GROUND INVESTIGATION REPORT

for the site at

35 GREVILLE ROAD, CAMDEN, LONDON NW6 5JB

on behalf of

UNAGI MANAGEMENT LIMITED

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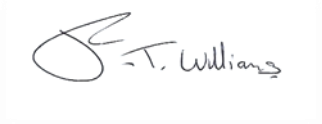
V1.01 August 2015		
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File Reference: Ground and Water/Project Files/ GWPR1303 35 Greville Road, London		

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1.0 INTRODUCTION

1.1 General

Ground and Water Limited were instructed by Unagi Management Limited on the 5th June 2015 to undertake a Ground Investigation on 35 Greville Road, London NW6 5JB. The scope of the investigation was detailed within the Ground and Water Limited fee proposal ref.: GWQ2482, dated 3rd June 2015.

1.2 Aims of the Investigation

The aim of the investigation was understood to be to supply the client and their designers with information regarding the ground conditions underlying the site to assist them in preparing an appropriate scheme for development.

The investigation was to be undertaken to provide parameters for the design of foundations by means of in-situ and laboratory geotechnical testing undertaken on soil samples recovered from trial holes.

The requirements of the London Borough of Camden, Camden Geological, Hydrogeological and Hydrological Study, Guidance for Subterranean Development (November 2010) was reviewed with respect to this report.

A Desk Study and full scale contamination assessment were not part of the remit of this report.

The techniques adopted for the investigation were chosen considering the anticipated ground conditions and development proposals on-site, and bearing in mind the nature of the site, limitations to site access and other logistical limitations.

1.3 Conditions and Limitations

This report has been prepared based on the terms, conditions and limitations outlined within Appendix A.

2.0 SITE SETTING

2.1 Site Location

The site comprised an approximately rectangular shaped plot of land, totalling ~800m^2 in area and orientated in a north-east to south-west direction. Mortimer Crescent was noted adjacent to the western boundary of the site with access to Marrick House to the north. The site was located in St Johns Wood/Kilburn, north-west London, within in the London Borough of Camden.

The national grid reference for the centre of the site was approximately TQ 25804 83516. A site location plan is given within Figure 1. A plan showing the boundary of the site is provided in Figure 2.

2.2 Site Description

The site comprised an end-terrace four storey brick built residential house with lower and upper ground floors. The front of the site, bordering Mortimer's Crescent, was dominated by a single storey double garage with the property to the rear. A single storey extension was noted to the south-west of the property with grassed soft landscaping beyond. An aerial view of the site is provided within Figure 3.

2.3 Proposed Development

At the time of reporting, August 2015, it was understood that the proposed development will comprise the construction of a basement below the entire footprint of the existing house. A plan showing the proposed development can be seen in Figure 4. The basement is anticipated to be formed at 4.30m below ground level (bgl).

2.4 Geology

The BGS Geological Map (Solid and Drift) for the North London area (Sheet No. 256), and Figures 3 and 4 of the Camden Geological, Hydrogeological and Hydrological Study, revealed that the site was underlain by the London Clay Formation.

London Clay Formation

The London Clay Formation comprises stiff grey fissured clay, weathering to brown near surface. Concretions of argillaceous limestone in nodular form (Claystones) occur throughout the formation. Crystals of Gypsum (Selenite) are often found within the weathered part of the London Clay Formation, and precautions against sulphate attack to concrete are sometimes required. The lowest part of the formation is a sandy bed with black rounded gravel and occasional layers of sandstone and is known as the Basement Bed.

A BGS borehole located ~50m south-east of the site revealed ~0.30m of Made Ground to overlie firm to very stiff, brown blue, becoming blue with depth, silty clays to 12.16m bgl. Claystone bands were noted.

No areas of Made Ground or Worked Ground were noted within a 250m radius of the site.

2.5 Slope Stability and Subterranean Developments

The site was not situated within an area where a natural or man-made slope of greater than 7^o was present (Figure 16 Camden Geological, Hydrogeological and Hydrological Study).

Figure 17 of the Camden Geological, Hydrogeological and Hydrological Study indicated that the site was not situated within an area prone to landslides.

Figure 18 of the Camden Geological, Hydrogeological and Hydrological Study indicated that the London Overground was situated running east to west ~220m north-west of the site. No major subterranean infrastructures (including existing and proposed tunnels) were noted within close proximity to the site.

2.6 Hydrogeology and Hydrology

A study of the aquifer maps on the Environment Agency website, and Figure 8 of the Camden Geological, Hydrogeological and Hydrological Study, revealed the site to be located on **Unproductive Strata** relating to the bedrock deposits of the London Clay Formation. No designation was given for any superficial deposits due to their likely absence.

Superficial (Drift) deposits are permeable unconsolidated (loose) deposits, for example, sands and gravels.The bedrock is described as solid permeable formations e.g. sandstone, chalk and limestone.

Unproductive strata are rock layers with low permeability that have negligible significance for water supply or river base flow. These were formerly classified as non-aquifers.

Examination of the Environment Agency records, and Figure 8 of the Camden Geological, Hydrogeological and Hydrological Study, showed that the site did not fall within a Groundwater Source Protection Zone as classified in the Policy and Practice for the Protection of Groundwater.

No surface water features were noted within a 250m radius of the site.

From analysis of hydrogeological and topographical maps groundwater was anticipated to be encountered at depth (>10m below existing ground level (bgl)) and it was considered that the groundwater was flowing in a south-westerly direction in alignment with the local topography.

Examination of the Environment Agency records showed that the site was **not** situated within flood zone or flood warning area.

2.7 Radon

BRE 211 (2007) Map 5 of the London, Sussex and west Kent area revealed the site was located within an area where mandatory protection measures against the ingress of Radon were **unlikely to be** required. The site **was not** located within an area where a risk assessment was required.

3.0 FIELDWORK

3.1 Scope of Works

Fieldwork was undertaken on the 3rd July 2015 and comprised the drilling of one Premier Windowless Sampler Borehole (BH1) to a depth of 6.00m bgl. A Heavy Dynamic Probe (DP1) was undertaken adjacent to BH1 to a depth of 10.00m bgl.

A small diameter combined bio-gas and groundwater monitoring well was installed within BH1 to 5.00m bgl. The construction of the well installed can be seen tabulated below.

Combined Bio-gas and Groundwater Monitoring Well Construction				
Trial Hole	**Depth of Installation (m bgl)**	**Thickness of slotted piping with gravel filter pack (m)**	**Depth of plain piping with bentonite seal (m bgl)**	**Piping external diameter (mm)**
BH1	5.00	4.00	1.00	19

The approximate locations of the trial hole can be seen within Figure 5.

Prior to commencing the ground investigation, a walkover survey was carried out to identify the presence of underground services and drainage. Where underground services/drainage were suspected and/or positively identified, exploratory positions were relocated away from these areas.

Upon completion of the site works, the trial hole was backfilled and made good/reinstated in relation to the surrounding area.

3.2 Sampling Procedures

Small disturbed samples were recovered from the trial hole at the depths shown on the trial hole record. Soil samples were generally retrieved from each change of strata and/or at specific areas of concern. Samples were also taken at approximately 0.5m intervals during broad homogenous soil horizons. A selection of samples were despatched for geotechnical testing purposes.

4.0 ENCOUNTERED GROUND CONDITIONS

4.1 Soil Conditions

All exploratory holes were logged by Phillip Allvey of Ground and Water Limited generally in accordance with BS EN 14688 'Geotechnical Investigation and Testing – Identification and Classification of Soil'.

The ground conditions encountered within the trial hole constructed on the site generally conformed to that anticipated from examination of the geology map. Made Ground was noted to overlie the London Clay Formation.

The ground conditions encountered during the investigation are described in this section. For more complete information about the Made Ground and the London Clay Formation at particular points, reference must be made to the individual trial hole log within Appendix B.

The trial hole location plan can be viewed in Figure 5.

For the purposes of discussion the succession of conditions encountered in BH1 in descending order can be summarised as follows:

Made Ground
London Clay Formation

Made Ground

Made Ground was encountered from ground level to 0.60m bgl and comprised a brown to dark brown gravelly very sandy to sandy clay. The sand was fine to medium grained and the gravel was occasional to abundant, fine to medium, sub-angular to sub-rounded flint, brick and tile fragments.

London Clay Formation

Soils described as representative of the London Clay Formation were encountered underlying the Made Ground and were proved for the remaining depth of the borehole, a maximum of 6.00m bgl. The deposits were described as a mid-brown and grey mottled silty clay. Selenite crystals and silt lenses were noted throughout.

For details of the composition of the soils encountered at particular points, reference must be made to the individual trial hole log within Appendix B.

4.3 Roots Encountered

Roots were noted to 3.00m bgl within BH1.

It must be noted that the chance of determining actual depth of root penetration through a narrow diameter borehole is low. Roots may be found to greater depths at other locations on the site, particularly close to trees and/or trees that have been removed both within the site and its close environs.

4.4 Groundwater Conditions

No groundwater was encountered during the construction of BH1. The standing groundwater level noted during two return visits to the site can be seen tabulated overpage.

Groundwater Observations					
Project Ref	Site Location	Borehole Ref.	Groundwater reading (m bgl)	Depth to base of borehole (m bgl)	Date
GWPR1303	35 Greville Road, London NW6 5JB	BH1	0.83m bgl	5.00m	04/08/2015
GWPR1303	35 Greville Road, London NW6 5JB	BH1	0.94m bgl	5.00m	10/08/2015

The standing water levels recorded in BH1 during the two return visits are likely to represent surface water or perched groundwater migrating through the Made Ground and/or silty lenses in the London Clay Formation and collecting within the installed standpipe.

Exact groundwater levels may only be determined through long term measurements from monitoring wells installed on-site. It should be noted that changes in groundwater level do occur for a number of reasons including seasonal effects and variations in drainage.

The site investigation was conducted in July and August 2015, when groundwater levels should be close to their annual minimum (i.e. lowest). The long-term groundwater elevation might increase at some time in the future due to seasonal fluctuation in weather conditions. Isolated pockets of groundwater may be perched within any Made Ground found at other locations around the site.

4.5 Obstructions

No artificial or natural sub-surface obstructions were noted during construction of the trial hole.

5.0 INSITU AND LABORATORY GEOTECHNICAL TESTING

5.1 In-Situ Geotechnical Testing

A Heavy Dynamic Probe (HDP) (DP1) was undertaken adjacent to BH1 to a depth of 10.00m bgl. The test results are presented with the borehole log in Appendix B.

Windowless Sampler Boreholes provide samples of the ground for assessment but they do not give any engineering data.

Heavy Dynamic Probing involves the driving of a metal cone into the ground via a series of steel rods. These rods are driven from the surface by a hammer system that lifts and drops a 50.0kg hammer onto the top of the rods through a set height (500mm), thus ensuring a consistent energy input. The numbers of hammer blows that are required to drive the cone down by each 100mm increment are recorded. These blow counts then provide a comparative assessment from which correlations have been published, based on dynamic energy, which permits engineering parameters to be generated. (The Dynamic Probe 'Heavy' (HDP) Tests were conducted in accordance with BS 1377; 1990; Part 9, Clause 3.2).

The cohesive soils of the London Clay Formation were classified based on the table below.

Undrained Shear Strength from Field Inspection/ Equivalent SPT blow counts $(N_1)_{60}$ as derived from dynamic probe: Cohesive Soils (EN ISO 14688-2:2004 & Stroud (1974))		
Classification	**Undrained Shear Strength (kPa)**	**Field Indications**
Extremely High	>300	-
Very High	150 – 300	Brittle or very tough
High	75 – 150	Cannot be moulded in the fingers
Medium	40 – 75	Can be moulded in the fingers by strong pressure
Low	20 – 40	Easily moulded in the fingers
Very Low	10 – 20	Exudes between fingers when squeezed in the fist
Extremely Low	<10	-

An interpretation of the in-situ geotechnical testing results is given in the table below.

Interpretation of In-situ Geotechnical Testing Results (DP1)					
Strata	**Equivalent SPT "N" Blow Counts**	**Equivalent Undrained Shear Strength (kPa) Cohesive Soils**	**Soil Type**		**Trial Hole/s**
			Cohesive	**Granular**	
London Clay Formation	4 – 20	20 - 100	Very Low – High	-	BH1 (0.60 – 10.00m bgl)

It must be noted that field measurements of undrained shear strength are dependent on a number of variables including disturbance of sample, method of investigation and also the size of specimen or test zone etc.

5.2 Laboratory Geotechnical Testing

A programme of geotechnical laboratory testing scheduled by Ground and Water Limited and carried out by K4 Soils Laboratory and QTS Environmental Limited was undertaken on samples recovered from the London Clay Formation. The results of the tests are presented in Appendix C.

The test procedures used were generally in accordance with the methods described in BS1377:1990.

Details of the specific tests used in each case are given below.

Standard Methodology for Laboratory Geotechnical Testing		
Test	**Standard**	**Number of Tests**
Atterberg Limit Tests	BS1377:1990:Part 2:Clauses 3.2, 4.3 & 5	4
Moisture Content	BS1377:1990:Part 2:Clause 3.2	4
BRE Special Digest 1 (incl. Ph, Electrical Conductivity, Total Sulphate, W/S Sulphate, Total Chlorine, W/S Chlorine, Total Sulphur, Ammonium as NH4, W/S Nitrate, W/S Magnesium)	BRE Special Digest 1 "Concrete in Aggressive Ground (BRE, 2005).	2

5.2.1 Atterberg Limit Tests

A précis of Atterberg Limit Tests undertaken on four samples of the London Clay Formation can be seen tabulated below.

Atterberg Limit Tests Results Summary							
Stratum/Trial Hole/Depth (m bgl)	**Moisture Content (%)**	**Passing 425 µm sieve (%)**	**Modified PI (%)**	**Soil Class**	**Consistency Index (Ic)**	**Volume Change Potential**	
						BRE	**NHBC**
London Clay Formation BH1/1.50	28	100	49	CV	0.96 (Stiff)	High	High
London Clay Formation BH1/2.50	31	100	53	CV	0.92 (Stiff)	High	High
London Clay Formation BH1/3.00	31	100	51	CV	0.94 (Stiff)	High	High
London Clay Formation BH1/4.00	29	100	50	CV	0.92 (Stiff)	High	High

NB: NP – Non-plastic
BRE Volume Change Potential refers to BRE Digest 240 (based on Atterberg results)
Soil Classification based on British Soil Classification System.
Consistency Index (Ic) based on BS EN IS0 14688-2:2004.

5.2.2 Comparison of Soil's Moisture Content with Index Properties

5.2.2.1 Liquidity Index Analyses

The results of the Atterberg Limit tests undertaken on four samples of the London Clay Formation were analysed to determine the Liquidity Index of the samples. This

gives an indication as to whether the samples recovered showed a moisture deficit and their degree of consolidation. The results are tabulated below.

The test results are presented within Appendix C.

Liquidity Index Calculations Summary					
Stratum/Trial Hole/Depth	**Moisture Content (%)**	**Plastic Limit (%)**	**Modified Plasticity Index (%)**	**Liquidity Index**	**Result**
London Clay Formation **BH1/1.50m bgl** (Dark orange brown and occasional blue grey silty CLAY with traces of fine rootlets and selenite crystals)	28	26	49	0.04	Heavily Overconsolidated
London Clay Formation **BH1/2.50m bgl** (Brown and occasional blue grey silty CLAY with traces of fine rootlets)	31	27	53	0.08	Heavily Overconsolidated
London Clay Formation **BH1/3.00m bgl** (Brown and occasional blue grey silty CLAY with traces of fine rootlets)	31	28	51	0.06	Heavily Overconsolidated
London Clay Formation **BH1/4.00m bgl** (Brown silty CLAY with traces of selenite crystals)	29	25	50	0.08	Heavily Overconsolidated

The results in the table above indicate that no potential moisture deficit is present within the heavily overconsolidated samples of the London Clay Formation tested.

5.2.2.2 Liquid Limit

A comparison of the soil moisture content and the liquid limit can be seen tabulated below.

Moisture Content vs. Liquid Limit				
Strata/Trial Hole/Depth/Soil Description	**Moisture Content (MC) (%)**	**Liquid Limit (LL) (%)**	**40% Liquid Limit (LL)**	**Result**
London Clay Formation **BH1/1.50m bgl** (Dark orange brown and occasional blue grey silty CLAY with traces of fine rootlets and selenite crystals)	28	75	30	**MC < 0.4 x LL (Potential Significant Moisture Deficit)**
London Clay Formation **BH1/2.50m bgl** (Brown and occasional blue grey silty CLAY with traces of fine rootlets)	31	80	32	**MC < 0.4 x LL (Potential Significant Moisture Deficit)**
London Clay Formation **BH1/3.00m bgl** (Brown and occasional blue grey silty CLAY with traces of fine rootlets)	31	79	31.6	**MC < 0.4 x LL (Potential Significant Moisture Deficit)**
London Clay Formation **BH1/4.00m bgl** (Brown silty CLAY with traces of selenite crystals)	29	75	30	**MC < 0.4 x LL (Potential Significant Moisture Deficit)**

The results in the table above indicated that a potential significant moisture deficit was present within all four samples of the London Clay Formation tested within BH1 (1.50m, 2.50, 3.00m and 4.00m bgl). The moisture content values were below 40%

of the liquid limits.

The samples were described as a brown and locally blue grey silty clay with traces of fine rootlets to 3.00m bgl. Traces of selenite crystals were noted within the samples at 1.50m and 4.00m bgl. The roots noted at 1.50m, 2.50m and 3.00m bgl could suggest that the moisture deficits within the samples were due to the moisture demand from surrounding roots/trees. The potential significant moisture deficit recorded within the sample at 4.00m is likely to be associated with the heavily overconsolidated nature of the soils rather than the moisture demand from roots/trees.

5.2.3 Moisture Content Profiling

Moisture content versus depth plots for BH1 can be seen within Figure 6. Within BH1 subtle variations in moisture content were noted with depth. The moisture content was noted to increase with depth to 2.50m bgl indicating a slight moisture deficit at the near surface. Roots were noted to 3.00m bgl and therefore the potential moisture deficit noted is likely to be associated with the moisture demand from roots/trees.

5.2.6 BRE Special Digest 1

In accordance with BRE Special Digest 1 'Concrete in Aggressive Ground' (BRE, 2005) two samples of the London Clay Formation (BH1/1.00m and BH1/5.50m bgl) were scheduled for laboratory analysis to determine parameters for concrete specification.

The results are given within Appendix C and a summary is tabulated below.

Summary of Results of BRE Special Digest Testing

Determinand	Unit	Minimum	Maximum
pH	-	7.9	8
Ammonium as NH_4	mg/kg	5.7	7.4
Sulphur	%	0.03	0.20
Chloride (water soluble)	mg/kg	40	117
Magnesium (water soluble)	mg/l	16	150
Nitrate (water soluble)	mg/kg	<3	5
Sulphate (water soluble)	mg/l	348	2890
Sulphate (total)	%	0.09	0.58

6.0 ENGINEERING CONSIDERATIONS

6.1 Soil Characteristics and Geotechnical Parameters

Based on the results of the intrusive investigation and geotechnical laboratory testing the following interpretations have been made with respect to engineering considerations.

- Made Ground was noted from ground level to 0.60m bgl.

  As a result of the inherent variability of Made Ground, it is usually unpredictable in terms of bearing capacity and settlement characteristics. Foundations should, therefore, be taken through any Made Ground and either into, or onto a suitable underlying natural stratum of adequate bearing characteristics.

  Made Ground may be found to deeper depth at other locations on the site, especially close to former structures/foundations and service runs.

- Soils described as the London Clay Formation were encountered underlying the Made Ground for the remaining depth of BH1, a maximum of 6.00m bgl.

  The deposits encountered were described as mid-brown and grey mottled silty clay. Selenite crystals and silt lenses were noted throughout.

  The London Clay Formation was shown to have very low to high undrained shear strength (20 – 100kPa).

  Geotechnical testing revealed the soils of the London Clay Formation to have a **high volume change potential** in accordance with both BRE240 and NHBC Standards Chapter 4.2. Consistency Index calculations indicated these soils to be stiff. The deposits of the London Clay Formation were shown to be heavily overconsolidated cohesive soils.

  Potential significant moisture deficits were present within all four samples of the London Clay Formation tested (BH1/1.50m, BH1/2.50m, BH1/3.00m and BH1/4.00m bgl). The moisture content values were below 40% of the liquid limit. The samples were described as a brown and locally blue grey silty clay with traces of fine rootlets to 3.00m bgl. Traces of selenite crystals were noted within the samples at 1.50m and 4.00m bgl. The roots noted at 1.50m, 2.50m and 3.00m bgl could suggest that the moisture deficits within the samples were due to the moisture demand from surrounding roots/trees. The potential significant moisture deficit recorded within the sample at 4.00m is likely to be associated with the heavily overconsolidated nature of the soils rather than the moisture demand from roots/trees.

  The heavily overconsolidated cohesive soils of the London Clay Formation were considered a suitable bearing stratum for moderately loaded footings/foundations. Settlements on loading are likely to be moderate.

The final design of foundations will need to take into account the volume change potential of the soil, the depth of root penetration and/or desiccation and the likely serviceability and settlement requirements of the proposed structure. These parameters for design are discussed in the next section of this report.

- No groundwater was encountered during the construction of the trial hole. The standing

groundwater levels were recorded as 0.83m and 0.94m bgl during two return visits to the site. These water levels are likely to represent migrating perched groundwater or surface water which has accumulated within the installed standpipe.

- Roots were noted to 3.00m bgl.

6.2 Basement Foundations

At the time of reporting, August 2015, it was understood that the proposed development will comprise the construction of a basement below the entire footprint of the existing house. The basement is anticipated to be formed at 4.30m bgl. A plan showing the proposed development can be seen in Figure 4.

The proposed development is likely to fall within Geotechnical Design Category 2 in accordance with Eurocode 7. The proposed foundation loads were not known to Ground and Water Limited at the time of reporting but are likely to range from 100 – 150kN/m^2.

Foundations constructed within the soils of London Clay Formation should be designed in accordance with soils of **high volume change potential** in accordance with BRE Digest 240 and NHBC Chapter 4.2.

Given the cohesive nature of the shallow deposits, foundations must therefore not be placed within cohesive root penetrated and/or desiccated soils and the influence of the trees surrounding the site must be taken into account (NHBC Standards Chapter 4.2). It is recommended that foundations are taken at least 300mm into non-root penetrated strata.

Roots were noted to 3.00m bgl within BH1. The proposed foundation level for the basement is over 300mm below this depth.

It is considered likely the proposed basements will be constructed with load bearing concrete retaining walls with semi-ground bearing concrete floors.

The following bearing capacities could be adopted for 5.0m long by 0.75m and 1.0m wide footings, or 1.50m by 1.50m pads at a depths of 4.30m bgl. The bearing capacities are tabulated below.

Limit State: Bearing Capacities Calculated (Based on DP1)		
Depth (m bgl)	**Foundation System**	**Limit Bearing Capacity (kN/m^2) (EC2)**
4.3m	5.00m by 0.75m Strip	128.35
	5.00m by 1.00m Strip	130.27
	1.50m by 1.50m Pad	140.17

Serviceability State: Settlement Parameters Calculated (Based on DP1)			
Depth (m bgl)	**Foundation System**	**Limit Bearing Capacity (kN/m^2)**	**Settlement (mm)**
4.3m	5.00m by 0.75m Strip	120	<13
	5.00m by 1.00m Strip	125	<17
	1.50m by 1.50m Pad	130	<17

It must be noted that a bearing capacity of less than 79kN/m^2 at 4.30m bgl respectively could result

in heave due to a reduction in effective stress at depth. This will need to be taken into account in the final design.

Excavations must be kept dry and either concreted or blinded as soon after excavation as possible. If water were allowed to accumulate on the formation level for even a short time not only would an increase in heave occur resulting from the soil increasing in volume by taking up water, but also the shear strength and hence the bearing capacity would also be reduced.

Groundwater was not encountered during the construction of the trial hole. The standing groundwater levels were recorded as 0.83m and 0.94m bgl during two return visits to the site. These water levels are likely to represent migrating perched groundwater or surface water which has accumulated within the installed standpipe. Therefore, groundwater is unlikely to be encountered during excavation of the basement. However, perched water is considered likely to be encountered, and therefore dewatering may be required, this must be taken into account in final design.

If the construction works take place during the winter months, when the groundwater level is expected to be at its higher elevation, perched water could accumulate thus dewatering could be required to facilitate the construction and prevent the base of the excavation blowing before the slab was cast. The advice of a reputable dewatering contractor, familiar with the type of ground and groundwater conditions encountered on this site, should be sought prior to finalising the design of the excavation for the basement.

General Recommendations for Spread Foundations:

- Foundation excavations must be carefully bottomed out and any loose soil or soft spots removed prior to the foundation concrete or blinding being placed. Failure to ensure that foundation excavations are suitably bottomed out could result in additional settlements.

- Inspection of foundation excavations, prior to concreting, must be made by a competent and suitably qualified person to check for any soft spots and to check for the presence of roots.

- The excavation must be kept dry as accumulation of water could result in increased settlements.

- Foundations must not be cast over foundations of former structures and/or other hard spots.

- Any groundwater or surface water ingress must be prevented from entering foundation trenches.

- Isolated Pad Foundations must be at least 1.5 times the width of the widest pad apart to keep to the anticipated settlements.

- Special foundation precautions will be required to prevent possible future shrinkage/swelling within clay strata affecting the integrity of the ground beams. A void, void former or compressible layer must be provided to accommodate potential movement below all ground beams. Compressible material or a void former should also be provided against the inside faces of ground beams.

- Final designs for the foundations should be carried out by a suitably qualified Engineer based

on the findings of this investigation and with reference to the anticipated loadings, serviceability requirements for the structure and the developments proximity to former, present and proposed trees.

6.3 Piled Foundations

Based on the results of the investigation it was considered unlikely that a piled foundations scheme would be required at this site.

6.4 Basement Excavations and Stability

Shallow excavations in the Made Ground and the London Clay Formation are likely to be marginally stable at best. Long, deep excavations, through these strata are likely to become unstable.

The excavation of the basement must not affect the integrity of the adjacent structures beyond the boundaries. The excavation must be supported by suitably designed retaining walls. It is considered unlikely that battering the sides of the excavation, casting the retaining walls and then backfilling to the rear of the walls would be suitable given the close proximity of the party walls.

The retaining walls for the basement will need to be constructed based on cohesive soils with an appropriate angle of shear resistance (Φ') for the ground conditions encountered.

Based on the ground conditions encountered within BH1 the following parameters could be used in the design of retaining walls. These have been designed based the results of geotechnical classification tests and reference to literature.

Retaining Wall/Basement Design Parameters					
Strata	**Unit Volume Weight (kN/m³)**	**Cohesion Intercept (c') (kPa)**	**Angle of Shearing Resistance (Ø)**	**Ka**	**Kp**
Made Ground	~15	0	12	0.66	1.52
London Clay Formation	~20 - 22	0	24	0.42	2.37

Unsupported earth faces formed during excavation may be liable to collapse without warning and suitable safety precautions should therefore be taken to ensure that such earth faces are adequately supported before excavations are entered by personnel.

6.5 Hydrogeological Effects

A study of the aquifer maps on the Environment Agency website, and Figure 8 of the Camden Geological, Hydrogeological and Hydrological Study, revealed the site to be located on **Unproductive Strata** relating to the bedrock deposits of the London Clay Formation. No designation was given for any superficial deposits due to their likely absence.

The ground conditions encountered within the trial holes constructed on the site generally conformed to that anticipated from examination of the geology map. Made Ground was noted to overlie the London Clay Formation.

Based on a visual appraisal of the soils encountered, the permeability of the London Clay Formation was considered to be low to very low.

Groundwater was not encountered during the construction of the trial hole. The standing groundwater levels were recorded as 0.83m and 0.94m bgl during two return visits to the site. These water levels are likely to represent migrating perched groundwater or surface water which has accumulated within the installed standpipe.

Based on the above it is considered unlikely that groundwater will be encountered during basement construction. However, perched water could accumulate during basement construction, especially after a period of prolongued rainfall.

Higher groundwater levels during winter months or during inclement weather may affect basement construction.

Once constructed, the Made Ground and the London Clay Formation are unlikely to act as a porous medium for water to migrate through; therefore, additional drainage around the basement should be considered.

6.6 Sub-Surface Concrete

Sulphate concentrations were measured in 2:1 water/soil extracts taken from the London Clay Formation fell into class DS-2 to DS–4 of the BRE Special Digest 1, 2005, *'Concrete in Aggressive Ground'.*

Table C1 of the Digest indicated an ACEC (Aggressive Chemical Environment for Concrete) classification of AC-1d to AC-2. For the classification given, the "mobile" and "natural" case was adopted given the presence of silt lenses and the residential use of the site. The sulphate concentration in the samples ranged from ~350 - 5800mg/l with a pH range of 7.90 – 8.16. The total potential sulphate concentrations ranged from 0.07 – 0.58%.

Concrete to be placed in contact with soil or groundwater must be designed in accordance with the recommendations of Building Research Establishment Special Digest 1, 2005, *'Concrete in Aggressive Ground'* taking into account the pH of the soils.

It is prudent to note that pyrite nodules may be present within the London Clay Formation. Pyrite can oxidise to gypsum and this normally only occurs in the upper weathered layer, but excavation allows faster oxidation and water soluble sulphate values can rapidly increase during construction. Therefore rising sulphate values should be taken into account should ferruginous staining/pyrite nodules be encountered within the London Clay Formation.

6.7 Surface Water Disposal

Infiltration tests were beyond the scope of the investigation.

Soakaways constructed within the cohesive soils of the London Clay Formation are unlikely to prove satisfactory due to low anticipated infiltration rates. Therefore an alternative method of surface water disposal is required.

Consultation with the Environment Agency must be sought regarding any use that may have an impact on groundwater resources.

The principles of sustainable urban drainage system (SUDS) should be applied to reduce the risk of flooding from surface water ponding and collection associated with the construction of the

basement.

6.8 Discovery Strategy

There may be areas of contamination that have not been identified during the course of the intrusive investigation. For example, there may have been underground storage tanks (UST's) not identified during the Ground Investigation for which there is no historical or contemporary evidence.

Such occurrences may be discovered during the demolition and construction phases for the redevelopment of the site.

Groundworkers should be instructed to report to the Site Manager any evidence for such contamination; this may comprise visual indicators, such as fibrous materials within the soil, discolouration, or odours and emission. Upon discovery advice must be taken from a suitably qualified person before proceeding, such that appropriate remedial measures and health and safety protection may be applied.

Should a new source of contamination be suspected or identified then the Local Authority will need to be informed.

6.9 Waste Disposal

Foundation excavations on-site are likely to produce waste which will require classification and then recycling or removal from site.

Under the Landfill (England and Wales) Regulations 2002 (as amended), prior to disposal all waste must be classified as;

- Inert;
- Non-hazardous, or;
- Hazardous.

The Environment Agency's Hazardous Waste Technical Guidance (WM2) document outlines the methodology for classifying wastes.

Once classified the waste can be removed to the appropriately licensed facilities, with some waste requiring pre-treatments prior to disposal.

INERT waste classification should be undertaken to determine if the proposed waste confirms to INERT or NON-HAZARDOUS Waste Acceptable Criteria (WAC).

6.10 Imported Material

Any soil which is to be imported onto the site must undergo chemical analysis to prove that it is suitable for the purpose for which it is intended.

The Topsoil must be fit for purpose and must either be supplied with traceable chemical laboratory test certificates or be tested, either prior to placing (ideally) or after placing, to ensure that the human receptor cannot come into contact with compounds that could be detrimental to human health.

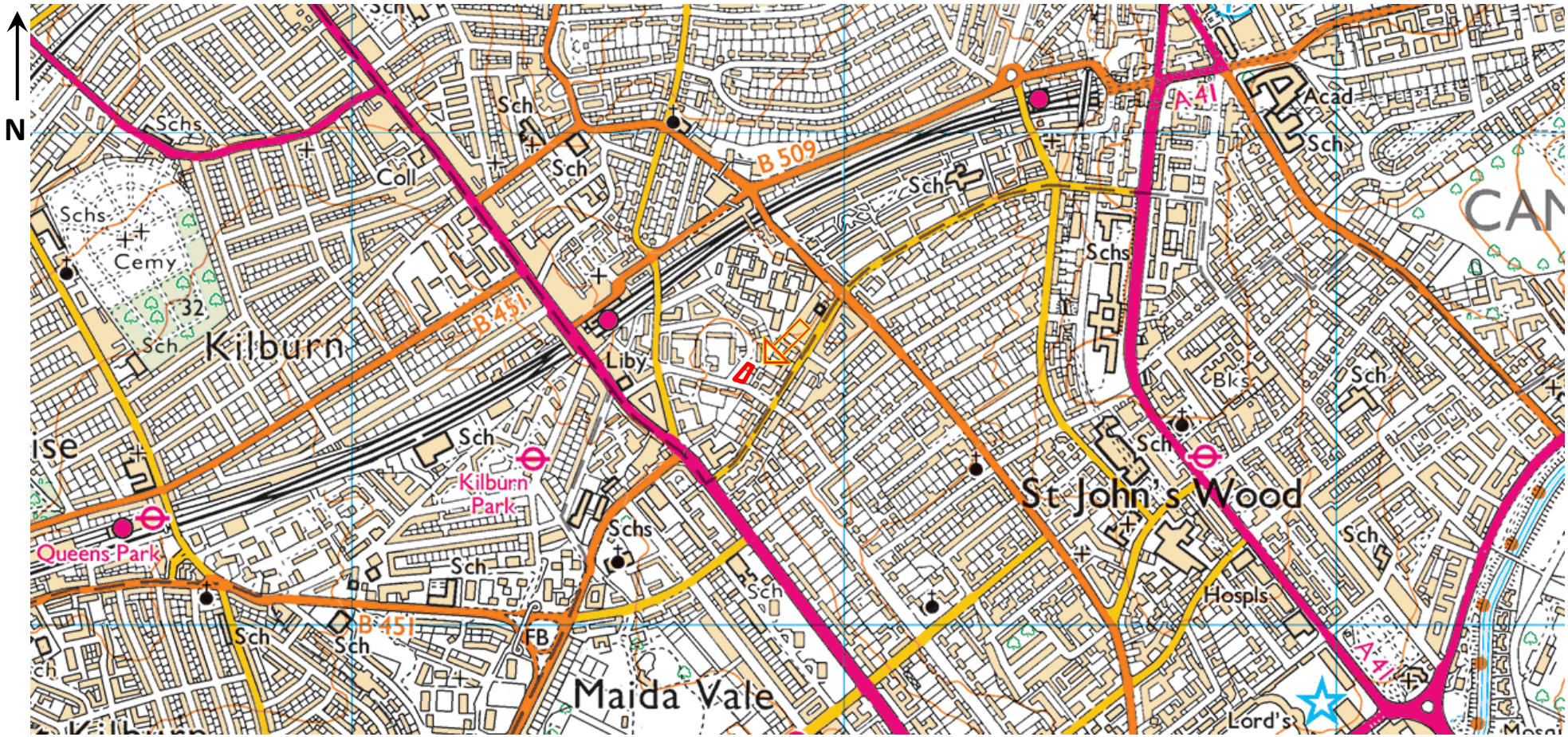
6.11 Duty of Care

Groundworkers must maintain a good standard of personal hygiene including the wearing of

overalls, boots, gloves and eye protectors and the use of dust masks during periods of dry weather.

To prevent exposure to airborne dust by both the general public and construction personnel the site should be kept damp during dry weather and at other times when dust were generated as a result of construction activities.

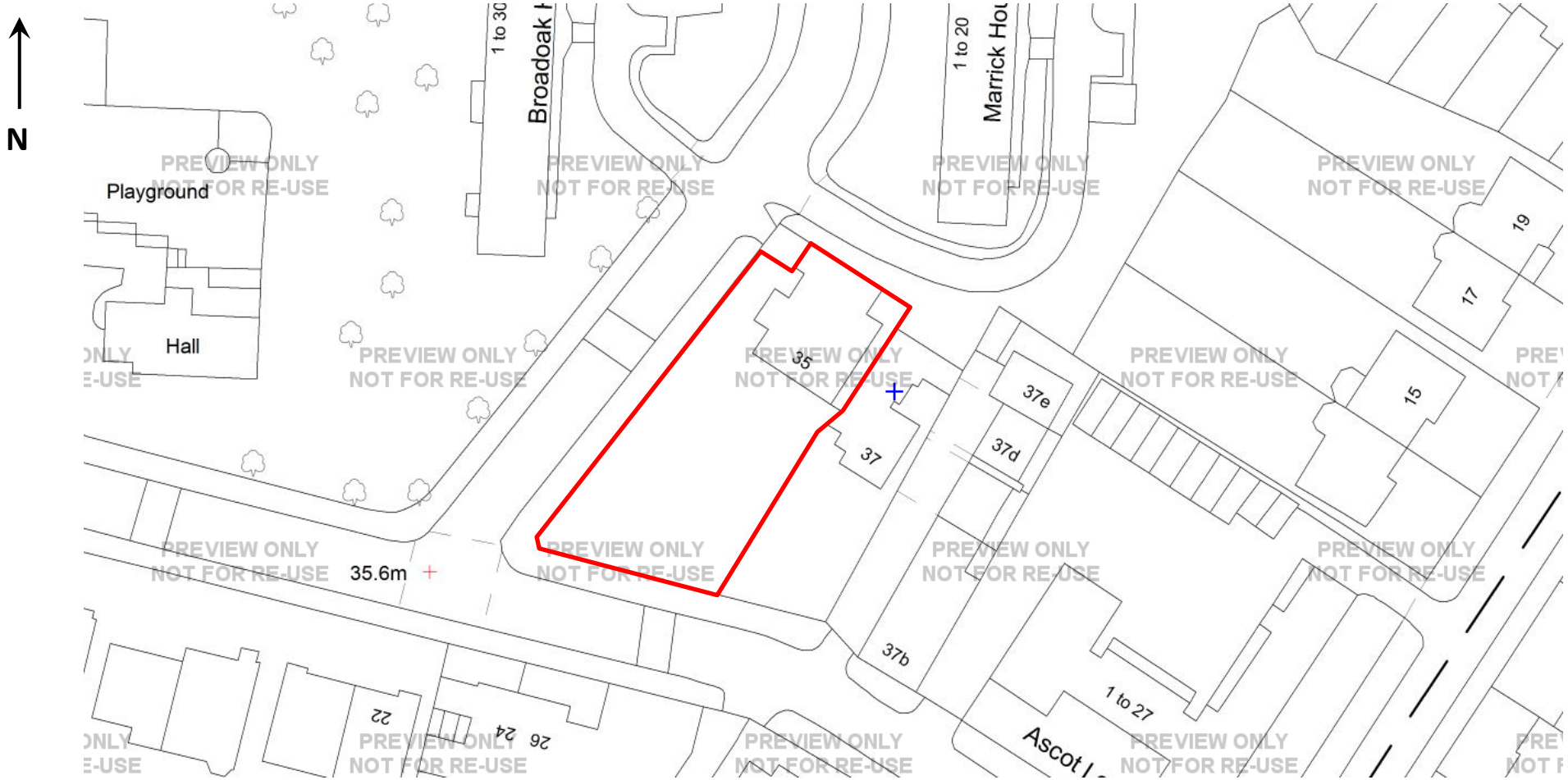
The site should be securely fenced at all times to prevent unauthorised access. Washing facilities should be provided and eating restricted to mess huts.



APPROXIMATE SITE BOUNDARY

NOTE: NOT TO SCALE

Project: 35 Greville Road, Camden, London NW6 5JB		**Figure 1**
Client: Unagi Management Limited	**Date:** August 2015	ground&water
Site Location Plan	**Ref:** GWPR1303	



APPROXIMATE SITE BOUNDARY

NOTE: NOT TO SCALE

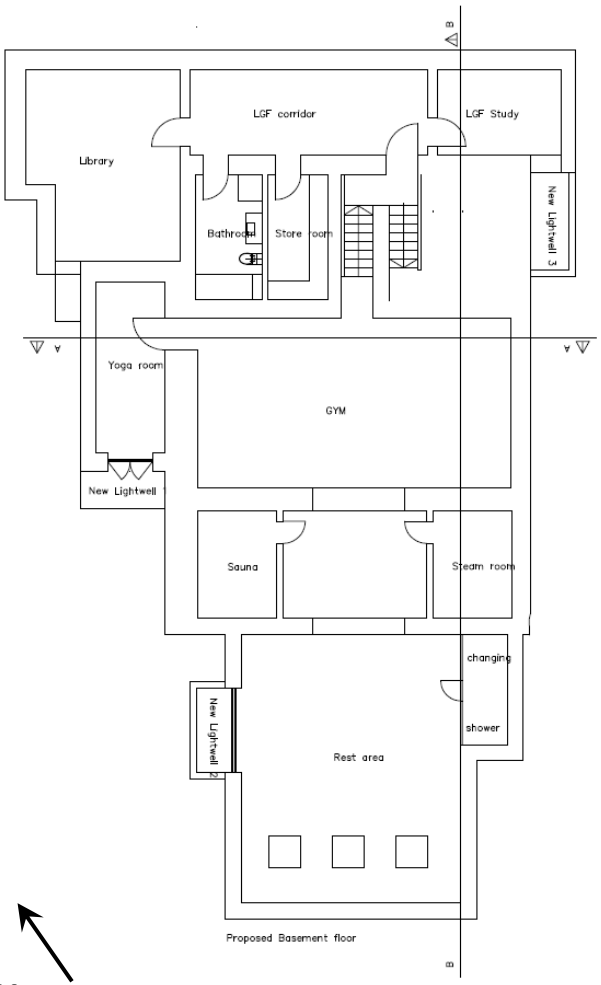
Project: 35 Greville Road, Camden, London NW6 5JB		**Figure 2**
Client: Unagi Management Limited	**Date:** August 2015	ground&water
Site Development Area	**Ref:** GWPR1303	

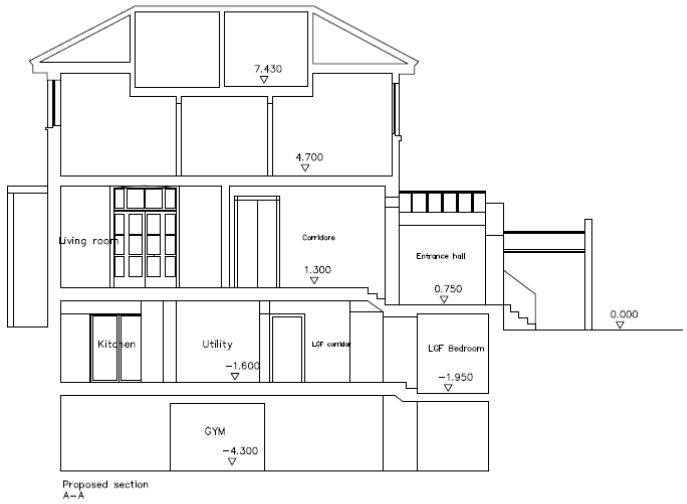
APPROXIMATE SITE BOUNDARY

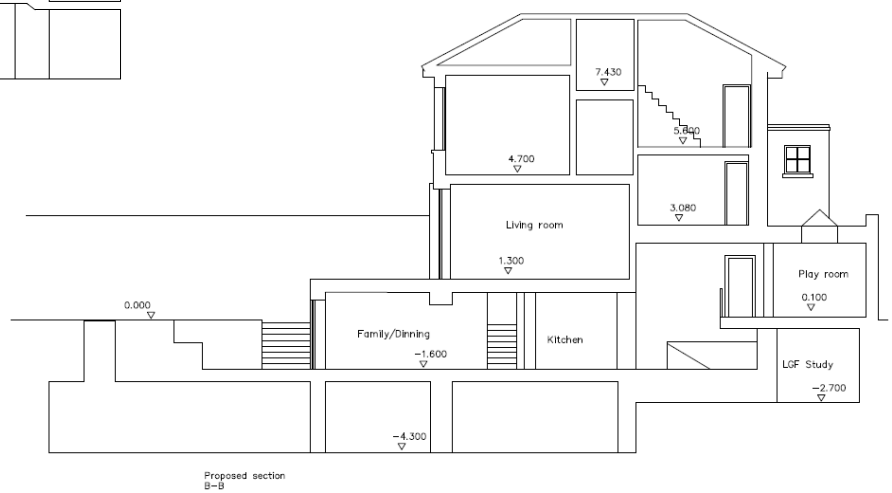
NOTE: NOT TO SCALE

Project: 35 Greville Road, Camden, London NW6 5JB	**Figure 3**
Client: Unagi Management Limited	**Date:** August 2015
Aerial View of the Site	**Ref:** GWPR1303

ground&water



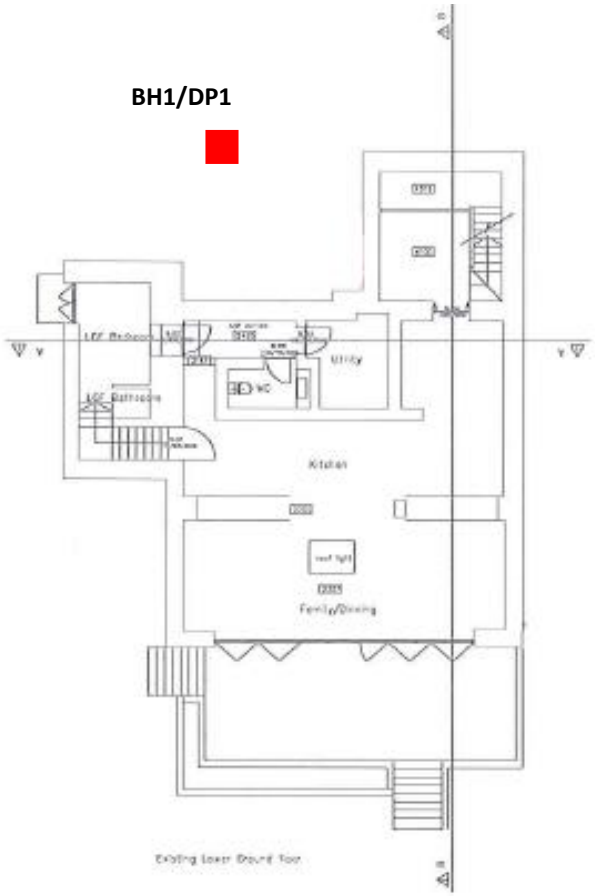




Project: **35 Greville Road, Camden, London NW6 5JB**		**Figure 4**
Client: **Unagi Management Limited**	**Date:** **August 2015**	ground&water
Proposed Development	**Ref:** **GWPR1303**	

N

BH1/DP1



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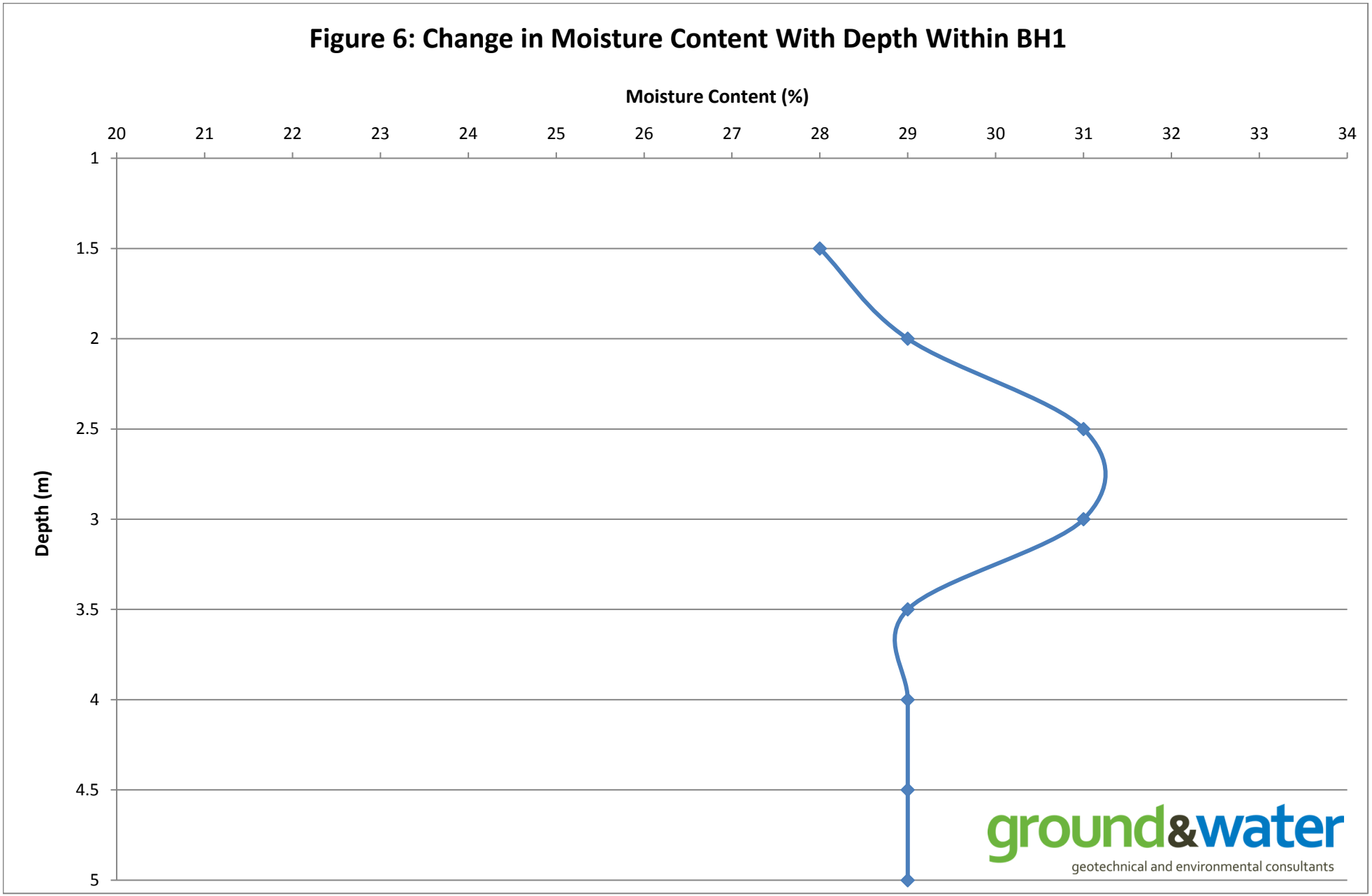
NOTES:

THIS DRAWING IS NOT TO BE SCALED.
ALL DIMENSIONS ARE TO BE CHECKED ON SIT
AND ANY DISCREPANCIES OR ERRORS ARE T
REPORTED TO THE ARCHITECT PRIOR TO
COMMENCEMENT OF WORKS

NOTE: NOT TO SCALE

Project: 35 Greville Road, Camden, London NW6 5JB		**Figure 5**
Client: Unagi Management Limited	**Date:** August 2015	ground&water
Trial Hole Location Plan	**Ref:** GWPR1303	

Figure 6: Change in Moisture Content With Depth Within BH1



APPENDIX A
Conditions and Limitations

The ground is a product of continuing natural and artificial processes. As a result, the ground will exhibit a variety of characteristics that vary from place to place across a site, and also with time. Whilst a ground investigation will mitigate to a greater or lesser degree against the resulting risk from variation, the risks cannot be eliminated.

The investigation, interpretations, and recommendations given in this report were prepared for the sole benefit of the client in accordance with their brief; as such these do not necessarily address all aspects of ground behaviour at the site. No liability is accepted for any reliance placed on it by others unless specifically agreed in writing.

Current regulations and good practice were used in the preparation of this report. An appropriately qualified person must review the recommendations given in this report at the time of preparation of the scheme design to ensure that any recommendations given remain valid in light of changes in regulation and practice, or additional information obtained regarding the site.

This report is based on readily available geological records, the recorded physical investigation, the strata observed in the works, together with the results of completed site and laboratory tests. Whilst skill and care has been taken to interpret these conditions likely between or below investigation points, the possibility of other characteristics not revealed cannot be discounted, for which no liability can be accepted. The impact of our assessment on other aspects of the development required evaluation by other involved parties.

The opinions expressed cannot be absolute due to the limitations of time and resources within the context of the agreed brief and the possibility of unrecorded previous in ground activities. The ground conditions have been samples or monitored in recorded locations and tests for some of the more common chemicals generally expected. Other concentrations of types of chemicals may exist. It was not part of the scope of this report to comment on environment/contaminated land considerations.

The conclusions and recommendations relate to 35 Greville Road, London NW6 5JB.

Trial hole is a generic term used to describe a method of direct investigation. The term trial pit, borehole or window sampler borehole implies the specific technique used to produce a trial hole.

The depth to roots and/or of desiccation may vary from that found during the investigation. The client is responsible for establishing the depth to roots and/or of desiccation on a plot-by-plot basis prior to the construction of foundations. Where trees are mentioned in the text this means existing trees, recently removed trees (approximately 15 years to full recovery on cohesive soils) and those planned as part of the site landscaping.

APPENDIX B
Fieldwork Logs

ground &water
geotechnical and environmental consultants

Ground and Water Ltd
Tel: 0333 600 1221
email: enquiries@groundandwater.co.uk
www.groundandwater.co.uk

Borehole No
BH1
Sheet 1 of 1

Project Name	Project No.	Co-ords: -	Hole Type
35 Greville Road	GWPR1303		WS
Location: London, NW6 5JB		Level: -	Scale 1:50
Client: Unagi Management Limited		Dates: 03/07/2015	Logged By PA

Well	Water Strikes	Samples & In Situ Testing Depth (m)	Type	Results	Depth (m)	Level (m AOD)	Legend	Stratum Description
		0.25	D					MADE GROUND: Brown/dark brown gravelly very sandy to sandy clay. Sand is fine to medium grained. Gravel is occasional to abundant, fine to medium, sub-angular to sub-rounded flint, brick and tile fragments.
		0.50	D		0.60			
		0.80	D					LONDON CLAY FORMATION: Mid brown and grey mottled silty CLAY. Selenite crystals noted. Silt lenses noted throughout.
		1.00	D					
		1.50	D					
		2.00	D					
		2.50	D					
		3.00	D					
		3.50	D					
		4.00	D					
		4.50	D					
		5.00	D					
		5.50	D					
		6.00	D		6.00			End of Borehole at 6.00 m
			Type	Results				

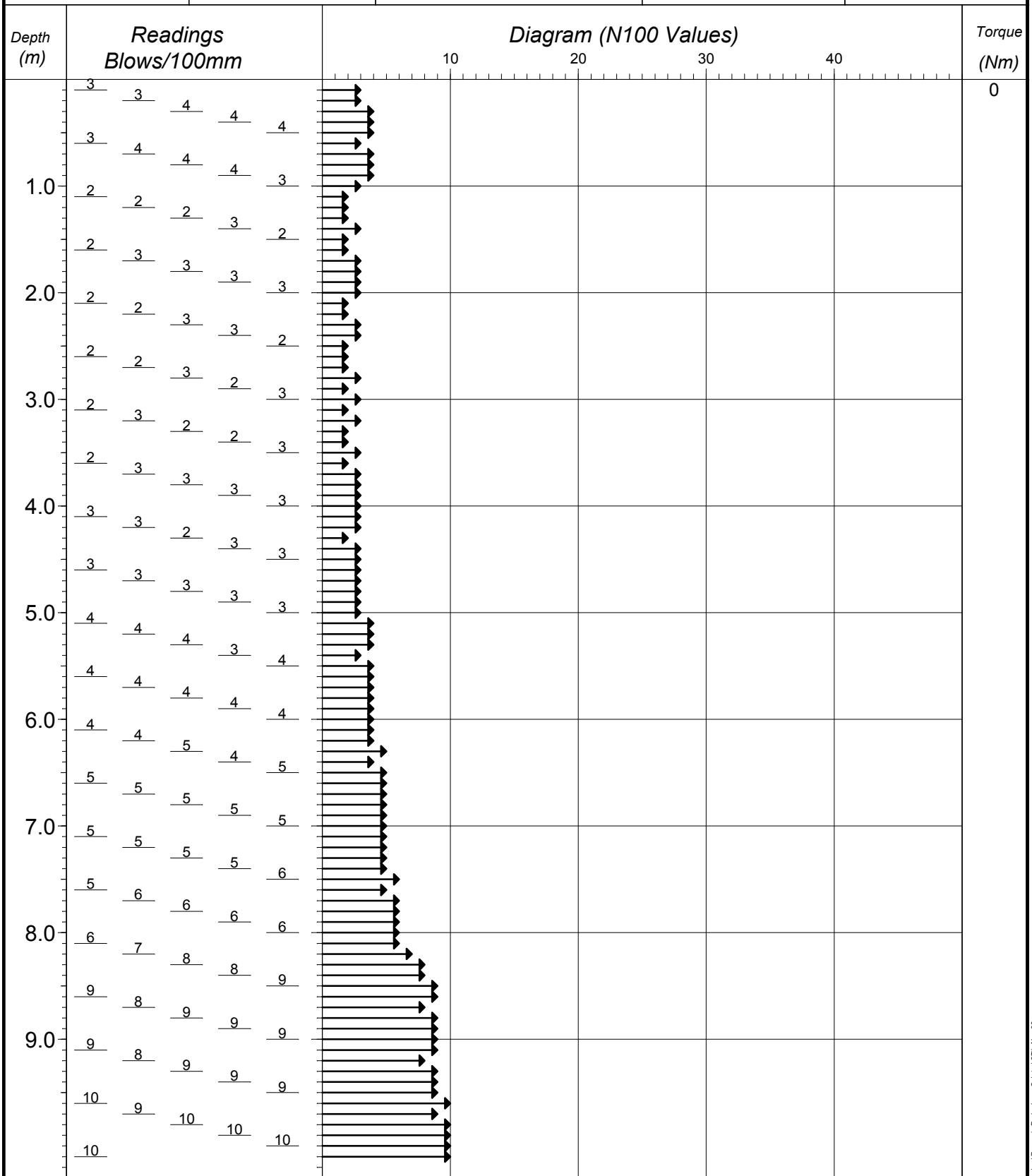
1 2 3 4 5 6 7 8 9

Remarks: Fine roots noted to 3.00m bgl.
No groundwater encountered.

AGS

DYNAMIC PROBING

Probe No **DP1**			
Client **Unagi Management Limited**			Sheet 1 of 1
Site **35 Greville Road**			Project No **GWPR1303**
E -	N -	Level -	Date **03/07/2015**	Logged by **SJM**



Fall Height **500**		Cone Base Diameter **35**	
Hammer Wt **50.00**		Final Depth **10.00**	
Probe Type **DPH**		Log Scale 1:50	

ground &water
geotechnical and environmental consultants

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www.groundandwater.co.uk

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APPENDIX C
Geotechnical Laboratory Test Results

Summary of Classification Test Results

Job No.	Project Name	Programme	
19218	35 Greveille Road, London NW6 5JB	Samples received	17/07/2015
		Schedule received	16/07/2015
Project No.	Client	Project started	17/07/2015
1303	Ground and Water Ltd	Testing Started	27/07/2015

Hole No.	Sample				Soil Description	NMC	Passing 425µm	LL	PL	PI	Remarks
	Ref	Top	Base	Type		%	%	%	%	%	
BH1		1.50		D	Dark orange brown and occasional blue grey silty CLAY with traces of fine footlets and selenite crystals	28	100	75	26	49	
BH1		2.00		D	Brown and grey silty CLAY	29					
BH1		2.50		D	Brown and occasional blue grey silty CLAY with traces of fine rootlets	31	100	80	27	53	
BH1		3.00		D	Brown and occasional blue grey silty CLAY with traces of fine rootlets	31	100	79	28	51	
BH1		3.50		D	Brown silty CLAY with traces of selenite crystals	29					
BH1		4.00		D	Brown silty CLAY with traces of selenite crystals	29	100	75	25	50	
BH1		4.50		D	Brown silty CLAY	29					
BH1		5.00		D	Brown and grey silty CLAY	29					

Test Methods: BS1377: Part 2: 1990:
Natural Moisture Content : clause 3.2
Atterberg Limits: clause 4.3 and 5.0

Test Report by K4 SOILS LABORATORY
Unit 8 Olds Close Olds Approach
Watford Herts WD18 9RU

Tel: 01923 711 288
Email: James@k4soils.com

Checked and Approved

Initials **J.P**

Date: 28/07/2015

Sulphate Content (Gravimetric Method) for 2:1 Soil: Water Extract and pH Value - Summary of Results

Tested in accordance with BS1377 : Part 3 : 1990, clause 5.3 and clause 9

Job No.	Project Name	Programme	
19218	35 Greveille Road, London NW6 5JB	Samples received	17/07/2015
		Schedule received	16/07/2015
Project No.	Client	Project started	17/07/2015
1303	Ground and Water Ltd	Testing Started	28/07/2015

Hole No.	Sample				Soil description	Dry Mass passing 2mm	SO3 Content	SO4 Content	pH	Remarks
	Ref	Top	Base	Type		%	g/l	g/l		
BH1		3.50		D	Brown silty CLAY with traces of selenite crystals	100	0.61	0.74	8.16	

Test Report by K4 SOILS LABORATORY
Unit 8 Olds Close Olds Approach
Watford Herts WD18 9RU
Tel: 01923 711 288
Email: James@k4soils.com

Checked and Approved

Initials J.P

Date: 28/07/2015

Phil Allvey
Ground & Water Ltd
2 The Long Barn
Norton Farm
Selborne Road
Alton
Hampshire
GU34 3NB

QTS Environmental Ltd
Unit 1
Rose Lane Industrial Estate
Rose Lane
Lenham Heath
Kent
ME17 2JN
t: 01622 850410
russell.jarvis@qtsenvironmental.com

QTS Environmental Report No: 15-33572

Site Reference:	35 Greville Road, London NW6 JB
Project / Job Ref:	GWPR1303
Order No:	None Supplied
Sample Receipt Date:	17/07/2015
Sample Scheduled Date:	17/07/2015
Report Issue Number:	1
Reporting Date:	23/07/2015

Authorised by:

Russell Jarvis
Director
On behalf of QTS Environmental Ltd

Authorised by:

Kevin Old
Director
On behalf of QTS Environmental Ltd

QTS Environmental Ltd
Unit 1, Rose Lane Industrial Estate
Rose Lane
Lenham Heath
Maidstone
Kent ME17 2JN
Tel : 01622 850410

4480

Soil Analysis Certificate							
QTS Environmental Report No: 15-33572	**Date Sampled**	03/06/15	03/06/15				
Ground & Water Ltd	**Time Sampled**	None Supplied	None Supplied				
Site Reference: 35 Greville Road, London NW6 JB	**TP / BH No**	BH1	BH1				
Project / Job Ref: GWPR1303	**Additional Refs**	None Supplied	None Supplied				
Order No: None Supplied	**Depth (m)**	1.00	5.50				
Reporting Date: 23/07/2015	**QTSE Sample No**	157990	157991				

Determinand	**Unit**	**RL**	**Accreditation**					
pH	pH Units	N/a	**MCERTS**	8.0	7.9			
Total Sulphate as SO_4	%	< 0.02	NONE	0.09	0.58			
W/S Sulphate as SO_4 (2:1)	mg/l	< 10	**MCERTS**	348	2890			
Total Sulphur	%	< 0.02	NONE	0.03	0.20			
Ammonium as NH_4	mg/kg	< 0.5	NONE	5.7	7.4			
W/S Chloride (2:1)	mg/kg	< 1	**MCERTS**	117	40			
Water Soluble Nitrate (2:1) as NO_3	mg/kg	< 3	**MCERTS**	5	< 3			
W/S Magnesium	mg/l	< 0.1	NONE	16	150			

Analytical results are expressed on a dry weight basis where samples are dried at less than $30^{O}C$
Analysis carried out on the dried sample is corrected for the stone content
Subcontracted analysis $^{(S)}$

QTS Environmental Ltd
Unit 1, Rose Lane Industrial Estate
Rose Lane
Lenham Heath
Maidstone
Kent ME17 2JN
Tel : 01622 850410

4480

Soil Analysis Certificate - Sample Descriptions	
QTS Environmental Report No: 15-33572	
Ground & Water Ltd	
Site Reference: 35 Greville Road, London NW6 JB	
Project / Job Ref: GWPR1303	
Order No: None Supplied	
Reporting Date: 23/07/2015	

QTSE Sample No	TP / BH No	Additional Refs	Depth (m)	Moisture Content (%)	Sample Matrix Description
$ 157990	BH1	None Supplied	1.00	19.6	Light brown clay
$ 157991	BH1	None Supplied	5.50	20.6	Light brown clay

Moisture content is part of procedure E003 & is not an accredited test

Insufficient Sample I/S

Unsuitable Sample U/S

$ samples exceeded recommended holding times

QTS Environmental Ltd
Unit 1, Rose Lane Industrial Estate
Rose Lane
Lenham Heath
Maidstone
Kent ME17 2JN
Tel : 01622 850410

4480

Soil Analysis Certificate - Methodology & Miscellaneous Information
QTS Environmental Report No: 15-33572
Ground & Water Ltd
Site Reference: 35 Greville Road, London NW6 JB
Project / Job Ref: GWPR1303
Order No: None Supplied
Reporting Date: 23/07/2015

Matrix	Analysed On	Determinand	Brief Method Description	Method No
Soil	D	Boron - Water Soluble	Determination of water soluble boron in soil by 2:1 hot water extract followed by ICP-OES	E012
Soil	AR	BTEX	Determination of BTEX by headspace GC-MS	E001
Soil	D	Cations	Determination of cations in soil by aqua-regia digestion followed by ICP-OES	E002
Soil	D	Chloride - Water Soluble (2:1)	Determination of chloride by extraction with water & analysed by ion chromatography	E009
Soil	AR	Chromium - Hexavalent	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry	E016
Soil	AR	Cyanide - Complex	Determination of complex cyanide by distillation followed by colorimetry	E015
Soil	AR	Cyanide - Free	Determination of free cyanide by distillation followed by colorimetry	E015
Soil	AR	Cyanide - Total	Determination of total cyanide by distillation followed by colorimetry	E015
Soil	D	Cyclohexane Extractable Matter (CEM)	Gravimetrically determined through extraction with cyclohexane	E011
Soil	AR	Diesel Range Organics (C10 - C24)	Determination of hexane/acetone extractable hydrocarbons by GC-FID	E004
Soil	AR	Electrical Conductivity	Determination of electrical conductivity by addition of saturated calcium sulphate followed by electrometric measurement	E022
Soil	AR	Electrical Conductivity	Determination of electrical conductivity by addition of water followed by electrometric measurement	E023
Soil	D	Elemental Sulphur	Determination of elemental sulphur by solvent extraction followed by GC-MS	E020
Soil	AR	EPH (C10 – C40)	Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soil	AR	EPH Product ID	Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soil	AR	EPH TEXAS (C6-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C40)	Determination of acetone/hexane extractable hydrocarbons by GC-FID for C8 to C40. C6 to C8 by headspace GC-MS	E004
Soil	D	Fluoride - Water Soluble	Determination of Fluoride by extraction with water & analysed by ion chromatography	E009
Soil	D	FOC (Fraction Organic Carbon)	Determination of fraction of organic carbon by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	D	Loss on Ignition @ 450oC	Determination of loss on ignition in soil by gravimetrically with the sample being ignited in a muffle furnace	E019
Soil	D	Magnesium - Water Soluble	Determination of water soluble magnesium by extraction with water followed by ICP-OES	E025
Soil	D	Metals	Determination of metals by aqua-regia digestion followed by ICP-OES	E002
Soil	AR	Mineral Oil (C10 - C40)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge	E004
Soil	AR	Moisture Content	Moisture content; determined gravimetrically	E003
Soil	D	Nitrate - Water Soluble (2:1)	Determination of nitrate by extraction with water & analysed by ion chromatography	E009
Soil	D	Organic Matter	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	AR	PAH - Speciated (EPA 16)	Determination of PAH compounds by extraction in acetone and hexane followed by GC-MS with the use of surrogate and internal standards	E005
Soil	AR	PCB - 7 Congeners	Determination of PCB by extraction with acetone and hexane followed by GC-MS	E008
Soil	D	Petroleum Ether Extract (PEE)	Gravimetrically determined through extraction with petroleum ether	E011
Soil	AR	pH	Determination of pH by addition of water followed by electrometric measurement	E007
Soil	AR	Phenols - Total (monohydric)	Determination of phenols by distillation followed by colorimetry	E021
Soil	D	Phosphate - Water Soluble (2:1)	Determination of phosphate by extraction with water & analysed by ion chromatography	E009
Soil	D	Sulphate (as SO4) - Total	Determination of total sulphate by extraction with 10% HCl followed by ICP-OES	E013
Soil	D	Sulphate (as SO4) - Water Soluble (2:1)	Determination of sulphate by extraction with water & analysed by ion chromatography	E009
Soil	D	Sulphate (as SO4) - Water Soluble (2:1)	Determination of water soluble sulphate by extraction with water followed by ICP-OES	E014
Soil	AR	Sulphide	Determination of sulphide by distillation followed by colorimetry	E018
Soil	D	Sulphur - Total	Determination of total sulphur by extraction with aqua-regia followed by ICP-OES	E024
Soil	AR	SVOC	Determination of semi-volatile organic compounds by extraction in acetone and hexane followed by GC-MS	E006
Soil	AR	Thiocyanate (as SCN)	Determination of thiocyanate by extraction in caustic soda followed by acidification followed by addition of ferric nitrate followed by colorimetry	E017
Soil	D	Toluene Extractable Matter (TEM)	Gravimetrically determined through extraction with toluene	E011
Soil	D	Total Organic Carbon (TOC)	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	AR	TPH CWG (ali: C5- C6, C6-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C34, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C35)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C35. C5 to C8 by headspace GC-MS	E004
Soil	AR	TPH LQM (ali: C5-C6, C6-C8, C8-C10, C10-C12, C12-C16, C16-C35, C35-C44, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C35, C35-C44)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C44. C5 to C8 by headspace GC-MS	E004
Soil	AR	VOCs	Determination of volatile organic compounds by headspace GC-MS	E001
Soil	AR	VPH (C6-C8 & C8-C10)	Determination of hydrocarbons C6-C8 by headspace GC-MS & C8-C10 by GC-FID	E001

D Dried
AR As Received