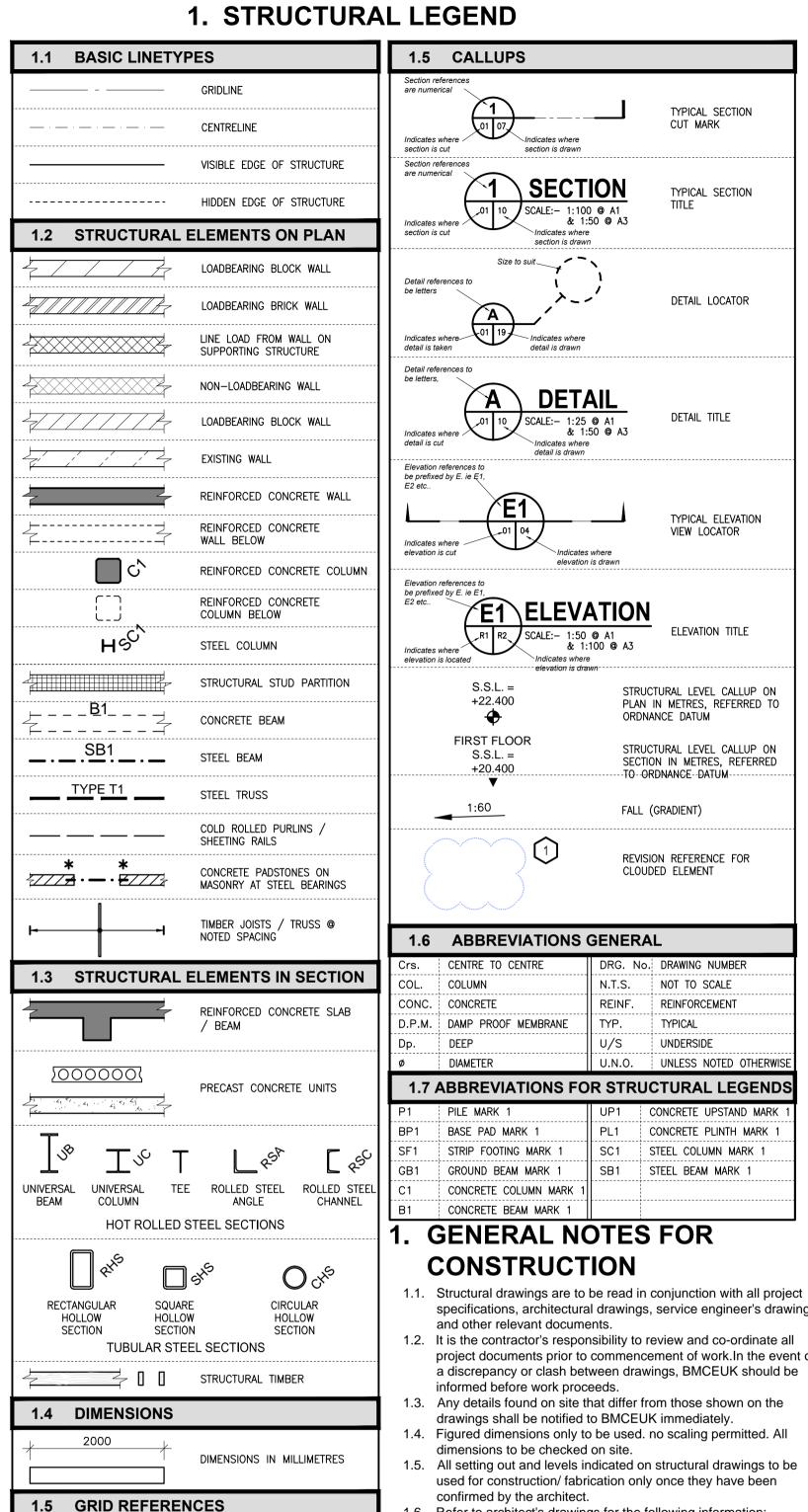
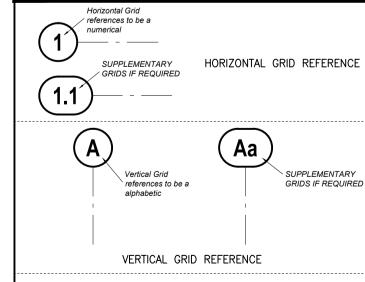
1. STRUCTURAL LEGEND 1.5 CALLUPS GRIDLINE are numerical CENTRELINE section is cut VISIBLE EDGE OF STRUCTURE Section references HIDDEN EDGE OF STRUCTURE 01 10 SCALE:- 1:100 @ A1





1.12. The contractor shall submit all manufacturer's drawings and specifications for equipment support, anchorage etc. to BMCEUK for review at least 2 weeks prior to placing an order for equipment.

1.13. The contractor's proposed substitutions, if any, shall be submitted to BMCEUK for review at least 4 weeks prior to works commencing.

1.14. Where materials, products and workmanship are not fully detailed or specified they shall be of a standard appropriate to the works and in accordance with good building practice.

1.15. All articles, materials and goods shall be new and of good quality, suitable for the required purpose and shall conform to the appropriate British Standard, where such exists. Where references to the above are made, it shall be inferred that the latest edition applies, together with subsequent amendments, unless otherwise specified. All proprietary systems to be installed in accordance with manufacturer's recommendations.

1.16. Nothing included or omitted on these drawings shall relieve the contractor of his duty to carry out the works in accordance with current standards of safety and good building practice.

1.17. The contractor is to notify the building control officer to carry out his inspections prior to covering up of structural elements and concreting of new foundations, slabs etc.

2. EXCAVATIONS

TYPICAL SECTION

TYPICAL SECTION

DETAIL LOCATOR

DETAIL TITLE

TYPICAL ELEVATION

VIEW LOCATOR

ELEVATION TITLE

STRUCTURAL LEVEL CALLUP ON

PLAN IN METRES, REFERRED TO

STRUCTURAL LEVEL CALLUP ON

SECTION IN METRES, REFERRED

-TO--ORDNANCE--DATUM-

REVISION REFERENCE FOR

FALL (GRADIENT)

CLOUDED ELEMENT

DRG. No.: DRAWING NUMBER

TYPICAL

UNDERSIDE

REINF.

TYP.

U/S

U.N.O.

SC1

SB1

specifications, architectural drawings, service engineer's drawings,

project documents prior to commencement of work. In the event of

a discrepancy or clash between drawings, BMCEUK should be

used for construction/ fabrication only once they have been

drawings shall be notified to BMCEUK immediately

1.6. Refer to architect's drawings for the following information:

1.6.d. Above ground waterproofing and insulation details.

1.7.a. Setting out and dimensions of all service openings.

1.8. Refer to contractor's drawings for the following information:

1.8.a. Temporary works required maintaining structural stability

1.8.b. Crane and hoist locations, together with associated access

1.8.e. All brackets, inserts, and fixings for cladding, lifts, lifting

contractor's responsibility and he shall take all necessary

contractor's temporary works details shall be submitted to

for the support of any additional loads imposed during

review at least 2 weeks prior to works commencing.

1.10. The structural members shown on drawings have been designed

1.11. All construction joints shown on the structural drawings shall be

measures to protect the safety of site operatives and the public.

The contractor shall maintain the structural integrity of all existing

BMCEUK for review at least 2 weeks prior to work commencing.

to carry in place design loads only. The contractor is responsible

incorporated into the structure. Details of additional construction

joints to facilitate construction shall be submitted to BMCEUK for

and new structures within or adjoining the works, at all stages. The

1.9. Construction methods, procedures, and sequences are the

1.7.d. Details and setting out of lightning protection.

1.7. Refer to service builderswork drawings for the following

1.6.c. Details of required surface finishes, chases, and arises.

for supporting services, access ladders and platforms.

GENERAL NOTES FOR

NOT TO SCALE

REINFORCEMENT

UNLESS NOTED OTHERWISE

CONCRETE UPSTAND MARK

CONCRETE PLINTH MARK

STEEL COLUMN MARK 1

STEEL BEAM MARK 1

CUT MARK

& 1:50 @ A3

Indicates where

DETAIL

0 / SCALE:- 1:25 @ A1

Indicates where detail is drawn

E1 ELEVATION

_R1 R2 SCALE:- 1:50 @ A1

^E1

S.S.L. =

FIRST FLOOR

S.S.L. =

+20.400

1:60

CENTRE TO CENTRI

BASE PAD MARK

STRIP FOOTING MARK 1

GROUND BEAM MARK 1

CONCRETE COLUMN MARK

CONSTRUCTION

and other relevant documents

informed before work proceeds.

dimensions to be checked on site

confirmed by the architect.

information

1.6.a. Setting out of gridlines on site.

1.6.b. Setting out of building envelope.

1.7.b. Cast in services, sleeves and frames.

1.7.e. Details and setting out of earthing pits.

during construction.

installations etc

construction.

platforms and restraints

1.8.d. Allocated storage areas for materials.

CONCRETE BEAM MARK

COLUMN

DEEP

+22.400

2.1. The bottom of all excavations are to be taken down to the levels required by BMCEUK drawings, or other instructions, and shall be to the satisfaction of the architect, engineer and local authority.

2.2. All excavated material not required for backfilling shall be removed from site. To be provided for by contractor.

by good sound timbering or other suitable methods to contractor's design. The removal of support shall be done in such a manner as not to endanger the works and shall not relieve the contractor of the responsibility for ensuring the stability of the works.

2.3. The side of excavations shall be properly supported and retained

2.4. The bottoms of all excavations shall be carefully trimmed and finished to the specified levels and all loose materials removed.

2.5. Should the excavated surface be cut up or softened under the action of ponded water or be broken up by any cause, the contractor shall at his own expense, excavate & remove soil down to solid formation and backfill with concrete or fill, as specified by BMCEUK, properly consolidated to the specified level.

2.6. If poor ground, cavities or soft spots are met within any part of the excavation, the contractor shall excavate to solid formation and fill up to the specified level with fill or concrete as directed by architect

2.7. Should the contractor excavate anywhere to a greater size or depth shown on the working drawings or should the sides of the excavation cave in anywhere, the contractor shall at his own expense fill and tightly pack the excess space with concrete or other approved material.

2.8. The contractor shall ensure that the formations are not damaged by weathering. Concrete or fill shall be placed in the same day the excavation has taken place unless the foundation is blinded with concrete or otherwise protected from damage. A layer of 50 mm lean mix blinded concrete shall be laid on the bottom of prepared formations under concrete bases or strip footings when completion of foundation is not carried out on day of the excavation.

2.9. The engineer shall be informed before any concrete or hardcore is placed and shall be given the opportunity of inspecting and approving the bottom of all excavations.

2.10. The contractor shall make provision for and deal with all water which may find its way into the works from any source whatsoever and shall excavate sumps, cut drains, provide & work pumps and provide & work all necessary materials, plant and equipment for dealing with any water encountered.

2.11. The contractor shall not pump or otherwise put water directly into

2.12. Where reinforcement for concrete construction is to be placed, a blinded layer of C16/20 (50 mm thick) concrete shall be laid to receive the reinforcement.

3. UNDERPINNING

3.1. The contractor shall be responsible for ensuring that his operations do not in any way impair the safety or conditions of the existing structures. He shall provide any temporary supports required for this purpose in addition to any temporary supports shown on the BMCEUK drawings.

3.2. Underpinning to be carried out in a 1,3,5,2,4 sequence as indicated on the BMCEUK plans. In no case shall the width of sections excavated exceed 1000mm. The total sum of unsupported lengths shall not exceed one fifth of the wall length. In no case shall a section be excavated immediately adjacent to one which has been completed.

3.3. Underpinning greater than a depth of 1.5 m to be carried out in separate lifts. Each lift to be not greater than 1.5 m deep. The lower bays should be staggered with those immediately above and be tied to adjacent horizontal and vertical bays with 4 x H20 bars (600 mm long) per interface.

3.4. The underside of existing wall footings to be cleaned and hacked free of soil or loose material before casting of concrete

3.5. Construct body of underpin using (C30/37, with AC-2 ACEC classification & DS-2 sulphate resisting cement, max 20 mm aggregate size) concrete. Underpinning to be cast in sections as indicated on BMCEUK drawings. As far as practically possible, excavation and underpinning to be carried out on the same day. Unconcreted sections shall be kept covered to prevent ingress of

3.6. New concrete underpin sections to be stopped 75 mm below underside of existing footings and final pinning up to wall carried out with 1:3 dry pack mortar well rammed in as soon as underpin has set hard.

3.7. Excavation of any section of underpinning shall not be commenced until at least 48 hrs after completion of any adjacent section of 1.7.c. Location and details of supports and plinths for plant, brackets work. Adjacent underpin concrete section to have reached a min

strength of 10 N/mm2. 3.8. The joint between adjacent sections of underpinning made by forming rough surface against which the first underpin section is cast, with H20 dowels at 300 ctrs hammered 300 mm into the excavation face. On construction of next underpin section, thoroughly clean exposed concrete face and projecting dowels before adjacent underpin is cast.

1.8.c. Temporary access routes for site operatives and site vehicles. CAST IN-SITU CONCRETE

4.1. All concrete is to comply with the latest edition of the national structural Concrete Specification for Buildings (NSCS), published by the Concrete Society and modified by BMCEUK project specification.

4.2. Schedule of concrete strengths U.N.O. on drawings:-4.2.a. For all reinforced concrete elements protects from weather & the ground: designated mix GEN 1 to BS EN 206 7 BS 8500-2

with DC-2 design class and AC-2 ACEC class. Reinforced concrete 28 day strength elements exposed to weather

other RC concrete elements 4.2.c. For external slabs with surface exposed to weather use: designated mix PAV2 air-entrained concrete mix to BS EN

206 & BS 8500-2. All unprotected reinforced concrete in contact with the ground: designated mix C32/40 with DC-2 design class and AC-2

4.3. Schedule of minimum cover

4.3.a. Unformed surfaces exposed to earth: 75mm 4.3.b. Unformed surfaces over vapor barrier: 50mm

4.3.c. Formed surfaces exposed to earth: 40mm 4.3.d. Formed surfaces exposed to weather: 50mm

4.3.e. Formed surfaces protected from weather / earth - beams, columns, slabs: 25mm u.n.o.

4.4. High yield bars (Fy = 500) deformed type 2 to BS 4449 Mild steel bars (Fy = 250) plain lap lengths to BS EN 1992-1-1:2004. Bars ≤ 32mm diameter. C28/35 concrete 'good' bond conditions

43xbar diameter

39xbar diameter beams: columns: 51xbar diameter walls: 56xbar diameter

NOTES: 1 If bar size = 40mm then reduce the lap length by 8% 2 For Grade 40 concrete reduce the lap length by 10% 3 For 'poor' bond conditions e.g. top mat rebar in beams / slabs > 250mm deep increase the lap length by 33% (beams)

4.5. Cube testing requirements, formwork and curing times to be in accordance with the specification.

4.6. Concrete finishes Unless noted otherwise on the drawings, concrete finishes shall be to BS EN 13670 where finishes are classed as basic, ordinary,

plain & special. Finishes on this project shall be as follows: 4.6.a. Formed finish for visible (exposed) concrete - plain finish 4.6.b. Formed finish for not visible (unexposed) concrete - ordinary

4.6.c. Unformed finish to internal areas - plain finish (power trowelled finish to slabs).

4.6.d. Unformed finish to external areas - ordinary finish (light brush finish to slabs). 4.7. The contractor shall provide information of their methods of controlling the curing of the concrete & shall detail these in a

method statement for submission to the engineer/architect in accordance with the specification. 4.8. Exposed slabs not receiving any other treatment are to be surface

sealed with BASF `FEBCLEAR SUPER' or similar approved applied in accordance with the manufacturers recommendations. 4.9. The contractor is to provide his proposed slab pouring sequence for approval at least 2 weeks prior to proposed 1st pour. This will include proposed construction joints; contractor is to check the requirements in the concrete specification. Typically, construction joints in suspended beams & slabs will only be accepted at 1/3 points of spans maximum. Maximum length of time before pouring

against previously work is 4 days. 4.10. The contractor is to notify the engineer 2 days in advance of each

concrete pour. 4.11. All concrete faces to be cast against formwork unless noted otherwise.

4.12. All holding down bolts, bolt boxes and cast-in plates to be detailed and supplied by steelwork sub-contractor and checked for position by steel contractor prior to being cast in place by concrete sub-contractor

4.13. Reinforcement estimates are as follows: Ground Beams 175kg/m³ Ground Floor Slab 125kg/m³ First Floor Slab 80kg/m³

4.14. Beam reinforcement based on overall beam volume (with beam depth measured from S.S.L. to beam soffit)

4.15. Slab reinforcement based on overall beam volume (with beam depth measured from S.S.L. to beam soffit)

4.16. The above rates make no allowance for support bars, chairs etc. to hold the rebar in place during concreting or shear links to slabs. 4.17. Contractor to check architects buildersworks drawings for detailed

setting out of edges, openings and stairs. 4.18. For corrosion protection, top coats, fire proofing, fire stopping and

waterproofing details refer to architect's drawings and specification.

STRUCTURAL STEEL

Structural steelwork shall be in accordance with the project specification and comply with the National Structural Steelwork Specification for building construction, latest edition, published by BCSA/SCI, as modified by the project notes and specifications.

Unless noted otherwise on the drawings steel to be Grade S355 weldable structural steel to BS EN 10025 & 10210 (latest edition). Bolts nuts etc. to be Grade 8.8 to BS 3692:2001.

5.3. Connections

The contractor is responsible for the design of all connections including base plates. Connections shown are indicative only. The connections shall be designed for the forces & moments shown on the drawings. Calculations & joint details to be submitted for the engineer for review 7 days prior to fabrication. Bolts in direct tension to be fitted with lock nuts. All connections to be designed for 75kN (+or-) axial & 75kN shear minimum (ultimate loads). Base plate connections to be designed for a lateral load equal to 2.5% of the axial column load. For large lateral load situations on the base plates the base plate is to be provided with a shear key or to be cast into a shallow pocket in the foundation. Where connections are detailed on the drawings the contractor is to confirm his acceptance of these details in writing prior to the start of fabrication.

5.4. Corrosion protection

5.4.1. Internal environment All internal steelwork shall be protected against corrosion as follows: (see also specification for further

Shot blast to SA 2 1/2. a - Within 2 hours of shot blasting apply 2 pack epoxy zinc

phosphate prefabrication primer to 20 microns DFT. b - Post fabrication clean down and spot prime all areas of

bare metal with prefabrication primer. c - Apply to the clean dry surface 1 coat of 2 pack epoxy high

build zinc phosphate primer to a dry film thickness of 75 microns. Allow 7 days to achieve maximum hardness before dispatch to site.

d - After erection prepare and carefully spot prime all damaged areas and bolt heads etc. with primer. e - Apply decorative paint finish where required by the

compatible with the underlying paint system. 5.4.2. External steelwork

architect. The decorative paint system used shall be

a - All external steelwork shall be protected against corrosion as follows (see specification for further details).

b - Blast clean to SA2 for roughness using chilled iron grit Grade G24. c - Hot dip galvanise to BS EN ISO 1461:2009 to achieve 90 micron DFT. (Note: no further drilling/fabrication of

d - Note: all bolts, fastenings etc. for galvanised steelwork to be galvanised and given the same paint build up as for galvanised members.

steelwork to be carried out after galvanising.

5.5. Shop drawings The contractor shall submit full workshop drawings for all structural steelwork members for review by the engineer at least 4 weeks 7.18. The contractor shall ensure that all lintels provided match the

5.6. Fire protection: All structural steel except rooF beams, achieve fire protection as required in the fire certificate by 75mm concrete encasement, intumescent paint system or another approved durable system. Exact details of the fire protection system to be supplied to the design team 2 weeks prior to steelwork fabrication. Intumescent paint system to be compatible with the primer.

5.7. Fire protection: Weld tests are required for all site / shop welds and shall be 9.2. carried out in accordance with the steelwork specification.

5.8. Site welding or site cutting of steelwork will only be allowed with the express approval of the engineer. Site welded connections designated by the engineer should be subject to ultra-sonic weld testing. Refer to steelwork specification for details.

5.9. Non shrink grout beneath all steel beam bearings, steel base plates or precast elements to have a minimum compressive strength of 60N/mm² to the engineer's approval.

5.10. Where any stainless steel brickwork support angles, proprietary stainless steel masonry support systems or stainless steel fabricated elements are provided, these are to be insulated from all mild steel elements using non-conductive waterproof gaskets and nylon or Teflon washers & brushes.

5.11. The steel fabricator shall inspect the prepared foundations and holding down bolts for position and level not less than 7 days before erection of steelwork starts. He shall then inform the engineer if he finds any discrepancies which are outside the deviations specified in the National Structural Steelwork Specification (black book) requesting that remedial works be carried out before erection commences. 5.12. The contractor is to allow for coordination with other contractors

whose work interfaces with the steel frame. Work specified by others is not shown on BMCEUK drawings.

5.13. All steelwork set out is to the centroid of the section (refer to blue

5.14. Column base plate assembly to be provided by steelworker to concrete subcontractor with required setting out plan to allow elements to be cast-in.

6. TIMBER

Structural timber shall meet the requirements of BS EN 338, BS EN 1912 and the project specification. The timber shall be stress graded and marked to BS 4978. It is the contractor's responsibility to provide timber that meets the requirements of this specification.

6.2. Structural timber to be Grade C24, unless noted otherwise. 6.3. No timber showing signs of decay or insect attack shall be used. No timber which could have come into contact with such infected

timbers shall be used 6.4. Preservation work shall be carried out in accordance with BS 8417. double vacuum treatment with organic solvent preservatives to be

used, all preservatives to architect's approval. 6.5. All materials and fixings shall be protected from the weather 6.6. Nails, fixings and metal clips to be hot dipped galvanised, sheradised or electro-galvanised post-fabrication. The minimum thickness of metal shall be 1.8 mm. Nails, fixings and metal clips

shall be in accordance with the latest British Standards. 6.7. Restraint straps to be provided at spacings and length indicated on drawings. all restraint straps shall be in accordance with BS EN

6.8. Provide noggins min. 38 mm thick and at least three quarters of depth of joist along lines of support and midspans. For spans greater than 4500 mm, provide noggins at 1/3 and 2/3 points.

6.9. Double up joists under new partitions running parallel to the joist span and bolt together with M12 bolts at 600 mm ctrs with oversize

6.10. For partitions running perpendicular to the joist span, provide solid noggins under new the partition base rail.

6.11. Trimmers to structural openings shall be jointed to the trimming joists with joist hangers to BS EN 845, unless noted otherwise.

6.12. No notching of joists shall occur without prior written approval from

6.13. Timbers should be supported on an even bed at bearings. packing, if required, should be provided under the full area of the bearing and be approved by BMCEUK.

MASONRY

7.1. All masonry to be designed in accordance with BS EN 1996-1:2005 & BS EN 1996-2:2006 and the project specification. 7.2. All materials for masonry ancillary items to be galvanised or

stainless steel in accordance with BS EN 1996-2.

7.3. Blockwork in accordance with BS EN 771-3:2011 and to have min compressive strength of 7.3N/mm2. 7.4. Brickwork in accordance with BS EN 771-3:2011 & to be standard

format bricks with min compressive strength of 20.0N/mm2. 7.5. All walls shown on BMCEUK drawings to be load bearing u.n.o. blockwork densities subject to confirmation with BMCEUK,

architect & acoustic consultant. Light weight blockwork shall not be

used unless prior approval by design team. 7.6. U.N.O. wall setting out and thicknesses to be as shown on architectural drawings and must be read in conjunction with architects specification.

7.7. Wall ties to be Type 2 U.N.O. in accordance with PD 6697 & stainless steel in accordance with BS EN 845-1. Ties to have min 50mm embedment with min 800N tensile capacity & min 1300N compressive capacity. Ties spaced at 450mm crs vertically & 750mm crs staggered horizontally. At openings ties spaced at 225mm from opening edge and at 1 per 300mm vertically.

7.8. Ancon IHR - B Sliding head restraint ties to be provided at 450mm crs head of masonry walls. Vertical restraint to be Ancon ties at 450mm crs where masonry secured to vertical columns with debonded sleeves.

7.9. All new masonry and repair masonry to existing structure shall be matched in colour, texture and dimensions and laid in the same

bond pattern as the remaining structure. 7.10. All new masonry and repair masonry and re-pointing to existing structure to be laid in a min 1:2:9 cement:lime:sand mortar. 7.11. Re-pointing: rake out and repoint joints to min depth of 40mm or

until loose mortar is removed 7.12. New bricks below DPC are to be Class B engineering brick sets in 1:3 CEM sand mortar with SRPC mortar. Blockwork to be laid in grade (1:1:6) mortar above ground and grade (1:4) CEM sand

mortar with SRPC mortar where buried. 7.13. Dry pack to be 1:3 cement: coarse sand and minimum of 35mm thick. Dry pack to be well rammed in where used at all locations 7.14. Stainless steel bed joint reinforcement shall be provided in two courses above and below new openings in solid masonry min

cross section area to be 49mm2 per m width. 7.15. Where new masonry construction abuts existing masonry construction, existing masonry to be plastered with a scud and fairing coat and stainless steel Staifix channels and dovetail slots at 450mm crs to be fixed to existing wall as starter for each new leaf of masonry.

7.16. Control joints in external masonry are generally located at 6m crs

concrete lintels or galvanised pressed steel lintels used strictly in

accordance with manufacturers details and to manufacturers safe

and extend from DPC to roof level. Exact location of joints to be agreed with architect. Ties to have one end debonded using debonding sleeve. 7.17. Lintels in masonry walls are to be proprietary pre-stressed

NOT FOR CONSTRUCTION

9. TEMPORARY WORKS

manufacturers details & recommendations.

provided in exposed brickwork external leaf.

9.1. The contractor is entirely responsible for maintaining the stability of all existing building and structures within and adjacent to the works and of all proposed works from the date of possession to practical completion of the works.

working loads. Lintel propping during construction & bearing to

required external wall finishes. e.g. - precast lintels shall not be

The contractor shall install and maintain all necessary temporary works for the duration of the project. Particular attention should be given to the bearing of temporary props.

10.METAL DECKING

10.1. Metal decking shall be hot dip galvanised structural steel to BS EN 10143 and BE EN 10346 with minimum coating mass = 275 g/m^2 . minimum yield strength = 350 N/mm².

10.2. Decking shall be trimmed where necessary at columns, cantilever connections etc. and additional support to be provided where necessary. All secondary supports for metal decking at column and beam intersections to be contractor designed and approved by BMCEUK.

10.3. Service openings:

a - For openings smaller than 300 mm square, the metal decking shall be laid continuously across the hole and the deck shall be cut away after concreting or core drilled to approved locations after completion of slab in accordance with the deck manufacturer's specifications b - Openings larger than 300 mm square are to have local

reinforcement or trimmers. Contractor is to make allowance as such in accordance with the deck manufacturer's specifications. 10.4. Contractor to ensure that all junctions between metal decking and

structural steel or concrete walls are sealed and that no grout

leakage will occur. 10.5. Shear studs shall be approved through deck shear connections in accordance with BS EN ISO 13918. Ahear studs shall be manufactured from low carbon steel, with minimum yield strength = 350 N/mm² and minimum tensile strength = 450 N/mm². Studs shall be 19 mm diameter, welded to beams in locations shown on drawings, in accordance with stud manufacturer's specifications. for through deck welding, top flanges must be clean and free of

10.6. Decking must be clear of grease and dirt which could adversely affect the bonding.

10.7. Contractor to make allowance for additional concrete due to deflection of decking.

> 2.12.14 ISSUED FOR TENDER ISSUED FOR COMMENT 2.09.14 DATE **DESCRIPTION** SSUE STATUS PRELIMINARY (P1, P2, P3 etc,,) PLANNING (PL1, PL2, PL3 etc, TENDER (T1,T2, T3 etc,,) CONSTRUCTION (0, 1, 2 etc.

> > barrett mahony

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STONE LTD.

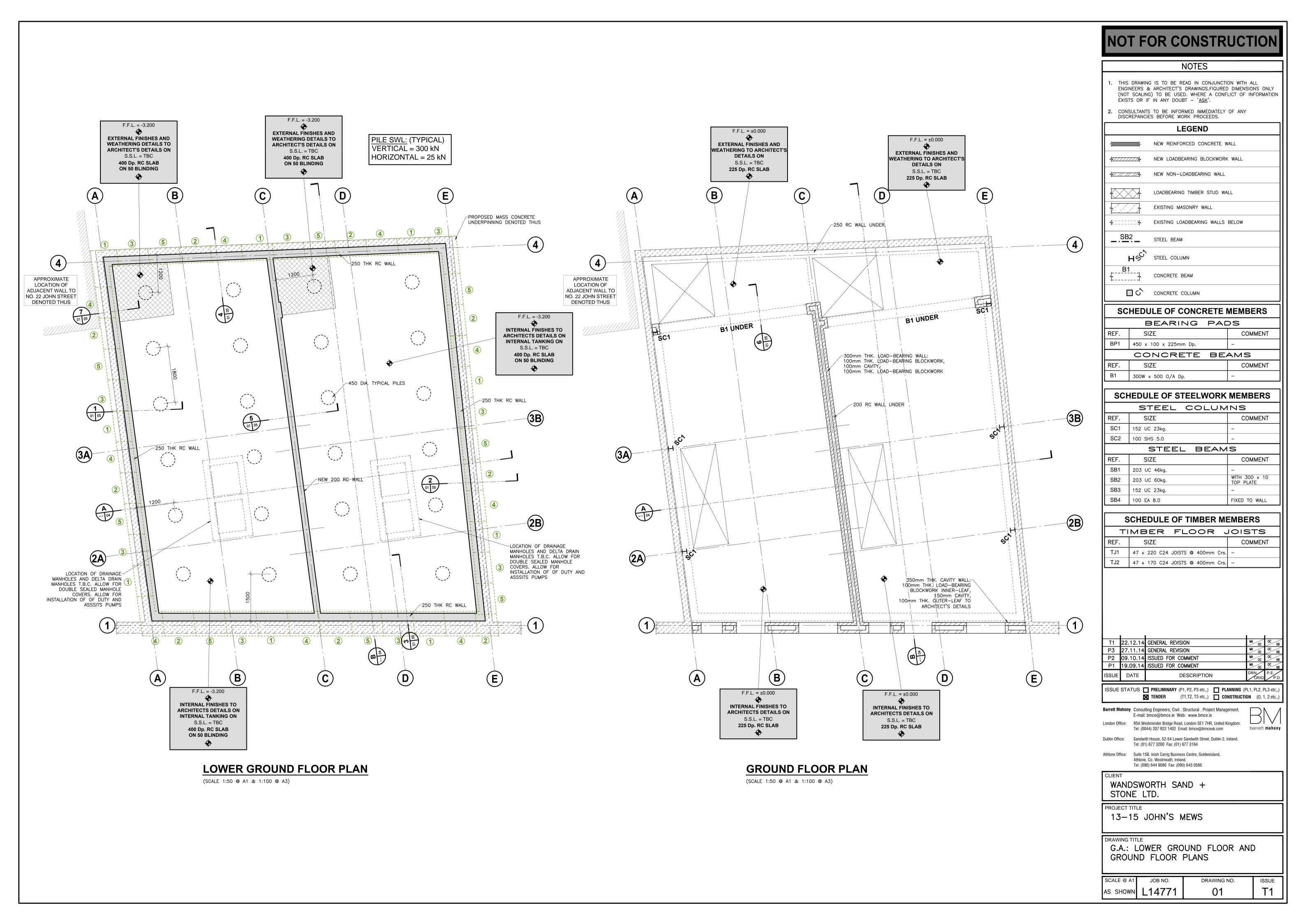
13 - 15 JOHN'S MEWS

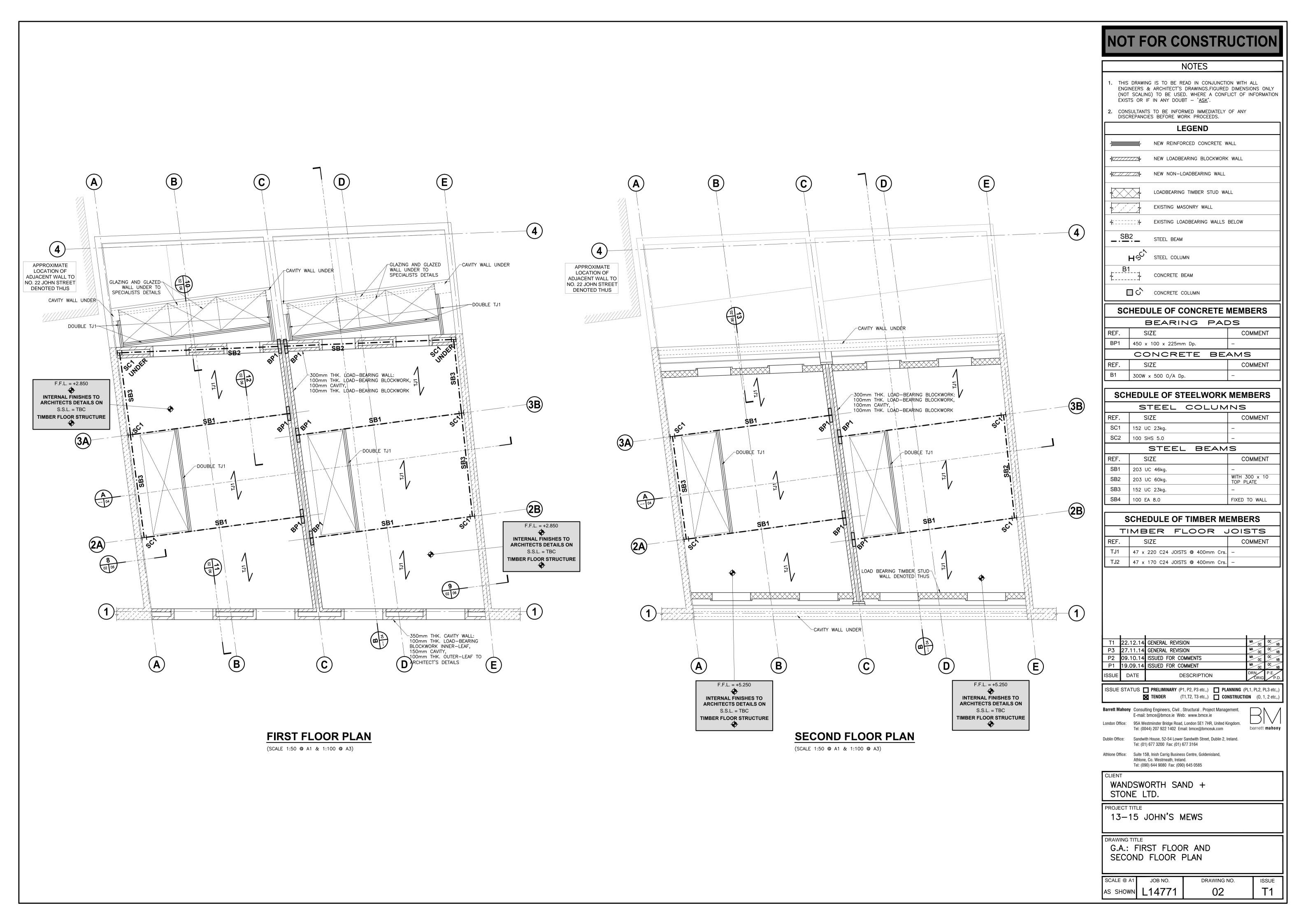
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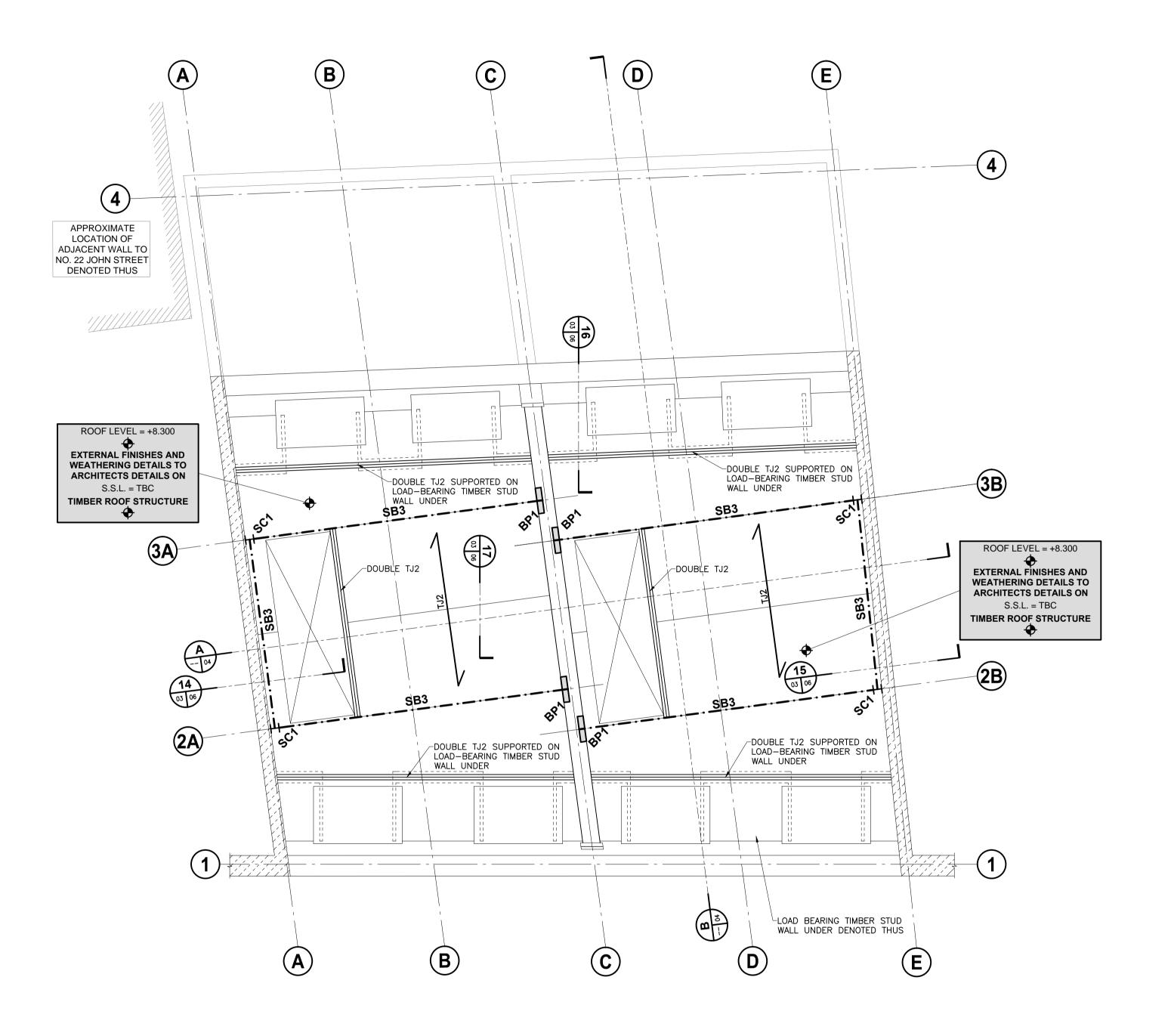
DRAWING TITLE GENERAL NOTES

JOB NO.

DRAWING NO. ISSUE L1477







ROOF PLAN

(SCALE 1:50 @ A1 & 1:100 @ A3)

NOT FOR CONSTRUCTION

NOTES

- THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL ENGINEERS & ARCHITECT'S DRAWINGS.FIGURED DIMENSIONS ONLY (NOT SCALING) TO BE USED. WHERE A CONFLICT OF INFORMATION EXISTS OR IF IN ANY DOUBT — 'ASK'.
- 2. CONSULTANTS TO BE INFORMED IMMEDIATELY OF ANY DISCREPANCIES BEFORE WORK PROCEEDS.

	LEGEND
	NEW REINFORCED CONCRETE WALL
4//////	NEW LOADBEARING BLOCKWORK WALL
4/////	NEW NON-LOADBEARING WALL
	LOADBEARING TIMBER STUD WALL
	EXISTING MASONRY WALL
₹ -	EXISTING LOADBEARING WALLS BELOW
SB2	STEEL BEAM
H _{&C,}	STEEL COLUMN
B1 	CONCRETE BEAM
□ c ²	CONCRETE COLUMN

SCHEDULE OF CONCRETE MEMBERS				
	BEARING	PAC	os	
REF.	SIZE		COMMENT	
BP1	450 x 100 x 225mm Dp.		-	
	CONCRETE	BE	AMS	
REF.	SIZE		COMMENT	

300W x 500 O/A Dp.

SCH	SCHEDULE OF STEELWORK MEMBERS				
	STEEL COLUMNS				
REF.	SIZE	COMMENT			
SC1	152 UC 23kg.	ı			
SC2	100 SHS 5.0	ı			
	STEEL BEAMS				
REF.	SIZE	COMMENT			
SB1	203 UC 46kg.	-			
SB2	203 UC 60kg.	WITH 300 x 10 TOP PLATE			
SB3	152 UC 23kg.	_			
SB4	100 EA 8.0	FIXED TO WALL			

TIMBER FLOOR JOISTS				
REF.	SIZE	COMMENT		
TJ1	47 x 220 C24 JOISTS @ 400mm Crs.			
TJ2	47 x 170 C24 JOISTS @ 400mm Crs.	_		

l ,		•		
T1	22.12.14	GENERAL REVISION	MA OC	OC VB
Р3	27.11.14	GENERAL REVISION	MA OC	OC VB
P2	09.10.14	ISSUED FOR COMMENT	MA OC	OC VB
P1	19.09.14	ISSUED FOR COMMENT	MA OC	OC VB
ISSUE	DATE	DESCRIPTION	DRN ORIG	P.E. P.D.
ISSUE	ISSUE STATUS T PRELIMINARY (P1, P2, P3 etc) T PLANNING (PL1, PL2, PL3 etc)			

SUE STATUS PRELIMINARY (P1, P2, P3 etc,,) PLANNING (PL1, PL2, PL3 etc,,) CONSTRUCTION (0, 1, 2 etc,,)

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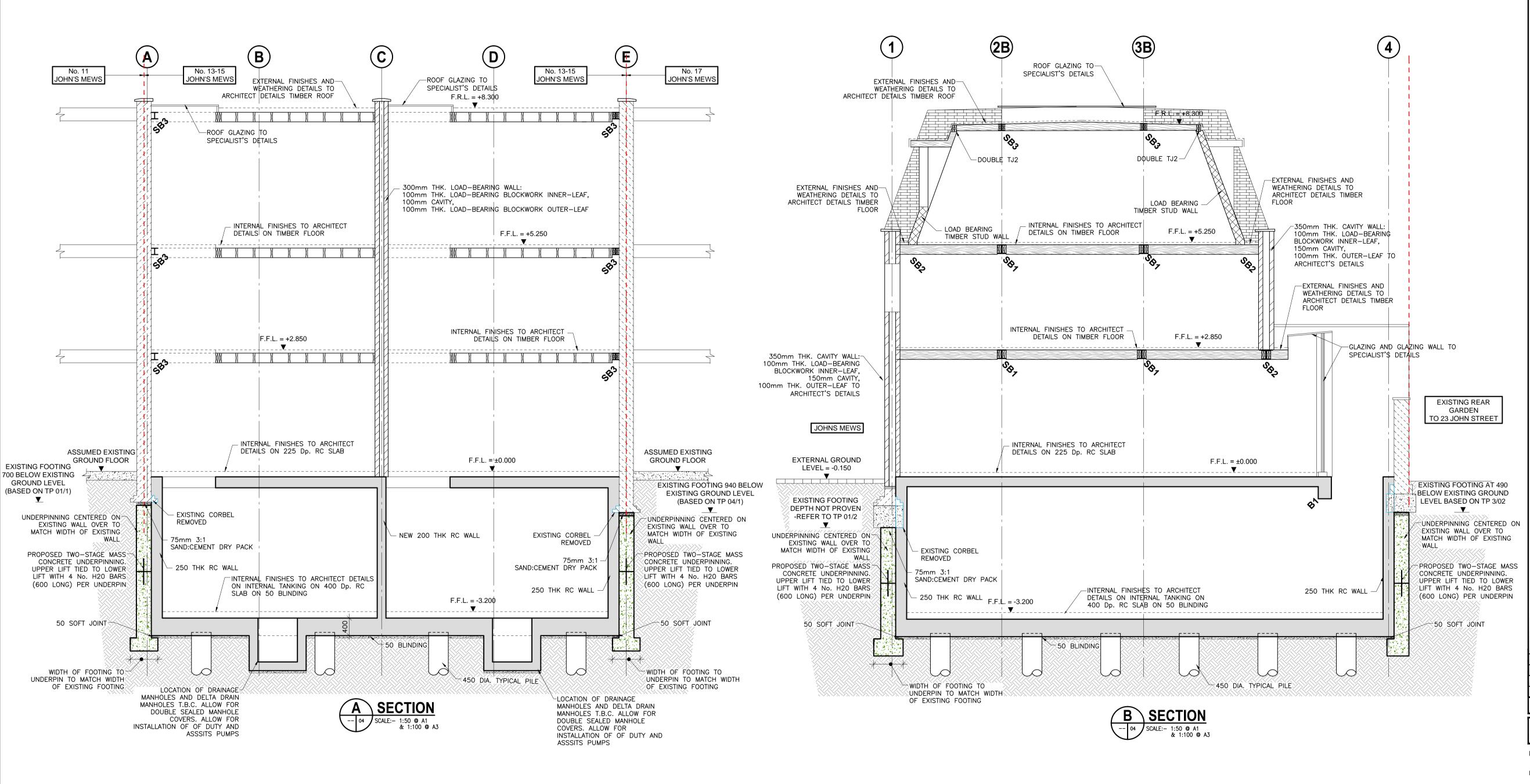
WANDSWORTH SAND + STONE LTD.

PROJECT TITLE

13-15 JOHN'S MEWS

G.A.: ROOF PLAN

SCALE @ A1	JOB NO.	DRAWING NO.	ISSUE
AS SHOWN	L14771	03	T1



NOT FOR CONSTRUCTION

NOTES

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- 2. CONSULTANTS TO BE INFORMED IMMEDIATELY OF ANY DISCREPANCIES BEFORE WORK PROCEEDS.

SCHEDULE OF CONCRETE MEMBERS

	BEARING	PAC	os
REF.	SIZE		COMMENT
BP1	450 x 100 x 225mm Dp.		_
	CONCRETE	BE	AMS
REF.	SIZE		COMMENT
B1	300W x 500 O/A Dp.		

SCHEDULE OF STEELWORK MEMBERS

	STEEL COLUMNS				
REF.	SIZE	COMMENT			
SC1	152 UC 23kg.	_			
SC2	100 SHS 5.0	_			
	STEEL BEAMS				
REF.	SIZE	COMMENT			
SB1	203 UC 46kg.	_			
SB2	203 UC 60kg.	WITH 300 x 10 TOP PLATE			
SB3	152 UC 23kg.	_			
SB4	100 EA 8.0	FIXED TO WALL			

SCHEDULE OF TIMBER MEMBERS

TIMBER FLOOR JOISTS				
REF.	SIZE		COMMENT	
TJ1	47 x 220 C24	JOISTS @ 400mm Crs.	_	
TJ2	47 x 170 C24	JOISTS @ 400mm Crs.	_	

	1				
T1	22.12.14	ISSUED FOR TENDER	MA OC	OC VB	
P3	27.11.14	GENERAL REVISION	MA OC	OC VB	
P2	09.10.14	ISSUED FOR COMMENT	MA OC	OC VB	
P1	19.09.14	ISSUED FOR COMMENT	MA OC	OC VB	
ISSUE	DATE	DESCRIPTION	DRN ORIG	P.E. P.D.	
ISSUE	ISSUE STATUS PRELIMINARY (P1, P2, P3 etc,,) PLANNING (PL1, PL2, PL3 etc,,) TENDER (T1,T2, T3 etc,,) CONSTRUCTION (0, 1, 2 etc,,)				

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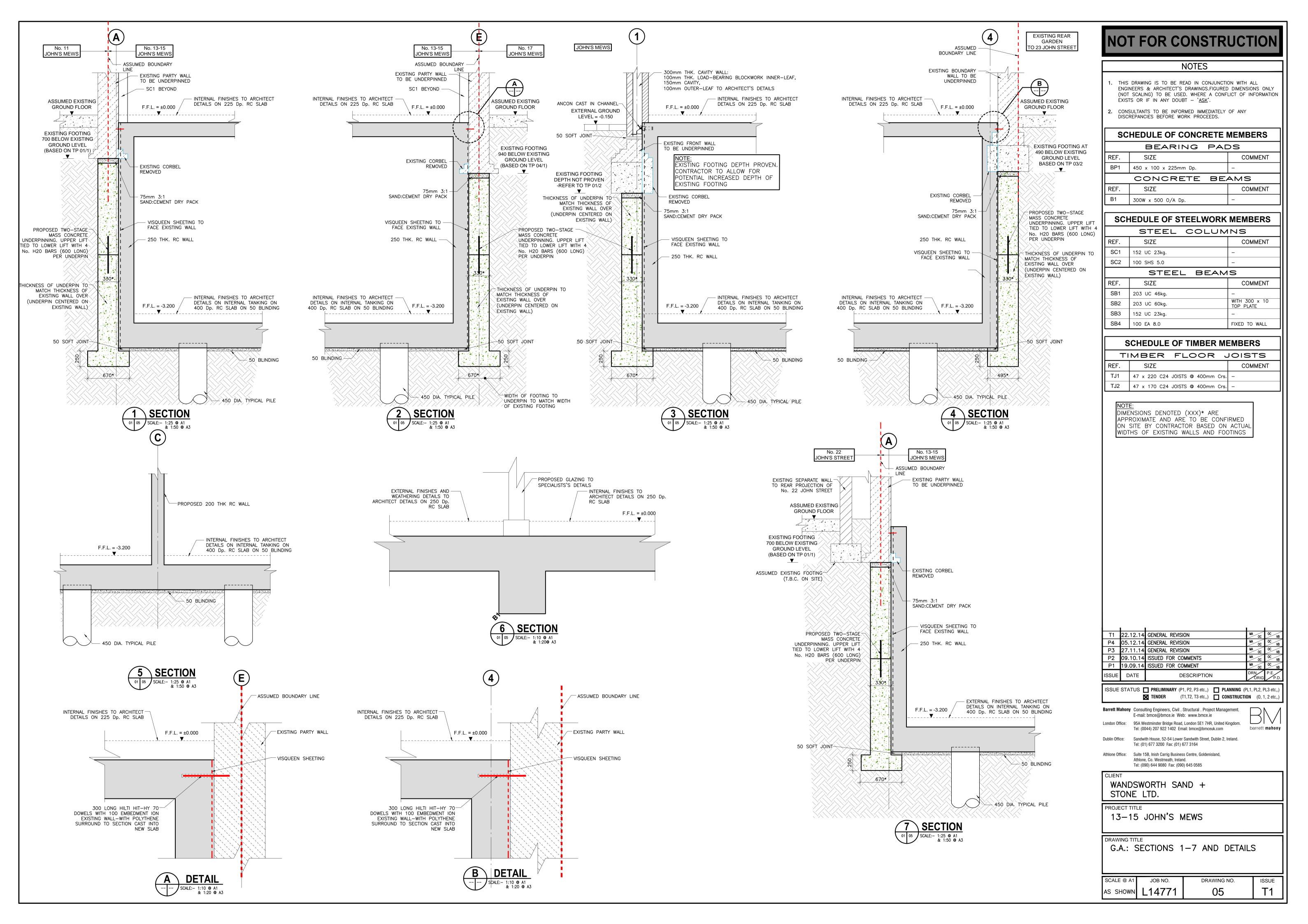
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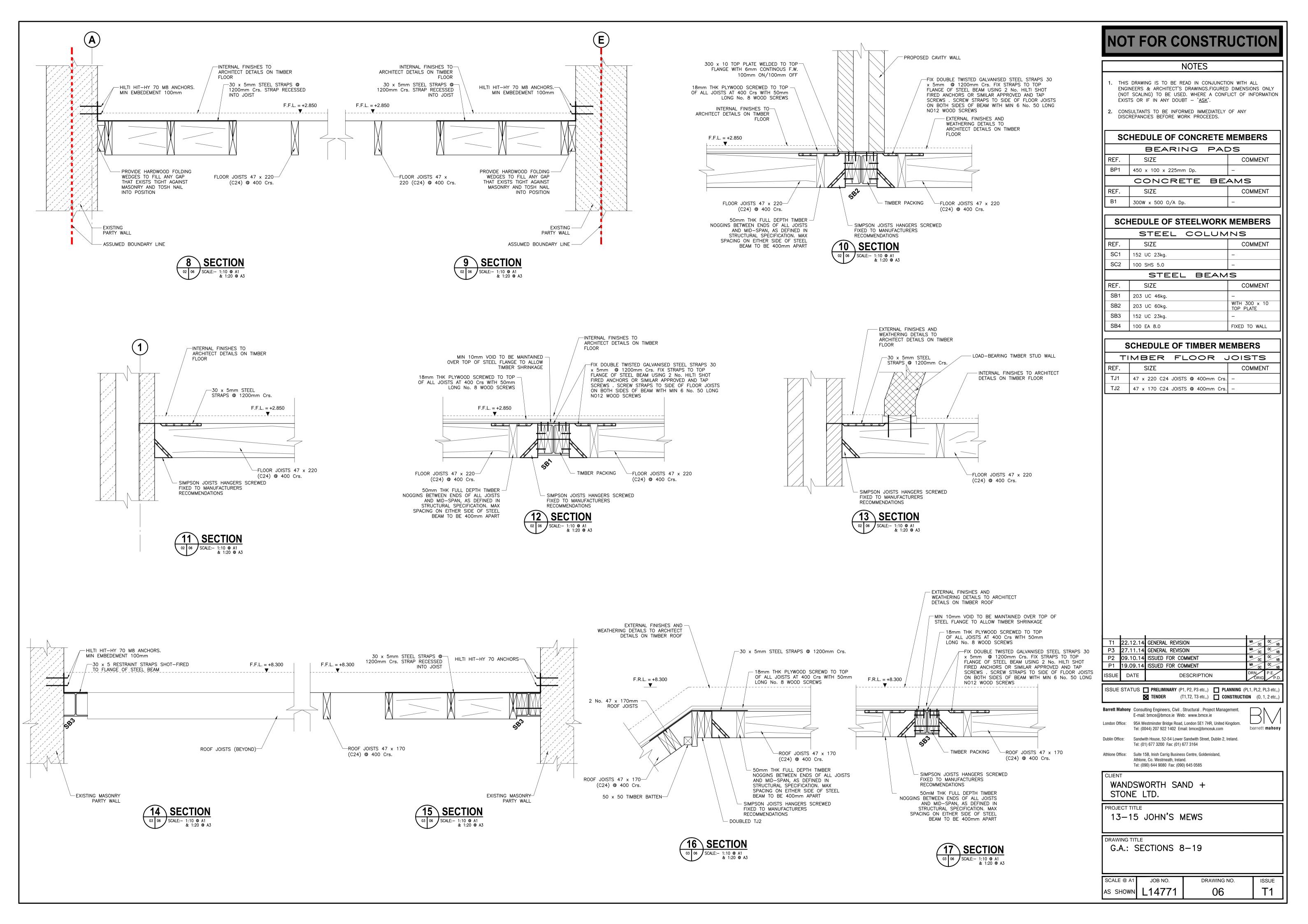
PROJECT TITLE

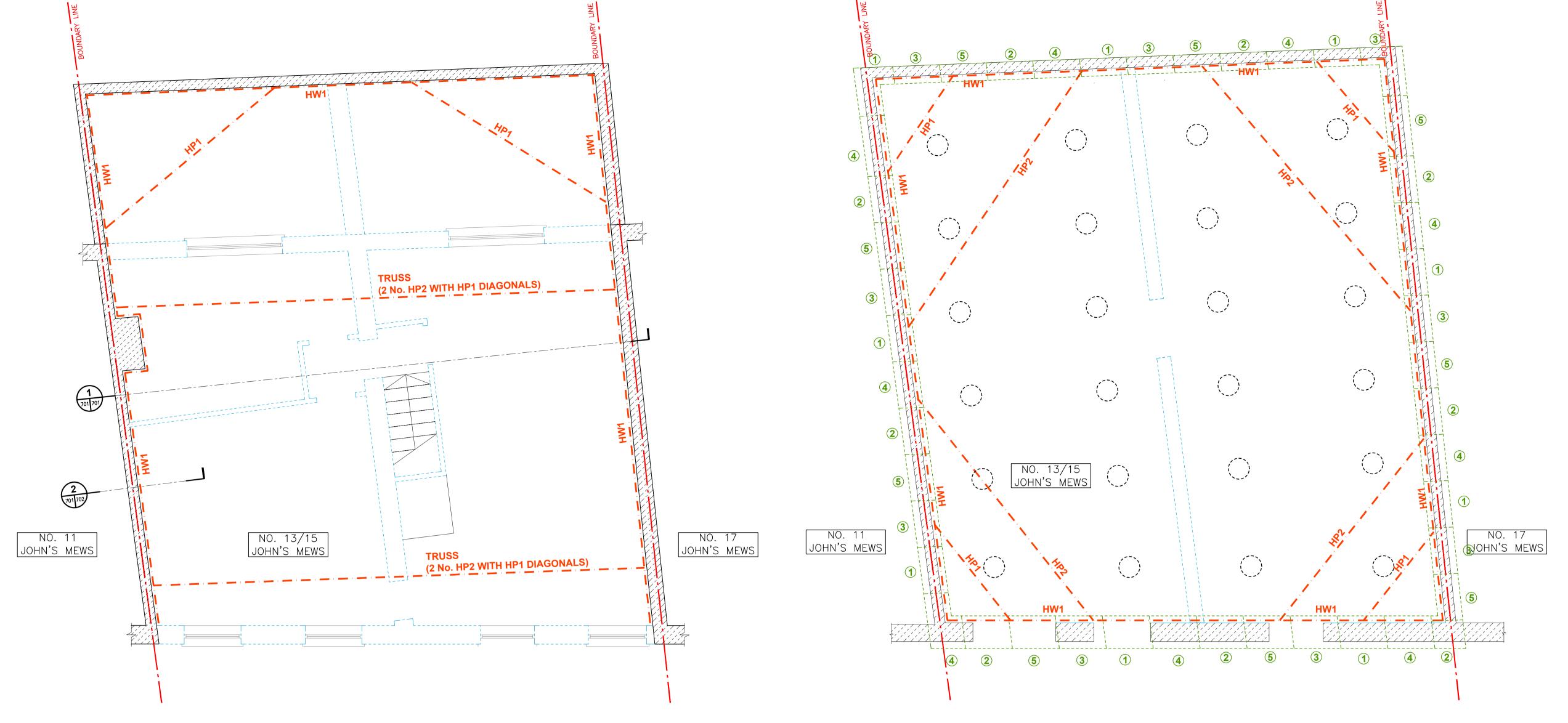
13-15 JOHN'S MEWS

G.A.: SECTIONS
A AND B

SCALE @ A1 JOB NO. DRAWING NO. ISSUE AS SHOWN L14771 04 T1







SUPERSTURCTURE T.W. PLAN (2 No. LEVELS THUS)

(SCALE 1:50 @ A1 & 1:100 @ A3)

SUBSTURCTURE T.W. PLAN (2 No. LEVELS THUS)

(SCALE 1:50 @ A1 & 1:100 @ A3)

METHOD STATEMENT FOR BASEMENT CONSTRUCTION AT No. 13-15 JOHN'S MEWS.

1. PHASE I: SUPERSTRUCTURE TEMPORARY WORKS

& DEMOLITION OF EXISTING STRUCTURE

- 1.1. CARRY OUT SOFT STRIP OF EXISTING STRUCTURE. REMOVE FINISHES, PLASTERBOARD, NON-LOADBEARING STUD WALL ETC. ALL FLOORBOARDS + JOISTS TO REMAIN IN POSITION UNTIL TEMPORARY WORKS HAVE BEEN INSTALLED.
- 1.2. BREAK OUT EXISTING GROUND FLOOR SLAB
- 1.3. NO FURTHER EXCAVATION SHALL TAKE PLACE UNTIL UNDERPINNING OF THE EXISTING WALLS HAS BEEN EXECUTED.
- 1.4. INSTALL TEMPORARY HORIZONTAL WALERS TO PARTY WALLS JUST BELOW EXISTING SECOND FLOOR (LOFT) LEVEL.
- 1.5. INSTALL TEMPORARY FLYING SHORE PROPS JUST BELOW EXISTING SECOND FLOOR (LOFT) LEVEL.
- 1.6. INSTALL TEMPORARY HORIZONTAL WALERS TO PARTY WALLS JUST ABOVE EXISTING FIRST FLOOR LEVEL.
- 1.7. INSTALL TEMPORARY FLYING SHORE PROPS JUST ABOVE EXISTING FIRST
- FLOOR LEVEL. 1.8. INSTALL TEMPORARY BRACING ELEMENTS BETWEEN SECOND FLOOR AND FIRST FLOOR PROPPING LEVELS TO FORM VERTICAL TRUSSES.
- 1.9. INSTALL HORIZONTAL WALER AND ASSOCIATED PROPPING TO TOP OF REAR BOUNDARY WALL TO BE RETAINED.
- 1.10. INSTALL PROPOSED TEMPORARY WEATHERING TO PARTY WALLS.
- 1.11. DEMOLISH EXISTING ROOF STRUCTURE.
- 1.12. DEMOLISH EXISTING MASONRY WALLS (EXCEPT BOUNDARY WALLS TO BE RETAINED) FROM ROOF LEVEL TO JUST ABOVE FIRST FLOOR LEVEL.
- 1.13. DEMOLISH EXISTING FIRST FLOOR.
- 1.14. DEMOLISH EXISTING MASONRY WALLS (EXCEPT BOUNDARY WALLS TO BE RETAINED) FROM FIRST FLOOR LEVEL TO GROUND FLOOR LEVEL.
- 1.15. DEMOLISH EXISTING FOOTINGS TO EXISTING SPINE WALL.

2. PHASE II: SUBSTRUCTURE TEMPORARY WORKS

& EXCAVATION FOR PROPOSED BASEMENT

- 2.1. UNDERPIN EXISTING WALL AS PER SEQUENCE INDICATED ON DRAWING L14771-01, IN ACCORDANCE WITH UNDERPINNING SPECIFICATION ON DRAWING L14771-00. AT THIS STAGE, THE UPPER LIFT UNDERPINNING ONLY SHALL BE CARRIED OUT. EACH BAY SHOULD BE BACKFILLED FOLLOWING FORMATION OF UNDERPIN, PRIOR TO PROCEEDING WITH THE NEXT UNDERPIN IN SEQUENCE.
- 2.2. INSTALL PILING MAT AT EXISTING GROUND LEVEL.
- 2.3. INSTALL PERMANENT PILES.
- 2.4. EXCAVATE TO 300 mm ABOVE UNDERSIDE OF UPPER LIFT OF UNDERPINS. 2.5. INSTALL TEMPORARY HORIZONTAL WALERS TO EXISTING WALLS JUST ABOVE INITIAL EXCAVATION LEVEL.
- 2.6. INSTALL TEMPORARY HORIZONTAL PROPS TO TEMPORARY WALERS JUST ABOVE INITIAL EXCAVATION LEVEL.
- 2.7. PROCEED WITH FORMATION OF LOWER LIFT OF UNDERPINS, DOWELLED TO UPPER LIFT OF UNDERPINS AS PER UNDERPINNING SPECIFICATION ON DRAWING L14771-00. EACH BAY SHOULD BE BACKFILLED FOLLOWING FORMATION OF UNDERPIN, PRIOR TO PROCEEDING WITH THE NEXT UNDERPIN IN SEQUENCE.
- 2.9. INSTALL TEMPORARY HORIZONTAL WALERS TO EXISTING WALLS JUST ABOVE SECOND EXCAVATION LEVEL.
- 2.10. INSTALL TEMPORARY HORIZONTAL PROPS TO TEMPORARY WALERS JUST ABOVE SECOND EXCAVATION LEVEL.

2.11. EXCAVATE TO PROPOSED FORMATION LEVEL FOR BASEMENT SLAB.

2.12. BREAK DOWN PILES TO THEIR REQUIRED CUT OFF LEVEL.

3. PHASE III: CONSTRUCTION & TEMPORARY

WORKS REMOVAL

- 3.1. PLACE BLINDING FOR PROPOSED BASEMENT SLAB.
- 3.2. INSTALL BELOW GROUND DRAINAGE ELEMENTS.
- 3.3. FIX REINFORCEMENT FOR BASEMENT SLAB.
- 3.4. CAST PROPOSED BASEMENT SLAB.
- 3.5. WHEN NEW BASEMENT SLAB HAS ATTAINED A STRENGTH OF 12 N/mm2, REMOVE LOW LEVEL TEMPORARY PROPS TO UNDERPINS.
- 3.6. FIX REINFORCEMENT FOR PROPOSED RISING ELEMENTS FROM BASEMENT TO GROUND FLOOR.
- 3.7. CAST PROPOSED RETAINING WALL TO 1500 mm ABOVE BASEMENT SSL.
- 3.8. WHEN NEW RETAINING WALLS HAVE ATTAINED A STRENGTH OF 12 N/mm2, REMOVE HIGH LEVEL TEMPORARY PROPS TO UNDERPINS.
- 3.9. CAST RETAINING WALLS UP TO GROUND FLOOR LEVEL AND CAST CONCRETE SPINE WALL.
- 3.10. FIX REINFORCEMENT FOR GROUND FLOOR SLAB AND CAST SLAB.
- 3.11. CONSTRUCT RISING ELEMENTS FROM GROUND FLOOR LEVEL TO FIRST FLOOR LEVEL.
- 3.12. CONSTRUCT PROPOSED FIRST FLOOR.
- 2.8. EXCAVATE TO 750 mm ABOVE FORMATION LEVEL FOR PROPOSED BASEMENT 3.13. REMOVE BOTTOM CHORD AND INTERNAL MEMBERS OF TEMPORARY TRUSS, LEAVING TOP CHORD IN POSITION.
 - 3.14. CONSTRUCT RISING ELEMENTS FROM FIRST FLOOR LEVEL TO SECOND FLOOR LEVEL.
 - 3.15. CONSTRUCT PROPOSED SECOND FLOOR.
 - 3.16. REMOVE REMAINING TEMPORARY WORKS.
 - 3.17. CONSTRUCT RISING ELEMENTS TO ROOF LEVEL. 3.18. CONSTRUCT PROPOSED ROOF STRUCTURE AND WEATHERING.

WORKS TO ARCHITECT'S SPECIFICATION

3.19. REMOVE TEMPORARY WEATHERING

3.20. PRIMARY STRUCTURAL WORKS ARE NOW COMPLETE. CARRY OUT FIT-OUT

NOT FOR CONSTRUCTION

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- CONSULTANTS TO BE INFORMED IMMEDIATELY OF ANY DISCREPANCIES BEFORE WORK PROCEEDS.
- ANY DISCREPANCY BETWEEN DETAILS INDICATED ON THIS DRAWING AND THOSE CONDITIONS ACTUALLY ENCOUNTERED ON SITE SHOULD BE HIGHLIGHTED BY THE CONTRACTORS SITE SUPERVISORY PERSONNEL
- **4.1** ALL STEELWORK TO BE GRADE 275 WITH FULLY WELDED CONNECTIONS THROUGHOUT

	SCHEDULE OF STEEL MEMBERS			
REF.	SIZE	COMMENTS		
HP1	150 UC 23 kg.	_		
HP2	203 UC 46 kg.	_		
HW1	230 x 90 PFC 32 kg.	1		

NOTE:

ALL TEMPORARY WORKS TO CONTRACTOR'S DESIGN AND DETAIL. THE CONTRACTOR IS RESPONSIBLE FOR TEMPORARY STABILITY OF ALL EXISTING AND ADJOINING STRUCTURES FOR THE DURATION OF THE WORKS. ALL TEMPORARY WORKS SHOWN IN DRAWING ARE INDICATIVELY ONLY.

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T1	22.12.14	ISSUED FOR TENDER	MA OC	OC VE
P4	27.11.14	GENERAL REVISION	MA OC	OC VB
P3	09.10.14	GENERALLY REVISED	MA OC	OC VB
P2	01.10.14	ISSUED FOR COMMENT	MA OC	S VB
P1	20.08.14	ISSUED FOR COMMENT	ARM OC	S VB
ISSUE	DATE	DESCRIPTION	DRN ORIG	P.E./ P.D

ISSUE STATUS PRELIMINARY (P1, P2, P3 etc,,) PLANNING (PL1, PL2, PL3 etc,,) **▼ TENDER** (T1,T2, T3 etc,,) **☐ CONSTRUCTION** (0, 1, 2 etc,

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WESTGATE + KEELE

No. 13-15 JOHN'S MEWS, LONDON, WC1N 2PA

DRAWING TITLE TEMPORARY WORKS + METHOD

STATEMENT PHASE I DEMOLITION

SCALE @ A1	JOB NO.	DRAWING NO.	ISSUE
AS SHOWN	L14771	701	T1

