

# Basement Impact Assessment: 56 Hawtrey Road

(Groundwater only)



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### **Prepared for**

Christine and Paul Koffel 56 Hawtrey Road London NW3 3SS

Report reference: 63489R1, May 2015

Report status: Final

Confidential Prepared by ESI Ltd



# Basement Impact Assessment: 56 Hawtrey Road

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#### 63489R1. Final

#### Groundwater

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### **Revision record:**

Issue	Report ref	Comment (SW/GW)	Author	Checker	Reviewer	Issue date	Issued to
1	63489 R1D1	Groundwater	JAK	HRS	всн	01/05/15	Christine Athanassious, Paul Koffel, Anna Williamson, Simon Bennett
2	63489 R1	Groundwater	JAK	HRS	всн	11/05/15	Christine Athanassious, Paul Koffel, Anna Williamson, Simon Bennett
3							
4							

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### REPORT SUMMARY

The assessment findings are summarised as follows:

2. Impacts to ground water flows and related flooding

High	
Med	
Low	

Key:

High	There is a high potential risk
Med	There is medium potential risk
Low	There is a low potential risk

#### **RECOMMENDATIONS** (FOR NEXT STEPS)

The water detected in the onsite borehole is most likely to be groundwater originating from the London Clay. There is a medium potential risk to the proposed basement construction as the recorded levels of groundwater are higher than the proposed depth of the basement, however this can be mitigated by appropriate design and construction techniques.

The potential impact to the groundwater environment is considered low as the London Clay severely limits any flow of groundwater.

There is a small possibility that the water detected in the onsite boreholes may be sourced from a recharge to the Made Ground that overlies the London Clay, in which case there is a low potential risk and no action would be required to mitigate impacts of the proposed development on groundwater.

On this basis the impacts have been conservatively assessed as Medium.

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#### 1 INTRODUCTION

#### 1.1 Background

ESI Ltd (ESI) was commissioned by Anna Williamson in February 2015 to undertake a Basement Impact Assessment (BIA) for the proposed development at 56 Hawtrey Rd, London, NW3 3SS (the Site). This is a mid-terrace three storey house located at the approximate national grid reference of 527049 184207 in the London Borough of Camden (Figure 1.1)

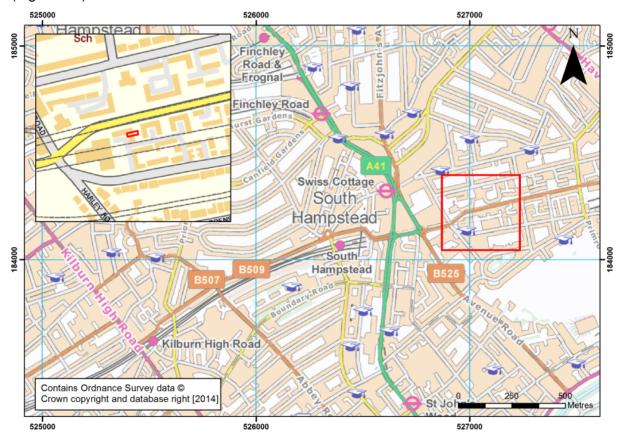


Figure 1.1 Site Location

This document is a desk study which considers the potential impact relating to the proposed basement development in terms of groundwater flow and flooding and complies with guidance issued by the London Borough of Camden. This report will be used for submission to the Planning Authority for approval of the proposed development.

#### 1.2 Scope of Works

The following scope of works was requested: an assessment of the impacts of the proposed development on ground water flow and levels. This report outlines the, hydrogeological conditions with relevance to construction of the basement at the property. The assessment conforms to the requirements of guidance set out by The London Borough of Camden which provides comprehensive guidance on planning applications for basement extensions. These guidelines for basement impact assessments (ARUP (2010), Camden Borough Council, (2011)) have been consulted in order to complete a screening analysis of key hydrological and hydrogeological issues that will satisfy the relevant planning requirements.

The works undertaken follow the procedure outlined below:

1) Screening – this process aims to identify sites that are a priority for investigation.

- 2) Scoping this process uses simple calculations to try to demonstrate whether the potential hazards identified in the screening stage pose a risk as a result of the development, and whether the actual risk is significant.
- 3) Site investigation to develop an understanding of the site and its immediate surroundings
- 4) Impact Assessment this stage evaluates the direct and indirect implications of the proposed project.

#### 1.3 Proposed Basement Works

The proposed development is for the excavation of a new single storey basement partially below the footprint of the existing residential property and fully under an above-ground extension to the property which has planning permission but has yet to be built. Site plans are shown in (Appendix A).

The depth of the completed basement is expected to be c. 3.4 m below ground level (mBGL) (note, the ground level at the Site is approximately 50 maOD).

The basement will have an external area of approximately 65 m<sup>2</sup>. The property, including the area of the yet to be built extension, has a total area of 85 m<sup>2</sup> therefore the proposed basement shall occupy an area that is approximately 77% of the total building footprint. The proposed basement shall extend from the rear of the extended property, occupying its full width but does not extend the full length of the property to the front facing side.

### **SCREENING**

The screening stage for Impact Assessment has been considered as set out in CPG4 (Camden Council, 2011) as follows:

Impact question	Answer	Justification	Reference
1a) Is the Site located directly above an aquifer?	No	The Site is located upon the London Clay Formation; a sedimentary bedrock comprising bioturbated or poorly laminated, slightly calcareous, silty to very silty clay, clayey silt and sometimes silt, with some layers of sandy clay. This may contain high porosity, low permeability horizons within generally low permeability and low porosity material that is classified as Unproductive Strata by the Environment Agency.	British Geological Survey, 2015. Environment Agency, 2015. Point Drilling Services, 2015
		The closest deep borehole log TQ28SE1769 (270 m west-north west of the site) shows that locally the London Clay has a thickness of about 83 m (Appendix B).	
		There is Made Ground, mainly comprising silty clay, ranging in thickness between 1.4 m and 2.15 m overlying the London Clay; this was logged during the ground investigation undertaken by Point Drilling Services on 13 <sup>th</sup> March 2015 (Appendix C). The ground investigation comprised three boreholes that were drilled in the rear garden (patio area) of the existing property; two to a depth of 5 mBGL that penetrate the London Clay and one to a depth of 2 mBGL that only extends through the Made Ground. Point Drilling Services found the London Clay to be stiff becoming very stiff slightly silty Clay with rare partings of Silt.	
1b) Will the proposed basement extend beneath the water table surface?	Uncertain	Given the nature of the London Clay in the vicinity of the Site significant groundwater flow in the London Clay beneath the Site is unlikely.	British Geological Survey, 2015.
		The proposed development is therefore unlikely to encounter a water table or extend beneath it.	Point Drilling Services, 2015
		No water strikes were reported during the drilling of the boreholes to 5 m depth during the ground investigation (Appendix C).	
		However water level dip measurements undertaken at the Site in the three weeks following the ground investigation show that water has entered the boreholes and that the water levels in the boreholes were about 2 mBGL, with a maximum recorded level of 1.65 mBGL on 30 March 2015 (refer to Appendix D). The source of this water is uncertain; since neither the Made Ground nor the London Clay should be sustaining significant groundwater flows. In each borehole water levels exceeded the base of the proposed basement (3.4 mBGL).	

2) Is the Site within 100m of a watercourse, well (used/disused) or potential spring line?	No	The closest open water course is the Grand Union Canal which lies approximately 1.0 km southeast of the Site. This watercourse is down gradient topographically from the Site.  The culverted ("lost") river Tyburn runs 70 m to the west of the proposed development and another culverted tributary of the Tyburn runs 150 m to the east of the proposed development  There are no wells within 100 m of the Site.	British Geological Survey, 2015. Ordnance Survey Mapping. 2015. Barton, 1992.
3) Will the proposed basement development result in a change in the proportion of hard surfaced / paved external areas?	No	The proposed basement will extend beyond the current footprint of the property and beneath a ground floor extension that has planning permission but has yet to be constructed. As such, the proposed basement will not alter the proportion of hard surfaced / paved external areas.	Site Plans.
4) As part of the Site drainage, will more surface water (e.g. rainfall and run-off) than at present be discharged to the ground (e.g. via soakaways and/or SUDS)?	No	There will be no change to the total area covered by hard standing or paving with the proposed development.	Site Plans.
5) Is the lowest point of the proposed excavation (allowing for any drainage and foundation space under the basement floor) close to, or lower than, the mean water level in any local pond or spring line.	No	There are no known ponds or spring lines within close proximity of the Site.	Ordnance Survey Mapping. 2015.

### 3 SCOPING

3.2 GROUND WATER (Subterranean (ground water) flow screening chart (Figure 1, CPG4 (Camden Council, 2011))						
Impact question	Answer	Justification	Reference			
1b) Will the proposed basement extend beneath the water table surface?	Uncertain	Many of the borehole logs from within a 500 m radius of the Site show that no groundwater was encountered at depths relating to the proposed basement during boring, this includes borehole log TQ28SE1769 located approximately 270 m north west of the site, (provided in Appendix B).	British Geological Survey, 2015. Point Drilling Services, 2015			
		There was no water encountered during the drilling of three boreholes, two of which were completed to depths of 5 m and the other to a depth of 2 m for the ground investigation at the Site on 13 <sup>th</sup> March 2015.				
		However water has been detected and monitored in all three boreholes at the Site between 27 <sup>th</sup> March and 2 <sup>nd</sup> April 2015 at an average level of about 2 mBGL and a maximum recorded level of 1.65 mBGL.				
		It is uncertain where the water detected in the borehole has come from, and there are two possibilities with different implications. 1) A flux of water in the Made Ground from recharge events could have penetrated the screened section of the borehole 2) There is a small amount of groundwater present in the London Clay and due to low permeability of the material this was not detected during the ground investigation. Subsequently it slowly filled the borehole.				
		Should the water have originated in the London Clay above the base of the proposed basement then it is very unlikely to indicate any significant groundwater flow locally due to the low permeability of the Clay; this means that neighbouring properties would be very unlikely to be affected. However, the implications for the construction of the basement would need to be considered. This is the most likely explanation since the peak levels were recorded following a significant rainfall event, when negligible amounts of water were recorded in the borehole located solely in the Made Ground.				
		Should the water have originated from surface water or from minor inflow from the Made Ground, there would be no risk either to neighbouring properties or to the proposed construction.				

### 4 SITE CONCEPTUAL MODEL

4.1 CONCEPTI	JAL UNDERSTANDING						
Geology	Superficial	Superficial There is shallow cover of Made Ground at the Site with a thickness in the range of 1.4 m to 2.15 m					
	Bedrock	Underlying the Made Ground at the Site is the London Clay Formation; a sedimentary bedrock comprising bioturbated or poorly laminated, slightly calcareous, silty to very silty clay, clayey silt and sometimes silt, with some layers of sandy clay. This is expected to have a local thickness of between 60 and 100 m and has been logged at a thickness of 83 m in a nearby borehole TQ28SE1769 (Appendix B). The clay is reported to become more fissured with depth.					
Aquifers	The London Clay is not classed as an aquifer by the Environment Agency, but as unproductive strata which is defined as rock layers or drift deposits with low permeability that has negligible significance for water supply or river base flow. (Environment Agency, 2014)						
Groundwater levels	There is uncertainty regarding the source of groundwater beneath the site based on the available data. Water was not encountered during the construction of three boreholes during the ground investigation in March 2015. However, water has since been detected in each of the boreholes at a depth of about 2 mBGL with maximum recorded levels at 1.65 mBGL during a 3 week period following the initial boreholes installation. There are two possible scenarios to cause this:						
	1) A flux of water draining vertically through the Made Ground from recharge events has penetrated the screened section of the borehole. There is anecdotal evidence which indicates that the peak in groundwater levels followed a heavy rainfall event in the preceding days (tele. comms. with Bob Lowther of Point Drilling Services, 2015). This would be a low risk scenario.						
	2) There is groundwater present in the London Clay and due to low permeability of the material and exceptionally low flow this was not detected during the ground investigation. This would be a medium risk scenario for the proposed construction, though it would remain low risk for neighbouring properties. This is the most likely explanation since the peak levels were recorded following a significant rainfall event, when negligible amounts of water were recorded in the borehole located solely in the Made Ground. If the water was originating in the Made Ground then higher levels would have been observed in the shallow borehole as the low permeability London Clay would prevent rapid infiltration.						
		resent in the London Clay at a level of 1.65 mBGL then the proposed basement would extend below the water table by 1.1 m would also be subject to seasonal variation, although groundwater levels at the time of monitoring would be expected to be at e winter recharge period					

#### **4.2 IMPACTS ON GROUNDWATER FLOWS**

If there is groundwater present in the clay then it is unlikely to have any significant flow due to its low permeability. Therefore the volume of water passing through the Site would be relatively low and there would be minimal impact.

Based upon the points above, the construction of the basement would cause a relatively minor obstruction of groundwater flow leading to slightly increased flows around the proposed basement.

As the development is not expected to cause any significant rise in groundwater height up gradient from the property, adjacent properties are not expected to be affected. There are not known to be any other basements adjacent to the site.

Down gradient properties are also not expected to be affected by the development.

Report Reference: 63489R1

Report Status: Final

### 5 CONCLUSIONS (IMPACT ASSESSMENT)

#### 5.1 Ground water

Potential impacts of the proposed basement development at 56 Hawtrey Rd have been considered as set out in the scope of works. The following summary conclusions are made:

- The proposed basement will be constructed to a depth of 3.4 m below ground level into the underlying Made Ground and London Clay.
- Groundwater was not present during or immediately after the site investigation, however
  it was observed in the onsite boreholes two weeks after the investigation and following a
  period of heavy rainfall.
- There are two possible scenarios to explain the presence of the water in the onsite boreholes:
  - 1. It is a flux of water from recharge events that has entered the borehole due to vertical drainage in the Made Ground and the position of the borehole screen.
  - 2. There is groundwater present in the clay.

It is most likely that the water originates from the London Clay as there was very little water recorded in the borehole in the Made Ground

- The overall risk from the proposed development is considered to be medium, as the groundwater present in the onsite boreholes is recorded at a level higher than the intended depth of the proposed basement.
- The potential impact to the groundwater environment is considered low as the London Clay severely limits any flow of groundwater

#### 5.2 Recommendations

It is recommended that due to the Site being classified as medium potential risk, mitigation would be required in the form of an appropriate basement design and construction method as the basement would extend below the groundwater level,.

#### **REFERENCES**

**Arup, 2008**. Royal Borough of Kensington & Chelsea Town Planning Policy on Subterranean Development Phase 1 - Scoping Study DRAFT, June 2008.

**Barton, N., 1992**. The Lost Rivers of London, revised edition. Historical Publications Ltd. London.

**British Geological Survey, 2015** Received 09/04/15 from http://mapapps.bgs.ac.uk/geologyofbritain/home.html.

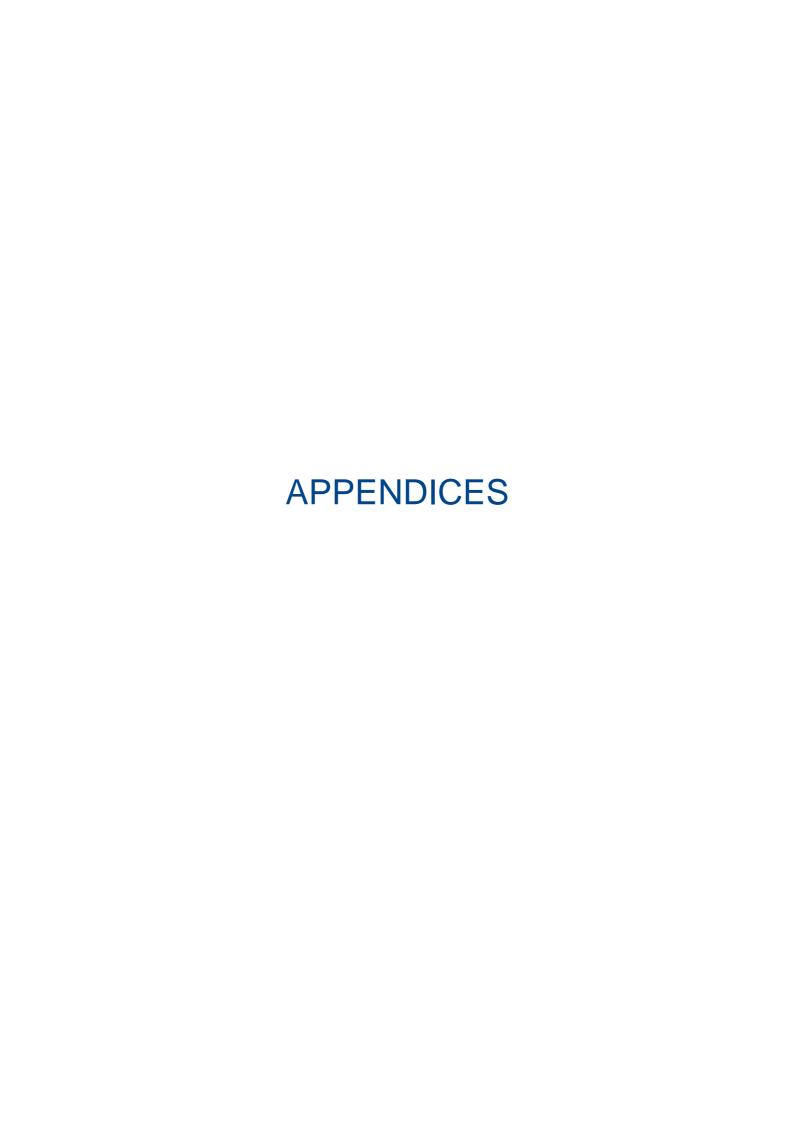
**Camden Council, 2011.** Camden Planning Guidance: Basements and lightwells. London Borough of Camden, CPG4.

Environment Agency, 2015. What's in your backyard website. Received from , April 2015.

**Ordnance survey mapping**, 1:10,000. © Crown copyright. All rights reserved. Licence number AL 100015683

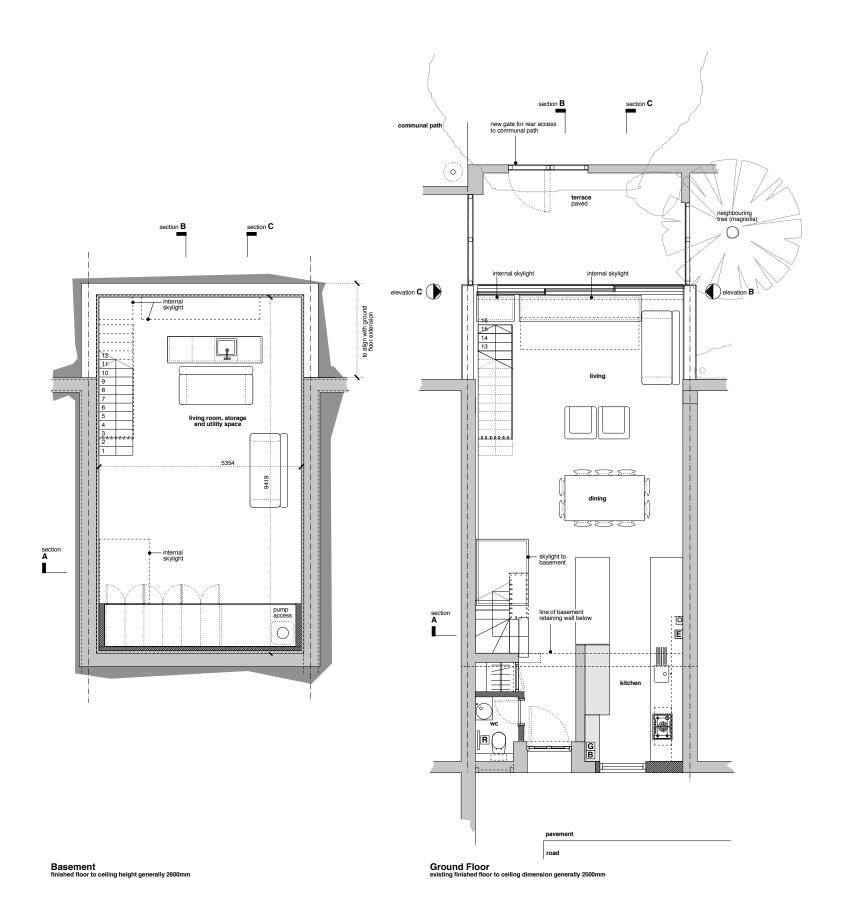
**London Borough of Camden, 2010.** Camden Geological, Hydrogeological and Hydrological Study.

Point Drilling Services, 2015 Site investigation borehole logs.



# **APPENDIX A**

**Hawtrey Road Proposed Development Plans** 



This drawing is copyright of Anna Williamson Architects Do not scale dimensions from this drawing. All dimensions are to be checked on site.

Report any dimensional discrepancies, errors or omissions prior to commencing on site.



Read in conjunction with: drawings P01-07, current revisions

Structural elements and foundations subject to confirmation by structural engineer

Party wall lines subject to confirmation by party wall surveyor

A 30 04 2015 Planning
Rev Date Description



Project 56 Hawtrey Road, NW3 3SS

General Arrangement Plans Proposed Basement-Ground

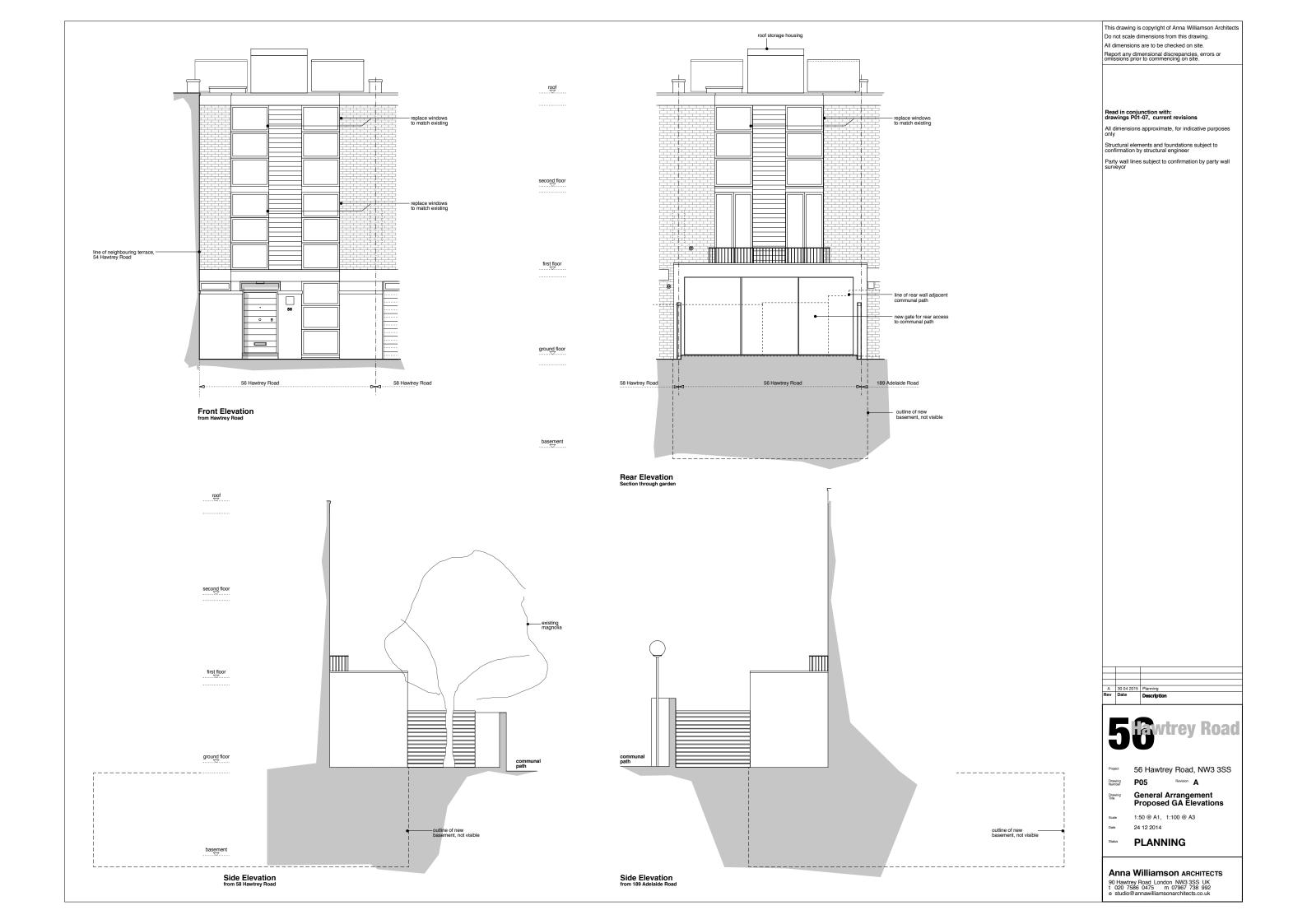
1:50 @ A1, 1:100 @ A3

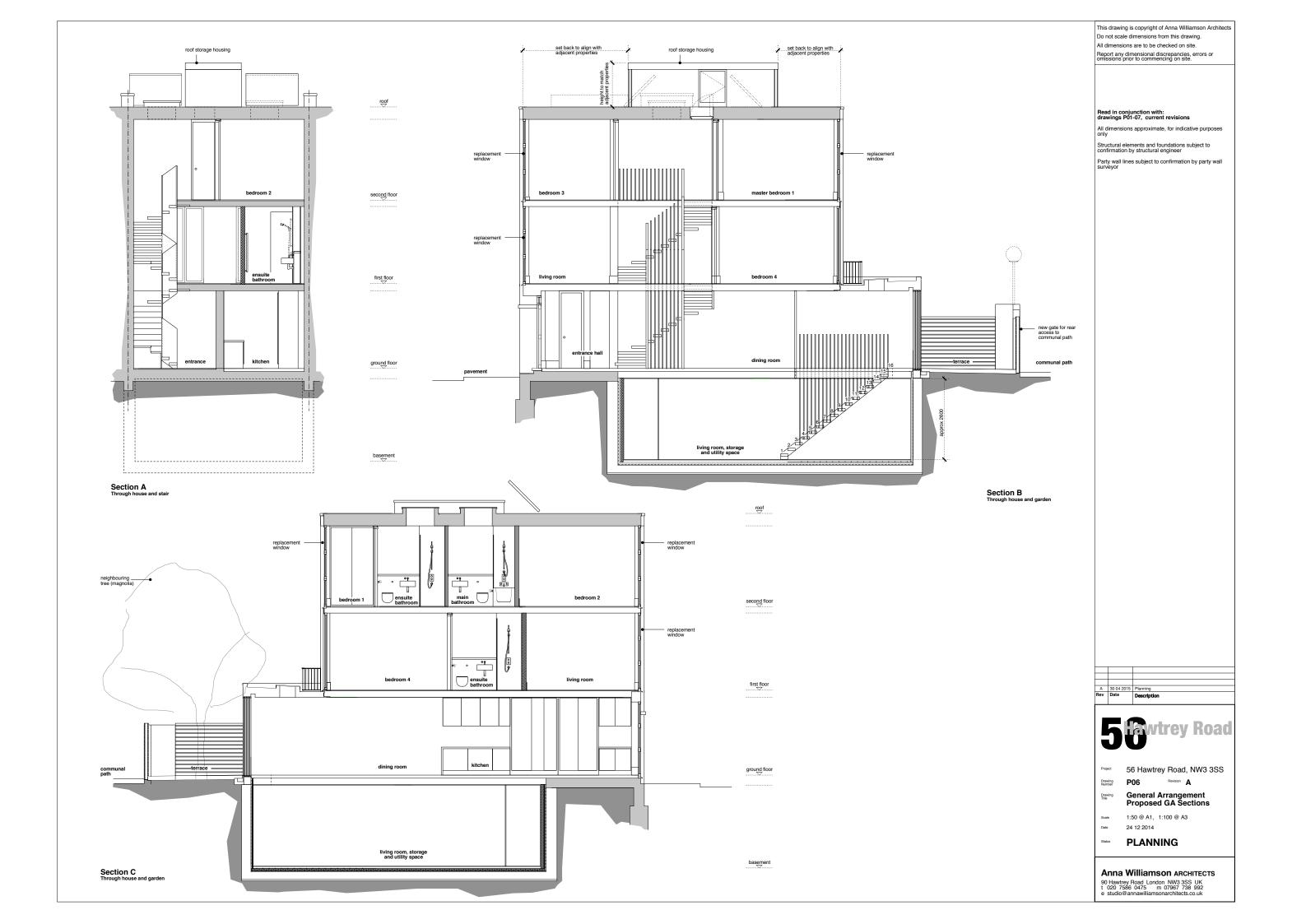
24 12 2014

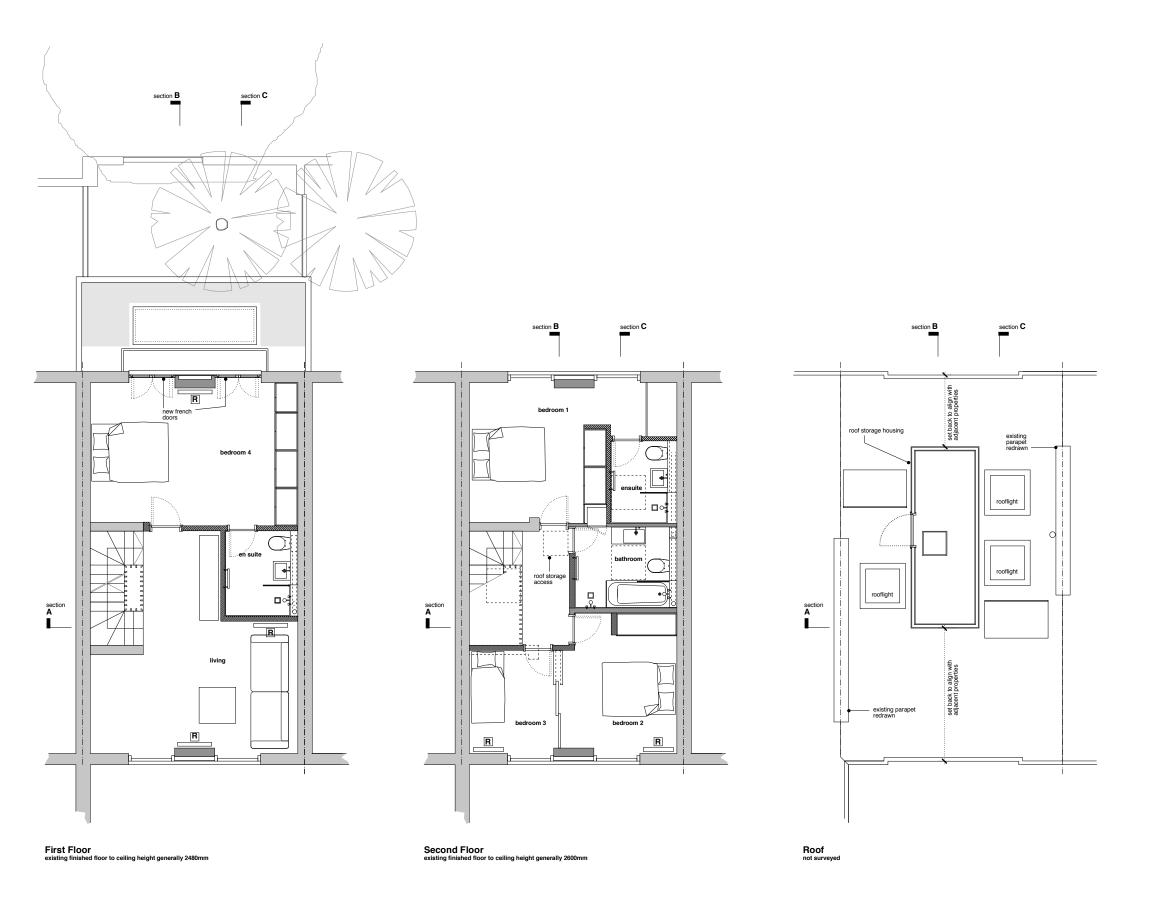
**PLANNING** 

Anna Williamson ARCHITECTS

90 Hawtrey Road London NW3 3SS UK t 020 7586 0475 m 07967 738 992 e studio@annawilliamsonarchitects.co.uk







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Read in conjunction with: drawings P01-07, current revisions

Structural elements and foundations subject to confirmation by structural engineer

Party wall lines subject to confirmation by party wall surveyor



Project 56 Hawtrey Road, NW3 3SS

General Arrangement Plans Proposed Level 1-Roof

1:50 @ A1, 1:100 @ A3 30 04 2015

PLANNING

Anna Williamson ARCHITECTS

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# **APPENDIX B**

**Local Borehole Log of TQ28SE1769** 

Hydrogeological Enquiries - Swiss Cottage Pump Test - Rosemary Fry ACC NO 46102. From: sijenkins@btconnect.com <hydroeng@bgs.ac.uk> To: Date: Tue, Aug 15, 2006 5:34 pm

Subject:

TW28/209

Rosemary

Please find attached the pump test data for Swiss Cottage. Not guite a dry hole but close. Regards

Swiss Cottage Pump Test - Rosemary Fry

Page 1

Steve Jenkins Contracts Manager Drilcorn Ltd T 0191 5273970 F 0191 5273115 M 07743 806302

yield less than I may hour during pump to recovery slow. Bosende in use but will read modifications.

1 m3/hour during pump test.

SWISS COTTAGE OPEN SPACE

28 /209

Owner	L B	CAMDEN		Licence No		Nat Grid Ref TQ 26	8 843	
Occupier			46	IGS Ref No		Status L.IC		
Ground Level		56	m OD		ft OD			
Level of Well To	op itish Geolog	ical Survey	m OD	British C	Seological Sift OD	Aquifer UPPER (	HALK	nny
Rest Water Lev	el	90	m bwt		ft bwt	Summary of Geological Section	Thickness	Depth
(Date 02/11/0	14)		m OD		ft OD	MADE GEOUND	0.5	0.5
Construction	25/06	104				TOP DIL	0.7	1.2
Depth	Denth	Linings (below	Linings (below well top)			LUMDON (LAY	83.0	84.2
bwt m	Diameter M M		To m	Diameter	Type	N. R. B.	12.0	96.2
BOsh Geologial St	250	0	117 <sup>British</sup>		SOLIO STEEL	THANNET SAND	4.2	100 4
9-117	244	0	157	113/103	UPVC	PUTTY CHALK	11.6	112.0
117-157	150			1		UPPER CHOCK	47.0	159.0
				+				
Abstraction Rat			Type of Pump					
	ph Geolog	ical Survey	Chem/Bact A	nal British C	YES NO		ritish Geological Sur	rey
g	pd		Well Driller	DRILL CORP				
10.0	3 8							

If insufficient space has been allowed, continue in 'Notes' overleaf

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British Geological S
- 4 JUL 2005

ENVIRON AGENCY

# **APPENDIX C**

**Hawtrey Road Ground Investigation Report** 

# FOINT DRILLING SERVICES

Birdsnest, Oakmead Road, St Osyth, Essex. CO16 8NL. Email: ylowther@aolcom

SITE INVESTIGATION TRI	AL PIT LOG	DATE: 14.3.15		
SITE ADDRESS: 56	S HAWTERY RD. CAN	10EN/SWISS COTTAGE.		
TRIALPIT LOG REF:	PLAN.	DESCRIPTION:		
1				
ADJ				
ROPERY	PROPERTY.	PROPER		
AND DESCRIPTION OF THE PARTY OF	TO DOOR.			
1				
	BHI	Bu1 2.0m		
	5.0 m	2.0m		
	REAR GA	ARDEN		
5.7m	(PATIT			
	847			
	BH2 5.0m			
V				
<	5.8m	7		
	(WALK WAY)	7		
	LARGE TREE POSS PLANE? V.VANE TEST.			

# POINT DRILLING SERVICES

BIRDSNEST, OAKMEAD ROAD ST OSYTH ESSEX CO16 8NL EMAIL: YLOWTHER@AOL.COM

TELEPHONE: 01255 821004 MOBILE: 07771 533 402

SITE	INVES	TIGATION	<b>BOREHOLE</b>	LOG

SITE ADDRESS:

56 HAWTERY RD. CAMDEN/SWISS COTTAGE.

DEPTH	(H/A TO 2.5m) DESCRIPTION 6" BH	SOIL/ROOT SAMPLE	TEST RESULT	DEPTH OF TEST
0	SLAB OVER SOFT, SAND.			
100mm	SOFT, DARK BROWN CLAYEY TOPSOIL.			
400	SOFT, DARK ORANGE BROWN, SILTY CLAY FILL WITH OCC BRICK & CONCAST PIECES.	5		
600	SOFT, ORANGE/BROWN, SUGHTLY SILTY CLAY FILL.			
1.5	SOFT, (LOOSE) TO AUGER DARK BROWN TOPSOILY SINTY CLAY FILL WITH OCC F, BRUCK COKE FRAGMENTS. (ROOTS SEEN to 2.0m O.L.A)	,		
2150	STIFF, MIDTSROWN GREY VEN SUGHTLY SILTY CLAY WITH OCC BRANGE SILT PARTINGS. (LONDON CLAY)			
3.5	AS ABOVE (VERY STIFF)			
5.0	BH END DRY LOPEN.  BU FINISH 11.30 AM  CHECKED 11.50 AM (DRY)  CHECKED 6.30 PM (DRY)			

KEY:

DISTURBED SAMPLE

V: VANE TEST.

MP: MACINTOSH PROBE TEST

R:ROOT SAMPLE

# POINT DRILLING SERVICES

BIRDSNEST, OAKMEAD ROAD ST OSYTH ESSEX CO16 8NL EMAIL: YLOWTHER@AOL.COM

TELEPHONE: 01255 821004 MOBILE: 07771 533 402

SITE INVESTIGATION BOREHOLE LOG

DATE:

14.3.15

SITE ADDRESS:

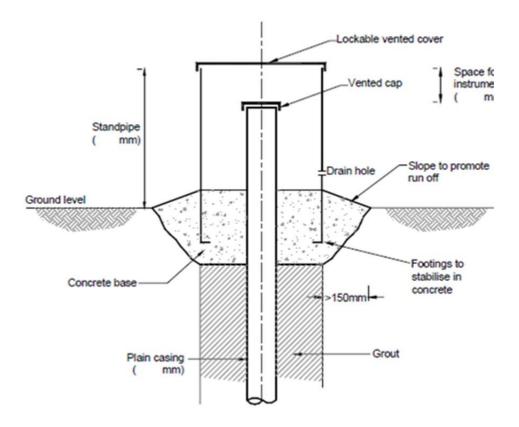
56 HAWTERY RD. CAMDEN SWISS COTTAGE.

DEPTH	(HIA to 2:0m) DESCRIPTION 6" BH	SOIL/ROOT SAMPLE	TEST RESULT	DEPTH OF TEST
0	SLAB OVER SOFT SAND			
100mm	SOFT, DARK BROWN, CLAYEY TOPSOIL			
350	SOFT DARK ORANGE BROWN SILTY CLAY WITH OCC BRICK & CONCRETE PIECES.			
500	SOFT, MID BROWN SUGHTLY SLUTY CLAY FILL WITH OCC BRICK, a LARGE COKE FRAGMENTS.			
1.4	FIRM MID BROWN GREY VEIN SUGHTLY SLITY CLAY WITH OCC OLANGE SLIT PARTINGS. (ROOTS SEEN to 2.0m O.L.A.)			2
2.0	AS ABOVE (STIFF)			
3.5	AS ABOVE (VERY STIFF)			
5.0	BH END DRY LOPEN			
	BH FINISH 3.00 PM (DRY) CHECKED 3.20 PM (DRY) CHECKED 6.30 PM (DRY)		*	
	BH 3 6"			
0	AS BH2 (ADJACENT)			
2.0	BH END (DRYLOPEN)			
	BH FINISH 5.10 PM CHECKED 5.30 PM (DRY) CHECKED 6.30 PM (DRY)			
FY ·	DISTURBED SAMPLE V- VANE TEST MP	· MACINITOSH P	DODE TEST	POOT SAMDLE

KEY:

DISTURBED SAMPLE V: VANE TEST. MP: MACINTOSH PROBE TEST R:ROOT SAMPLE

### Recommended Borehole Design



# **APPENDIX D**

**Groundwater Levels** 

### Groundwater levels (dip measurements)

Location	By House	End of Garden	By House	
Borehole name	BH1	BH2	BH3 (Shallow)	Comment
Depths (mBGL)	5	5	2	Depth to base
14/03/2015	dry	dry	dry	Drilling date
27/03/2015	2.3	2.3	dry	
30/03/2015	2	1.65	1.95	Maximum recorded levels
02/04/2015	2.1	1.7	' dry	