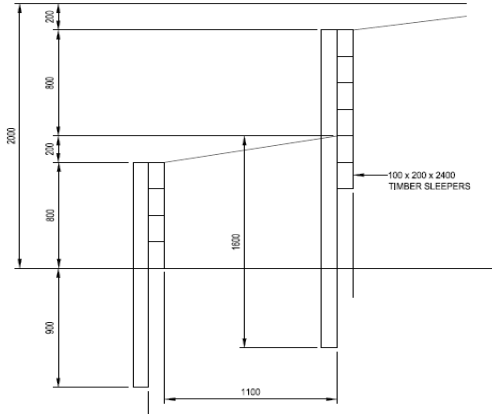


TIMBER RETAINING WALL DESIGN

1) TIMBER JOIST/SLEEPER - (200 WIDE X 100 DEEP) Oak

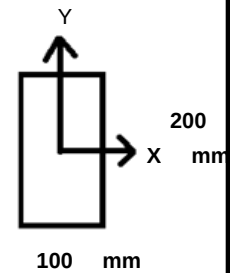
GENERAL DATA

Timber Grade:	TH2
Timber Class:	1
E min (Mpa) :	7000
Member extends:	75mm bearing
Timber Density (kN/Cum):	7.0
Bending Stress (Mpa) :	7.8



MEMBER DATA

Depth 'h' [mm] =	200
Width 'b' [mm] =	100
Bearing [mm] =	75
Total 'l' [mm] =	2400
Ixx [cm4] =	6666.67
Zxx [cm3] =	666.67
Iyy [cm4] =	1666.67
Zyy [cm3] =	333.33
Self weight, kN =	0.34



STRESS MODIFICATION FACTORS

Class factor K2 =	0.80
Load duration factor K3 =	1.25
Bearing stress factor K4 =	1.10
Shear at notched end (top) K5 =	1.00
Shear at notched end (btm) K5 =	1.00
Total depth factor K7 =	1.00
Load share factor K8 =	1.00
Allowable Bending Stress; σ (Mpa) :	7.80
Allowable Shear Stress; σ' (Mpa) :	2.00

LOADING

UDL (Self weight) =	0.14	kN/m
UDL (p-Surcharge) =	2.00	kN/m ²
Angle of Repose =	30°	K= 0.33 (Assumed)
Density of Soil (d) =	16.0	kN/m ³

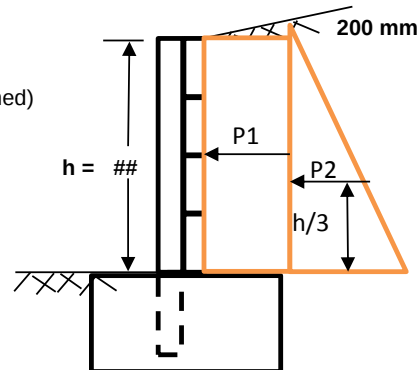
$$P1 = K \times p \times h = 0.528 \text{ kN/m}$$

$$P2 = K \times d \times h \times h / 2 = 2.64 \text{ kN/m}$$

however assume load at bottom 200 deep beam

$$P3 = K \times p \times 0.2 = 0.132 \text{ kN/m}$$

$$P4 = K \times d \times 0.2 = 1.056 \text{ kN/m} \quad w = 1.188 \text{ kN/m}$$



Reaction on support =	1.426	kN
Bending Moment in sleeper =	1.711	kN/m

BENDING STRESS CHECK

Applied Bending Stress =	5.13	N/mm ²
Allowable Design Bending Stress =	7.80	N/mm ²
(K2 x K3 x K7 x K8 x σ)		
Usage factor =	0.66	

SHEAR STRESS CHECK

Applied Shear Stress =	0.07	N/mm ²
Allowable Design Shear Stress =	2.00	N/mm ²
(K2 x K3 x K5 x K8 x σ')		
Usage factor =	0.04	

DEFLECTION CHECK

Actual Deflection = $5 w l^4 / 384 E I_{yy}$	4.40	mm
Allowable deflection; $l / 325$ =	7.38	mm
Usage factor =	0.60	

CLIENT : **Mr & Mrs Smithson, 40 Platt's Lane**
London, NW3 7NT.

SUBJECT: Timber sleeper Design

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2) TIMBER POST - (200 Wide X 150 Deep)

Properties : Self weight, kN = 0.21
 $I_{yy} = 200 \times 150^3 / 12 = 6E+07 \text{ mm}^4$

Design Load on Post = $(P1 + P2) \times l = 3.17 \times 2 = 7.60 \text{ kN (Shear)}$

Design Moment on Post = $P1 \times l \times (h/2+0.1) + P2 \times l \times ((h+0.2)/3+0.1) = 3.38 \text{ kNm}$
(100 mm extra length)

BENDING STRESS CHECK

Applied Bending Stress = 4.51 N/mm²
Allowable Design Bending Stress = 7.80 N/mm²
(K2 x K3 x K7 x K8 x σ)
Usage factor = **0.58**

SHEAR STRESS CHECK

Applied Shear Stress = 0.25 N/mm²
Allowable Design Shear Stress = 2.00 N/mm²
(K2 x K3 x K5 x K8 x σ')
Usage factor = **0.13**

DEFLECTION CHECK

Actual Deflection = $p l^3 / 3 E I_{yy} = 0.57 \text{ mm}$
Allowable deflection; $h / 325 = 2.46 \text{ mm}$
Usage factor = 0.23

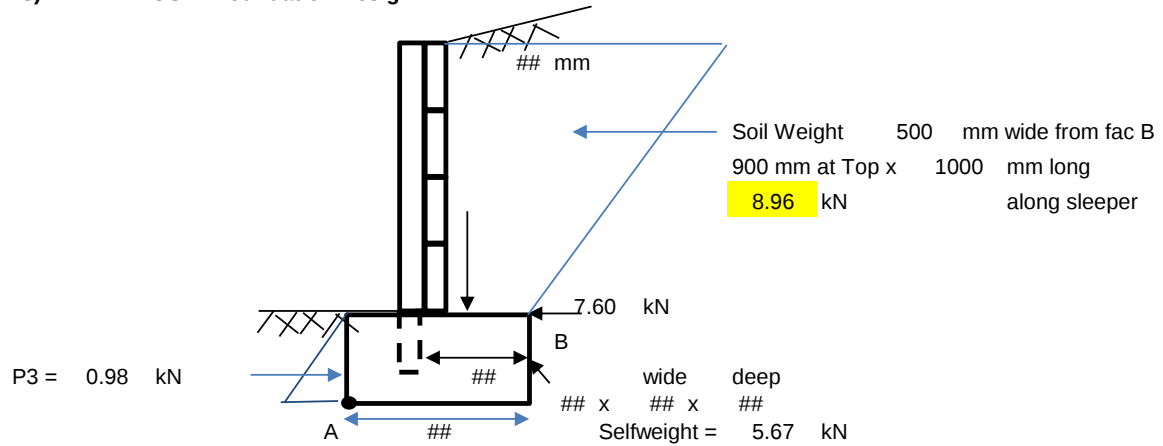
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3) TIMBER POST - Foundation Design



Loading On Foundation top

Lateral Load due to retaining soil = 7.60 kN

Moment above foundation top = 3.38 kNm

Vertical Load on Foundation = 1.55 kN (From Post and Sleeper) @ 225 from point A
5.67 kN Selfweight @ 450 from point A
10.84 kN (Surcharge + Soil) @ 650 from point A

Overturning Moment about point A

$$= 7.60 \times 0.35 + 3.3792 = 6.04 \text{ kNm}$$

Restoring Moment about point A

$$= 1.55 \times 0.225 + 5.67 \times 0.45 + 10.84 \times 0.65 + 0.98 \times 0.117 = 10.06 \text{ kNm}$$

Usage factor = 1.67 With Surcharge

Usage factor = 2.03 Without Surcharge

Summary

Sleeper Size = 200 mm Deep X 100 mm wide x 2.4m Long

Post Size = 200 mm wide X 150 mm deep x 1.0m Long

Foundation Size = 900 mm wide X 750 mm Long x 350 mm Deep

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