



VINCENT & RYMILL
LAKESIDE COUNTRY CLUB
FRIMLEY GREEN
SURREY GU16 6PT

Project 21 KIDDERPORE GARDENS NW3				Job Ref. 12A11	
Section PRELIMINARY WALL / BASE CALCULATIONS				Sheet no./rev. 1	
Calc. by TV	Date 13/05/2015	Chk'd by	Date	App'd by	Date

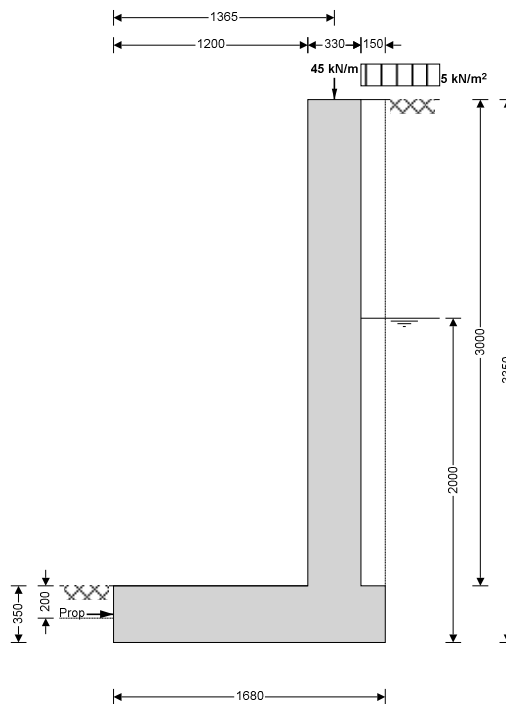
PRELIMINARY WALL BASE CALCULATIONS

1.PARTY WALL - RETAINING WALL AND BASE DESIGN

RETAINING WALL ANALYSIS & DESIGN (BS8002:1994)

RETAINING WALL ANALYSIS (BS 8002:1994)

TEDDS calculation version 1.2.01.03



Wall details

Retaining wall type

Cantilever

Height of wall stem

$h_{stem} = 3000$ mm

Wall stem thickness

$t_{wall} = 330$ mm

Length of toe

$l_{toe} = 1200$ mm

Length of heel

$l_{heel} = 150$ mm

Overall length of base

$l_{base} = 1680$ mm

Base thickness

$t_{base} = 350$ mm

Height of retaining wall

$h_{wall} = 3350$ mm

Depth of downstand

$d_{ds} = 0$ mm

Thickness of downstand

$t_{ds} = 350$ mm

Position of downstand

$l_{ds} = 1330$ mm

Depth of cover in front of wall

$d_{cover} = 0$ mm

Unplanned excavation depth

$d_{exc} = 200$ mm

Height of ground water

$h_{water} = 2000$ mm

Density of water

$\gamma_{water} = 9.81$ kN/m³

Density of wall construction

$\gamma_{wall} = 23.6$ kN/m³

Density of base construction

$\gamma_{base} = 23.6$ kN/m³

Angle of soil surface

$\beta = 0.0$ deg

Effective height at back of wall

$h_{eff} = 3350$ mm

Mobilisation factor

$M = 1.5$

Moist density

$\gamma_m = 21.0$ kN/m³

Saturated density

$\gamma_s = 23.0$ kN/m³

Design shear strength

$\phi' = 24.2$ deg

Angle of wall friction

$\delta = 18.6$ deg

Design shear strength

$\phi'_b = 24.2$ deg

Design base friction

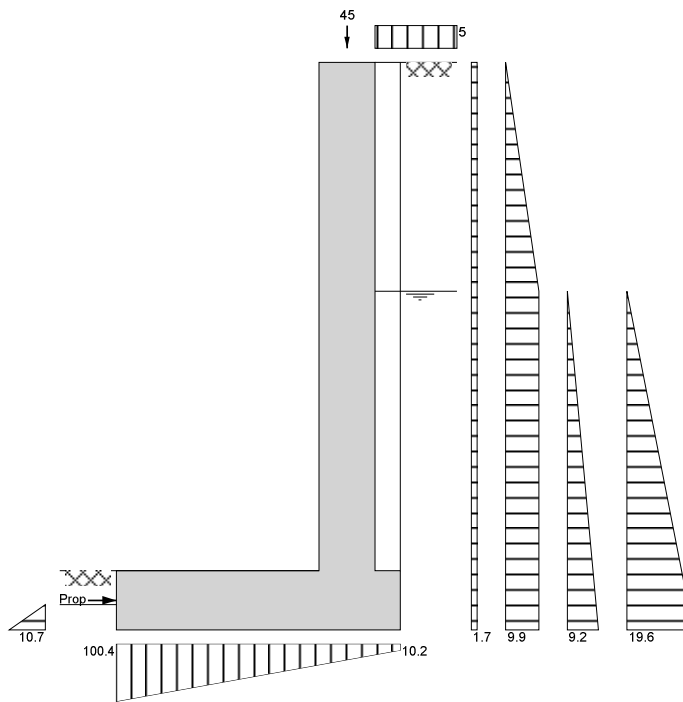
$\delta_b = 18.6$ deg



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Moist density	$\gamma_{mb} = 18.0 \text{ kN/m}^3$	Allowable bearing	$P_{bearing} = 125 \text{ kN/m}^2$
Using Coulomb theory			
Active pressure	$K_a = 0.369$	Passive pressure	$K_p = 4.187$
At-rest pressure	$K_0 = 0.590$		
Loading details			
Surcharge load	Surcharge = 5.0 kN/m²		
Vertical dead load	$W_{dead} = 45.0 \text{ kN/m}$	Vertical live load	$W_{live} = 0.0 \text{ kN/m}$
Horizontal dead load	$F_{dead} = 0.0 \text{ kN/m}$	Horizontal live load	$F_{live} = 0.0 \text{ kN/m}$
Position of vertical load	$l_{load} = 1365 \text{ mm}$	Height of horizontal load	$h_{load} = 0 \text{ mm}$



Loads shown in kN/m, pressures shown in kN/m²

Calculate propping force			
Propping force	$F_{prop} = 29.4 \text{ kN/m}$		
Check bearing pressure			
Total vertical reaction	$R = 92.9 \text{ kN/m}$	Total moment	$M_{total} = 56.9 \text{ kNm/m}$
Distance to reaction	$x_{bar} = 612 \text{ mm}$	Eccentricity of reaction	$e = 228 \text{ mm}$
		Reaction acts within middle third of base	
Bearing pressure at toe	$p_{toe} = 100.4 \text{ kN/m}^2$	Bearing pressure at heel	$p_{heel} = 10.2 \text{ kN/m}^2$
	PASS - Maximum bearing pressure is less than allowable bearing pressure		



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RETAINING WALL DESIGN (BS 8002:1994)

TEDDS calculation version 1.2.01.03

Ultimate limit state load factors

Dead load factor $\gamma_{f_d} = 1.4$ Live load factor $\gamma_{f_l} = 1.6$
Earth pressure factor $\gamma_{f_e} = 1.4$

Calculate propping force

Propping force $F_{prop} = 29.4$ kN/m

Design of reinforced concrete retaining wall toe (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40$ N/mm² Strength of reinforcement $f_y = 500$ N/mm²

Base details

Minimum reinforcement $k = 0.13$ % Cover in toe $c_{toe} = 50$ mm

Design of retaining wall toe

Shear at heel $V_{toe} = 116.4$ kN/m Moment at heel $M_{toe} = 140.7$ kNm/m

Compression reinforcement is not required

Check toe in bending

Reinforcement provided **16 mm dia.bars @ 125 mm centres**

Area required $A_{s_toe_req} = 1166.0$ mm²/m Area provided $A_{s_toe_prov} = 1608$ mm²/m

PASS - Reinforcement provided at the retaining wall toe is adequate

Check shear resistance at toe

Design shear stress $v_{toe} = 0.399$ N/mm² Allowable shear stress $v_{adm} = 5.000$ N/mm²

PASS - Design shear stress is less than maximum shear stress

Concrete shear stress $v_{c_toe} = 0.656$ N/mm²

$v_{toe} < v_{c_toe}$ - No shear reinforcement required

Design of reinforced concrete retaining wall heel (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40$ N/mm² Strength of reinforcement $f_y = 500$ N/mm²

Base details

Minimum reinforcement $k = 0.13$ % Cover in heel $c_{heel} = 50$ mm

Design of retaining wall heel

Shear at heel $V_{heel} = 16.9$ kN/m Moment at heel $M_{heel} = 4.2$ kNm/m

Compression reinforcement is not required

Check heel in bending

Reinforcement provided **B785 mesh**

Area required $A_{s_heel_req} = 455.0$ mm²/m Area provided $A_{s_heel_prov} = 785$ mm²/m

PASS - Reinforcement provided at the retaining wall heel is adequate

Check shear resistance at heel

Design shear stress $v_{heel} = 0.057$ N/mm² Allowable shear stress $v_{adm} = 5.000$ N/mm²

PASS - Design shear stress is less than maximum shear stress

Concrete shear stress $v_{c_heel} = 0.513$ N/mm²

$v_{heel} < v_{c_heel}$ - No shear reinforcement required



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Design of reinforced concrete retaining wall stem (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40 \text{ N/mm}^2$ Strength of reinforcement $f_y = 500 \text{ N/mm}^2$

Wall details

Minimum reinforcement $k = 0.13 \%$

Cover in stem $c_{stem} = 50 \text{ mm}$ Cover in wall $c_{wall} = 50 \text{ mm}$

Design of retaining wall stem

Shear at base of stem $V_{stem} = 19.0 \text{ kN/m}$ Moment at base of stem $M_{stem} = 110.0 \text{ kNm/m}$

Compression reinforcement is not required

Check wall stem in bending

Reinforcement provided **16 mm dia.bars @ 100 mm centres**

Area required $A_{s_stem_req} = 978.7 \text{ mm}^2/\text{m}$ Area provided $A_{s_stem_prov} = 2011 \text{ mm}^2/\text{m}$

PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress $V_{stem} = 0.070 \text{ N/mm}^2$ Allowable shear stress $V_{adm} = 5.000 \text{ N/mm}^2$

PASS - Design shear stress is less than maximum shear stress

Concrete shear stress $V_{c_stem} = 0.736 \text{ N/mm}^2$

$V_{stem} < V_{c_stem}$ - No shear reinforcement required

Check retaining wall deflection

Max span/depth ratio $ratio_{max} = 11.54$ Actual span/depth ratio $ratio_{act} = 11.03$

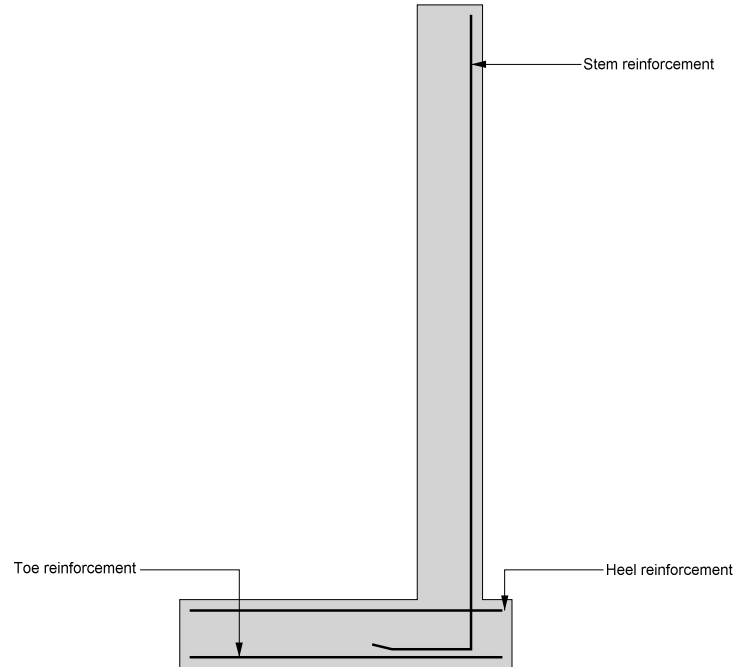
PASS - Span to depth ratio is acceptable



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Indicative retaining wall reinforcement diagram



Toe bars - 16 mm dia.@ 125 mm centres - (1608 mm²/m)

Heel mesh - B785 - (785 mm²/m)

Stem bars - 16 mm dia.@ 100 mm centres - (2011 mm²/m)



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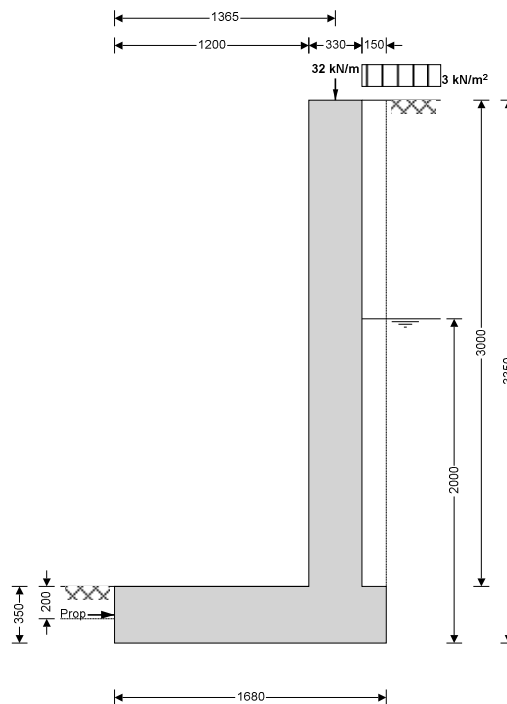
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2. OTHER EXTERNAL WALLS – WALL AND BASE DESIGN

RETAINING WALL ANALYSIS & DESIGN (BS8002)

RETAINING WALL ANALYSIS (BS 8002:1994)

TEDDS calculation version 1.2.01.03



Wall details

Retaining wall type

Height of wall stem

Length of toe

Overall length of base

Height of retaining wall

Depth of downstand

Position of downstand

Depth of cover in front of wall

Height of ground water

Density of wall construction

Angle of soil surface

Mobilisation factor

Moist density

Design shear strength

Design shear strength

Moist density

Using Coulomb theory

Active pressure

At-rest pressure

Cantilever

$h_{\text{stem}} = 3000$ mm

$l_{\text{toe}} = 1200$ mm

$l_{\text{base}} = 1680$ mm

$h_{\text{wall}} = 3350$ mm

$d_{\text{ds}} = 0$ mm

$l_{\text{ds}} = 1250$ mm

$d_{\text{cover}} = 0$ mm

$h_{\text{water}} = 2000$ mm

$\gamma_{\text{wall}} = 23.6$ kN/m³

$\beta = 0.0$ deg

$M = 1.5$

$\gamma_m = 21.0$ kN/m³

$\phi' = 24.2$ deg

$\phi'_b = 24.2$ deg

$\gamma_{\text{mb}} = 18.0$ kN/m³

$K_a = 0.369$

$K_0 = 0.590$

Wall stem thickness

Length of heel

Base thickness

Thickness of downstand

Unplanned excavation depth

Density of water

Density of base construction

Effective height at back of wall

Saturated density

Angle of wall friction

Design base friction

Allowable bearing

Passive pressure

$t_{\text{wall}} = 330$ mm

$l_{\text{heel}} = 150$ mm

$t_{\text{base}} = 350$ mm

$t_{\text{ds}} = 350$ mm

$d_{\text{exc}} = 200$ mm

$\gamma_{\text{water}} = 9.81$ kN/m³

$\gamma_{\text{base}} = 23.6$ kN/m³

$h_{\text{eff}} = 3350$ mm

$\gamma_s = 23.0$ kN/m³

$\delta = 18.6$ deg

$\delta_b = 18.6$ deg

$P_{\text{bearing}} = 125$ kN/m²

$K_p = 4.187$

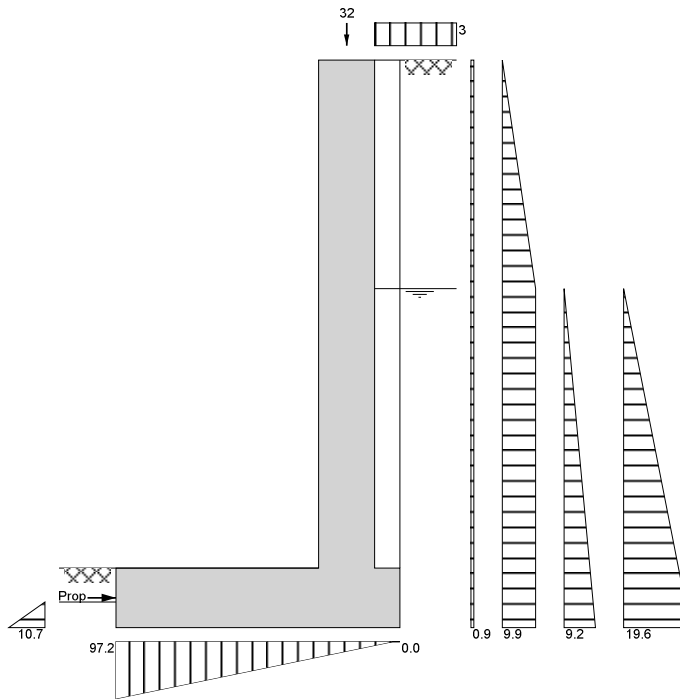


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Loading details

Surcharge load	Surcharge = 2.5 kN/m²	Vertical live load	$W_{live} = 0.0 \text{ kN/m}$
Vertical dead load	$W_{dead} = 32.0 \text{ kN/m}$	Horizontal live load	$F_{live} = 0.0 \text{ kN/m}$
Horizontal dead load	$F_{dead} = 0.0 \text{ kN/m}$	Height of horizontal load	$h_{load} = 0 \text{ mm}$
Position of vertical load	$l_{load} = 1365 \text{ mm}$		



Loads shown in kN/m, pressures shown in kN/m²

Calculate propping force

Propping force $F_{prop} = 30.9 \text{ kN/m}$

Check bearing pressure

Total vertical reaction $R = 79.6 \text{ kN/m}$
Distance to reaction $x_{bar} = 546 \text{ mm}$

Total moment $M_{total} = 43.4 \text{ kNm/m}$
Eccentricity of reaction $e = 294 \text{ mm}$

Reaction acts outside middle third of base

Bearing pressure at toe $p_{toe} = 97.2 \text{ kN/m}^2$ Bearing pressure at heel $p_{heel} = 0.0 \text{ kN/m}^2$

PASS - Maximum bearing pressure is less than allowable bearing pressure



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RETAINING WALL DESIGN (BS 8002:1994)

TEDDS calculation version 1.2.01.03

Ultimate limit state load factors

Dead load factor $\gamma_{f_d} = 1.4$ Live load factor $\gamma_{f_l} = 1.6$
Earth pressure factor $\gamma_{f_e} = 1.4$

Calculate propping force

Propping force $F_{prop} = 30.9$ kN/m

Design of reinforced concrete retaining wall toe (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40$ N/mm² Strength of reinforcement $f_y = 500$ N/mm²

Base details

Minimum reinforcement $k = 0.13$ % Cover in toe $c_{toe} = 50$ mm

Design of retaining wall toe

Shear at heel $V_{toe} = 97.6$ kN/m Moment at heel $M_{toe} = 127.6$ kNm/m

Compression reinforcement is not required

Check toe in bending

Reinforcement provided **16 mm dia.bars @ 125 mm centres**

Area required $A_{s_toe_req} = 1057.4$ mm²/m Area provided $A_{s_toe_prov} = 1608$ mm²/m

PASS - Reinforcement provided at the retaining wall toe is adequate

Check shear resistance at toe

Design shear stress $v_{toe} = 0.334$ N/mm² Allowable shear stress $v_{adm} = 5.000$ N/mm²

PASS - Design shear stress is less than maximum shear stress

Concrete shear stress $v_{c_toe} = 0.656$ N/mm²

$v_{toe} < v_{c_toe}$ - No shear reinforcement required

Design of reinforced concrete retaining wall heel (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40$ N/mm² Strength of reinforcement $f_y = 500$ N/mm²

Base details

Minimum reinforcement $k = 0.13$ % Cover in heel $c_{heel} = 50$ mm

Design of retaining wall heel

Shear at heel $V_{heel} = 16.3$ kN/m Moment at heel $M_{heel} = 4.1$ kNm/m

Compression reinforcement is not required

Check heel in bending

Reinforcement provided **B785 mesh**

Area required $A_{s_heel_req} = 455.0$ mm²/m Area provided $A_{s_heel_prov} = 785$ mm²/m

PASS - Reinforcement provided at the retaining wall heel is adequate

Check shear resistance at heel

Design shear stress $v_{heel} = 0.055$ N/mm² Allowable shear stress $v_{adm} = 5.000$ N/mm²

PASS - Design shear stress is less than maximum shear stress

Concrete shear stress $v_{c_heel} = 0.513$ N/mm²

$v_{heel} < v_{c_heel}$ - No shear reinforcement required



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Design of reinforced concrete retaining wall stem (BS 8002:1994)

Material properties

Strength of concrete $f_{cu} = 40 \text{ N/mm}^2$ Strength of reinforcement $f_y = 500 \text{ N/mm}^2$

Wall details

Minimum reinforcement $k = 0.13 \%$
Cover in stem $c_{stem} = 50 \text{ mm}$ Cover in wall $c_{wall} = 50 \text{ mm}$

Design of retaining wall stem

Shear at base of stem $V_{stem} = 13.7 \text{ kN/m}$ Moment at base of stem $M_{stem} = 98.1 \text{ kNm/m}$
Compression reinforcement is not required

Check wall stem in bending

Reinforcement provided **16 mm dia.bars @ 125 mm centres**
Area required $A_{s_stem_req} = 873.2 \text{ mm}^2/\text{m}$ Area provided $A_{s_stem_prov} = 1608 \text{ mm}^2/\text{m}$
PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress $V_{stem} = 0.050 \text{ N/mm}^2$ Allowable shear stress $V_{adm} = 5.000 \text{ N/mm}^2$
PASS - Design shear stress is less than maximum shear stress
Concrete shear stress $V_{c_stem} = 0.683 \text{ N/mm}^2$
 $V_{stem} < V_{c_stem}$ - No shear reinforcement required

Check retaining wall deflection

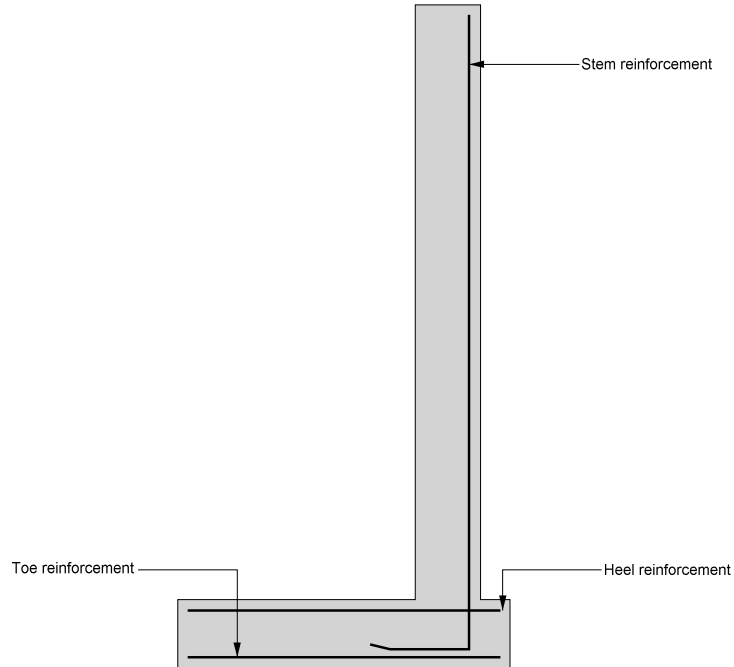
Max span/depth ratio $ratio_{max} = 11.61$ Actual span/depth ratio $ratio_{act} = 11.03$
PASS - Span to depth ratio is acceptable



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Indicative retaining wall reinforcement diagram



Toe bars - 16 mm dia.@ 125 mm centres - (1608 mm²/m)

Heel mesh - B785 - (785 mm²/m)

Stem bars - 16 mm dia.@ 125 mm centres - (1608 mm²/m)