line for Vertical Movement			Ratio Horizontal Strain	Strain Horizontal Displacemen	t Displacement	Curvature	Category		
Calculations [m] 0		[m] [m] 1 53.117 6.1574 Hogging	[%] [%] 176.87E-6 0.0010977	Curve [%]	Curve	[m]	0		
-		2 59.274 6.5715 Sagging	0.0011389 -0.0075903	0.0015190 153.81E-		5 37997.	(Negligible) 0 (Negligible)		
Tencile horizont.		3 65.846 1.3020 Sagging	47.733E-6 -0.0035152	703.28E-6 64.351E-	6 33.623E-	5 326160.	(Negligible) 0 (Negligible)		
iensiie norizontal	strains are	e +ve, compressive horizon	tar strains are -ve.						
Specific Building Da	mage Results	- Critical Values for All Segn	ents within Each Sub-St	ructure					
		structure: Southern Elevat							
Vertical Defle Offset from Rat Line for			Max. Maximum t Tensile Gradient of Strain Horizontal	Maximum Min. Gradient of Radius Vertical Curvatu		Damage Cat	egory		

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[m]	[%] [%] [mm] [%] [m] [[m]		
		2691.0 0 (Negligible)		
Vertical De:	8 to 40 Sub-structure: Eastern Elevation flection Average Maximum Maximum Max. Maximum Maximum Min. Min.	Damage Category		
offset from 1 Line for Vertical Movement Calculations	Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radius of Strain Strain Horizontal Vertical Curvature Curvatur Displacement Displacement (Hogging) (Sagging Curve Curve Curve	re		
[m] 0	[%] [%] [mm] [%] [m] [m] [m] 0.0 0.0 0.0 1.9223 0.0 0.0 0.0 -	- 0 (Negligible)		
structure: No 3	8 to 40 Sub-structure: Northern Elevation			
	flection Average Maximum Maximum Max. Maximum Maximum Min. Mir Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radius Strain Horizontal Vertical Curvature Curva Displacement Displacement (Hogging) (Sagg Curve Curve	us of ature		
Calculations [m]	[%] [%] [mm] [%] [m] [m	m] 691.0 0 (Negligible)		
	0.010838 0.037500 404.462-6 2.1800 0.041962 -3/4.862-6 404.462-6 11210. 20 8 to 40 Sub-structure: Western Elevation	(megrigibie)		
Vertical De:	flection Average Maximum Maximum Max. Maximum Maximum Min. Min.	Damage Category		
Line for Vertical	Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radius of Strain Strain Horizontal Vertical Curvature Curvatur Displacement Displacement (Hogging) (Sagging	re		
Movement Calculations [m]	Curve Curve [%] [m] [m]			
Structure: No 3	8 to 40 Sub-structure: Party Wall			
Vertical Des Dffset from 1 Line for Vertical Movement	flection Average Maximum Maximum Max. Maximum Maximum Min. Min. Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radius of Strain Horizontal Vertical Curvatur Displacement Displacement Hogging) (Sagging Curve Curve	re		
[m] 0	[%] [%] [mm] [%] [m] [m]	- 0 (Negligible)		
	uilding to NW Sub-structure: Southern Elevation			
Vertical Des Offset from 1	flection Average Maximum Maximum Max. Maximum Maximum Min. Mi Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radi			
Line for Vertical Movement Calculations	Strain Strain Horisontal Vertical Curvature Curv Displacement Displacement (Hogging) (Sag Curve Curve	vature		
[m]	[%] [%] [mm] [%] [mm] [%] .0012636 -0.015672 146.73E-6 0.86784 0.0039804 223.06E-6 146.73E-6 38921. 4	[m] 41882. 0 (Negligible)		
	uilding to NW Sub-structure: Eastern Elevation			
Offset from 1 Line for Vertical Movement	flection Average Maximum Maximum Max. Maximum Maximum Min. Min Ratio Horizontal Slope Settlement Tensile Gradiunt of Gradient of Radius Of Radiu Strain Horizontal Vertical Curvature Curva Displacement Displacement (Hogging) (Sagg Curve Curve	us of ature		
Calculations [m] 0 0	[%] [%] [mm] [%] [mm] .0022023 0.022069 241.62E-6 0.86784 0.024097 -237.13E-6 241.62E-6 18458.	m] - O (Negligible)		
Structure: Outbo	uilding to NW Sub-structure: Northern Elevation			
	flection Average Maximum Maximum Max. Maximum Maximum Min. Min. Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Radius o Strain Strain Horizontal Vertical Curvature Curvatur Displacement Displacement (Hogging) (Sagging	re		
Movement Calculations [m]	Curve Curve (m) [%]			
Structure: Outbo	uilding to NW Sub-structure: Western Elevation			
Offset from 1 Line for Vertical Movement	flection Average Maximum Maximum Max. Maximum Maximum Min. Mir Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius Strain Strain Horizontal Vertical Curvature Curva Displacement Displacement (Hogging) (Sagg Curve Curve	us of ature		
[m] 0	[%] [%] [mm] [%] [m] [%] [m] [m] [%] [m] [m] [m] [m] [m] [m] [m] [m] [m] [m	m] - O (Negligible)		
Structure: Netwo	ork Rail Line Sub-structure: Sub 10			
	flection Average Maximum Maximum Max. Maximum Maximum Min. Mi Ratio Horizontal Slope Settlement Tensile Gradient of Gradient of Radius of Strain Strain Horizontal Vertical Curvature Curv Displacement Displacement (Hogging) (Sag Curve Curve	vature		
Calculations [m]	[%] [%] [mm] [%] [m]	[m] 37997. 0 (Negligible)		
Specific Building	Damage Results - Critical Segments within Each Structure			
Structure Name	Sub-Structure Segment Slope Settlement Ter	ax. Min. Min. Da nsile Radius of Radius of rain Curvature Curvature	amage Category	
No 38 to 40		(Hogging) (Sagging) [%] [m] [m]	ligible)	
	Maximum Slope Solution 1 - 5000 - 5072 mSging Control - 6 1.2773 0.0 Maximum Northern 1 0.0 4.3628 Sagging 404.46E-6 2.1800 0.0 Settlement Elevation 1 0.0 4.3628 Sagging 404.46E-6 2.1800 0.0			
	Max. Tensile Southern 2 7.6372 12.000 Sagging 404.46E-6 2.1800 0.0 Strain Elevation			
	Min. Radius of Southern 1 4.0000 7.6372 Hogging 404.46E-6 1.2773 0.0 Curvature Elevation (Hogging)			
	Min. Radius of Southern 2 7.6372 12.000 Sagging 404.46E-6 2.1800 0.0 Curvature Elevation (Sagging)			
Outbuilding to NW	(adgjung) Maximum Slope Eastern 1 0.0 4.0000 Hogging 241.62E-6 0.86784 0.0 Elevation Maximum Southern 1 0.0 2.2065 Sagging 146.73E-6 0.86784 0.00			
	Settlement Elevation Max. Tensile Eastern 1 0.0 4.0000 Hogging 241.62E-6 0.86784			
	Strain Elevation Min. Radius of Eastern 1 0.0 4.0000 Hogging 241.62E-6 0.86784 0.0 Curvature Elevation 1 0.0 4.0000 Hogging 241.62E-6 0.86784 0.0	024097 18458 0 (Neg)	ligible)	

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Structure Name		l Critica ture Segment	:			Strain	Min. Radius of Curvature (Hogging)	Curvature	Damage Ca	tegory	
	(Hogging) Min. Radius of Southern Curvature Elevation		1 0.0 2.	2065 Saggi	ng 146.73E-6	0.86784 0.0031447	-		(Negligible)		
Network Rail	(Sagging) Maximum Slope Sub 10		2 59.274 65	.846 Saggi	ng 41.144E-6	0.28014 0.0015190	-	37997. 0	(Negligible)		
Line	Maximum Sub 10		2 59.274 65	.846 Saggi		0.28014 0.0015190	-		(Negligible)		
	Settlement Max. Tensile Sub 10 Strain		2 59.274 65	.846 Saggi	ng 41.144E-6	0.28014 0.0015190	-	37997. 0	(Negligible)		
	Strain Min. Radius of Sub 10 Curvature		1 53.117 59	.274 Hoggi	ng 27.401E-6	0.22648 0.0013419	378380.	- 0	(Negligible)		
	(Hogging) Min. Radius of Sub 10 Curvature (Sagging)		2 59.274 65	.846 Saggi	ng 41.144E-6	0.28014 0.0015190	-	3 7997. 0	(Negligible)		
Specific Buildir	ng Damage Results - All Combin	ed Segments									
Structure: No	38 to 40 Sub-structure: So	outhern Elevat	ion								
Vertical 0 Offset from 5 Line for Vertical	Combined Start Length Curvatu Segment		h Average Horizontal Strain		Damage Category	r					
Movement Calculations [m]	[m] [m] have segments combined.	[%]	[%]	[%]							
Structure: No	38 to 40 Sub-structure: Ea	stern Elevati	lon								
Vertical (Combined Start Length Curvatu	re Deflection	h Average	Max.	Damage Category	r					
Offset from S Line for Vertical Movement	Segment	Ratio	Horizontal Strain	. Tensile Strain							
[m] No structures	[m] [m] have segments combined.	[%]	[%]	[%]							
	38 to 40 Sub-structure: No										
Vertical (Offset from S Line for Vertical Movement	Combined Start Length Curvatu Segment	re Deflection Ratio	n Average Horizontal Strain		Damage Category	r					
[m] No structures	[m] [m] have segments combined.	[%]	[%]	[%]							
	38 to 40 Sub-structure: We Combined Start Length Curvate			Max.	Damage Category	,					
Line for Vertical Movement Calculations	Segment	RACIO	Strain								
[m]	[m] [m] have segments combined.	[%]	[%]	[%]							
	38 to 40 Sub-structure: Pa										
Offset from S Line for Vertical Movement			Horizontal Strain	Tensile	Damage Category	,					
Calculations [m] No structures	[m] [m] have segments combined.	[%]	[%]	[%]							
Structure: Out	tbuilding to NW Sub-structu	re: Southern	Elevation								
Offset from S	Combined Start Length Curvatu Segment		Horizontal	Tensile	Damage Category	r					
Line for Vertical Movement			Strain	strain							
Calculations [m]	[m] [m] have segments combined.	[%]	[%]	[%]							
	tbuilding to NW Sub-structu										
Offset from S Line for Vertical	Combined Start Length Curvatu Segment	re Deflection Ratio	n Average Horizontal Strain	Tensile	Damage Category	,					
Movement Calculations [m] No structures	[m] [m] have segments combined.	[%]	[%]	[%]							
Structure: Out	tbuilding to NW Sub-structu	re: Northern	Elevation								
Vertical 0 Offset from 8 Line for Vertical	Combined Start Length Curvatu Segment	re Deflection Ratio	n Average Horizontal Strain		Damage Category	,					
Movement Calculations [m]	[m] [m] have segments combined.	[%]	[%]	[%]							
Structure: Out	tbuilding to NW Sub-structu	ure: Western :	levation								
Vertical 0 Offset from 8 Line for Vertical	Combined Start Length Curvatu	re Deflection			Damage Category	r					
Movement Calculations [m]	[m] [m] have segments combined.	[%]	[%]	[%]							
Structure: Net	work Rail Line Sub-structu	ure: Sub 10									
Vertical 0 Offset from 5 Line for Vertical Movement	Combined Start Length Curvatu Segment		h Average Horizontal Strain	Tensile	Damage Category	, ,					

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[m] [m] [m] [%] [%] [%] No structures have segments combined.