# 14 Netherhall Gardens

Below Ground Works

Ref No: 23959 Date: December 2014



















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# 1.0 Summary

Fluid Structures are the Structural Engineer for the 14 Netherhall Gardens Project.

14 Netherhall Gardens consists of two structures which will be joined by a common basement. The first structure (block 01) is an existing townhouse period property which will be internally refurbished. Currently, the timber floors and roof are supported on masonry walls. The external walls and façade will be retained whilst the internal structure will be replaced by a steel frame with concrete floor slabs on metal trough decking.

The second structure (block 02) will be a new four storey apartment building. The main structural frame of the building is reinforced concrete columns supporting two way spanning RC flat slabs. The exterior wall will be of cavity construction with concrete blockwork forming the internal leaf and a brickwork outer leaf. The roof shall be supported by steel framework. The RC columns supporting the 1<sup>st</sup> to 3<sup>rd</sup> floors are not at the same plan location as the columns supporting the ground floor slab, therefore the ground floor slab has been designed to transfer loads from the superstructure to RC columns in the basement.

A new basement will be formed beneath both structures. The basement beneath the new four storey block will include a contiguous piled wall around the perimeter of the basement.

Due to the presence of a nearby network rail tunnel, the new basement under the existing building will be constructed by underpinning the existing walls in a traditional "hit and miss" sequence. Internal columns will be supported on pad foundations located under the new basement slab. The existing internal walls will be resupported using a system of needles and beams bearing onto the new columns. The existing party/boundary walls are also to be retained and will be underpinned prior to the construction of the RC retaining wall and contiguous pile wall.

The new apartment block shall bear onto piled foundations, which shall be located to adequately spread the load of the structure being carried by the columns onto a 600mm thick basement transfer slab. The existing building shall have a ground bearing basement slab with pad footings provided under the load bearing walls and columns.

Any piles that could potentially increase the load on the network rail tunnel (allowing for a 45° cone of influence from the tunnel roof) will be sleeved to ensure there shall be no increase in the load on the tunnel due to the piled foundations.

There shall be vehicular access to the basement via a ramp.

The works are summarised as follows:

- Demolition of existing building on the site of block 02.
- Removal of the non-structural interior of block 01.
- Underpinning and temporary supporting of existing walls.
- Piling, excavation and construction of the basement.
- Floor by floor construction of the replacement steel frame and composite decking in block 01.
- Floor by floor construction of the RC frame and RC slabs of block 02.
- Refurbishment / replacement of the roof in Block 01 and construction of the steel frame roof in block 02.

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• Other works including the facades, ramp, retaining walls, underpinning party walls etc.



## 2.0 Outline Construction Sequence

- 1) Install Underpinning in 'hit and miss' sequence.
- 2) Remove floorboards and joists to ground floor level and install needles and beams each side of walls to allow walls to be supported by beams and columns. Install temporary works where required to allow installation and support beams and needles with temporary works to suit step 3 below.
- Remove hydraulic lift and equipment and demolish existing liftshaft from top down, providing temporary support where required at each level.
- Install internal basement columns, pad foundations and liftshaft / pit. Temporarily prop locally as required. Note that pad foundations will be incorporated into basement slab so starter bars will need to be left exposed.
- 5) Connect beams and needles to columns and underpinning and jack loads onto beams.
- 6) Once jacking is complete excavate to form basement and complete basement slab.
- 7) Install remaining ground floor beams and decking and pour ground floor slab.
- 8) Erect temporary works from ground floor to allow new first floor beams to be installed.
- Locally remove sections of first floor to allow installation of new first floor beams and padstones along with perimeter angles.
- 10) Once new beams are installed and tied to the walls, remove existing timber floor and install metal deck and pour first floor slab.
- 11) Once the first floor slab has cured, install temporary works to allow access to second floor and locally remove sections of second floor to allow installation of second floor beams.
- 12) Install Second floor beams and padstones.
- 13) Once the beams are installed and tied to the walls, remove existing timber floor and install metal deck and pour floor slab.
- 14) Erect external scaffold and strip existing roof where required.
- 15) Once floor slab has cured, install temporary works to allow access for the third floor steelwork installation.
- 16) Install third floor steel beams and padstones.
- 17) Install metal deck and pour concrete slab.
- Once the slab has cured, install temporary works to allow access for roof steelwork installation and temporary support.
- 19) Install roof steelwork and alter / re-support timber roof.

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# 3.0 Ground Water Impact

The ground investigation and basement impact assessment carried out by GEA determined that there should be no impact to the current water table. See Appendix D.

(J12015 – 9.3)

# 4.0 Loading on Belsize Tunnel

Calculations conducted to assess the effect of the increased load on the Belsize tunnel due to the proposed works have determined that the additional stress experienced by the tunnel will be negligible (~ 1% increase).

See Appendix C.

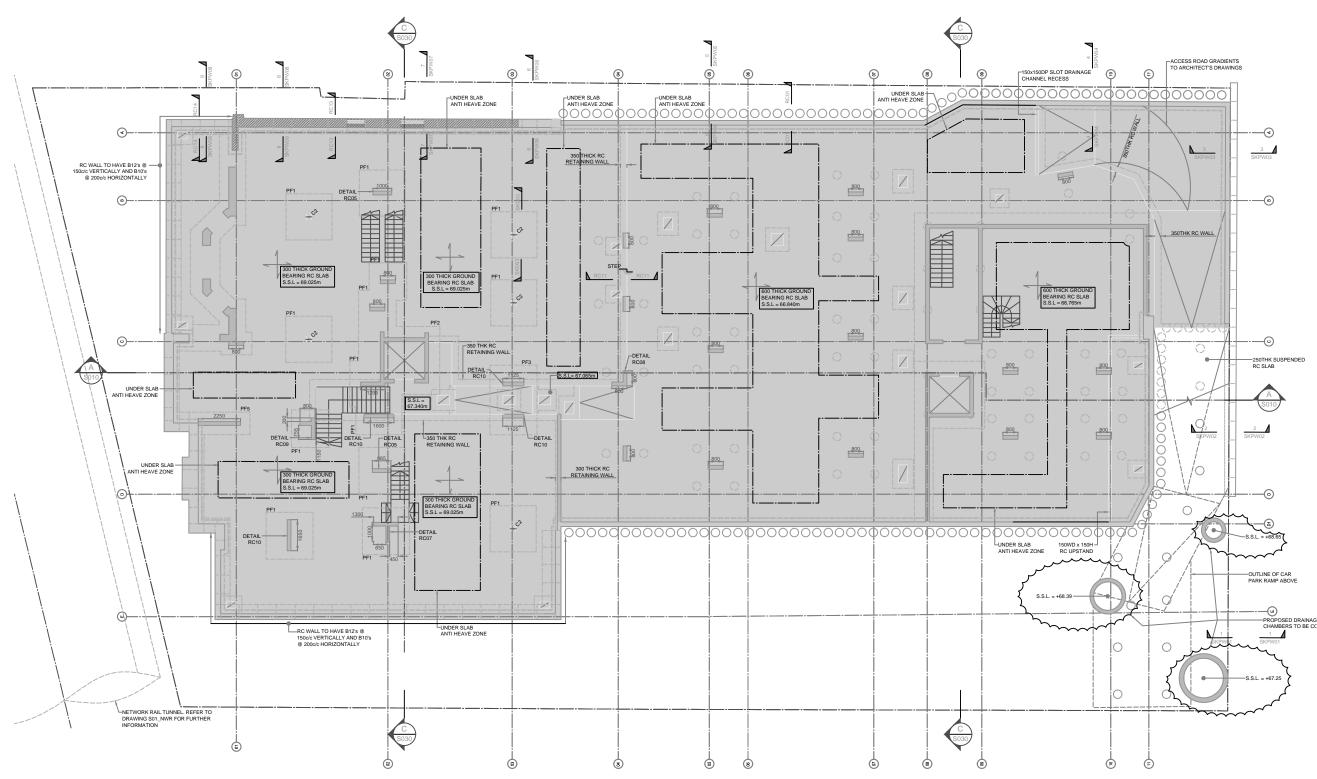
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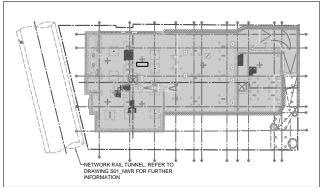


Appendix A

Structural Drawings & Sketches

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NETWORK RAIL TUNNEL LOCATION PLAN SCALE 1:500

#### NOTES:

. DRAWING IS TO BE READ IN CONJUNCTION WITH THE GENERAL NOTES

1. DRAWING IS TO BE READ IN CONJUNCTION WITH THE GENERAL NOTE DRAWING GNOT SCALE FROM THIS DRAWING.
 2. DO NOT SCALE FROM THIS DRAWING.
 3. ALL DIMENSIONS ARE IN mm U.N.O.
 4. DRAWING IS TO BE READ IN CONJUNEER, STRUCTURAL ENGINEER, TRUCTURAL ENGINEER AND THE ADDITION WITH ALL RELEVANT DRAW 4. DRAWING IS TO BE READ IN CONJUNEER, STRUCTURAL ENGINEER, TRUNSPORT ENGINEER AND THANES WATER LIMITED.
 5. ALL LEVELS ARE TO THE ARCHITECT'S DRAWINGS 6. SITE WELDING WILL NOT BE PERMITTED UNDER ANY CIRCUMSTANCE WITHOUT PRIOR WRITTEN AJTHORISATION FROM THE ENGINEER. 7. ALL CONCRETE IS TO BE GRADE C40.
 8. MIN LAP OF REINFORCING BARS IS 40xbard.
 9. UBARS TO ALL SLAB EDGES, COLUMN EDGES, WALL AND COLUMN INTERSECTIONS, EDGES AND TOPS.

9. ULANS TO ALL SLAB EDGES, COLUMN EDGES, WALL AND COLUMN INTERSECTIONS. EDGES AND TOPS. 10. RC ALL PERIMITIES UNING WALLS ARE 250mm THICK U.N.O. ALL PERIMITIES UNING WALLS ARE 200mm THICK X 600mm LONG U.N.O. ALL RITERNAL FOLUMNS ARE 200mm THICK X 600mm LONG U.N.O. ALL INTERNAL FOLUMNS ARE 200mm THICK X 600mm LONG U.N.O. 11. MINIMUM COVER TO REINFORCEMENT: 200mm INTERNAL FACE 400mm EXTERNAL FACE 400mm EXTERNAL FACE 400mm EXTERNAL FACE 400mm EXTERNAL FACE 400m EXTERNAL 400m EXTERNAL FACE 400m EXTERNAL FACE

VALLS MULST BE TED AT HEAD LEVEL AS SHOWN ON DRAWINGS No. SKBW01 TO SKB203. 17. ALL FINISHES, WEATHERING, WATERPROOF MEMBRANES, DAMP PROOF COURSES AND FREPROOFING ARE TO ARCHITECTS DETAILS. 18. THE CONSTRUCTION SEQUENCE. INDICATED ON DRAWINGS IS AN OUTLINE SEQUENCE AND THE CONTRACTOR IS RESPONSIBLE FOR PREARING AND EXECUTING A DETAILED SEQUENCE WITH RELEVANT TEMPORARY WORKS CALCULATIONS AND DRAWINGS DESIGNED SUITABILITY QUALIFIED AND EXPERIENCED TEMPORARY WORKS ENGINEER. IN. THE CONTRACTOR WILL BE RESPONSIBLE FOR PREAS IN. THE CONTRACTOR WILL BE RESPONSIBLE FOR PREAS NECESSARY APPROVALS FROM NETWORK RAIL TO ALLOW WORKS ON SITE TO PROOFED.

20. THE TOP OF PILES WITHIN THE NETWORK RAIL INFLUENCE ZONE ARE TO BE SLEEVED SO THAT THE PILES DO NOT SURCHARGE THE TUNNEL REFER T

SLEEVED SO THAT THE PILES DO NOT SURGINARSE THE TONNEE RELEASE AWINGS S01\_WWR. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL STEEL TO STEEL INNECTION DESIGN AND REINFORCEMENT DETAIL DRAWINGS.



COLUMN SCHEDULE: - ALL STEELWORK IS TO BE GRADE \$355

# C1: 100x100x10 SHS C2: 254x254x132 UC C3: 152x152x37 UC C4: 203x203x46 UC

### PADSTONES SCHEDULE:

 P1:
 450Lx225Dx225W

 P1a:
 450Lx225Dx300W

 P2:
 900Lx450Dx225W

 P3:
 1125Lx450Dx250W

 P3:
 1125Lx450Dx205W

 P3:
 1125Lx450Dx25W

 P3:
 1125Lx450Dx205W

#### PADFOOTING SCHEDULE:

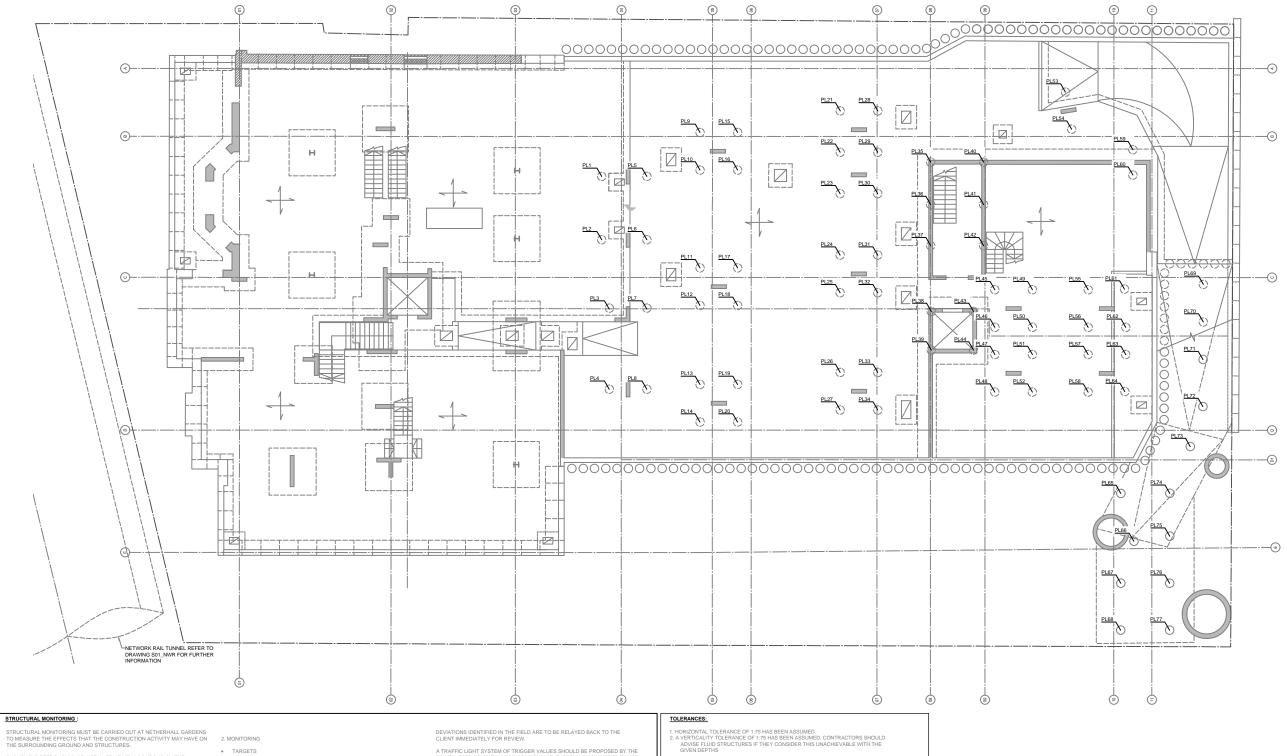
ALL PADFOOTINGS ARE 600DP AND REINFORCED WITH B10'S TOP AND B16'S @ 200c/c BOTTOM IN EACH DIRECTION.

- PF1: 2500x2500 PF2: 3600x3600 PF3: 3700x2500 PF4: 1200x3200

#### BASEMENT NOTES:

1. ALL FOUNDATION DETAILS ARE SUBJECT TO REVIEW UPON RECEIPT OF THI SITE INVESTIGATION REPORT. 2. BASEMENT SLAB IS TO BE CAST ON 75mm THICK CONCRETE BLINDING. 3. ANTI HEAVE ZONES ARE TO BE CORDEX CELLCORE HX S 160 1824 OF 8. MILAR APPROVED. 4. ALL SUMPS ARE TO HAVE 300 THICK RC WALLS. FOR LOCATIONS AND DIMENSIONS PLACER REFER TO DRAINAGE ENCINEERS DRAWINGS.

BASEMENT REINFORCEMENT NOTES:	STAGE E REPORT ISSUE				
1. ALL RC UNDERPINNING FOUNDATIONS AND WALLS ARE TO BE REINFORCED WITH BIOS @ 2006/VERTCALLY AND HORZONTALLY IN EACH FACE UNO.     2. FOR ALL COLUMN AND WALL REINFORCEMENT DETAILS REFER TO DETAIL     DRAWING RCOLT TO RCI3E.     3. FOR ALL SLAB REINFORCEMENT DETAIL REFER TO DRAWING RC_P090.	T1 ISSUED FOR STAGE E         DRN: SA [CHK: SB   01/12/14				
TRITON WATERPPROOF RC NOTES:					
1. ALL CONCRETE EXCLUDING PILES TO BE CONSTRUCTED STRICTLY IN ACCORDANCE WITH TRITON WATERPROOFING SYSTEM SPECIFICATIONS. 2. ALL WATERPROOF CONCRETE TO BE FITTION TT SUPER ADMIX TO TRITON SPECIFICATIONS WITH THE FOLLOWING MIX:	Critical, diffugued entry of				
TRITON TT SUPER MIX: ALKALI CONTENT < 0.1% CHLORIDE (ON < 0.05% 350kg/m² CeMI MIN CEMENT CONTENT MAX WATER CONTENT RATIO = 0.5 NO GROUND GRANULATED BLAST FURNACE SLAB PERMITTED.	FLUID. STRUCTURES ENGINEERS & TECHNICAL DESIGNERS Second Floor 21 St. Georges Road London Telephone: 020 78207				
3. WATERPROOF CONCRETE ELEMENTS TO INCLUDE PAD FOUNDATIONS, LIFT PIT, SUMPS, EXTERNAL GROUND FLOOR SLAB, RETAINING WALLS AND RC UNDERPINNING 4. ALL WATERPROOF CONCRETE CONSTRUCTION INCLUDING JOINTS, SERVICE PENETRATIONS, ETC TO BE IN ACCORDANCE WITH TRITON SPECIFICATIONS.	Fox: 020 7582784₿ E−mail: [firstname]@fluidstructures.com Web: www.fluidstructures.com				
WATERPROOF RC KEY:					
DENOTES EXTENTS OF WATERPROOF CONCRETE TO	Project: 14 NETHERHALL GARDENS				
ACHIEVE GRADE 3 BASEMENT DESIGNED IN ACCORDANCE WITH BS 8102. SUGGESTED B+C PROTECTION.	Title: BASEMENT PLAN				
DENOTES WALL FOR GRADE 3 BASEMENT DESIGNED IN ACCORDANCE WITH BS 8102, SUGGESTED B+C PROTECTION.	Job No: 23959 DRN: IRS Date: SEP'14				
	Scale: 1:100 @A1; 1:200@A3 CHK: SB DRG. No.: P090 T1				



PILE LENGTH, CUT OFF AND TOE LEVEL:

. INDICATIVE PILE LENGTHS AS NOTED ON DRAWING. ACTUAL PILE LENGTH TO PILE DESIGNERS

SPECIFICATION. 2. PILE CUT-OFF LEVEL: FOR SECANT PILED WALL TOE LEVEL TO SUIT LATERAL STABILITY AND WATER TABLE RECUIREMENTS. 3. PILES UNDER RAFT, CUT-OFF LEVEL GENERALLY 40MM ABOVE RAFT SLAB FORMATION LEVEL -

DIES UNDER KAFT, CUT-OFF LEVEL GENERALLY 40MM ABOVE RAFT SLAB FO SEE DRAWINGS. I. DEPTH OF PILE TOE BENEATH SITE DATUM (MINIMUM): REFER TO DRAWINGS O THER REQUIREMENTS:

EYTIN UP HEE YOL BENEATH SITE DATUM (MINIMUM): REFER TO DR HIER REQUIREMENTS:
 PILE LENGTH TO BE SUITABLE TO PROVIDE SUFFICIENT LONGTERM SHEAR. RESISTANCE:
 SHEAR LOADS: - DESIGN FOR A MINIMUM OF 250KN UNFACTORED SHEAR LOAD UNLESS NOTED OTHERWISE ON UNFACTORED SHEAR LOAD UNLESS NOTED OTHERWISE ON SHEAR DUE TO SOIL, SURGHARGE PRESSURES, ETC. LATERAL DEFLECTION OF PERIMETER PILED WALLS NOT TO EXCEED LIMITS SHOWN ON DRAWINGS DURING EXCAVATION OR CONSTRUCTION OF BASEMENT. PILED WALLS TO BE LATERALLY PROPPED DURING EXCAVATION TO CONTROL LATERAL DEFLECTION. JACKING POINTS TO BE PROVIDED BETWEEN PROPS AND PILED WALLS TO ALLOW DEFLECTION TO BE CONTROLLED IF NECESSARY.

SURVEY TARGETS SHOULD BE INSTALLED AT KEY LOCATIONS ON THE PROPOSED PILED WALLS AS INDICATED ON FLUID DRAWING PLOSD. THEY SHOLLD BE SURVEYED IN THE -SOMENSIONAL PLANE WHICH WILL PROVIDE DISPLACEMENT MEASUREMENTS IN THE HORIZONTAL AND VERTICAL PLANE TO DETERMINE THE DEFLECTION OF THE WALL DURING EXCAVATION AND BASEMENT CONSTRUCTION.

ALL MOVEMENT MONITORING MUST BE CHECKED REGULARLY AGAINST THE PILE DESIGN BY THE CONTRACTOR TO ENSURE THAT THE CONSTRUCTION WORKS DO NOT HAVE AN ADVERSE EFFECT ON THE SURROUNDING BUILDING 1. INSTALLATION

CONTROL

CONTROL STATIONS ARE TO BE ESTABLISHED AT STRATEGIC LOCATIONS, FROM WHICH THE MONITORING POINTS ARE TO BE SURVEYED. THE COORDINATE SYSTEM MUST BE SPECIFIC TO THE MONITORING.

RETRO TARGETS FOR CONTROL SHOULD BE PLACED ON SURROUNDING STRUCTURES OUTSIDE OF THE SITE'S ZONE OF INFLUENCE, AND THESE POINTS MUST BE ACCURATELY FIXED IN THE 3-DIMENSIONAL PLANE.

REFLECTIVE TARGETS

REFLECTIVE SURVEY TARGETS SHOULD BE INSTALLED ON THE TOPS OF INDICATED PILES.

TARGETS

AT LEAST TWO ROUNDS OF ANGLES ARE TO BE SURVEYED IN BOTH FACES FOR EACH SURVEY POINT. THE DATA IS TO BE RECORDED ELECTRONICALLY DIRECTLY ONTO THE INSTRUMENT. THE INSTRUMENT SHOLLD BE CAPABLE OF DISPLAYING DIFFERENCES IN REAL-TIME. ALLOWING THE SURVEYOR TO ASSESS THE EXTENT OF ANY DEVIATION IN THE FIELD, AND IF NECESSARY, REPORT LARGE DEVIATIONS IMMEDIATELY TO THE CLIENT.

3. FREQUENCY OF READINGS

A MONITORING FREQUENCY WILL NEED TO BE AGREED BEFORE THE COMMENCEMENT OF WORKS. GENERALLY, FORTNIGHTLY TO WEEKLY MEASUREMENTS SHOLD BE CONSIDERED DURING THE STRUCTURALLY CRITICAL PHASES, SUCH AS PILING, EXCAVATION AND BASEMENT CONSTRUCTION.

MONTHLY MONITORING FOR 6 TO 12 MONTHS FOLLOWING BASEMENT CONSTRUCTION IS NORMALLY ACCEPTABLE TO ASCERTAIN IF THERE IS ANY LONG TERM MOVEMENT STILL PREVALENT.

CONTINGENCY SHOULD BE SET ASIDE TO ALLOW FOR ADDITIONAL VIS INCREASED FREQUENCIES, SHOULD TRIGGER VALUES BE EXCEEDED. VISITS AT

4 REPORTING

ON COMPLETION OF EACH CYCLE OF READINGS THE DATA SHOULD BE COMPILED IN A REPORT AND ISSUED WITHIN 24 HOURS. ANY LARGE

A TRAFFIC LIGHT SYSTEM OF TRIGGER VALUES SHOULD BE PROPOSED BY THE CONTRACTOR PRIOR TO WORK COMMENCING. THESE VALUES MUST TAKE INTO ACCOUNT THE ONUS ON THE CONTRACTOR AND HIS TEAM TO AVOID ANY MOVEMENT RESULTING IN CRACKING, SETTLEMENT OR HEAVE TO ADJACENT STRUCTURES.

EACH REPORT SHALL CONTAIN THE FOLLOWING:

- EXECUTIVE SUMMARY
- SCHEDULE OF VISITS INCLUDING: DATE OF SURVEY WEATHER CONDITIONS DURING SURVEY SURVEYOR RESPONSIBLE FOR THE SURVEY.
- APPLICABLE NOTES AND ACCURACIES

 SURVEY RESULTS SHOWING:
 TABULATED DATA HIGHLIGHTING EXCEEDING SPECIFIED TABULATED DATA HIGHLIGHTING EXCEEDING SPECIFIED TRIGGER LIMITS. GRAPHS SHOWING VERTICAL AND HORIZONTAL MOVEMENTS WITH TIME

LOCATION PLANS

PILE SCHEDULE							
PILE REF	NOTES						
PI 1	LOADING (KN) 1000	CUT OFF LEVEL 68.805	NOTES				
PL2	900	68.805					
PL3	900	68.805					
PL4	750	66.140					
PL5	1000	66.140					
PL6	900	66.140					
PL7	900	66.140					
PL8 PL9	750	66.140					
PL9 PL10	900	66.140 66.140					
PL11	1000	66.140					
PL12	1000	66.140					
PL13	800	66.140					
PL14	800	66.140					
PL15	900	66.140					
PL16	900	66.140					
PL17	1000	66.140					
PL18 PL19	1000 800	66.140					
PL19 PL20	800	66.140 66.140					
PL20 PL21	700	66.140					
PL22	700	66.140					
PL23	700	66.140					
PL24	900	66.140					
PL25	900	66.140					
PL26	1100	66.140					
PL27	1100	66.140					
PL28 PL29	700	66.140 66.140					
PL29 PL30	700	66.140					
PL31	900	66.140					
PL32	900	66.140					
PL33	1100	66.140					
PL34	1100	66.140					
PL35	1500	66.140					
PL36	1500	66.140					
PL37	1000	66.140					
PL38 PL39	1000	66.140					
PL39 PL40	1000 1500	66.140 66.140					
PL40 PL41	1500	66.140					
PI 42	1000	66.140					
PL43	1000	66.140					
PL44	1000	66.140					
PL45	800	66.140					
PL46	800	66.140					
PL47	1000	66.140					
PL48	1000	66.140					
PL49 PL50	800	66.140					
PL50 PL51	1000	66.140 66.140					
PL51	1000	66.140					
PL53	2100	66.140	1				
PL54	2100	66.140					
PL55	1000	66.140					
PL56	1000	66.140					
PL57	900	66.140					
PL58	900	66.140					
PL59 PL60	1600	66.140 66.140	-				
PL60 PL61	1000	66.140					
PL61 PL62	1000	66.140					
PL63	900	66.140					
PL64	900	66.140					
PL65	100	69.167	VEHICLE RAMP				
PL66	100	69.380	VEHICLE RAMP				
PL67	100	69.565	VEHICLE RAMP				
PL68	100	69.561	VEHICLE RAMP				
PL69	150	68.239	VEHICLE RAMP				
PL70 PL71	150 100	68.406	VEHICLE RAMP				
PL/1 PL72	100	68.574 68.783	VEHICLE RAMP VEHICLE RAMP				
PL72 PL73	150	68.960	VEHICLE RAMP				
PL74	100	69.167	VEHICLE RAMP				
PL75	100	69.357	VEHICLE RAMP				
PL76	100	69.565	VEHICLE RAMP				
PL77	100	69.561	VEHICLE RAMP				

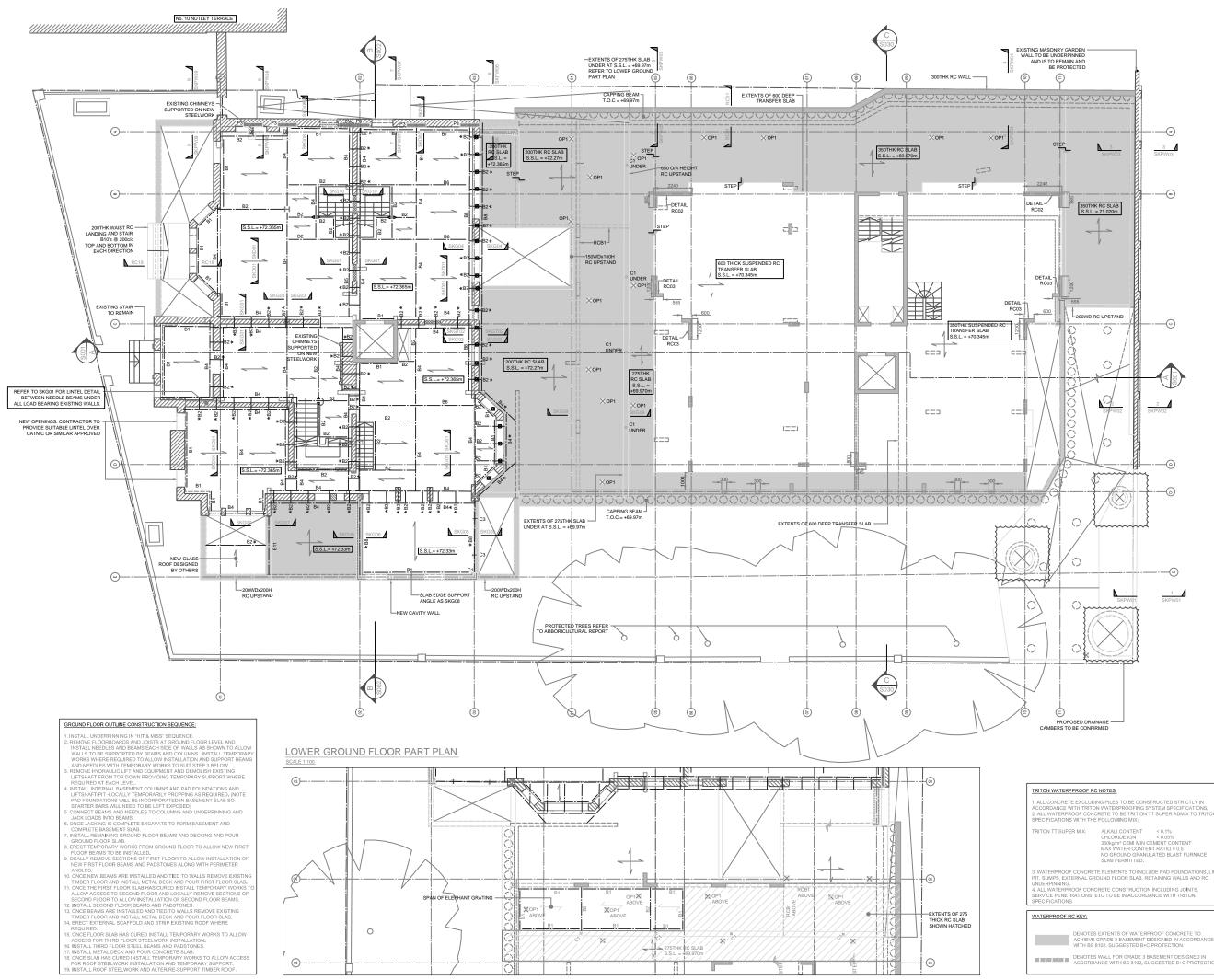
# STAGE E REPORT ISSUE

DRN: DB CHK: SB 01/12/14 T1 ISSUED FOR STAGE E

FLUID. STRUCTURES ENGINEERS & TECHNICAL DESIGNERS Second Floor 21 st. Georges Road London Telephone: 020 7820 E-moil: [firstname]@fluidstructures Web: www.fluidstructures.co

Project: 14 NETHERHALL GARDENS

Title: BASEMENT FLOOR PLAN - PILING LAYOUT						
Job No:	23959	DRN:	DB	Date:	NOV '14	
Scale:	1:100 @A1; 1:200 @A3	CHK:	SB	DRG. No.:	PL_P090	T1



#### NOTES:

DRAWING IS TO BE READ IN CONJUNCTION WITH THE GENERAL NOTES

DRAWING GNDT. I. DO NOT SCALE FROM THIS DRAWING. ALL DIMENSIONS ARE IN mm U.N.O. DRAWING IST DO BE READ IN CONJUNCTION WITH ALL RELEVANT DI Y THE ARCHITECT. SERVICES ENGINEER, STRUCTURAL ENGINEER, RANSPORT ENGINEER AND THAMES WATER LIMITED. ALL LEVELS ARE TO THE ARCHITECTS DRAWINGS S. THE WELDING WILL NOT BE PRMITTED UNDER ANY CIRCUMSTAN

SITE WELDING WILL NOT BE PERMITTED UNDER ANY CIRCUMSTANC THOUT PRIOR WITTEN AUTHORISATION FRADMINTE ENGINEER. ALL CONCRETE IS TO BE GRADE C40. MIN LAP OF REINFORCING BARS IS 40x8ar0. U BARS TO ALL SLAG BOGES, COLUMN EDGES, WALL AND COLUMN TERRECTIONS, EDGES AND TOPS.

J MEMBERS LIST: ALL PERIMETER LINING WALLS ARE 250mm THICK U.N.O. ALL NC WALLS ARE 200 THICK U.N.O. ALL INTERNAL COLUMNS ARE 200mm THICK x 600mm LONG U.N.O. INIUM COVER TO REINFORCEMENT:

20mm INTERNAL FACE 40mm EXTERNAL FACE L SLAB PENETRATIONS

2010/11 MI EFEVALE FAGE 2010/01/2012 STEPRALE STEPRALE AND PROVIDE A PRICE PER PENETRATION 2010/01/2012 STEPRALE STEPRALE AND PROVIDE A PRICE PER PENETRATION 2010/01/2012 STEPRALE STEPRALE STATUS 2010/01/2012 STEPRALE STEPRALE STATUS 2010/01/2012 STATUS 201

3 SKB203. ALL FINISHES. WEATHERING, WATERPROOF MEMBRANES, DAMP PROOF JURSES AND FIREPROOFING ARE TO ARCHITECT'S DETAILS. THE CONSTRUCTION SEQUENCE INDICATE ON DRAWINGS IS AN OUTLINE QUENCE AND THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND IECULATIONS AND DRAWINGS DESIGNED SUITABILITY QUALIFIED AND IECUCATOR WILL BE RESPONSIBLE FOR OBTINING ALL CESSARY APPROVALS FROM NETWORK RAIL TO ALLOW WORKS ON SITE TO ROCEED.

PROCEED. 20. THE TOP OF PILES WITHIN THE NETWORK RAIL INFLUENCE ZONE ARE TO BE SLEEVED SO THAT THE PILES DO NOT SURCHARGE THE TUNNEL REFER T DRAWINGS 501 NWR. 25. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL STEEL TO STEEL CONNECTION DESIGN AND REINFORCEMENT DETAIL DRAWINGS.



BEAM SCHEDULE:

STEEL BEAMS - ALL STEELWORK IS TO BE GRADE \$355

ALL STEEL BEAMS ARE SLIMFLOR U.N.O. REFER TO DETAIL DRAWINGS SKD0 TO SKD07 & SKG03 FOR FURTHER INFORMATION

# ALL STEEL TO STEEL CONNECTIONS ARE TO CATER FOR AN UNFACTORED REACTION OF 250kN

\* DENOTES STANDARD STEEL BEAM

B6:254x254x107 UC B7: 254x254x132 UC B8: 250x150x10 RHS BR: 100x100x10 FLAT PLATE BRACING 
 B1:
 200x100x10 EA

 B2:
 152x152x37 UC

 B3:
 203x203x46 UC

 B4:
 254x254x73 UC

 B5:
 254x254x89 UC

#### RC BEAMS

RCB1: 200WDx900 O/A DEPTH DOWNSTANDING RC BEAM - SKG09 RCB2: 600WDx400 O/A DEPTH DOWNSTANDING RC BEAM - RC19

COLUMN SCHEDULE: - ALL STEELWORK IS TO BE GRADE \$355

# C1: 100x100x10 SHS C2: 254x254x132 UC C3: 152x152x37 UC C4: 203x203x46 UC

# PADSTONES SCHEDULE:

P1: 450Lx225Dx225W P1a: 450Lx225Dx300W P2: 900Lx450Dx225W P3: 1125Lx450Dx250W P3: 1125Lx450Dx250W P3a: 1125Lx450Dx300W P3b: 1125Lx450Dx450W

### GROUND FLOOR NOTES:

1. MINIMUM REINFORCEMENT FOR ALL CONCRETE ELEMENTS IS TO BE B10's ( 200c/c IN EACH FACE AND EACH DIRECTION U.N.O. 2. REFER TO DETAIL DRAWINGS RC01 TO RC19 FOR FURTHER DETAILS.

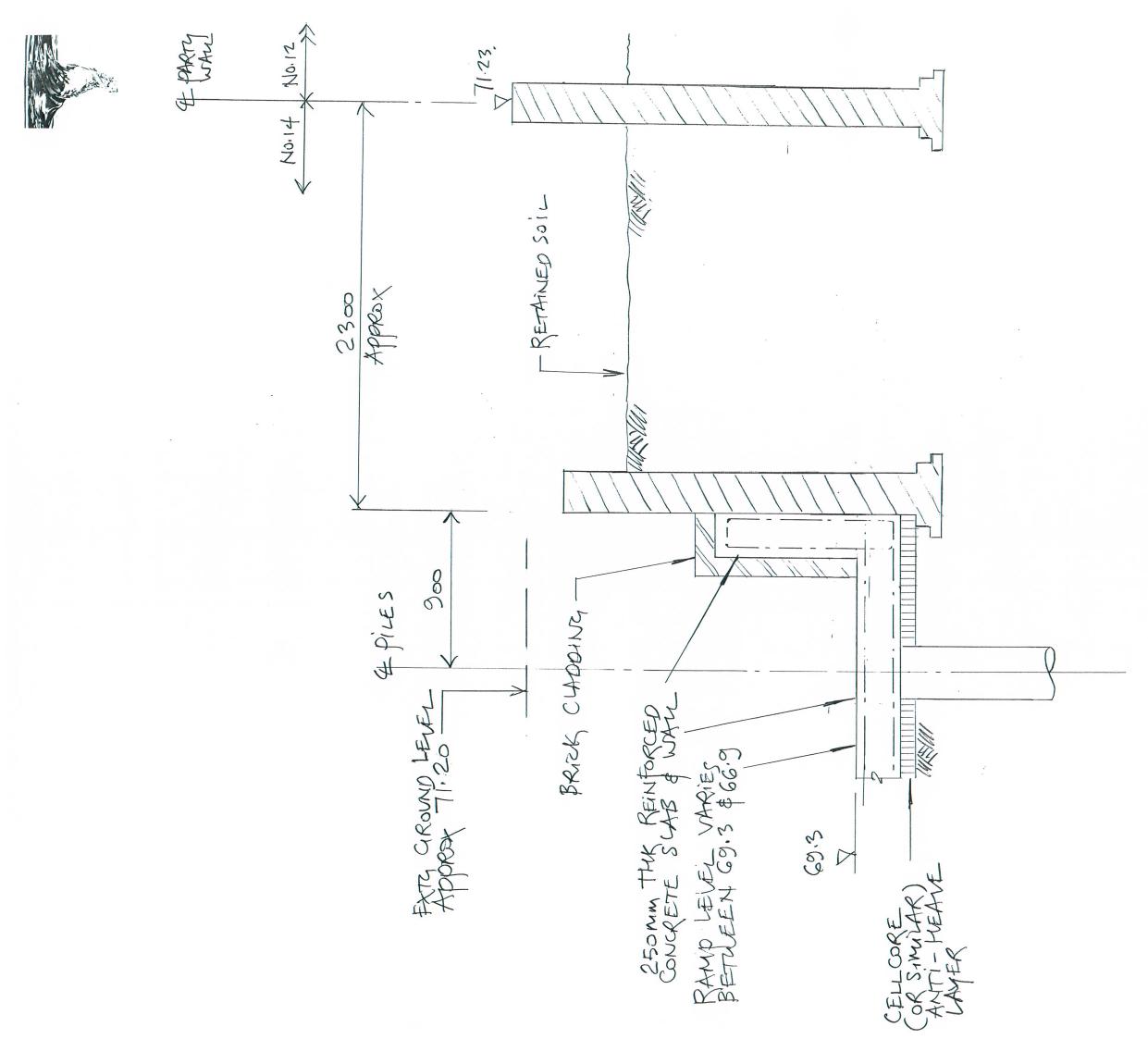
# STAGE E REPORT ISSUE

T1 ISSUED FOR STAGE E DRN: SA CHK: SB 01/12/14

FLUID. STRUCTURES ENGINEERS & TECHNICAL DESIGNERS Second Floor 21 St. Georges Road London Telephone: 020 78207 Fax: 020 78287 E-mail: [firstname]@fluidstructures Web: www.fluidstructures.com

Project: 14 NETHERHALL GARDENS

Title:	GROUND FLOOR PLAN					
Job No:	23959	DRN:	SA	Date:	SEP`14	
Scale:	1:100 @A1; 1:200 @A3	CHK:	SB	DRG. No.:	P100	T1



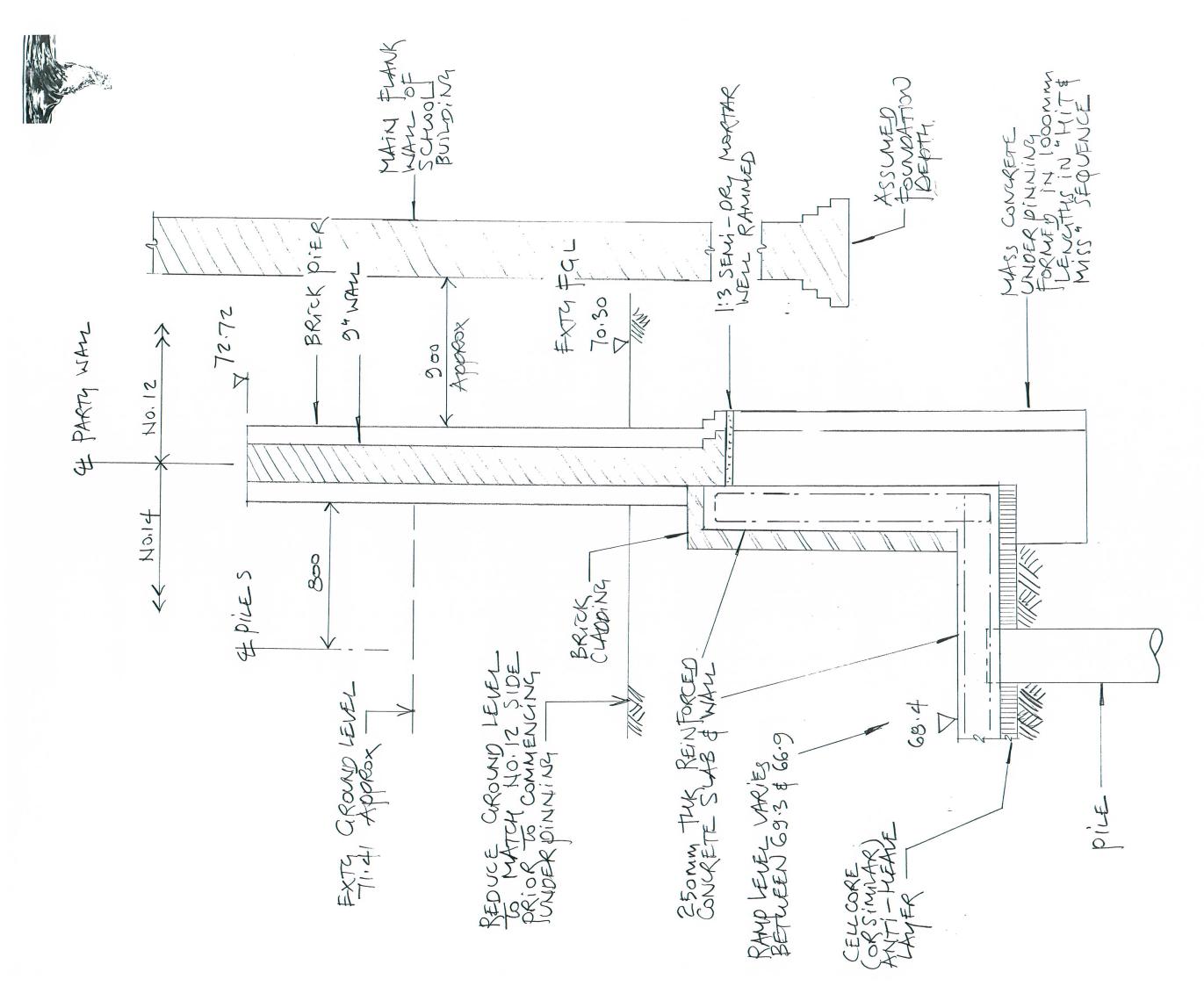
21 St George's Road London SE1 6E5 Telephone: 020 78207766 Email: [firstname]@fluidstructures.com Web: www.fluidstructures.com

ENGINEERS AND TECHNICAL DESIGNERS

FLUID.STRUCHTEE

Drn By: Chk By: SB NETHERHALL GONS ECTION 1-1 Date: 8/14 REV: Project: SECTION THE: SECTION ob No. 23959 MG No. SKPWOI

SECTION

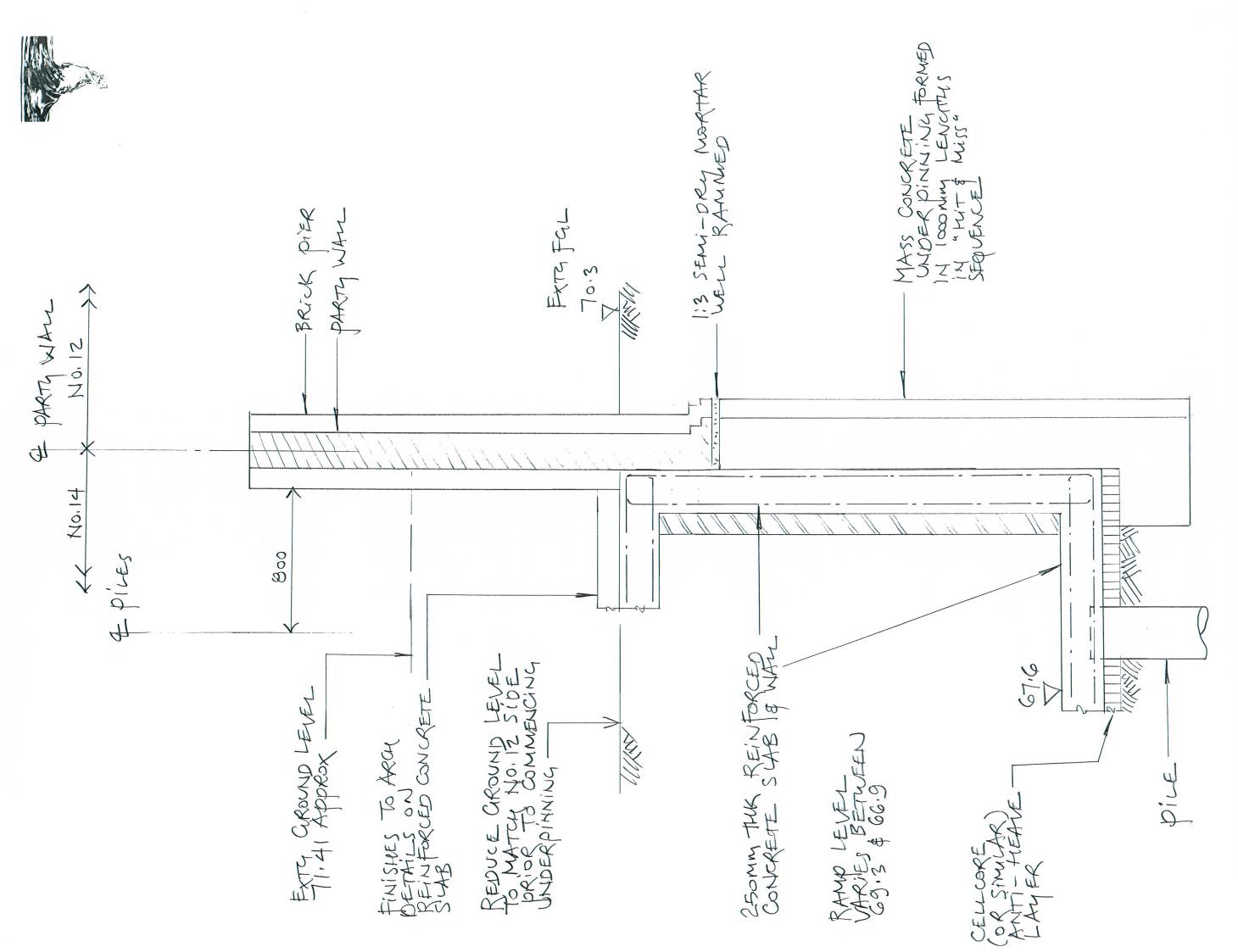


Drn By: 2 Chk By: 2B SNad Date: 0/14 REV: A NETTICN 2-2 TITLE: SECTION ob No. 23959 MG No. 58 PWOR <sup>2</sup>roject:

באסואבהא אואט ובכחואובאו באיט אואט ובכחואובאט באסטאנאט 21 St George's Road London SE1 6E5 Telephone: 220 78207766 Email: [firstname]@fluidstructures.com Web: www.fluidstructures.com

ENGINEERS AND TECHNICAL DESIGNERS

PLUD.S



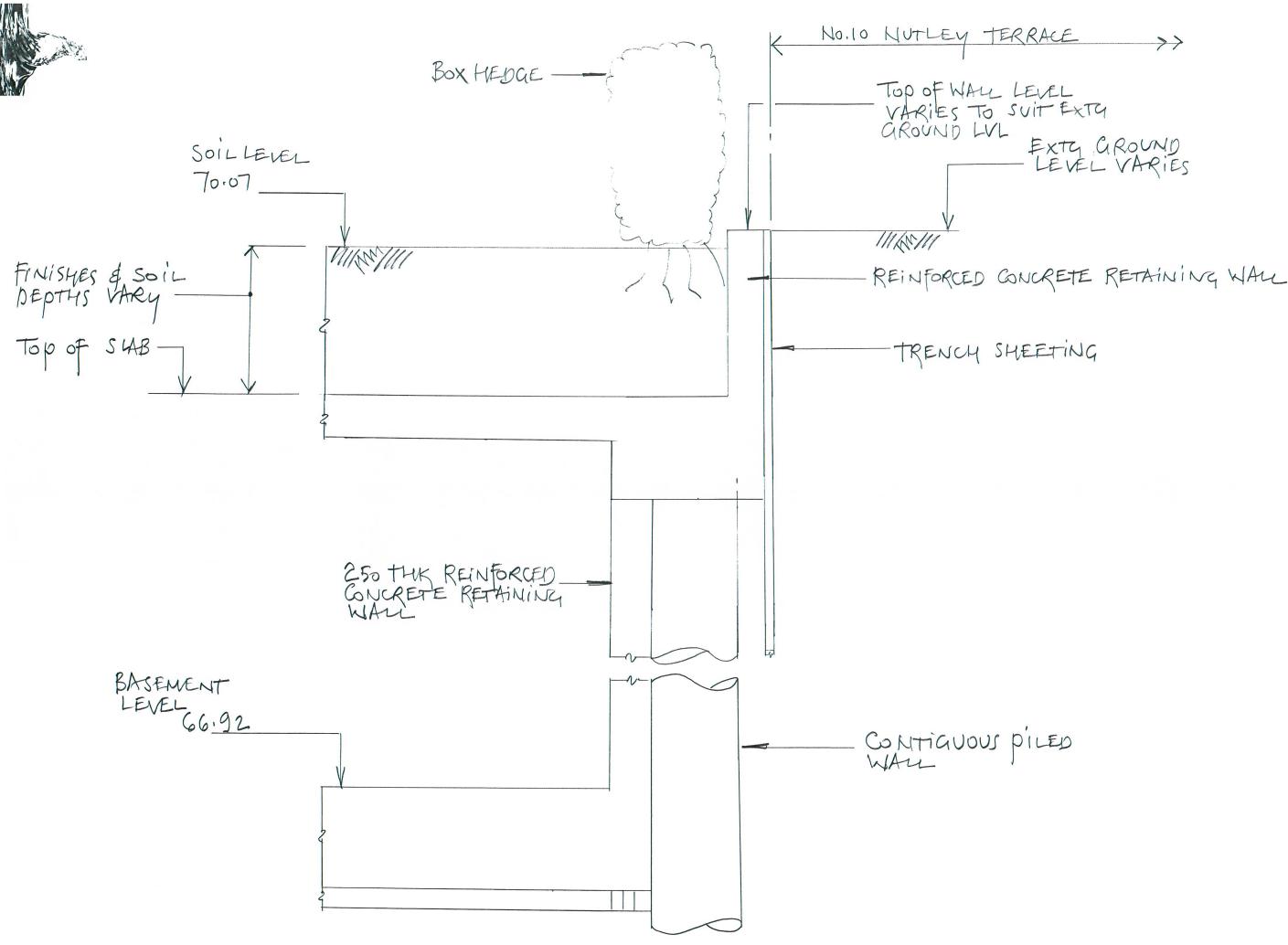
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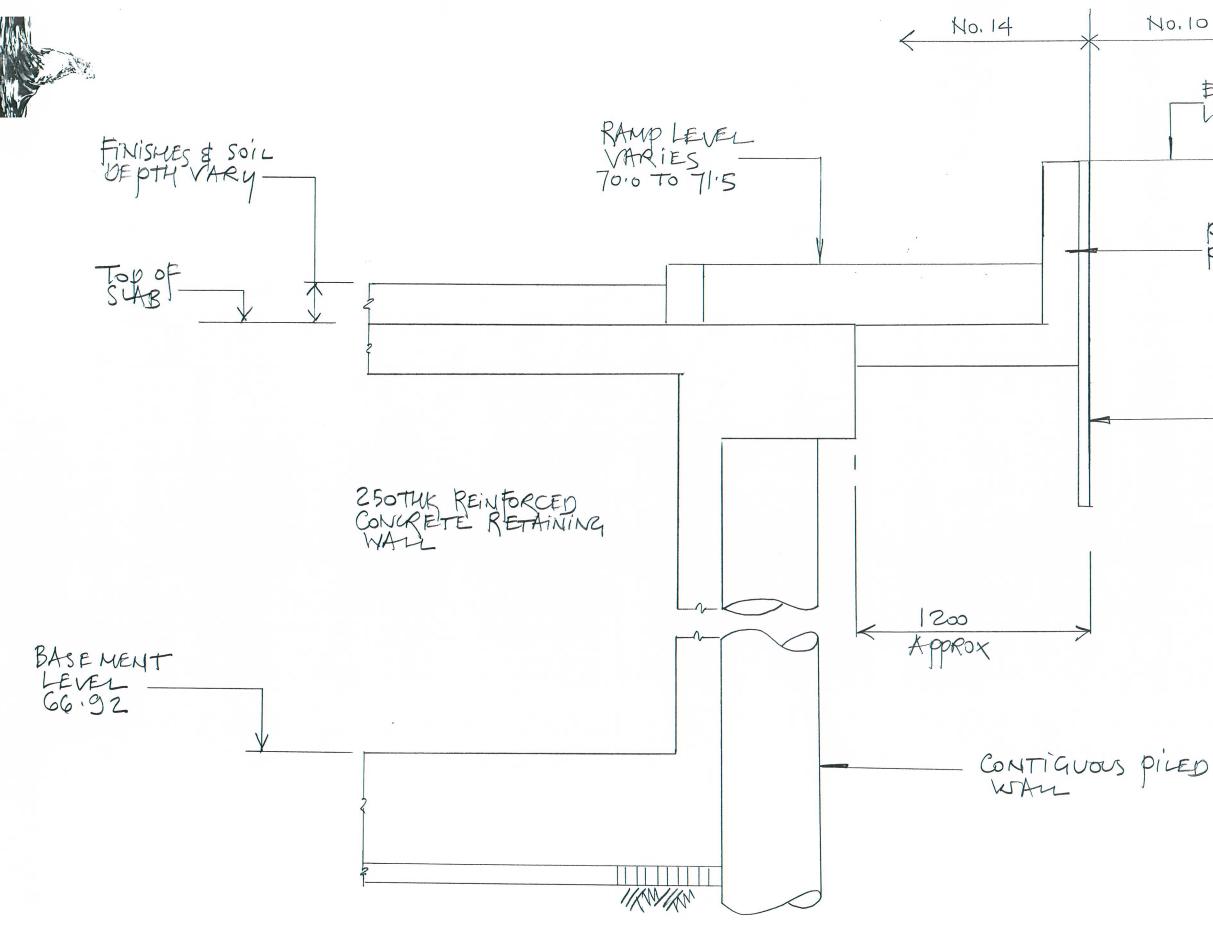


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SECTION 5-5

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TRENCH SMEETING

Drn By: H Chk By: ALL CHROENS 5-5 Date: Bliff REV: B SK pw of NET-LERY SECTION 23959 20 39 Project: Title: Ibb No. DRG No.