

Unit 15, East Hanningfield Industrial Estate
Old Church Road, East Hanningfield, Essex CM3 8AB
Telephone: 01245 400 930 Fax: 01245 400 933
Email: info@siteinvestigations.co.uk Website: www.siteinvestigations.co.uk



# Factual Report

Client: Zurich Insurance C/o

Pyle Consulting

Site: 56 Parkway

London NW17AH

CSI Ref: FACT/4137/Visit 1

Dated: 7<sup>th</sup> January 2014

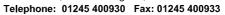


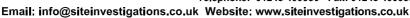
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## **FACTUAL REPORT CONTENT**

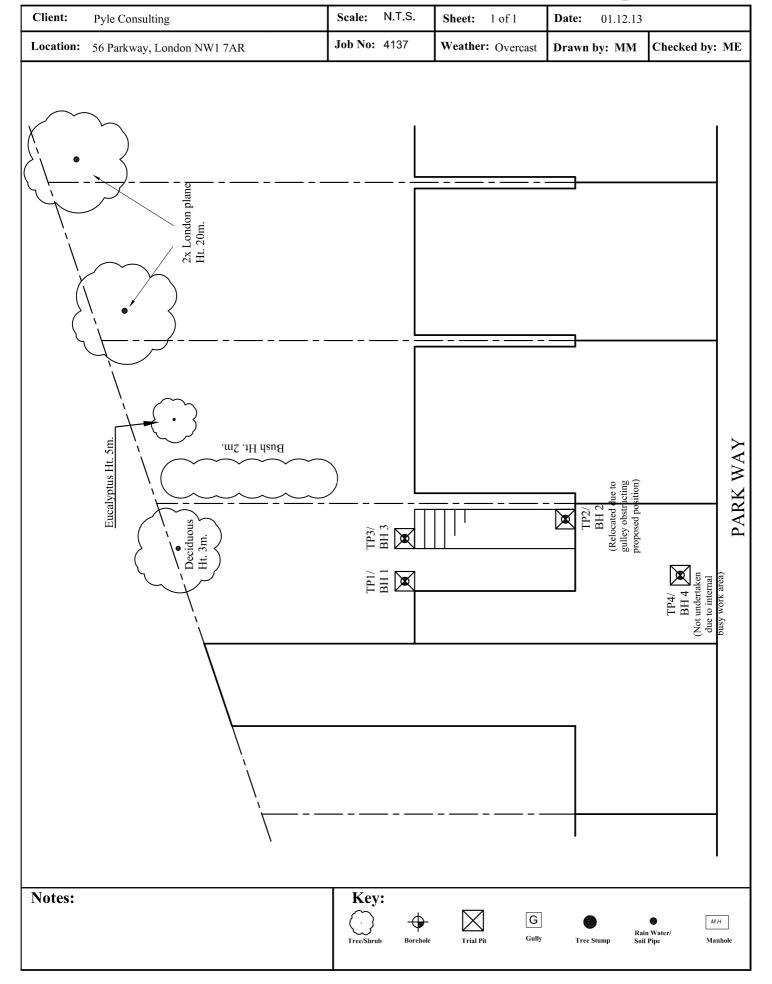
1.0	SITE PLAN
2.0	TRIAL PITS / BOREHOLE LOGS
3.0	ROOT IDENTIFICATION
4.0	GEOTECHNICAL SOIL TESTING
5.0	CCTV DRAINAGE REPORT
6.0	REPORT NOTES

Unit 15 East Hanningfield Industrial Estate Old Church Road, East Hanningfield, Essex CM3 8AB







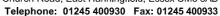


1 of 1

Date:

Unit 15 East Hanningfield Industrial Estate Old Church Road, East Hanningfield, Essex CM3 8AB

Sheet No:





N.T.S.

Scale:

Client:

Remarks:

Pyle Consulting



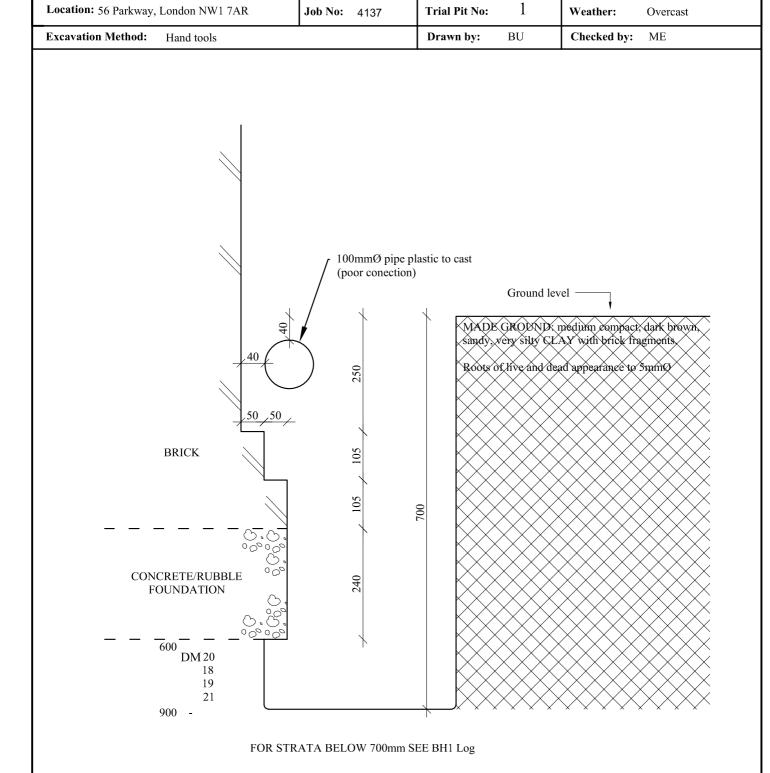
07.12.13

**J** Jar sample

V Pilcon Vane (kPa)

W Water Sample

M Mackintosh Probe



**D** Small disturbed sample

**U** Undisturbed sample (U100)

N Standard Penetration Test Blow Count

**B** Bulk disturbed sample

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Email: info@siteinvestigations.co.uk Website: www.siteinvestigations.co.uk

Client:	Pyle Consulting		N.T.S.	Sheet No			ther: Overcast	<b>Date:</b> 07	1.12.13
Site:	56 Parkway, London NW1 7AR		<b>2:</b> 4137	Borehole			ng method: Hand Auger		
Depth Mtrs.	Description of Strata	Thick- ness		Sample	Test Type F		Root Information	Depth to Water	Depth Mtrs
	AS TP1	0.7					Roots of live appearance to 1mmØ to 2.2m.		
0.7	MADE GROUND: medium compact, dark brown, sandy, very silty clay with brick fragments.	0.2							
0.9			× ×	D	V 74 76				1.0
			- · - ^ · - · - · · · · · · · · · · · ·	D	V 10				1.5
	Stiff, mid brown, silty CLAY with partings of orange and brown silt and fine sand.	2.1		D	V 11 11				2.0
				D	V 11 12				2.5
3.0	Borehole ends at 3.0 m			D	V 13 13				3.0
Drawn Remark	by: BU Approved by: ME  ss: Borehole dry and open on completion.		D Sn B Bu U Un	nall Disturl ılk Disturb disturbed S	Sample (U10	J V 0) M	Jar Sample Pilcon Vane (kPa) Mackintosh Probe enetration Test Blow Count		

Chelmer Site Investigations
Unit 15 East Hanningfield Industrial Estate
Old Church Road, East Hanningfield, Essex CM3 8AB



helmer Lite **x** investigations

Telephone: 01245 400930 Fax: 01245 400933 Email: info@siteinvestigations.co.uk Website: www.siteinvestigations.co.uk

Client: Pyle Consulting	Scale:	N.T.S.	Sheet No:	1 of 1	Date:	07.12.13
Location: 56 Parkway, London NW1 7AR	Job No:	4137	Trial Pit No:	2	Weather:	Overcast
Excavation Method: Hand tools			Drawn by:	BU	Checked by:	ME
CONCRETE	40	290 70 340	009 00	MADE GRO sandy very s Roots of live Firm, brown, and orange si Roots of live	appearance to 3h	partings of brown
FO	R STRATA	A BELOW 80	0mm SEE BH2 Lo	og		
Remarks:			ey: Small disturbed s	amnle	J	Jar sample

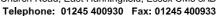
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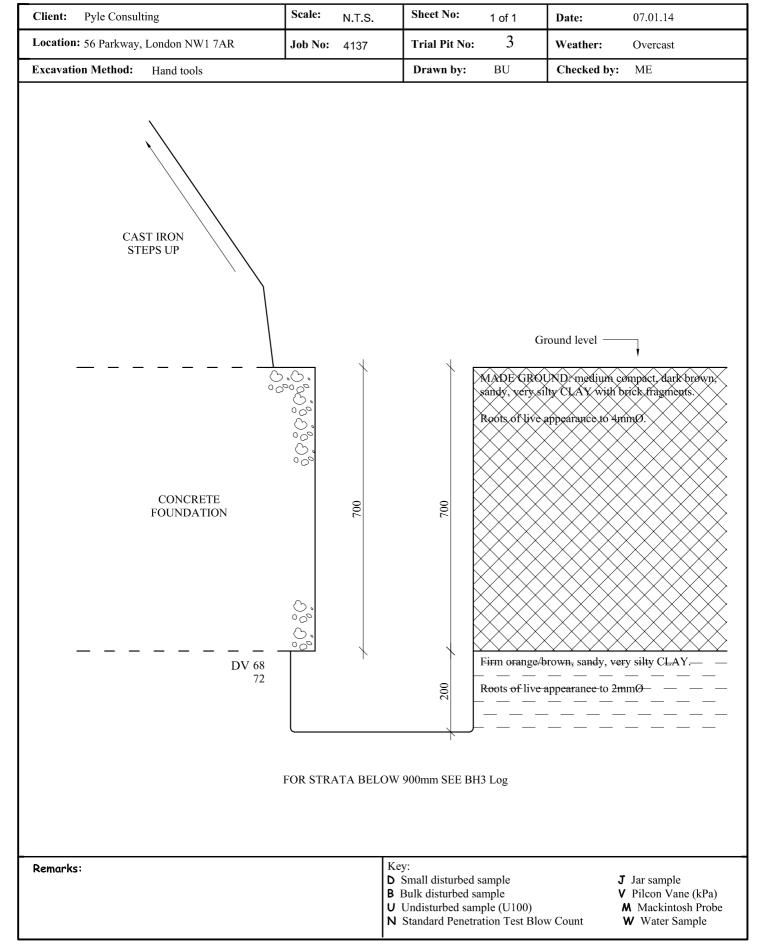
Client:	Pyle Consulting	Scale:	N.T.S.	Sheet No	: 1 of 1	Weat	ther: Overcast	<b>Date:</b> 0.	7.01.14
Site:	56 Parkway, London NW1 7AR	<u> </u>	<b>2:</b> 4137	Borehole			ng method: Hand Auger	24000 01	7.101111
Depth Mtrs.	Description of Strata	Thick- ness		Sample	Test Type F		Root Information	Depth to Water	Depth Mtrs
0.8	AS TP2	0.8					Roots of live appearance to 1mmØ to 1.6m.		
0.8	Firm, brown, silty CLAY with partings of orange and brown silt and fine sand.  Becoming stiff from 1.0m.		×_ ·_ · · · ·	D	V 78 82				1.0
				D	V 11 12				1.5
		2.2	· — — — — — — — — — — — — — — — — — — —	D	V 13 13				2.0
				D	V 12 12				2.5
3.0	Borehole ends at 3.0 m		· . _x _ ·	D	V 11 12				3.0
Drawn Remark	by: BU Approved by: ME  is: Borehole dry and open on completion.		D Sr B Bı U Un	nall Distur ılk Disturb disturbed S	Too Dense to bed Sample ed Sample Sample (U10 e N Stan	J V 0) M	Jar Sample Pilcon Vane (kPa) Mackintosh Probe enetration Test Blow Count	:	

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Email: info@siteinvestigations.co.uk Website: www.siteinvestigations.co.uk

Client:	Pyle Consulting	Scale:	N.T.S.	Sheet No	: 1 of 1	Weat	ther: Overcast	Date: 0.	7.01.14
Site:	56 Parkway, London NW1 7AR	Job No	: 4137	Borehole	No: 3	Borir	ng method: Hand Auger		
Depth Mtrs.	Description of Strata	Thick- ness	Legend	Sample	Test Type F		Root Information	Depth to Water	Depth Mtrs
	AS TP3	0.9					Roots of live appearance to 1mmØ to 1.4m.		
0.9			  ×	D	V 98 10	302			1.0
	Stiff, brown, silty CLAY with partings of brown and orange silt and fine sand.			D	V 11 12				1.5
	Becoming very stiff from 2.0m.	2.1	·	D	V 14	.0+ .0+			2.0
				D	V 14 14	-0+ -0+			2.5
3.0	Borehole ends at 3.0 m		 ×	D	V 14 14	-0+ -0+			3.0
Drawn Remark	by: BU Approved by: ME  is: Borehole dry and open on completion.		D Sr B Bı U Un	nall Disturl ılk Disturb disturbed S	Sample (U10	J V 0) M	Jar Sample Pilcon Vane (kPa) Mackintosh Probe enetration Test Blow Count		



**Chelmer Site Investigations** Unit 15 East Hanningfield Ind. Est. Old Church Rd, E. Hanningfield

22/05/2014

Essex CM3 8AB

Dear Sirs

### 56 Parkway

The samples you sent in relation to the above on 19/05/2014 (received by us on 21/05/2014 (yesterday)) have been examined. The structure was referable as follows:

### TP, u/s, 600mm

1 root: PLATANUS (Plane). This sample was in POOR condition. Dead\*.

10 pieces of BARK only - insufficient material for identification.

1 root: PLATANUS (Plane). A further sample, not examined in detail appeared similar under low magnification. Dead\*.

### TP2, u/s, 600mm

1 root: PLATANUS (Plane). Alive, recently\*.

### BH2, 800-1600mm

1 root: PLATANUS (Plane). A further sample, not examined in detail appeared similar under low magnification. Alive, recently\*.

### TP3, u/s, 700mm

1 root: PLATANUS (Plane). Alive, recently\*.

### BH3, 900-1400mm

1 root: PLATANUS (Plane). Alive, recently\*.

I trust this is of help. Please call us if you have any queries; our Invoice is enclosed.

Yours faithfully

Dr Ian B K Richardson

Based mainly on the Iodine test for starch. Starch is present in some cells of a living woody root, but is more or less rapidly broken down by soil micro-organisms on death of the root, sometimes before decay is evident. This result need not reflect the state of the parent tree.



Dr lan B K Richardson

James Richardson

BSc (Hons. Biology) **Enterprise House** 

Reading RG6 7BB

Your ref:

Our ref:

49-51 Whiteknights Road

Web: www.botanical.net

BSc, PhD, CBiol, MiBiol, MiHort, FLS

Tel: (0118) 986 9552 (Direct line)

E-mail: richardsons@botanical.net

4137

73/4501

Job Number: CGL03797
Client: Pyle Consulting
Client Reference: CSI4137
Site Name: 56 Parkway, London NW1

© ≅helmer ⊠eotechnical ±aboratories

Date Received: 20/01/2014
Date Testing Started: 20/01/2014
Date Testing Completed: 21/01/2014
Laboratory Used: Chelmer Geotechnical, CM3 8AB

Notes :-				TP/BH1	TP/BH1	TP/BH1	TP/BH1	TP/BH1	TP/BH1	вн/тр/ws	Sa
				3.0	2.5	2.0	1.5	1.0	0.7	Depth	Sample Ref
				47454	47453	47452	47451	47450	47449	OID	
				D	D	D	D	D	D	Sample Type	
				31	30	28	34	34	35	Content (%) [1]	NA Signature
				<5	<5	<5	<5	<b>~</b> 5	<5	> 0.425mm > (%) [ 2 ]	7
				79		77	86	81		Liquid Limit (%) [3]	
				21		22	21	23		Plastic Limit (%) [4]	
				58		55	65	58		Plasticity Index (%) [ 5 ]	
				0.17		0.11	0.19	0.20		Liquidity Index (%) [ 5 ]	
				58		55	65	58		Plastic Limit Plasticity Index Liquidity Index Plasticity Index (%) [4] (%) [5] (%) [6]	NA>-460-2
				CΛ		۷	۷	CV		Soil Class [7]	
										Contact Time (h) [8]	7
										Soil Sample Suction (kPa)	
				135	120	120	110	75	70	Vane Strength (kPa) [ 9 ]	5000
										Content (%) [10]	
Key										pH Value [11]	
										SO <sub>3</sub> [12]	Sul
										SO <sub>4</sub> [13]	Sulphate Content
										Class [14]	ent

Juced :- MT Checked By
------------------------

lab-001

[1] BS 1377 : Part 2 : 1990, Test No 3.2

[5] BS 1377 : Part 2 : 1990, Test No 5.4 [6] BRE Digest 240: 1993

[10] BS 1377: Part 3: 1990, Test No 4 [11] BS 1377: Part 2: 1990, Test No 9

[9] Values of shear strength were determined in situ by Chelmer Site Investigations using a Pilcon hand vane or Geonor vane (GV).

Note that if the  $SO_4$  content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class respectively unless water soluble magnesium testing is undertaken to prove otherwise

ENP U/S \$

> Essentially Non-Plastic Water sample

Underside Foundation

[14] BRE Special Digest One (Concrete in Aggressive Ground) 2005

 $[13] SO_4 = 1.2 \times SO_3$ 

[12] BS 1377 : Part 3 : 1990, Test No 5.6

O

Disturbed sample

Bulk sample

 $\subset$ Φ

U100 (undisturbed sample)

[8] In-house method S9a adapted from BRE IP 4/93

[7] BS 5930 : 1981 : Figure 31 - Plasticity Chart for the classification of fine soils

[4] BS 1377 : Part 2 : 1990, Test No 5.3 [3] BS 1377 : Part 2 : 1990, Test No 4.4 [2] Estimated if <5%, otherwise measured

BS 1377:1990

Job Number: CGL03797
Client: Pyle Consulting
Client Reference: CSI4137
Site Name: 56 Parkway, London NW1

Chelmer
Ceotechnical
Laboratories

Date Received: 20/01/2014

Date Testing Started: 20/01/2014

Date Testing Completed: 21/01/2014

Laboratory Used: Chelmer Geotechnical, CM3 8AB

Oito		O I GINW	Olic Hallic . 30 I alkway, Edildoli Hyyri	4										Labola	Labolatory Costa: Criteriner Ceotechnical, Civio Cob		CIGCIIII	Jai, Civio	
San	Sample Ref																Sulp	Sulphate Content	nt
				Moisture	Soil Faction					Modified		Filter Paper		Insitu Shear	Organic				
BH/TP/WS Depth			UID Sample Type	Content (%) [1]	> 0.425mm (%) [ 2 ]	Liquid Limit	Plastic Limit	Plastic Limit Plasticity Index Liquidity Index Plasticity Index	Liquidity Index	Plasticity Index	Soil Class	Contact Time	Contact Time Soil Sample Vane Strength  (h) [ 8 ] Suction (kPa) (kPa) [ 9 ]	Vane Strength	Content (%) [ 10 ]	pH Value	SO <sub>3</sub> SO <sub>4</sub>		Class [141
TP/BH2 0.7 47455	0.7	47455	D	35	Çı	89	22	67	0.19	67	C۷			70					
TP/BH2 1.0 47456	1.0	47456	D	34	<5	84	21	63	0.21	63	СУ			80					

Т														
[5] BS 1377 : Part 2 : 1990 [6] BRE Digest 240 : 1993	[3] BS 137 [4] BS 137	[2] Estimate	Notes :- [1] BS 137				TP/BH2	TP/BH2	TP/BH2	TP/BH2	TP/BH2	TP/BH2	вн/тр/ws	
7: Part 2: jest 240: 1	7: Part 2: 7: Part 2:	ed if <5%, o	7 : Part 2 :				3.0	2.5	2.0	1.5	1.0	0.7	Depth	Sample Ref
[5] BS 1377 : Part 2 : 1990, Test No 5.4 [6] BRE Digest 240 : 1993	[3] BS 1377 : Part 2 : 1990, Test No 4.4 [4] BS 1377 : Part 2 : 1990, Test No 5.3	[2] Estimated if <5%, otherwise measured	Notes :- [1] BS 1377 : Part 2 : 1990, Test No 3.2				47460	47459	47458	47457	47456	47455	UID	of .
Vo 5.4	Vo 4.4 Vo 5.3	easured	lo 3.2				D	D	D	D	D	D	Sample Type	
[10] BS 1377 : F [11] BS 1377 : F	[9] Values of sh Pilcon hand var	[8] In-house me	[7] BS 5930 : 19				31	32	36	31	34	35	Moisture Content (%) [1]	
[10] BS 1377 : Part 3 : 1990, Test No 4 [11] BS 1377 : Part 2 : 1990, Test No 9	<ul><li>(9) Values of shear strength were determined in situ by Chelmer Site Investigations using a Plicon hand vane or Geonor vane (GV).</li></ul>	[8] In-house method S9a adapted from BRE IP 4/93	[7] BS 5930 : 1981 : Figure 31 - Plasticity Chart for the classification of fine soils				<5	<5	<5	<5	<5	<5	Soil Faction > 0.425mm (%) [ 2 ]	
st No 4 st No 9	e determined in : ne (GV).	d from BRE IP 4	Plasticity Chart f						73	75	84	89	Liquid Limit (%) [3]	
	situ by Chelmer	/93	for the classifica						20	21	21	22	Plastic Limit (%) [4]	
	Site Investigation		tion of fine soils						53	54	63	67	Modified Plasticity Index Liquidity Index Plasticity Index (%) [5] (%) [6]	
	ns using a								0.30	0.17	0.21	0.19	Liquidity Index (%) [5]	
									53	54	63	67	Modified Plasticity Index (%) [6]	
respectively unlotherwise	[14] BRE Special Note that if prudent to cons		[12] BS 1377 : F						۷	۸J	۷	CV	Soil Class [7]	
ess water solubl	al Digest One (C f the SO <sub>4</sub> conten ider the sample	(SO <sub>3</sub>	1377 : Part 3 : 1990, Test No 5.6										Filter Paper Contact Time (h) [8]	
e magnesium te	oncrete in Aggre t falls into the DS as falling into the		st No 5.6										Soil Sample Suction (kPa)	
respectively unless water soluble magnesium testing is undertaken to prove otherwise	[14] BRE Special Digest One (Concrete in Aggressive Ground) 2005 Note that if the SO <sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class						120	125	135	120	80	70	Insitu Shear Vane Strength (kPa) [9]	
en to prove	005 s, it would be m class												Organic Content (%) [ 10 ]	
ENP U/S	€ ⊂	B	Kev D										pH Value [11]	
Essentia Undersia	U100 (undistu Water sample	Bulk sample	Disturbe										SO <sub>3</sub>	S
Essentially Non-Plastic Underside Foundation	U100 (undisturbed sample) Water sample	nple	Disturbed sample										SO <sub>4</sub>	Sulphate Content
ion	sample)												Class [14]	ntent

Notes:-			Key	
[1] BS 1377 : Part 2 : 1990, Test No 3.2	[7] BS 5930 : 1981 : Figure 31 - Plasticity Chart for the classification of fine soils	[12] BS 1377 : Part 3 : 1990, Test No 5.6	D	Disturbed sample
[2] Estimated if <5%, otherwise measured	[8] In-house method S9a adapted from BRE IP 4/93	[13] $SO_4 = 1.2 \times SO_3$	œ	Bulk sample
[3] BS 1377 : Part 2 : 1990, Test No 4.4	[9] Values of shear strength were determined in situ by Chelmer Site Investigations using a	[14] BRE Special Digest One (Concrete in Aggressive Ground) 2005	C	U100 (undisturbed sample)
[4] BS 1377 : Part 2 : 1990, Test No 5.3	rlicon nand varie or Geonor varie (GV).	Note that if the SO <sub>4</sub> content falls into the DS-4 or DS-5 class, it would be	\$	Water sample
[5] BS 1377 : Part 2 : 1990, Test No 5.4	[10] BS 1377 : Part 3 : 1990, Test No 4	respectively unless water soluble magnesium testing is undertaken to prove	ENP	Essentially Non-Plastic
[6] BRE Digest 240 : 1993	[11] BS 1377 : Part 2 : 1990, Test No 9	otherwise	N/S	Underside Foundation
Comments :-				
Produced :- MT	Checked By :- AK	D	Date Checked :- 23-Jan-14	;- 23-Jan-14

lab-001 Chelmer Site Investigations 2012

BS 1377 : 1990

Job Number: CGL03797 Client: Pyle Consulting Client Reference: CSI4137 Site Name: 56 Parkway, London NW1

Seotechnical
aboratories

Date Received: 20/01/2014

Date Testing Started: 20/01/2014

Date Testing Completed: 21/01/2014

Laboratory Used: Chelmer Geotechnical, CM3 8AB

			13	З	З	З	З	З		Ś
			3.0	2.5	2.0	1.5	1.0	0.7	'S Depth	Sample Ref
			47466	47465	47464	47463	47462	47461	UID	f
			D	D	D	D	D	D	Sample Type	
			32	32	29	32	35	34	Content (%) [1]	Mointing
			<5	<5	<5	<5	<5	<5	> 0.425mm > (%) [2]	on Eartion
				74	73		88	84	Liquid Limit (%) [3]	
				22	17		21	22	Plastic Limit (%) [4]	
				52	56		67	62	Plasticity Index (%) [5]	
				0.18	0.21		0.20	0.20	Liquidity Index (%) [5]	
				52	56		67	62	Plastic Limit Plasticity Index Liquidity Index Plasticity Index (%) [4] (%) [5] (%) [5] (%) [6]	NAO diffical
				СУ	СУ		СУ	CV	Soil Class [7]	
									Contact Time (h) [8]	Tiltor Donor
									Soil Sample Suction (kPa)	
			120	>140	>140	120	100	70	Vane Strength (kPa) [9]	Issit Oboor
									Organic Content (%) [ 10 ]	Orași
									pH Value [11]	
									SO <sub>3</sub> [ 12]	Sul
									SO <sub>4</sub>	Sulphate Content
									Class [14]	tent

TP/BH3 TP/BH3 TP/BH3 TP/BH3 TP/BH3 TP/BH3 BH/TP/WS

Date Checked :- 23-Jan-14	te Checked	Da	Checked By ;- AK	Produced :- MT
				Comments :-
Underside Foundation	U/S	otherwise	[11] BS 1377 : Part 2 : 1990, Test No 9	[6] BRE Digest 240 : 1993
Essentially Non-Plastic	ENP	respectively unless water soluble magnesium testing is undertaken to prove	[10] BS 1377 : Part 3 : 1990, Test No 4	[5] BS 1377 : Part 2 : 1990, Test No 5.4
Water sample	8	Note that if the SO <sub>4</sub> content falls into the DS-4 or DS-5 class, it would be	רוויטוו ומווע אמודכיט פטווט אמודכ (טיא).	[4] BS 1377 : Part 2 : 1990, Test No 5.3
U100 (undisturbed sample)	C	[14] BRE Special Digest One (Concrete in Aggressive Ground) 2005	[9] Values of shear strength were determined in situ by Chelmer Site Investigations using a	[3] BS 1377 : Part 2 : 1990, Test No 4.4
Bulk sample	8	[13] $SO_4 = 1.2 \times SO_3$	[8] In-house method S9a adapted from BRE IP 4/93	[2] Estimated if <5%, otherwise measured
Disturbed sample	D	[12] BS 1377 : Part 3 : 1990, Test No 5.6	[7] BS 5930 : 1981 : Figure 31 - Plasticity Chart for the classification of fine soils	[1] BS 1377 : Part 2 : 1990, Test No 3.2
	Key	H		Notes:-

lab-001 Chelmer Site Investigations 2012

helmer **Seotechnical \_aboratories** 

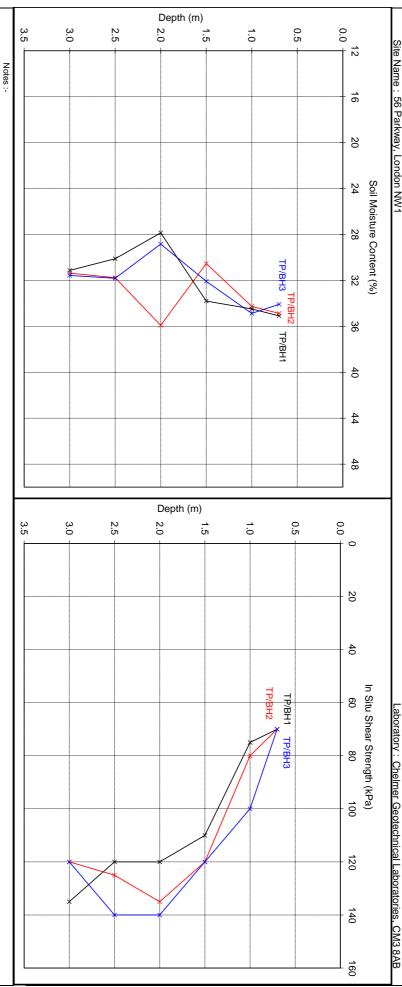
Moisture Content/Shear Strength Profile

Job Number: CGL03797

Client: Pyle Consulting Client Reference: CSI4137

Site Name: 56 Parkway, London NW1





2. If plotted, 0.4 LL and PL+2 ( after Driscoll, 1983 ) should only be applied to London Clay plotted and the alternative profile additionally shown as an appropriately coloured broken line, the remainder ( calculated in accordance with BS 1377: Part 2 : 1990, cl.3.2.4 note 1 ) is also 1. If the Soil Fraction > 0.425mm exceeds 5% the Equivalent Moisture Content of

a maximum reading of 140 kPa.

Chelmer Site Investigations using a Pilcon Hand Vane the calibration of which is limited to

Unless otherwise stated, values of Shear Strength were determined in situ by

and similarly over consolidated clays) at shallow depths.

Comments :-

Checked By :- AK

Date Checked :- 23-Jan-14

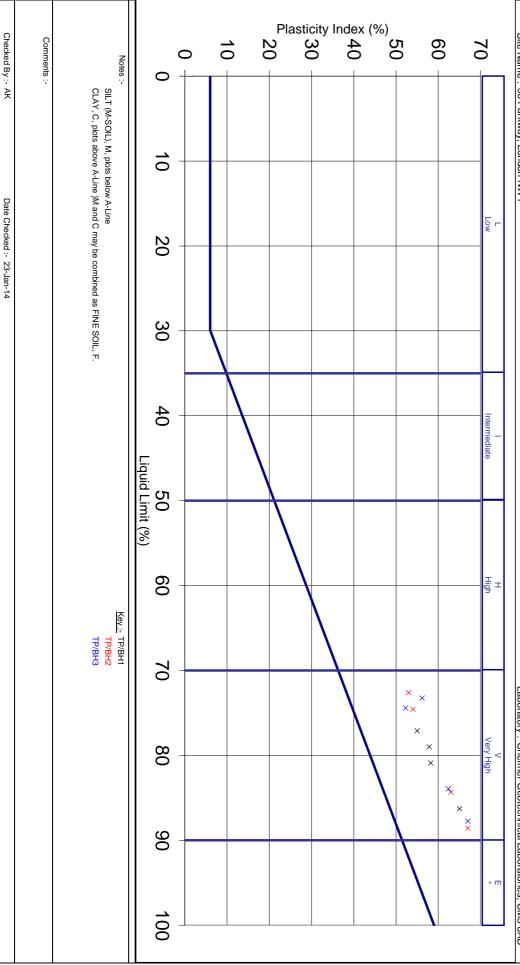
Plasticity Chart for the classification of fine soils and the finer part of coarse soils in Compliance with BS5930 : 1999

Job Number: CGL03797 Client: Pyle Consulting Client Reference: CSI4137 Site Name: 56 Parkway, London NW1

Chelmer
Ceotechnical
Laboratories

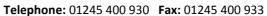
Date Received: 20/01/2014
Date Testing Started: 20/01/2014
Date Testing Completed: 21/01/2014

Laboratory: Chelmer Geotechnical Laboratories, CM3 8AB



Unit 15, East Hanningfield Industrial Estate, Old Church Road

East Hanningfield, Essex CM3 8AB







# DRAINAGE REPORT

Client:

**Pyle Consulting** 

Site Address:

56 Parkway London NW1 7AH

Date of Drainage Survey:

7th January 2014

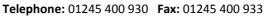
Chelmer Reference:

1019

Chelmer Project No.1019 Site Address: 56 Parkway, London, NW1 7AH

Unit 15, East Hanningfield Industrial Estate, Old Church Road

East Hanningfield, Essex CM3 8AB





 $\textbf{Email}: in fo@site investigations.co.uk \quad \textbf{Website}: www.site investigations.co.uk$ 

### **REPORT CONTENTS**

1.0	Drainage investigation summary
2.0	Drainage layout plan
3.0	CCTV survey report and recommendations
4.0	Manhole detail and condition summary
5.0	Summary of recommendations with repair costs

Chelmer Project No.1019 Site Address: 56 Parkway, London, NW1 7AH Date: 07-01-2014 Page **2** of **11** 

Unit 15, East Hanningfield Industrial Estate, Old Church Road East Hanningfield, Essex CM3 8AB

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### 1.0 DRAINAGE INVESTIGATION SUMMARY

This property has a combined drainage system with services collected from the rear of the property into an interceptor manhole, MH1, before flowing out and connecting to a common sewer. This common sewer appears to flow across the rear of the properties, passing under the back addition.

There were no operational issues noted at the time of the survey, however some defects were noted and are outlined in this report.

A CCTV survey was carried out to establish the condition of the drainage system. The fine detail of this CCTV survey is given on Pages 5–7, together with a summary of the findings and relevant recommendations where appropriate (recommendations based solely on *visual* condition of pipework).

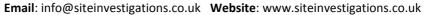
A summary of all the recommendations are given on Page 9, together with costing for the proposed works.

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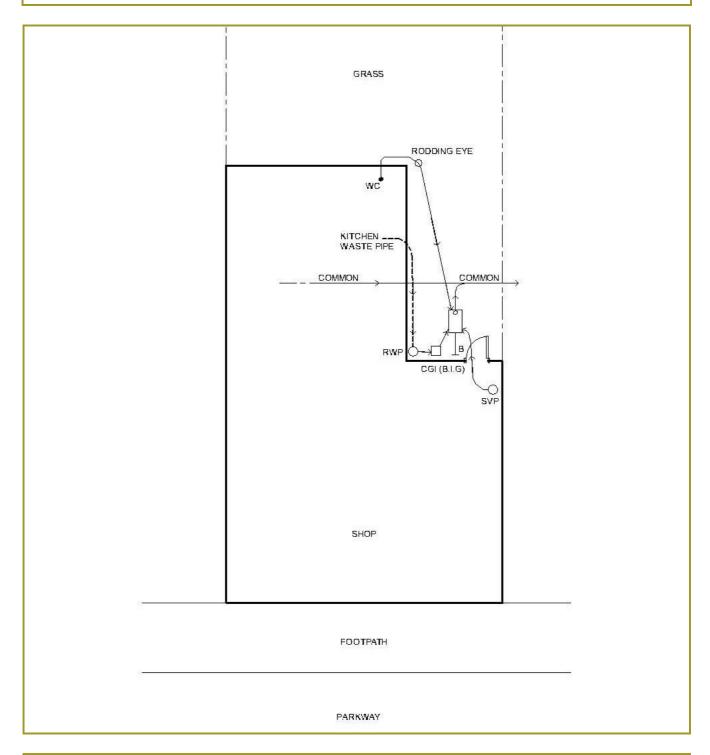
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## 2.0 DRAINAGE LAYOUT PLAN (NTS)



BD = Backdrop

CG = Combined grey waste and storm gully

R/E = Rodding Eye

SWG = Surface water gully

G = Grey water waste gully

RWG = Rainwater gully

SVP = Soil and vent pipe

RWP = Rainwater pipe

MH = Manhole

RWS = Rainwater pipe onto surface

VP = Vent pipe WP = Waste pipe

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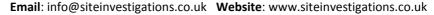
Date: 07-01-2014

Chelmer Project No.1019

KEY:

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### 3.0 CCTV SURVEY INSECTION REPORT AND RECOMMENDATIONS

SURVEY 1:	MH1 UPSTREAM TO SVP: 100mm CLAY: FOUL DUTY				
Meterage		Observations and comments			
00.0	Start survey	Start survey			
00.8	Slight offset jo	Slight offset joint (stepped with flow)			
00.9 – 01.4	Bend right	Bend right			
01.2	Slight offset jo	Slight offset joint (pulled right) and jointing intrusions			
01.3	Radial crack	Radial crack			
02.0 - 02.5	Bend left	Bend left			
02.6	At rest bend to SVP				
Invert level at MH1:	650mm	Operational condition: No operational issues noted at time	of survey		

### Recommendations:

There were no operational issues in this line at the time of the survey, however the survey did identify a crack in the pipework just after the first bend upstream from MH1 and given this defect there is likely to be leakage from the drain.

Accordingly, consideration should be given to lining from MH1 upstream to approximately 1.5m in order to seal this defect. Please note that due to the bend in the pipework this will need to be a 'flexible' liner (CIPP).

(See Repair Item A, Section 5.0)

SURVEY 2:	MH1 UPSTREAM 'B': 100mm CLAY: REDUNDANT				
Meterage		Observations and comments			
00.0	Start survey	Start survey			
00.0 - 00.4	Debris, up to 3	Debris, up to 30%			
00.4	End of pipework – brick visible ahead. Line appears redundant				
Invert level at MH1:	650mm	Operational condition: N/A			

### Recommendations:

The CCTV survey indicated this to be a redundant connection into MH1. The end of the pipe does not appear to be properly capped-off therefore if the system blocks there could be leakage into the ground from this inlet.

All services should be carefully checked again and once this inlet is confirmed to be redundant it should be properly capped-off from within MH1.

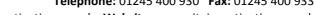
(See Repair Item B, Section 5.0)

cont./

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### 3.1 CCTV SURVEY INSECTION REPORT AND RECOMMENDATIONS

SURVEY 3:	MH1 UPSTREAM TO CG1: 100mm CLAY: COMBINED DUTY				
Meterage	Observations and comments				
00.0	Start survey				
00.3	Slight offset joint (stepped with flow). At back-inlet gully CG1				
Invert level at MH1:	650mm	Operational condition: No operational issues noted at time of surv			

### Recommendations:

This run is very short and no operational issues noted, however there is an open pipe from the RWP/waste pipes that connects into the gully as a back-inlet. This is in very poor condition and leakage could be seen during our site attendance. Accordingly this back-inlet and the gully CG1 should be broken out and renewed and the layout rationalised into MH1.

(See Repair Item C, Section 5.0)

SURVEY 4:	RODDING EYE UPSTREAM TO WC: 100mm PLASTIC: FOUL DUTY			
Meterage	Observations and comments			
00.0	Start survey			
00.7 – 01.0	Bend up to WC			
Invert level:	GL	Operational condition:	No operational issues noted at time of survey	

### Recommendations:

This section of pipework is unusually installed in that the pipework is at ground level. There were no visual defects or operational issues in the section surveyed therefore further works are not considered essential, however the arrangement is not ideal.



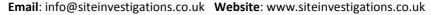
Pipework to WC is visible at ground level

cont./

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### 3.2 CCTV SURVEY INSECTION REPORT AND RECOMMENDATIONS

SURVEY 5:	R/ EYE DOWNSTREAM TO MH1: 100mm PLASTIC/CAST IRON/CLAY: FOUL DUTY			
Meterage	Observations and comments			
00.0	Start survey			
00.3	Change of material to cast iron			
04.0	Change of material to clay			
04.1 – 04.3	Bend down to MH1 (enters MH1 at high level – see MH1 overview photograph, Page 8)			
Invert level:	GL	Operational condition: No operational issues noted at time of survey		

### Recommendations:

See general notes for Survey 4 regarding pipework being at ground level.

SURVEY 6:	MH1 DOWNSTREAM TO MAIN SEWER: 100mm CLAY: COMBINED DUTY				
Meterage	Observations and comments				
00.0	Start survey	Start survey			
00.1	Interceptor trap outle	Interceptor trap outlet at 6 o'clock. Radial crack/fracture			
00.3	Radial crack				
00.4 - 00.5	Bend left				
00.6	At junction to common sewer				
Invert level at MH1:	650mm	Operational condition:	No operational issues noted at time of survey		

### Recommendations:

The survey identified some serious visual defects in this short section of pipework through to the common sewer. There is a fracture around the back of the interceptor trap and a further radial crack in the pipework indicating this run will leak.

Consideration should therefore be given to excavating and renewing the pipework through to the common sewer, including the interceptor trap.

Please note that the common sewer passes under the back addition. If the integrity of this sewer needs to be established it should be CCTV surveyed, however this will need to be instructed through Thames Water Utilities Limited as they are responsible for this sewer.

(See Repair Item D, Section 5.0)

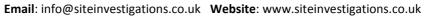
**END OF CCTV SURVEY** 

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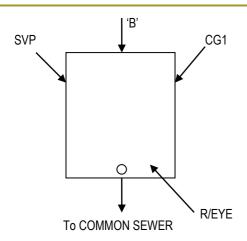
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### 4.0 MANHOLE SUMMARY



### <u>MH1</u>

Invert level (relative to cover level): 650mm

Cover dimension: 500 x 500mm Chamber dimension: 500 x 500mm Construction: Brick and render

Recommendation: None

Condition: No defects noted



MH1 overview

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### 4.0 SUMMARY OF RECOMMENDATIONS AND REPAIR COSTS

Item	Section of drainage	Chelmer CCTV survey number	Recommendation		Cost (£) excluding VAT
A	MH1 – SVP	1	Part line (CIPP) using flexible liner		£325.00
В	MH1 – 'B'	2	Check all services to confirm line is redundant, then cap-off properly from within MH1		£75.00
С	MH1 – CG1	3	Excavate and renew CG1 and back-inlet from RWP/waste pipes		£601.28
D	MH1 – Downstream	6	Excavate and renew (Please note that it <i>may</i> be necessary to break out part of the front manhole wall to allow sufficient working space to renew the interceptor trap in MH1)		£810.85
	High Priority Sub-total			£1,812.13	
	Medium Priority VAT			£362,43	
	Low Priority TOTAL				£2,174.56

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### TERMS AND CONDITIONS

### Please note the Quotation is based on the following:-

Within this quotation we have not included for any works other than for those stated above.

All works are subject to Chelmer Site Investigations Ltd Terms and Conditions of contract, schedule of variations and day working charges.

Access to the area of work is agreed prior to date of work, as a relevant fee will apply for an aborted site visit.

Areas of work are easily accessible and clear of obstructions.

When undertaking any internal works we will require the use of electricity. However, if none is available at time of works then an additional charge will be made.

Timber decking is to be lifted by householder/owner prior to date of works or a further fee will apply.

No allowances have been made to use crawler camera drain unit to survey pipelines greater than 225mm diameter, price on application.

Subject to availability, we would be able to undertake these works within 7 days upon receipt of written instruction.

### **Payment Terms**

Please be advised that when undertaking works for private/domestic clients full payment is required either before, or on the day of works.

For all new commercial clients/companies a 50% payment will be required before works commence.

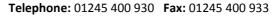
If you would like to accept this quotation we would be grateful if the details attached could be completed by the person/s settling our invoice and returned to ourselves.

Please note this quotation is valid for 90 days from the date of this letter.

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**Email**: info@siteinvestigations.co.uk **Website**: www.siteinvestigations.co.uk

We/I accept the quotation and terms as set out in Chelmer Project No.1019 and agree your payment terms. The invoice should be addressed as follows:-**Company Name:** (Invoice payee) Invoice Address: Tele No: Fax No: Email: **Purchase Order No:** Signed Print Name: \_\_\_\_\_ Date: \_\_\_\_\_

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## **REPORT NOTES**

### **Equipment Used**

Hand tools, Mechanical Concrete Breaker and Spade, Hand Augers, 100mm/150mm diameter Mechanical Flight Auger Rig, GEO205 Flight Auger Rig, Window Sampling Rig, and Large or Limited Access Shell & Auger Rig upon request and/or access permitting.

### On Site Tests

By Pilcon Shear-Vane Tester (Kn/m²) in clay soils, and/or Mackintosh Probe in granular soils or made ground and/or upon request Continuous Dynamic Probe Testing and Standard Penetration Testing.

### Note:

Details reported in trial-pits and boreholes relate to positions investigated only as instructed by the client or engineer on the date shown.

We are therefore unable to accept any responsibility for changes in soil conditions not investigated i.e. variations due to climate, season, vegetation and varying ground water levels.

Full terms and conditions are available upon request.