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FACTUAL GROUND INVESTIGATION REPORT

11 WADHAM GARDENS
CAMDEN
LONDON NW3

Report Reference No. C12520a

On behalf of:-

**Mr. M. Steinberg
11 Wadham Gardens
Camden
London
NW3 3DN**

November 2011

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MR. M. STEINBERG

JAMPEL DAVISON & BELL LIMITED

CONSULTING ENGINEERS

GROUND INVESTIGATION REPORT

AT

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CAMDEN

LONDON NW3

Report Reference No. C12520fac

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INTRODUCTION

Mr. M. Steinberg proposes to construct a new basement under the footprint of the existing three-storey house at 11 Wadham Gardens, London NW3. It is proposed to extend this basement beneath the rear garden where it will house a swimming pool. It is envisaged that the swimming pool excavation would extend to approximately 7.00m depth and the remaining basement beneath the house will be taken to about 4.50m below existing ground level.

A site investigation was undertaken by Ground Engineering Limited on behalf of Mr. M Steinberg, following instructions from Consulting Engineers, Jampel Davison & Bell and to the requirements of Geotechnical Consulting Group. The Ground Engineering Limited Desk Study report referenced C12520 was issued in November 2011 and should be consulted for full details of the site.

This report provides factual information from the intrusive ground investigation which was required to provide information on the underlying ground conditions. A summary of the desk study report is included within this report, together with a preliminary conceptual model.

LOCATION, TOPOGRAPHY AND GEOLOGY OF THE SITE

Number 11 Wadham Gardens is a detached three-storey house, located on the north side of Wadham Gardens, Camden, London NW3. The property is centred at National Grid Reference TQ 2715 8405, approximately 500m north-west of Primrose Hill and 700m east of South Hampstead railway station.

The extended red-brick house was sited towards the southern end of the 48m by 18m plot, with a front drive and small garden fronting the north side of Wadham Gardens. The front garden was flanked by a hedge and a mature London Plane tree grew within the adjacent pavement to the south-east of the property.

A narrow gated path provided access along the eastern flank of the house to the rear garden which was largely laid to lawn and surrounded by hedges, shrubs and trees including Lilac, Palm and Pyracantha. A 6m high *Acer* tree stood close to the location of the proposed basement structure. Within the neighbouring properties to the west and north, stood a 10m high Maple, some 6m distant; a 12m high Poplar, approximately 20m distant; and an 11m high Oak about 25m distant.

The property was flanked to the east and west by existing adjacent extended residential properties and their gardens.

The Primrose Hill (fast lines) Tunnel is routed east-west beneath the northern part of the site, with an associated air shaft about 12m to the east of the site within the adjacent rear garden of No.13 Wadham Gardens.

The property stands on a relatively level plot at an elevation of about 47.5mOD on land gently sloping down to the south within a subdued valley between Primrose Hill and Ordnance Hill. The subdued valley between Primrose Hill and Ordnance Hill delineates the former course of the southward flowing 'Tyburn' or 'Aye Brook'.

The surrounding ground rose towards Primrose Hill about 450m to the south-east; Haverstock Hill about 900m to the north-east; Ordnance Hill about 500m to the south-west; and

the more elevated Hampstead Heath (134mOD) about 2.5km to the north. Wadham Gardens rises slightly to the west to about 48.5mOD at its junction with Harley Road.

The geological maps, sheet 256 (2006) at 1:50,000 scale, and London Sheet IV.NE (1936) at 1:10,560 scale, show the site to be underlain directly by the solid geology of the London Clay Formation. Well records from the district indicate the London Clay to be in the order of 44m to 78m thick. The more recent map indicates a propensity for Head Deposits around the flanks of Primrose Hill within 200m of the site. The Head Deposit was formed by the downslope movement of saturated soils under periglacial climatic conditions.

SUMMARY OF DESK STUDY REPORT (C12520)

Summary of Historical Background

The site area was originally part of the Chalcots Estate, owned by Eton College, which was gradually sold off from around 1830 and the surrounding land developed for residential properties. The Primrose Hill (fast lines) Tunnel, opened in 1879, was routed beneath the former Eton & Middlesex Cricket Ground and extended beneath the northern part of the site, with an air shaft about 12m to the east of the site. Wadham Gardens including No.11, was constructed between 1896 and 1915, with the air shaft incorporated within the adjacent rear garden of No.13. Number 11 Wadham Gardens was subsequently extended to its rear on several occasions.

The tunnel drawings indicate the crown of the tunnel to be at approximately 7.00m depth beneath the site. The tunnel has an internal height of approximately 7.50m, an internal maximum width of approximately 7.00m, and a lining thickness in the order of 0.90m. Overhead electrification was added to the tunnel in 1959. The tunnel was inclined at 1:1056 towards the eastern Euston portal. Drainage within the tunnel is recorded via an earthenware pipe approximately 1m below rail level, draining to this eastern portal.

Summary of Surface Drainage

The historical surface water drainage follows the topography to the south between Primrose Hill and Ordnance Hill. The Camden River Restoration History Report 'The History of Lost Rivers in Camden' (March 2010) notes this eastern source of the Tyburn as a spring behind Hampstead Town Hall on Haverstock Hill, and notes this stream to flow along Belsize Avenue, across Adelaide Road and Avenue Road to the intersection of Norfolk Road and Woronzow Road, where it joined another tributary from the north-west.

A combined sewer (914mm x 635mm) is routed beneath the centre of Wadham Gardens, immediately south of the site, draining to the south-west to the front of the site. This

has an invert level of about 3.00m beneath the road to the front of No.11. This sewer may intercept water from older surface water drainage features.

Summary of Environmental Database Searches

There are records of five (5) potentially contaminative sites within 250m of the site. Four entries are for electricity sub-stations, the closest of which is 22m to the south. The remaining entry is for 'Piano Advisory Service' at 181 Adelaide Road, 146m to the north.

The site is not recorded as being underlain by any made ground, superficial or drift deposits. The site is underlain by the solid geology of the London Clay Formation, classified as Unproductive strata.

The site lies within a Zone 2 (Outer Catchment) Source Protection Zone.

There is one (1) Detailed River Network entry within 250m of the site. This entry is for an 'Extended Culvert' extending north-south and passing the site 8m to the east. This is believed to be erroneous, since the data is derived partly from OS Master Map features, which may have used the adjacent air shaft as a node for the plotted culvert. This is named as St. Agnes's Well (which is located within Hyde Park and was part of the Westbourne river system) and extends south for over 5km from a pond at Parliament Hill, towards Paddington Station, before heading south-west.

Preliminary Conceptual Model

Assessment of the potential linkage between ground contamination sources, human and environmental receptors have been assessed based on the desk study research documented in the preceding sections of this report. A generalised preliminary conceptual model relative to the construction phase and completed development is presented overleaf in Table 1.

Table 1: Preliminary Conceptual Model Relative to Construction & Use of Future Development

Receptors	Pathway	Estimated Potential for Linkage with Contaminant Sources			
		Drainage & Tunnel	Soil Beneath Site	Soil Gas	Ground Contamination Outside Site Boundary
Human Health – ground workers	Ingestion and Inhalation of contaminated Soil, Dust and Vapour	Low likelihood	Low likelihood	Low likelihood	Low likelihood
Human Health – users of completed development	Ingestion and Inhalation of contaminated Soil, Dust and Vapour	Low likelihood	Low likelihood	Unlikely	Unlikely
Water Environment	Migration through ground into surface water or groundwater	Low likelihood	Low likelihood	Unlikely	Low likelihood
Flora	Vegetation on site growing on contaminated soil	Unlikely	Low likelihood	Unlikely	Unlikely
Building Materials	Contact with contaminated soil	Low likelihood	Low likelihood	N/A	Unlikely

<u>Key to Table 1</u> Estimated Potential for Linkage with Contaminant Source	Definition
High likelihood	There is a pollution linkage and an event that either appears very likely in the short term and almost inevitable over the long term, or there is evidence at the receptor of harm or pollution.
Likely	There is a pollution linkage and all the elements are present and in the right place, which means that it is probable that an event will occur. Circumstances are such that an event is not inevitable, but possible in the short term and likely over the long term.
Low likelihood	There is a pollution linkage and circumstances are possible under which an event could occur. However, it is by no means certain that even over a longer period such an event would take place, and is less likely in the shorter term.
Unlikely	There is a pollution linkage but circumstances are such that it is improbable that an event would occur even in the very long term.
N/A	Not Applicable

SITE WORK

The site work consisted of a cable percussive borehole (BH1) located in the front garden and a window sampled borehole (WS2) sunk within the rear garden. The site work was conducted on 18th and 19th October 2011 at the positions shown on the borehole location plan to the rear of this report.

The borehole records give the descriptions and depths of the various strata encountered, details of all samples taken, installation details and the groundwater conditions observed during boring, excavation, on completion and within the standpipe and piezometer installations.

Ground levels at each exploratory position have been interpolated from arbitrary datum spot heights provided on a plan (Drawing No. 6048/01/A) provided by the engineer.

Public utility service drawings were sourced and consulted prior to determining the exploratory hole positions. Copies of these records were included in report C12520. Prior to excavation, a service scan was made at each position using a CAT (Cable Avoidance Tool) to check for the absence of detectable buried services that may otherwise have been damaged by the investigation. An inspection pit was hand excavated to 1.20m at both borehole positions in order to confirm the absence of any buried services.

Cable Percussive Borehole

A dismantlable cable percussive boring rig was used on 18th and 19th October 2011, to sink a borehole within the front lawn. The rig was assembled on plastic sheeting and boards to protect the garden. The borehole was then advanced through the base of the inspection pit using weighted claycutter boring tools suspended from a light winch cable to 22.00m depth and working within 150mm diameter steel casing inserted to 2.20m depth.

Representative small disturbed and bulk samples of soil were taken from the boring tools at regular intervals throughout the depth of the borehole.

Undisturbed samples 100mm in diameter were taken in clay at regular intervals within aluminium tubes. The ends of the samples were sealed to maintain them in as representative condition as possible during transit to the laboratory.

Standard penetration tests were undertaken in clay between the undisturbed samples in order to give an indication of the in-situ strength of the material. The test was made by driving a split spoon sampler, 50mm diameter into the soil at the base of the borehole by means of an automatic trip hammer weighing 63.50kg falling freely through 760mm. The penetration resistance was determined as the number of blows required to drive the tool the final 300mm of a total penetration of 450mm into the soil ahead of the borehole. The results have been tabulated following the borehole records and the SPT 'N' values are presented on the borehole record and have been plotted against depth in Figure 1.

On completion of BH1, a de-aired ceramic piezometer tip was installed within a pea gravel surround at 7.30m depth. The borehole was backfilled with arisings from the base of the hole to 9.00m where a bentonite seal was placed to 7.50m depth. The gravel pack surrounding the piezometer extended from 7.50m to 6.50m depth, above which a 1.00m thick bentonite seal was placed. The 19mm plain piezometer pipe extended from the porous tip to ground level, where another 1.00m bentonite seal was installed, below which the annulus was backfilled with arisings. A gas tap was fitted to the top of the pipe and a protective stopcock cover was concreted in place at ground level above the installation.

Window Sample Borehole

Borehole WS2, sunk to a depth of 10.45m by a dismantlable window sampling rig capable of accessing the rear garden using the narrow side pathway and gates.

The borehole was formed by a hydraulically operated super heavy dynamic probing rig. The window sampling equipment consisted of drive-in sample tubes of specially constructed and strengthened steel, lined with a plastic core-liner. The barrels were initially of 87mm internal diameter and were reduced in diameter with successive barrels with increasing depth. Upon extraction, a continuous 'undisturbed' profile of the soil was obtained within the

plastic liners. The plastic liners were subsequently split by a geotechnical engineer who sub-sampled them, conducted hand shear vane or pocket penetrometer tests, with the remaining samples re-sealed within the plastic liners.

Standard penetration tests (SPT) were undertaken between liners at approximately 1.00m intervals in order to give an indication of the in-situ strength of the material. The test was made by driving a 50mm diameter split spoon sampler attachment into the soil at the base of the borehole by means of an automatic trip hammer weighing 63.50kg falling freely through 750mm. The penetration resistance was determined as the distance driven for 50 blows into the soil ahead of the borehole.

On completion of borehole WS2 a 50mm diameter gas and groundwater monitoring standpipe was installed to 5.00m depth, where a 1.00m thick bentonite seal had been placed to 6.00m. The pipe was perforated to within 1.00m of ground level and the annulus backfilled with pea gravel. A bentonite seal was placed from ground surface to 1.00m depth and a gas tap fitted. A protective stopcock cover was concreted in place at ground level above the installation.

Return Monitoring Visits

Return visits were made to monitor the gas and groundwater levels within the installations on 27th October and 24th November 2011. The results are provided following the borehole records. The gas concentrations were measured using a Gaslog GFM430 instrument.

During the first visit a groundwater sample was taken from the installation in WS2 and sealed in glassware. Groundwater was not extracted from the small diameter installation in BH1.

LABORATORY WORK

The samples were inspected in the laboratory and assessments of the soil characteristics have been taken into account during preparation of the borehole records. The soil descriptions have been made in accordance with BS5930:1999 including amendment No.2 (2010). Geotechnical laboratory testing was scheduled, approved by Geotechnical Consulting Group, and tested in accordance with BS1377:1990.

The moisture contents of selected soil samples were determined. The results have been plotted against depth in Figure 2.

The particle density of selected samples was determined using a pycnometer.

The index properties of selected soil samples were determined as a guide to soil classification and behaviour. The liquid limit was determined by a cone penetrometer.

Test specimens were prepared at full diameter from selected undisturbed samples. Immediate undrained triaxial compression tests were undertaken on the samples under a single confining cell pressure. The moisture content and bulk density of each specimen was also determined. The values of apparent shear strength have been plotted together with the hand shear vane test results from WS2, against depth in Figure 3.

Selected samples of soil and groundwater were analysed to determine the concentration of soluble sulphates. The pH values were also determined. The acid soluble sulphate content and total sulphur concentrations of selected samples of clay were determined. The chloride and nitrate contents of a sample of groundwater were also determined.

An indication of the settlement characteristics of selected undisturbed samples of London Clay was obtained from the consolidation apparatus or oedometer. Each test was performed on a 75mm diameter specimen, approximately 19mm thick, contained in a steel ring. The specimen was saturated and the swelling pressure balanced prior to applying a constant load with drainage at both ends. When primary compression was complete, the load was increased and this repeated for three increments of load. The sample was then unloaded. The rate and total amount of consolidation were continually monitored using a computer controlled E.L.E.

Datasystem 7 Unit. The result was plotted and analysed by the computer for each increment of load to obtain the coefficients of compressibility (m_v), and of consolidation (c_v), which govern the amount and rate of settlement, respectively.

Chemical analysis of three soil samples recovered from the exploratory holes was undertaken, by an independent laboratory, primarily for characterisation purposes. The samples were tested for a suite encompassing a wide range of potential contaminants outlined by the Environment Agency (EA) and National House Building Council (NHBC) document R&D 66; 2008 'Guidance for the Safe Development of Housing on Land Affected by Contamination'. In addition, the percentage Soil Organic Matter (SOM) and hexavalent chromium content of the samples was also determined.

A sample of made ground was tested for the full Waste Acceptance Criteria (WAC) CEN leachate tests.

GROUND ENGINEERING LIMITED

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GROUND ENGINEERING LIMITED Tel: 01733-566566 www.groundengineering.co.uk			Site: 11 WADHAM GARDENS, LONDON NW3				BOREHOLE BH1									
Date: 18/10/11 to 19/10/11			Hole Size: 150mm dia to 22.00m				Ground Level: 49.40m. S.D.									
Samples and in-situ Tests			(Date)	Inst.	Description of Strata		Legend	Depth m	S.D. Level m							
Depth m	Type	Blows	Casing													
0.10 0.20-0.50 0.50-1.00	D1 B1 B2				MADE GROUND - Firm, friable, dark brown/grey slightly gravelly, sandy CLAY. Gravel consisting of angular to sub-angular flint, concrete, brick and ash fragments.			0.20	49.20							
					MADE GROUND - Firm, brown, slightly sandy, slightly gravelly CLAY. Gravel consisting of angular to sub-rounded flint, concrete, brick, coal and ash fragments.			1.00	48.40							
1.20 1.20-1.65	D2 U1	25			Firm, medium strength, orange brown/light grey mottled slightly gravelly, silty CLAY. Gravel consisting of fine and medium, angular to rounded flint.											
1.60 1.65-2.05 1.80-2.10	D3 B3 S	N8			(HEAD DEPOSIT)											
2.05 2.10-2.55	D4 U2	28	1.50	▼s												
2.55-3.00 2.55 2.70-3.00 3.00 3.00-3.45	B4 D5 S D6 U3	N13 35	2.20 2.20		Stiff, fissured, medium strength, brown/orange brown/grey mottled CLAY with occasional decayed root traces to 5.00m. Occasional fine gravel size selenite below 4.00m.			2.80	46.60							
3.45-4.00 3.45 3.60-3.90	B5 D7 S	N12	2.20													
3.90 4.00-4.45	D8 U4	38	2.20													
4.45-5.00 4.45 4.60-4.90	B6 D9 S	N15	2.20													
4.90 5.00-5.45	D10 U5	38	2.20	▼s	...Becoming high strength below 5.00m depth.											
5.45 5.60-5.90	D11 S	N16	2.20													
5.90 6.00-6.45	D12 U6	42	2.20		(WEATHERED LONDON CLAY)											
6.45 6.60-6.90	D13 S	N17	2.20													
6.90 7.10 7.10-7.55	D14 D15 U7	35	2.20	▼	Weak, light brown calcareous SILTSTONE. (WEATHERED LONDON CLAY)			7.00 7.10	42.40 42.30							
7.55 7.70-8.00	D16 S	N16	2.00		Stiff, very closely fissured, high strength, brown CLAY with occasional fine and medium gravel size selenite. Occasional orange brown stained fissures and orange brown silt partings below 8.50m. (WEATHERED LONDON CLAY)											
8.00 8.00-8.45	D17 U8	40	2.20													
8.45-9.00 8.45 8.60-8.90	B7 D18 S	N16	2.20													
9.10-9.55	U9	40	2.20					9.50	39.90							
9.55-10.00 9.55 9.70-10.00	B8 D19 S	N17	2.20		Stiff, closely fissured, high strength, dark brown/dark grey CLAY with rare fine gravel size shell fragments. (LONDON CLAY)			10.00	39.40							
REMARKS								Project No 12520								
1. Excavating a pit from 0.00m to 1.20m for 1 hour 2. Fibrous live roots observed to 1.60m depth 3. Borehole cased to 2.20m depth 4. Piezometer installed at 7.30m depth								Scale 1:50	Page 1/3							
KEY			N - SPT Blows for 0.3m			Groundwater Strikes			Groundwater Observations							
D - Disturbed Sample B - Bulk Sample U - Undisturbed Sample W - Water Sample S/C - SPT Spoon/Cone			* - Blows for quoted penetration V - Vane Shear Test Cohesion () kPa ▼ Level on completion c▼ Level casing withdrawn ▼ Water Strike ▼ Water Rise			Depth m			Depth m							
						No	Struck	Rose to	Rate	Cased	Sealed	Date	Hole	Casing	Water	
						1	7.10	7.00	slow	2.20	not		18/10/11 19/10/11 19/10/11 19/10/11 27/10/11	10.00 10.00 22.00 22.00 7.30	2.20 2.20 2.20 0.00 7.00	dry 10.00 dry dry 5.11

GROUND ENGINEERING L I M I T E D Tel: 01733-566566 www.groundengineering.co.uk			Site: 11 WADHAM GARDENS, LONDON NW3				BOREHOLE BH1				
			Date: 18/10/11 to 19/10/11		Hole Size: 150mm dia to 22.00m		Ground Level: 49.40m. S.D.				
Samples and in-situ Tests			(Date) Casing	Inst.	Description of Strata	Legend	Depth m	S.D. Level m			
Depth m	Type	Blows									
10.70-11.15	U10	45	2.20	BENEATH INSTALLATION	Stiff, closely fissured, high strength, dark brown/ dark grey CLAY with rare fine gravel size shell fragments. Rare light grey silt partings at 11.80m.	✕	10.00	39.40			
11.15	D20			BENEATH INSTALLATION		✕					
11.65-11.95	S	N20	2.20	BENEATH INSTALLATION		✕					
11.95	D21			BENEATH INSTALLATION		✕					
12.50-12.95	U11	43	2.20	BENEATH INSTALLATION		✕					
12.95	D22			BENEATH INSTALLATION		✕					
13.65-13.95	S	N21	2.20	BENEATH INSTALLATION		✕					
13.95	D23			BENEATH INSTALLATION		✕					
14.50-14.95	U12	47	2.20	BENEATH INSTALLATION		✕					
14.95	D24			BENEATH INSTALLATION		✕					
15.65-15.95	S	N25	2.20	BENEATH INSTALLATION		✕					
15.95	D25			BENEATH INSTALLATION		✕					
16.50-16.95	U13	58	2.20	BENEATH INSTALLATION		✕					
16.95	D26			BENEATH INSTALLATION		✕					
17.65-17.95	S	N29	2.20	BENEATH INSTALLATION		✕					
17.95	D27			BENEATH INSTALLATION		✕					
18.50-18.95	U14	60	2.20	BENEATH INSTALLATION		✕					
18.95	D28			BENEATH INSTALLATION		✕					
19.65-19.95	S	N31	2.20	BENEATH INSTALLATION		✕					
19.95	D29			BENEATH INSTALLATION		✕					
...Becoming very stiff, closely fissured and high strength below 15.00m. (LONDON CLAY)						✕	17.50	31.90			
						✕					
Very stiff, very closely fissured, high strength dark brown/dark grey CLAY with occasional grey silt partings up to 2mm thick. Rare finely comminuted shell fragments. (LONDON CLAY)						✕	20.00	29.40			
						✕					
REMARKS								Project No 12520			
								Scale 1:50	Page 2/3		
KEY D - Disturbed Sample N - SPT Blows for 0.3m B - Bulk Sample * - Blows for quoted U - Undisturbed Sample penetration W - Water Sample V - Vane Shear Test S/C - SPT Spoon/Cone Cohesion () kPa Water Strike Level on completion Water Rise Level casing withdrawn Standpipe Level				Groundwater Strikes				Groundwater Observations			
				Depth m				Depth m			
				No	Struck	Rose to	Rate	Cased	Sealed	Date	Hole
						24/11/11	7.30	7.00	2.20		

GROUND ENGINEERING L I M I T E D Tel: 01733-566566 www.groundengineering.co.uk			Site: 11 WADHAM GARDENS, LONDON NW3				BOREHOLE BH1								
Date: 18/10/11 to 19/10/11			Hole Size: 150mm dia to 22.00m				Ground Level: 49.40m. S.D.								
Samples and in-situ Tests			(Date)	Inst.	Description of Strata	Legend	Depth m	S.D. Level m							
Depth m	Type	Blows	Casing												
20.50-20.95	U15	62	2.20		Very stiff, very closely fissured, high strength, dark brown/dark grey CLAY with occasional grey silt partings up to 4mm thick. (LONDON CLAY)		20.00	29.40							
20.95	D30														
21.65-21.95	S	N38	2.20												
22.00	D31														
Borehole completed at 22.00m depth							22.00	27.40							
REMARKS								Project No 12520							
								Scale 1:50	Page 3/3						
KEY D - Disturbed Sample B - Bulk Sample U - Undisturbed Sample W - Water Sample S/C - SPT Spoon/Cone Water Strike Water Rise				N - SPT Blows for 0.3m * - Blows for quoted penetration V - Vane Shear Test Cohesion () kPa Level on completion Level casing withdrawn Standpipe Level				Groundwater Strikes Depth m No Struck Rose to Rate Cased Sealed				Groundwater Observations Date Depth m Hole Casing Water			

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Hole Size: 87mm dia to 3.00m
77mm dia to 5.00m
57mm dia to 10.45m



Ground Level: 49.90m. S.D.

REMARKS	1. Starter pit excavated from GL to 1.20m depth 2. No live roots observed 3. Hole sides stable 4. Borehole cased to 1.00m depth 5. Gas monitoring standpipe installed to 5.00m depth
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Project No
12520

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GROUND ENGINEERING L I M I T E D Tel: 01733-566566 www.groundengineering.co.uk			Site: 11 WADHAM GARDENS, LONDON NW3					WINDOW SAMPLE WS2			
Date: 19/10/11			Hole Size: 87mm dia to 3.00m 77mm dia to 5.00m 57mm dia to 10.45m					Ground Level: 49.90m. S.D.			
Samples and in-situ Tests			(Date)	Inst.	Description of Strata			Legend	Depth m	S.D. Level m	
Depth m	Type	Result	Water								
10.15-10.45	S	N30	1.00		Stiff, closely fissured, dark brown/dark grey CLAY. (LONDON CLAY) Borehole completed at 10.45m depth				10.00	39.90	
									10.45	39.45	
REMARKS											
										Project No 12520	
										Scale 1:50	Page 2/2
KEY D - Disturbed Sample B - Bulk Sample U - Undisturbed Sample W - Water Sample ☒ - Water Strike ▼ - Depth to Water on completion J - Jar Sample M - Mackintosh Probe V - Vane Shear Test Cohesion () kPa P () - Hand Penetrometer Cohesion () kPa ▼s Standpipe Level					Groundwater Strikes Depth m No Struck Rose to Rate Cased Sealed			Groundwater Observations Date Hole Casing Water			

Borehole Number	Depth (m)	Casing Depth (m)	Depth to Water (m)	Type of Test *	Seating Drive: Blows/Penetration (mm)	Test Drive: 300mm Blows for each successive 75 mm Penetration				N Value	Extrapolated Value
BH1	1.65 - 2.10			S	2/150	2	2	2	2	8	
	2.55 - 3.00	2.20		S	3/150	2	3	4	4	13	
	3.45 - 3.90	2.20		S	3/150	2	3	3	4	12	
	4.45 - 4.90	2.20		S	3/150	2	4	4	5	15	
	5.45 - 5.90	2.20		S	3/150	3	4	4	5	16	
	6.45 - 6.90	2.20		S	3/150	3	4	5	5	17	
	7.55 - 8.00	2.00		S	3/150	3	4	4	5	16	
	8.45 - 8.90	2.20		S	3/150	3	4	4	5	16	
	9.55 - 10.00	2.20		S	3/150	3	4	5	5	17	
	11.50 - 11.95	2.20		S	5/150	4	4	6	6	20	
	13.50 - 13.95	2.20		S	5/150	4	5	6	6	21	
	15.50 - 15.95	2.20		S	7/150	5	6	7	7	25	
	17.50 - 17.95	2.20		S	7/150	5	7	8	9	29	
	19.50 - 19.95	2.20		S	7/150	6	6	9	10	31	
	21.50 - 21.95	2.20		S	7/150	6	9	10	13	38	
WS2	1.20 - 1.65			S	1/150	1	1	2	2	6	
	2.00 - 2.45	1.00		S	3/150	1	2	2	2	7	
	3.00 - 3.45	1.00		S	3/150	2	2	2	3	9	
	4.00 - 4.45	1.00		S	4/150	3	3	3	3	12	
	5.00 - 5.45	1.00		S	5/150	3	3	4	4	14	
	6.00 - 6.45	1.00		S	6/150	3	4	4	5	16	
	7.00 - 7.45	1.00		S	4/150	3	4	4	4	15	
	8.00 - 8.45	1.00		S	4/150	2	3	4	4	13	
	9.00 - 9.45	1.00		S	5/150	4	4	4	5	17	
	10.00 - 10.45	1.00		S	7/150	7	8	7	8	30	
* C denotes test using a solid cone S denotes test using a split barrel sampler											
Results of Standard/Cone Penetration Tests										12520	
11 WADHAM GARDENS, LONDON NW3										Table No	

Gas & Groundwater Monitoring Record

Site: **11 Wadham Gardens, London NW3**

Report Ref: **C12520**

Date	Borehole No.	Methane (% v/v)		Methane LEL (%LEL)		Carbon Dioxide (% v/v)		Oxygen (% v/v)		Flow Rate (l/hr)	Atmosph. Pressure (mb)	Depth of Well (mbgl)	Depth to Groundwater (mbgl)	Comments
		Peak	Steady	Peak	Steady	Peak	Steady	Min.	Max.					
27/10/2011	BH1	<0.1	<0.1	<0.1	<0.1	0.4	0.1	17.9	20.8	<0.1	999		5.11	
	WS2	<0.1	<0.1	<0.1	<0.1	0.1	<0.1	20.8	21.2	<0.1	999		0.34	
24/11/2011	BH1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	20.2	20.2	<0.1	1026		2.20	
	WS2	<0.1	<0.1	<0.1	<0.1	0.9	0.9	20.3	20.3	<0.1	1026		0.51	

LEL – Lower Explosive Limit

GROUND ENGINEERING LIMITED, NEWARK ROAD, PETERBOROUGH, PE1 5UA

LABORATORY TEST RESULTS

CONTRACT 11 WADHAM GARDENS, LONDON NW3

Bore-hole	Sample	Depth m	Classification				Density		Triaxial Compression						Sulphates (SO ₄)				Remarks
			Liquid Limit %	Plastic Limit %	Plasticity Index %	Moisture Content %	Bulk Mg/m ³	Dry Mg/m ³	Type	Principal Stress Difference kPa	Cell Pressure kPa	Shear Strength kPa	Angle of Shear Resistance degrees	Soil		Water			
														Total % Dry Wt.	Aqueous Extract mg/l	mg/l	pH		
BH1	D2	1.20				22													SOIL CLASSIFICATION = CH 6% retained on 425µm sieve
	U1	1.20 - 1.65	51	14	37	24	2.08	1.68	Q	122	50	61	0						
	D3	1.60				21													
	D4	2.05				27													
	U2	2.10 - 2.55	48	16	32	27	2.13	1.68	Q	128	50	64	0						
	D5	2.55				24													SOIL CLASSIFICATION = CI 0% retained on 425µm sieve
	U3	3.00 - 3.45	73	22	51	28	2.02	1.57	Q	134	60	67	0						
	B5	3.45 - 4.00														429		7.7	
	U4	4.00 - 4.45	78	26	52	33	1.93	1.46	Q	104	80	52	0						SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U5	5.00 - 5.45	72	24	48	29	1.98	1.54	Q	178	100	89	0						SOIL CLASSIFICATION = CV 0% retained on 425µm sieve Particle Density = 2.80Mg/m3

Aqueous Extract 2:1 Water:Soil

U - UNDISTURBED SAMPLE
D - DISTURBED SAMPLE
B - BULK SAMPLE
W - WATER SAMPLE
C.U. - CONSOLIDATED UNDRAINED
C.D. - CONSOLIDATED DRAINED
Q. - IMMEDIATE UNDRAINED
Q.M. - IMMEDIATE UNDRAINED MULTISTAGE

12520

LABORATORY TEST RESULTS

CONTRACT 11 WADHAM GARDENS, LONDON NW3

Bore-hole	Sample	Depth m	Classification				Density		Triaxial Compression					Sulphates (SO ₄)				Remarks
			Liquid Limit %	Plastic Limit %	Plasticity Index %	Moisture Content %	Bulk Mg/m ³	Dry Mg/m ³	Type	Principal Stress Difference kPa	Cell Pressure kPa	Shear Strength kPa	Angle of Shear Resistance degrees	Soil			pH	
														Total % Dry Wt.	Aqueous Extract mg/l	Water mg/l		
BH1	U6	6.00 - 6.45	73	25	48	31	1.97	1.50	Q	202	120	101	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U7	7.10 - 7.55	74	25	49	29	1.99	1.54	Q	167	145	84	0					SOIL CLASSIFICATION = CV 3% retained on 425µm sieve Particle Density = 2.78Mg/m3
	U8	8.00 - 8.45	79	26	53	31	1.95	1.49	Q	169	160	84	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	B6	8.45 - 9.00													2639		7.6	
	U9	9.10 - 9.55	77	26	51	33	1.92	1.44	Q	150	180	75	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U10	10.70 - 11.15	80	26	54	31	1.95	1.49	Q	173	220	86	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U11	12.50 - 12.95	80	28	52	29	1.97	1.53	Q	218	250	109	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U12	14.50 - 14.95	79	30	49	29	2.00	1.56	Q	218	290	109	0					SOIL CLASSIFICATION = CV 0% retained on 425µm sieve

Aqueous Extract 2:1 Water:Soil

U - UNDISTURBED SAMPLE
D - DISTURBED SAMPLE
B - BULK SAMPLE
W - WATER SAMPLE
C.U. - CONSOLIDATED UNDRAINED
C.D. - CONSOLIDATED DRAINED
Q. - IMMEDIATE UNDRAINED
Q.M. - IMMEDIATE UNDRAINED MULTISTAGE

12520

LABORATORY TEST RESULTS

CONTRACT 11 WADHAM GARDENS, LONDON NW3

Bore-hole	Sample	Depth m	Classification				Density		Triaxial Compression					Sulphates (SO ₄)				Remarks
			Liquid Limit %	Plastic Limit %	Plasticity Index %	Moisture Content %	Bulk Mg/m ³	Dry Mg/m ³	Type	Principal Stress Difference kPa	Cell Pressure kPa	Shear Strength kPa	Angle of Shear Resistance degrees	Total % Dry Wt.	Soil Aqueous Extract mg/l	Water mg/l	pH	
BH1	D24	14.95												706			8.1	SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	U13	16.50 - 16.95	78	27	51	31	2.00	1.53	Q	242	330	121	0					
	U14	18.50 - 18.95	70	25	45	24	2.13	1.73	Q	292	370	146	0					SOIL CLASSIFICATION = CH/CV 0% retained on 425µm sieve
	D29	19.95													536		8.3	
	U15	20.50 - 20.95	70	26	44	26	1.97	1.56	Q	265	410	132	0					SOIL CLASSIFICATION = CH/CV 0% retained on 425µm sieve

Aqueous Extract 2:1 Water:Soil

U - UNDISTURBED SAMPLE
D - DISTURBED SAMPLE
B - BULK SAMPLE
W - WATER SAMPLE
C.U. - CONSOLIDATED UNDRAINED
C.D. - CONSOLIDATED DRAINED
Q. - IMMEDIATE UNDRAINED
Q.M. - IMMEDIATE UNDRAINED MULTISTAGE

12520

LABORATORY TEST RESULTS

CONTRACT 11 WADHAM GARDENS, LONDON NW3

Bore-hole	Sample	Depth m	Classification				Density		Triaxial Compression					Sulphates (SO ₄)				Remarks
			Liquid Limit %	Plastic Limit %	Plasticity Index %	Moisture Content %	Bulk Mg/m ³	Dry Mg/m ³	Type	Principal Stress Difference kPa	Cell Pressure kPa	Shear Strength kPa	Angle of Shear Resistance degrees	Total % Dry Wt.	Soil Aqueous Extract mg/l	Water mg/l	pH	
WS2	D4	0.90				38												SOIL CLASSIFICATION = CV 0% retained on 425µm sieve
	D5	1.20				37												
	D6	1.30																
	D7	1.90	74	26	48	33								106			8.0	
	D8	2.00				33												
	D9	2.90	78	28	50	35												
	D10	3.00				34												
	D11	3.90	79	29	50	34												
	D13	4.90	71	26	45	31												
	D16	6.70	77	28	49									2349			7.7	
	D20	9.50												2066			7.7	SOIL CLASSIFICATION = CV 0% retained on 425µm sieve

Aqueous Extract 2:1 Water:Soil

U - UNDISTURBED SAMPLE
D - DISTURBED SAMPLE
B - BULK SAMPLE
W - WATER SAMPLE
C.U. - CONSOLIDATED UNDRAINED
C.D. - CONSOLIDATED DRAINED
Q. - IMMEDIATE UNDRAINED
Q.M. - IMMEDIATE UNDRAINED MULTISTAGE

12520

TEST CERTIFICATE

One-Dimensional Consolidation

Properties

(Tested in accordance with BS1377 : Part 5 1990)

Client: Ground Engineering Ltd
 Client Address: Newark Road
 Peterborough
 Cambridgeshire
 Postcode: PE1 5UA
 Contact: Chris Ebeling
 Site Name: 11 Wadham Gardens
 Site Address: London NW3

Newark Road Peterborough

t:01733 555525 f:01733 315280

e: peterborough@enverity.co.uk

Certificate Number: PL3452-1-7/731
 Client Reference Number: C12520
 Date Sampled: Unknown
 Date Received: 27.10.2011
 Date Tested: 02.11.2011
 Sampling Certificate No: N/A
 Certificate of Sampling: N/A
 Sampled By: Client

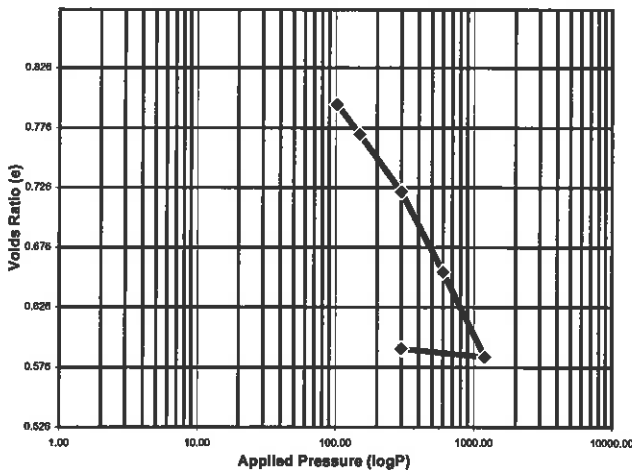
Test Details

Location: BH1
 Sample Ref: U3
 Sample Description: Stiff brown CLAY
 Particle Density (Mg/m³): 2.75 Measured
 Mean Lab Temp. (°C): 22
 Variations from Standard: None
 Lab Reference: PL3452-1-7
 Depth (m): 3.00 m

Specimen Details

	INITIAL	FINAL
Height (mm):	18.48	16.38
Bulk Density (Mg/m ³):	1.95	2.15
Moisture Content (%):	27	24
Dry Density (Mg/m ³):	1.53	1.73
Voids Ratio:	0.795	0.591
Degree of Saturation (%):	94.0	112.9
Diameter (mm):	74.96	N/A
Swelling Pressure (kPa):	102	N/A
Method of time fitting used:	Log Time	N/A

Voids Ratio against logarithm of Applied Pressure



Applied Pressure (kPa)	Coefficient of Compressibility m_v (m ² /MN)	Coefficient of Consolidation c_v (m ² /year)
102		
150	0.29	0.15
300	0.18	0.20
600	0.13	0.19
1200	0.07	0.20
300	0.01	---

Comments:

Approved [x] M.Hartnup - Laboratory Manager
 Signatory: [] G.Meadows - Team Leader

Signed:

for and on behalf of
 Enverity Ltd

Date Reported: 15/11/2011

Opinions and interpretations expressed herein are outside the scope of the UKAS Accreditation.
 This report may not be reproduced other than in full without the prior written approval of the issuing laboratory.

Form Number: EN/C/731 Issue 1

Registered in England and Wales
 Reg Number 6930692
 Registered Office: Diasma
 Willie Snaith Rd
 Newmarket CB8 7SQ

TEST CERTIFICATE

One-Dimensional Consolidation

Properties

(Tested in accordance with BS1377 : Part 5 1990)

Client: Ground Engineering Ltd
 Client Address: Newark Road
 Peterborough
 Cambridgeshire
 Postcode: PE1 5UA
 Contact: Chris Ebeling
 Site Name: 11 Wadham Gardens
 Site Address: London NW3

Newark Road Peterborough

t:01733 555525 f:01733 315280

e: peterborough@enverity.co.uk

Certificate Number: PL3452-1-10/731
 Client Reference Number: C12520
 Date Sampled: Unknown
 Date Received: 27.10.2011
 Date Tested: 02.11.2011
 Sampling Certificate No: N/A
 Certificate of Sampling: N/A
 Sampled By: Client

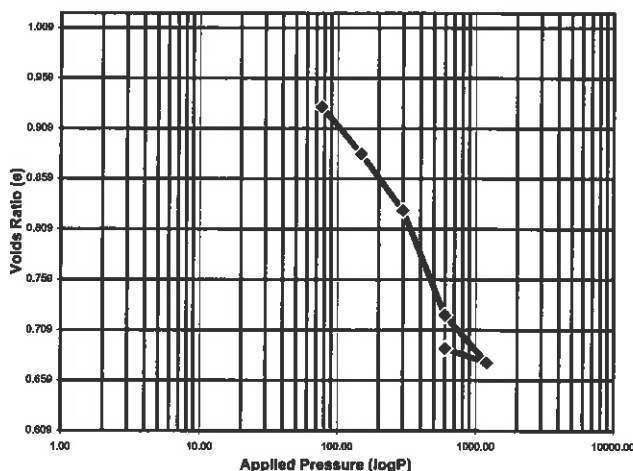
Test Details

Location: BH1
 Sample Ref: U5
 Sample Description: Stiff brown CLAY
 Particle Density (Mg/m³): 2.8 Measured
 Mean Lab Temp. (°C): 22
 Variations from Standard: None
 Lab Reference: PL3452-1-10
 Depth (m): 5.00 m

Specimen Details

	INITIAL	FINAL
Height (mm):	18.93	16.58
Bulk Density (Mg/m ³):	1.89	2.12
Moisture Content (%):	30	28
Dry Density (Mg/m ³):	1.45	1.66
Voids Ratio:	0.931	0.691
Degree of Saturation (%):	91.2	114.0
Diameter (mm):	74.99	N/A
Swelling Pressure (kPa):	77	N/A
Method of time fitting used:	Log Time	N/A

Voids Ratio against logarithm of Applied Pressure



Applied Pressure (kPa)	Coefficient of Compressibility m_v (m ² /MN)	Coefficient of Consolidation c_v (m ² /year)
77		
150	0.33	0.61
300	0.20	0.56
600	0.19	0.40
1200	0.05	0.44
600	0.01	—

Comments:

Approved [x] M.Hartnup - Laboratory Manager
 Signatory: [] G.Meadows - Team Leader

Signed:

for and on behalf of
 Enverity Ltd

Date Reported: 15/11/2011

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 Reg Number 6930692
 Registered Office: Diasma
 Willie Snaith Rd
 Newmarket CB8 7SQ

TEST CERTIFICATE

One-Dimensional Consolidation

Properties

(Tested in accordance with BS1377 : Part 5 1990)

Client: Ground Engineering Ltd
Client Address: Newark Road
Peterborough
Cambridgeshire
Postcode: PE1 5UA
Contact: Chris Ebeling
Site Name: 11 Wadham Gardens
Site Address: London NW3

Newark Road Peterborough
t:01733 555525 f:01733 315280
e: peterborough@enverity.co.uk

Certificate Number: PL3452-1-12/731
Client Reference Number: C12520
Date Sampled: Unknown
Date Received: 27.10.2011
Date Tested: 02.11.2011
Sampling Certificate No: N/A
Certificate of Sampling: N/A
Sampled By: Client

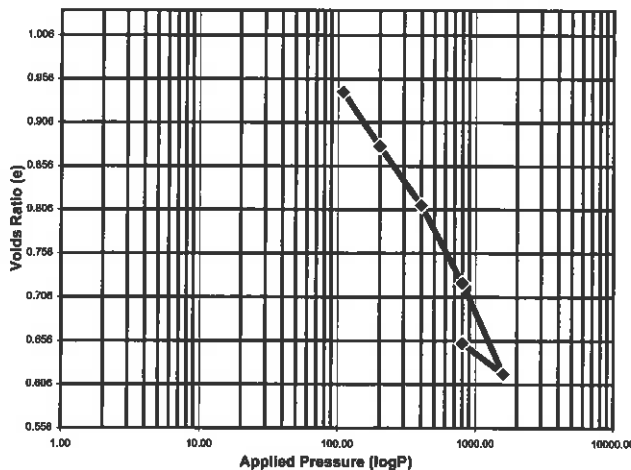
Test Details

Location: BH1
Sample Ref: U7
Sample Description: Stiff brown CLAY
Particle Density (Mg/m^3): 2.78 Measured
Mean Lab Temp. ($^{\circ}\text{C}$): 22
Variations from Standard: None
Lab Reference: PL3452-1-12
Depth (m): 7.10 m

Specimen Details

	INITIAL	FINAL
Height (mm):	18.78	15.99
Bulk Density (Mg/m^3):	1.89	2.18
Moisture Content (%):	32	29
Dry Density (Mg/m^3):	1.43	1.68
Voids Ratio:	0.941	0.653
Degree of Saturation (%):	95.1	125.6
Diameter (mm):	74.95	N/A
Swelling Pressure (kPa):	109	N/A
Method of time fitting used:	Log Time	N/A

Voids Ratio against logarithm of Applied Pressure



Applied Pressure (kPa)	Coefficient of Compressibility m_v (m^2/MN)	Coefficient of Consolidation c_v (m^2/year)
109		
200	0.36	0.40
400	0.18	0.45
800	0.12	0.40
1600	0.08	0.45
800	0.03	---

Comments:

Approved [x] M.Hartnup - Laboratory Manager
Signatory: [] G.Meadows - Team Leader

Signed:

for and on behalf of
Enverity Ltd

Date Reported: 15/11/2011

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Form Number: EN/C/731 Issue 1

Registered in England and Wales
Reg Number 6930692
Registered Office: Diasma
Willie Snailth Rd
Newmarket CB8 7SQ

Ground Engineering Limited
Newark Road
Peterborough

PE1 5UA

FAO Chris Ebeling
07 November 2011

Dear Chris Ebeling

Test Report Number 150339
Your Project Reference 11 Wadham Gardens, London NW3 - C12520

Please find enclosed the results of analysis for the samples received 28 October 2011.

All soil samples will be retained for a period of one month and all water samples will be retained for 7 days following the date of the test report. Should you require an extended retention period then please detail your requirements in an email to customerservices@chemtest.co.uk. Please be aware that charges may be applicable for extended sample storage.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely

K Jones

Authorised Signatory

<input type="checkbox"/> Darrell Hall	Director
<input type="checkbox"/> Phil Hellier	Director
<input checked="" type="checkbox"/> Keith Jones	Technical Manager
<input type="checkbox"/> John Crawford	Quality Manager
<input type="checkbox"/> Malcolm Avis	Director



Notes to accompany report:

- The sign < means 'less than'
- Tests marked 'U' hold UKAS accreditation
- Tests marked 'M' hold MCertS (and UKAS) accreditation
- Tests marked 'N' do not currently hold UKAS accreditation
- Tests marked 'S' were subcontracted to an approved laboratory
- n/e means 'not evaluated'
- i/s means 'insufficient sample'
- u/s means 'unsuitable sample'
- Comments or interpretations are beyond the scope of UKAS accreditation
- The results relate only to the items tested

**Results of analysis of 10 samples
received 28 October 2011**

PE1 5UA
FAO Chris Ebeling

Report Date
07 November 2011

11 Wadham Gardens, London NW3 - C12520

All tests undertaken between 31/10/2011 and 03/11/2011

* Accreditation status

This report should be interpreted in conjunction with the notes on the accompanying cover page.

Column page 1

Report page 1 of 2

LIMS sample ID range AG63710 to AG63719

LABORATORY TEST REPORT

PE1 5UA

Report Date
07 November 2011

FAO Chris Ebeling

11 Wadham Gardens, London NW3 - C12520

Results of analysis of 10 samples
received 28 October 2011

Login Batch No

AG63719

Sample ID

Sample No

Sampling Date

Depth

Matrix

SOP ↓ Determinand ↓

2010 pH

2175 Sulfur (total TRL report 447)

2300 Cyanide (free)

Cyanide (total)

2325 Sulfide

2625 Organic matter

2120 Boron (hot water soluble)

Sulfate (2:1 water soluble) as SO₄

2490 Chromium (hexavalent)

2430 Sulfate (total BS1377 HCl extract)

2450 Arsenic

Cadmium

Chromium

Copper

Mercury

Nickel

Lead

Selenium

Zinc

2700 Naphthalene

Acenaphthylene

Acenaphthene

Fluorene

Phenanthrene

Anthracene

Fluoranthene

Pyrene

150339

AG63719

WS2

D6

27/10/2011

1.3m

SOIL

BH1

D29

27/10/2011

19.95m

SOIL

Units ↓

CAS No ↓

M

M

M

M

M

M

M

M

M

N

M

M

M

M

M

M

M

M

M

M

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mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

mg kg⁻¹

0.46

0.02

1.8

1.2

1.9

1.6

0.08

0.07

LABORATORY TEST REPORT

Results of analysis of 10 samples
received 28 October 2011

11 Wadham Gardens, London NW3 - C12520

Report Date

07 November 2011

150339

		AG63710	AG63711	AG63712	AG63713	AG63714	AG63715	AG63716	AG63717
		BH1	BH1	BH1	BH1	BH1	WS2	WS2	WS2
		B1	B2	B5	B6	D24	D16	D1	D20
		27/10/2011	27/10/2011	27/10/2011	27/10/2011	27/10/2011	27/10/2011	27/10/2011	27/10/2011
		0.2m - 0.5m	0.5m - 1m	3.45m - 4m	8.45m - 9m	14.95m	6.7m	0.2m	9.5m
		SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
2700 Benzo[a]anthracene	56553	1.1	0.17					0.72	
Chrysene	218019	1.1	0.18					0.81	
Benzo[b]fluoranthene	205992	1.2	< 0.1					0.88	
Benzo[k]fluoranthene	207089	0.79	< 0.1					0.54	
Benzo[a]pyrene	50328	1.6	< 0.1					3.6	
Dibenzo[a,h]anthracene	53703	< 0.1	< 0.1					< 0.1	
Indeno[1,2,3-cd]pyrene	193395	0.81	< 0.1					0.66	
Benzo[g,h,i]perylene	191242	0.66	< 0.1					0.49	
Total (of 16) PAHs		16	2.4					11	
2920 Phenols (total)		< 0.3	< 0.3					< 0.3	

LABORATORY TEST REPORT

Results of analysis of 10 samples
received 28 October 2011

11 Wadham Gardens, London NW3 - C12520

Report Date
07 November 2011

150339

AG6371B	AG63719
WS2	BH1
D6	D29
27/10/2011	27/10/2011
1.3m	19.95m
SOIL	SOIL

2700	Benzo[a]anthracene	56553	mg kg ⁻¹	M
	Chrysene	218019	mg kg ⁻¹	M
	Benzo[b]fluoranthene	205992	mg kg ⁻¹	M
	Benzo[k]fluoranthene	207089	mg kg ⁻¹	M
	Benzo[a]pyrene	50328	mg kg ⁻¹	M
	Dibenzof[a,h]anthracene	53703	mg kg ⁻¹	M
	Indeno[1,2,3-cd]pyrene	193395	mg kg ⁻¹	M
	Benzo[g,h,i]perylene	191242	mg kg ⁻¹	M
	Total (of 16) PAHs		mg kg ⁻¹	M
2920	Phenols (total)		mg kg ⁻¹	N

Ground Engineering Limited
Newark Road
Peterborough

PE1 5UA

FAO C Ebeling
18 November 2011

Dear C Ebeling

Test Report Number 160405
Your Project Reference 11 Wadham Gardens, London NW3 - C12520

Please find enclosed the results of analysis for the samples received 10 November 2011.

All soil samples will be retained for a period of one month and all water samples will be retained for 7 days following the date of the test report. Should you require an extended retention period then please detail your requirements in an email to customerservices@chemtest.co.uk. Please be aware that charges may be applicable for extended sample storage.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely



Authorised Signatory

<input type="checkbox"/> Darrell Hall	Director
<input checked="" type="checkbox"/> Phil Hellier	Director
<input checked="" type="checkbox"/> Keith Jones	Technical Manager
<input type="checkbox"/> John Crawford	Quality Manager
<input type="checkbox"/> Malcolm Avis	Director



2183



Notes to accompany report:

- The sign '<' means 'less than'
- Tests marked 'U' hold UKAS accreditation
- Tests marked 'M' hold MCertS (and UKAS) accreditation
- Tests marked 'N' do not currently hold UKAS accreditation
- Tests marked 'S' were subcontracted to an approved laboratory
- n/e means 'not evaluated'
- i/s means 'insufficient sample'
- u/s means 'unsuitable sample'
- Comments or interpretations are beyond the scope of UKAS accreditation
- The results relate only to the items tested

LABORATORY TEST REPORT

Results of analysis of 1 sample
received 10 November 2011

11 Wadham Gardens, London NW3 - C12520

Login Batch No

ChemtestLIMS ID

Sample ID

Sample No

Sampling Date

Depth

Matrix

SOP ↓ Determinand ↓

1010 pH

1220 Chloride

Nitrate

1220 Sulfate

Units ↓

CAS No ↓

PH

16887006

14797558

14808798

U

U

U

U

160405

AG68635

WS2

W1

07/11/2011

0.34m

WATER

7.7

59

100

200

Ground Engineering Limited
Newark Road
Peterborough

PE1 5UA

FAO Chris Ebeling
08 November 2011

Dear Chris Ebeling

Test Report Number 150340

Your Project Reference 11 Wadham Gardens, London NW3 - C12520

Please find enclosed the results of analysis for the samples received 28 October 2011.

All soil samples will be retained for a period of one month and all water samples will be retained for 7 days following the date of the test report. Should you require an extended retention period then please detail your requirements in an email to customerservices@chemtest.co.uk. Please be aware that charges may be applicable for extended sample storage.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely

Authorised Signatory

<input checked="" type="checkbox"/> Darrell Hall	Director
<input type="checkbox"/> Phil Hellier	Director
<input type="checkbox"/> Keith Jones	Technical Manager
<input type="checkbox"/> John Crawford	Quality Manager
<input type="checkbox"/> Malcolm Avis	Director



2183



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- The results relate only to the items tested



Chemtest

LABORATORY TEST REPORT

CEN 10:1 CUMULATIVE TWO STAGE BATCH TEST

Ground Engineering Limited
Newark Road
Peterborough

PE1 5UA

FAO Chris Ebeling

Result of analysis of 2 samples
received 28 October 2011

11 Wadham Gardens, London NW3 - C12520

Report Date
8 November 2011

Log Batch No **150340**
LIMS ID **AG63721**
Sample ID **WS2**
Sample No **D2**
Sampling Date **28/10/2011**
Depth **0.4m**

Landfill Waste Acceptance Criteria Limits

Solid Waste Analysis

Determinand ↓	SOP ↓	*				Inert Waste Landfill	Stable Non-reactive Hazardous Waste in Non- Hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	%	M	0.85		3	5	6
Loss on ignition	2610	%	N	3.32				10
Total BTEX	2761	mg kg ⁻¹	M	<0.005		6		
Total PCBs (7 congeners)	2811	mg kg ⁻¹	N	<1		1		
TPH Total WAC	2670	mg kg ⁻¹	M	< 10		500		
Total (of 17) PAHs	2700	mg kg ⁻¹	N	3.7		100		
pH	2010		M	8.0			>6	
Acid Neutralisation Capacity	2015	mol kg ⁻¹	N	0.018			To evaluate	To evaluate

Eluate Analysis

Determinand ↓	SOP ↓	*	2:1 Eluate mg l ⁻¹	8:1 Eluate mg l ⁻¹	2:1 Eluate mg kg ⁻¹	Cumulative 10:1 Eluate mg kg ⁻¹	Limit values for compliance leaching test using BS EN 12457-3 at L/S 10 l/kg
Arsenic	1450	N	0.001	<0.001	<0.05	<0.05	0.5
Barium	1450	N	0.038	0.022	<0.5	<0.5	20
Cadmium	1450	N	<0.0005	<0.0005	<0.01	<0.01	0.04
Chromium	1450	N	<0.001	<0.001	<0.05	<0.05	0.5
Copper	1450	N	0.005	0.003	<0.05	<0.05	2
Mercury	1450	N	0.0008	0.0005	<0.01	0.01	0.01
Molybdenum	1450	N	0.006	0.006	<0.05	0.06	0.5
Nickel	1450	N	0.007	0.005	<0.05	0.05	0.4
Lead	1450	N	0.002	0.004	<0.005	0.04	0.5
Antimony	1450	N	0.002	0.001	<0.01	0.01	0.06
Selenium	1450	N	0.006	0.003	0.01	0.03	0.1
Zinc	1450	N	0.01	0.005	<0.5	<0.5	4
Chloride	1220	N	13	3	26	35.8	800
Fluoride	1220	N	1.1	1.1	2.2	11	10
Sulphate	1220	N	46	9.5	91.9	116	1000
Total Dissolved Solids	1040	N	240	110	479	1170	4000
Phenol Index	1920	N	<0.03	<0.03	<0.5	<0.5	1
Dissolved Organic Carbon	1610	N	29	9	57.9	102	500

Solid Information

Dry mass of test portion/kg	0.175
Moisture (%)	23.1

Leach Test Information

Leachant volume 1st extract/l	0.297	4-Nov-2011
Leachant volume 2nd extract/l	1.4	4-Nov-2011
Eluate recovered from 1st extract/l	0.1013	

All tests undertaken between 28-Oct-2011 and 8-Nov-2011

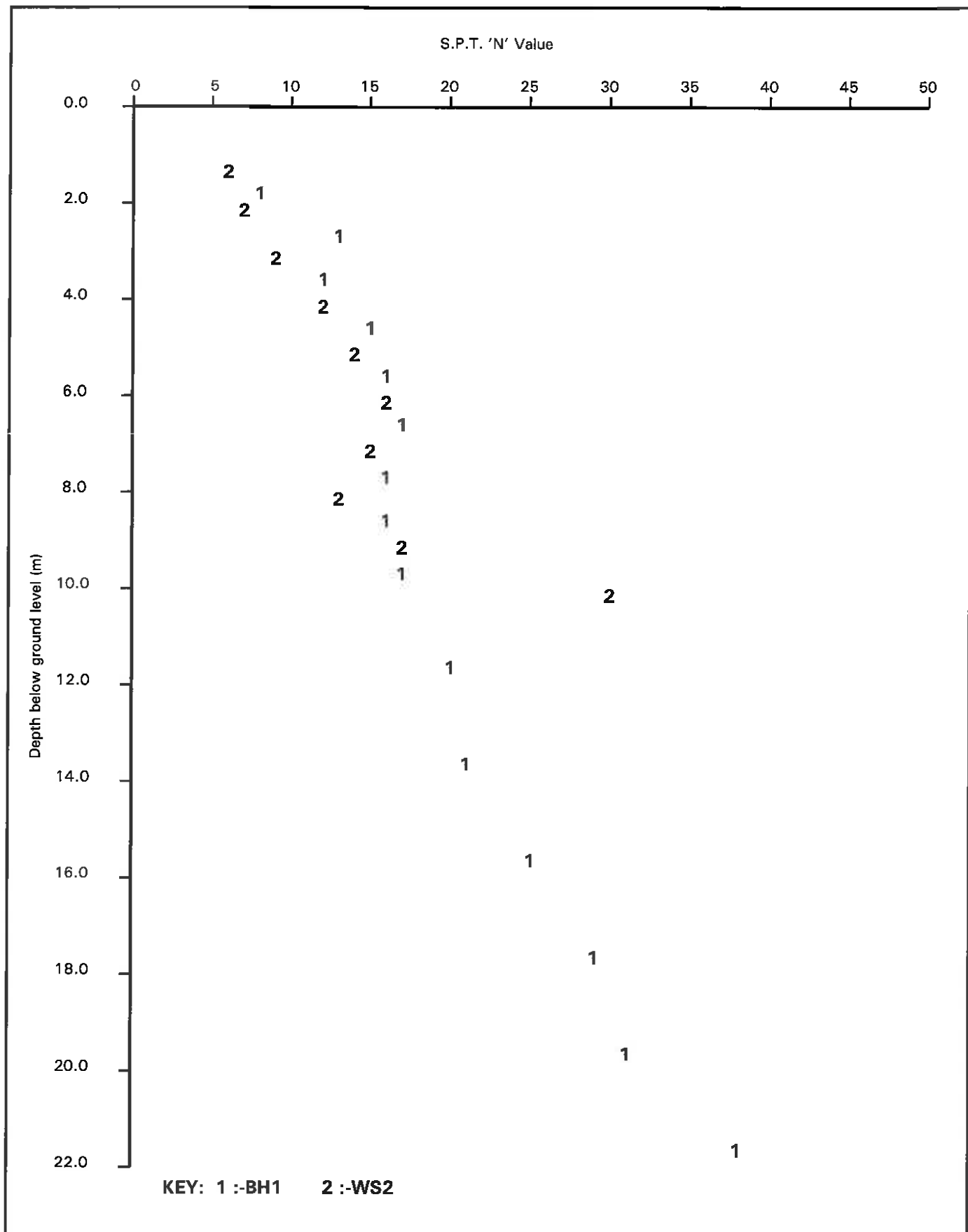
* Accreditation status

This report should be interpreted in conjunction with the notes on the accompanying cover page.

Column page 1

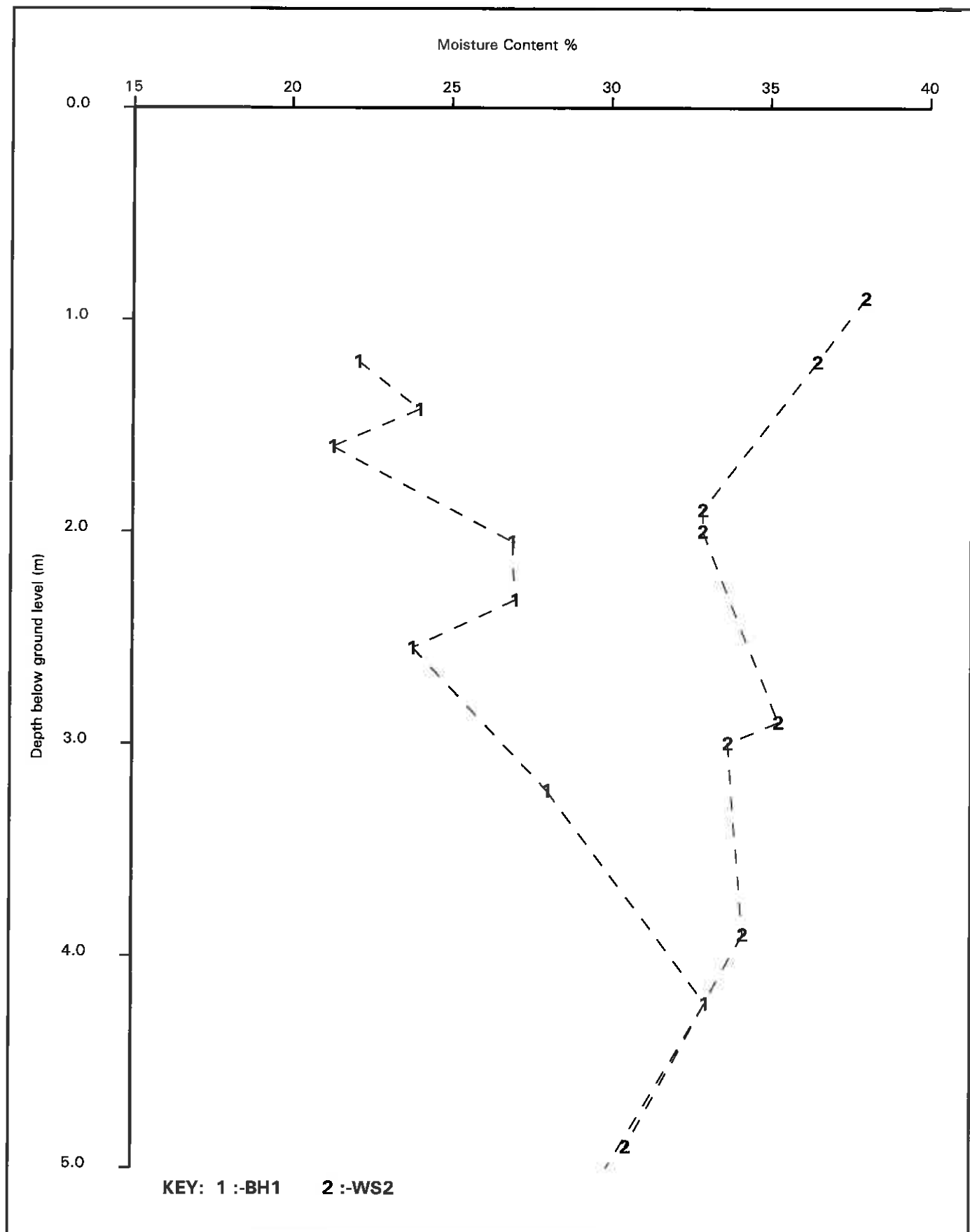
Report page 1 of 1

LIMS sample ID range AG63720 to AG63721



S.P.T. 'N' Value vs Depth below ground level (m).

SITE		11 WADHAM GARDENS, LONDON NW3	
CLIENT		MR. M. STEINBERG	Contract Number 12520
GROUND ENGINEERING L I M I T E D		Tel: 01733-566568 www.groundengineering.co.uk	Date 24/11/11 Figure 1



Moisture Content % vs Depth below ground level (m).

SITE

11 WADHAM GARDENS, LONDON NW3

CLIENT

MR. M. STEINBERG

Contract
Number 12520

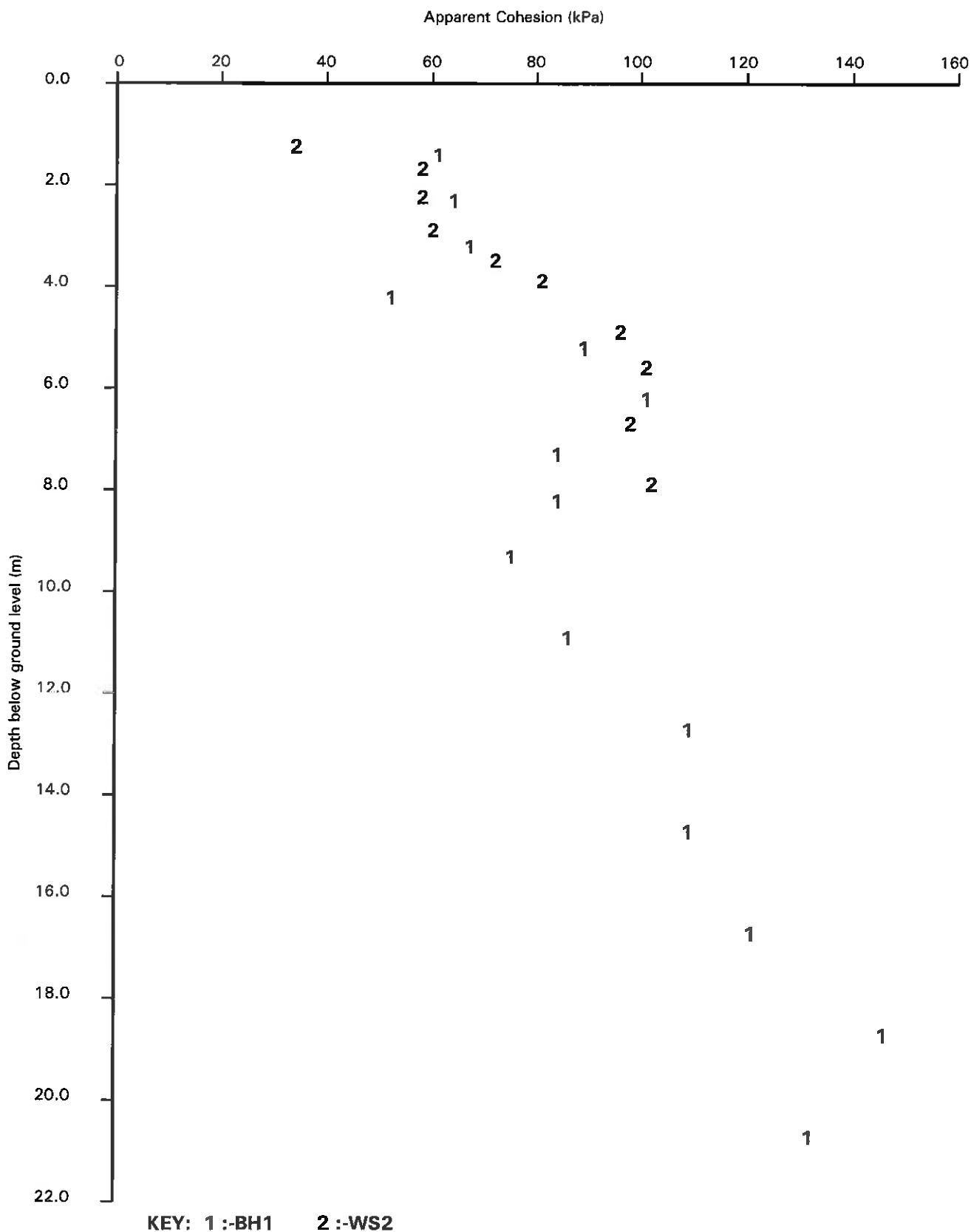
GROUND ENGINEERING

L I M I T E D

Tel: 01733-566566
www.groundengineering.co.uk

Date
24/11/11

Figure
2



Apparent Cohesion (kPa) vs Depth below ground level (m).

SITE

11 WADHAM GARDENS, LONDON NW3

CLIENT

MR. M. STEINBERG

Contract
Number

12520

GROUND ENGINEERING

L I M I T E D

Tel: 01733-566566

www.groundengineering.co.uk

Date

24/11/11

Figure

3

elevation
above
Ordnance
Datum
-metres

50
49
48
47
46
45
44
43
42
41
40
39
38
37
36
35
34
33
32
31
30
29
28
27













BH1



WS2



KEY TO LEGENDS

-  MADE GROUND
-  Soft or firm, slightly gravelly CLAY
HEAD DEPOSIT
-  Firm CLAY
WEATHERED LONDON CLAY
-  Firm or stiff, fissured CLAY
WEATHERED LONDON CLAY
-  Weak SILTSTONE
WEATHERED LONDON CLAY
-  Stiff, fissured CLAY with occasional silt partings
LONDON CLAY
-  Fibrous roots
-  Groundwater Encountered
-  Groundwater Rise
-  Level on Completion
-  Level Casing Withdrawn
-  Standpipe Level

SITE 11 WADHAM GARDENS, LONDON NW3

Contract No. 12520

CLIENT MR. M. STEINBERG

Soil Profile

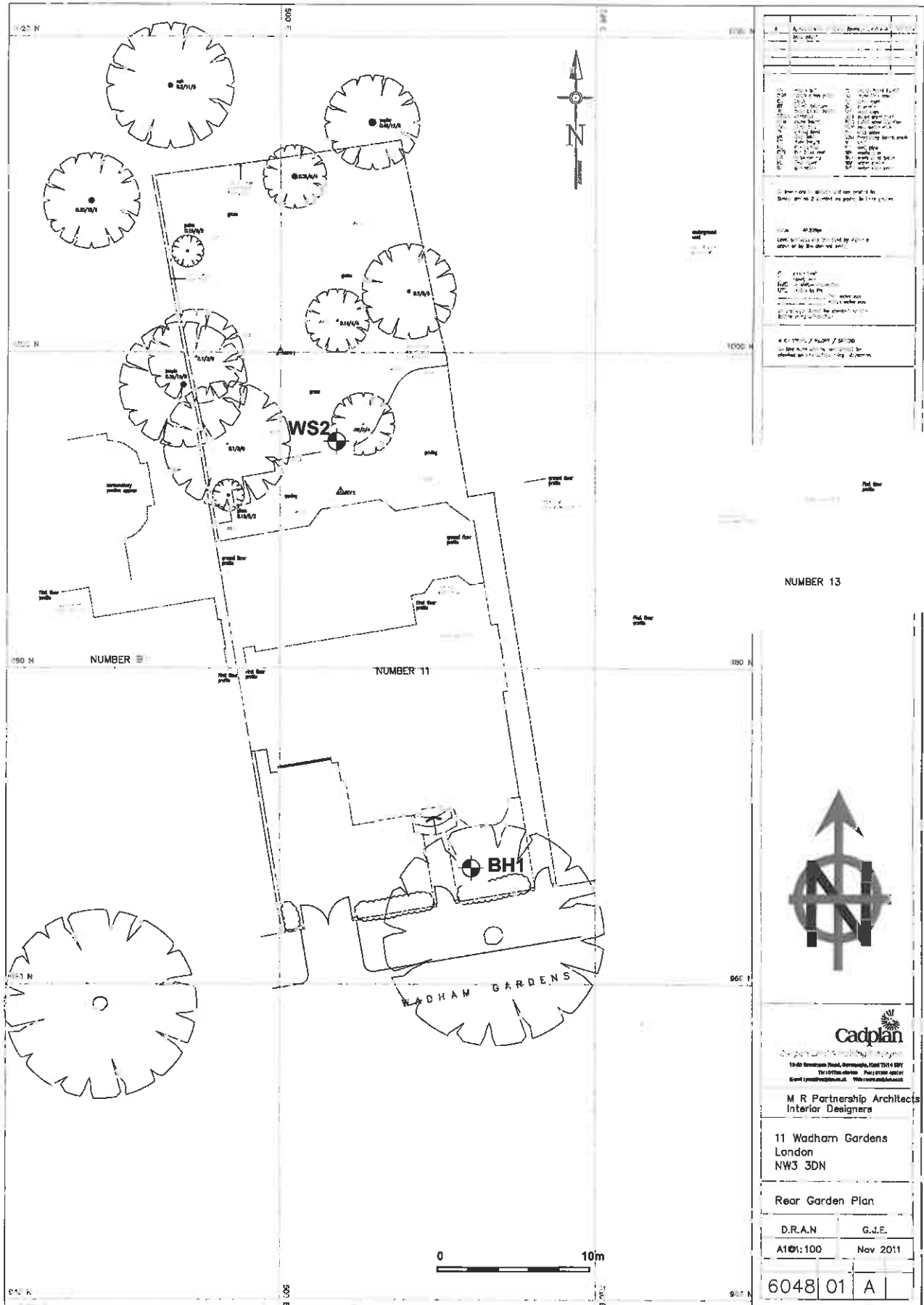
Vertical Scale 1:100

GROUND ENGINEERING LIMITED, PETERBOROUGH. Tel (01733) 566566

Date 24/11/11

Fig. No 4

Borehole Location Plan



Project: 11 Wadham Gardens, London NW3

Client: Mr. M. Steinberg

**GROUND
ENGINEERING
LIMITED**

Peterborough

Tel : 01733 566566

Project No.

C12520a