## **Cunningham Lindsey**

Subs Scanning Cntr, Woodhead House, Centre 27 Business Park, Woodhead Rd Birstall WF17 9TD Telephone 01489 567700 Facsimile 01489 565816

Policyholder

Subject Property Address:

97, Greencroft Gardens

**LONDON** 

NW6 3PG

**INSURANCE CLAIM** 

**CONCERNING SUBSIDENCE DAMAGE** 

## **ENGINEERING APPRAISAL REPORT**

This report is prepared on behalf of for the purpose of investigating a claim for subsidence. It is not intended to cover any other aspect of structural inadequacy or building defect that may otherwise have been in existence at the time of inspection.

Date: 23/07/2012

Cunningham Lindsey Ref: SOHPC/KLN/4291445

### Our Ref: SOHPC/KLN/4291445

### **INTRODUCTION**

This report has been prepared by our Chartered Engineer, Raymond Borrow BSc, CEng, MICE, and is being investigated in accordance with our Project Managed Service.

Unless stated otherwise all directions are referred to as looking towards the front door from the outside the property.

### **DESCRIPTION OF BUILDING**

The subject property is a semi detached property converted into 9 flats in a residential estate location on a plot that is level.

The overall layout is recorded on our site plan.

There are trees within influencing distance of the property located to both the front and rear of the property shown on our site plan dated 9<sup>th</sup> February 2012. The drainage system is a combined system which is shown on the attached plan.

## **CIRCUMSTANCES OF DISCOVERY OF DAMAGE**

The policyholder and homeowner, and the policyholder and the policyholder and homeowner, and the policyholder and homeowner, and the policyholder and homeowner, and the policyholder and the policyholde

A previous subsidence claim for the same area of damage was progressed in 2006.

The damage recurrence appeared suddenly.

The policyholder then advised insurers.

### NATURE AND EXTENT OF DAMAGE

Sketches showing the layout of the site and the damage are attached.

Description and Mechanism

The principal damage takes the form of tapering diagonal cracks in the region of 1 - 3 mm in widh

Internal damage takes the form of cracking to internal walls in the area of the communal hall, stairs and landing, Second Floor Flat 5, hall and rear right bedroom, Ground Floor Flat 2 front right lounge, Rear Addition Flat 9 lounge, kitchen and hall

There is current cracking evident to the exterior walls

The indicated mechanism of movement is both downwards movement to rear addition and rotation from the rear of the main building together with downwards movement to the rear right of the property and downwards movement to front right corner of the main building

Significance

The level of damage is slight, and is classified as category 2 in accordance with BRE Digest 251 - Assessment of damage in low-rise buildings

**Onset and Progression** 

advised that damage recurrence first commenced in Summer 2011.

We consider that the crack damage has occurred recently, but that distortions are historic.

It is likely that movement will be of a cyclical nature with cracks opening in the summer and closing in the winter.

### SITE INVESTIGATIONS

The ground investigation was carried out by CET Safehouse Ltd on 16<sup>th</sup> May 2012for details of the trial pit and borehole locations, together with test results, please refer to the attached CET factual report dated 30<sup>th</sup> May 2012.

### Trial Pit 1/Borehole 1

This was located at the front right of the building.

The underside of the foundation is at 1600 mm below ground level with the foundation comprising a brick corbel on a brick foundation. The soil beneath the property foundations has been identified as stiff, mid brown, grey veined silty clay with partings of orange and brown silt and fine sand, occasional gravel and stone nodules.

Samples of root taken from beneath the foundations have been analysed and originate from an Tilia spp. i.e. lime tree.

There are Lime trees located to the front right of the property which is in the ownership of the adjoining property at 99, Greencroft Gardens.

#### Trial Pit 2/Borehole 2

This was located at the rear right of the building.

The underside of the foundation is at 2175 mm below ground level with the foundation comprising a brick foundation. The soil beneath the property foundations has been identified as made ground soft, moist, mid brown, very silty clay with occasional gravel and brick, concrete and clinker fragments to a depth of 2600 mm below ground level with a firm, mid brown, grey veined silty clay beneath.

Samples of root taken from beneath the foundations have been analysed and originate from an Acer spp. are maples, including sycamore, Norway maple, and Japanese maples.

There is a Sycamore tree located to the front left of the property which is in the ownership of FFF Estates Ltd / Lawnpond Ltd.

#### Trial Pit 3/Borehole 3

This was located at the rear of the rear right extension.

The underside of the foundation is at 800 mm below ground level with the foundation comprising a concrete foundation. The soil beneath the property foundations has been identified as made ground: medium to compact, mid brown, silty clay with occasional gravel and brick, concrete and clinker fragments to a depth of 1100 mm below ground level with a firm, mid brown, grey veined silty clay beneath.

Samples of root taken from beneath the foundations have been analysed and originate from an Tilia spp. i.e. lime Tree.

There are Lime trees located to the rear of the property which is in the ownership of

#### Trial Pit 4/Borehole 4

This was located at the rear left of the rear right extension.

The underside of the foundation is at 700 mm below ground level with the foundation comprising a brick corbel on a concrete foundation. The soil beneath the property foundations has been identified as made ground: medium to compact, mid brown, silty clay with occasional gravel and brick, concrete and clinker fragments to a depth of 1800 mm below ground level with a firm, mid brown, grey veined silty clay beneath.

Samples of root taken from beneath the foundations have been analysed and originate from an Tilia spp. i.e. lime Tree.

There are Lime trees located to the rear of the property which is in the ownership of

The site investigation results confirm that sub soil beneath the building foundations at the various locations around the property is desiccated due to the effects of the nearby trees.

No drainage Investigations have been undertaken as the drainage system has been previously repaired.

### **MONITORING**

Level monitoring has been underway since 24<sup>th</sup> May 2012 and we await the first set of readings.

#### CAUSE OF DAMAGE

Taking an overview of all the site investigation referred to above, it is our opinion that the cause of damage results from clay shrinkage subsidence brought about by the action of roots from the Sycamore and Lime trees located in both the risk premises and the adjoining property at 99, Greencroft Gardens

We base this view on the fact that the foundations of the property in the area of damage have been built at a relatively shallow depth, bearing onto shrinkable clay subsoil. The soil is susceptible to movement as a result of changes in volume of the clay with variations in moisture content and analysis of the site investigation results indicates that the soil has been affected by shrinkage.

Sycamore and Limetree roots are present in the clay subsoil beneath the foundations.

In this case, were are satisfied that the damage has therefore been caused by clay shrinkage subsidence following moisture extraction by the Sycamore and Lime trees.

Continuation / 6 Our Ref: SOHPC/KLN/4291445

We are also satisfied that there is no factor, other than the Sycamore and Lime, that is causing the damage.

## **RECOMMENDATIONS**

We await further arboricultural advice from OCA UK Limited concerning the Sycamore and Lime trees outlined in the report above.

For Cunningham Lindsey

Raymond Borrow BSc, CEng, MICE Project Manager
Keeley Yeatman
Senior Customer Support
Direct dial: 01923 230209

E-mail: keeley.yeatman@cl-uk.com

## **FACTUAL REPORT**

## **OF**

# **INVESTIGATION**

AT:- 97, Greencroft Gardens

LONDON

ON:- 16 May 2012

FOR:-

c/o Cunningham Lindsey - Solent

REF:- 4291445-

JOB NO:- 123879

REPORT ISSUED:- 30/05/2012

## SPECIALIST CONTRACTING DIVISION

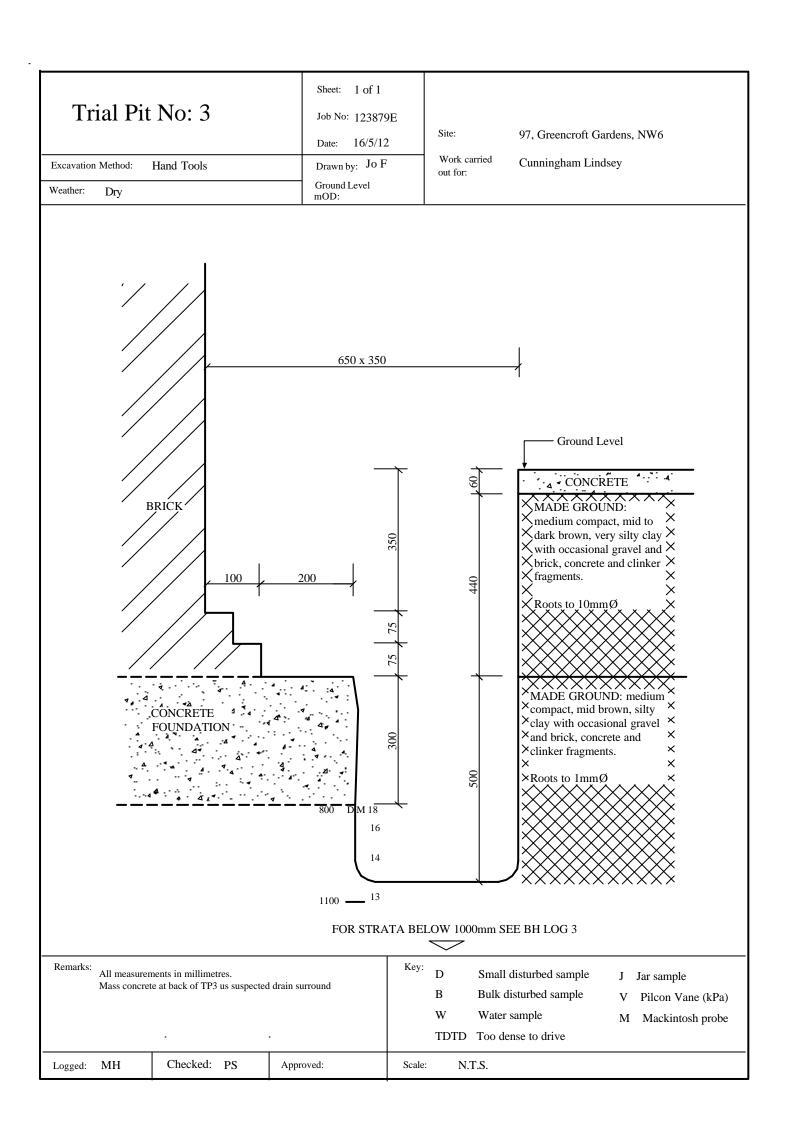
## CET SAFEHOUSE LIMITED

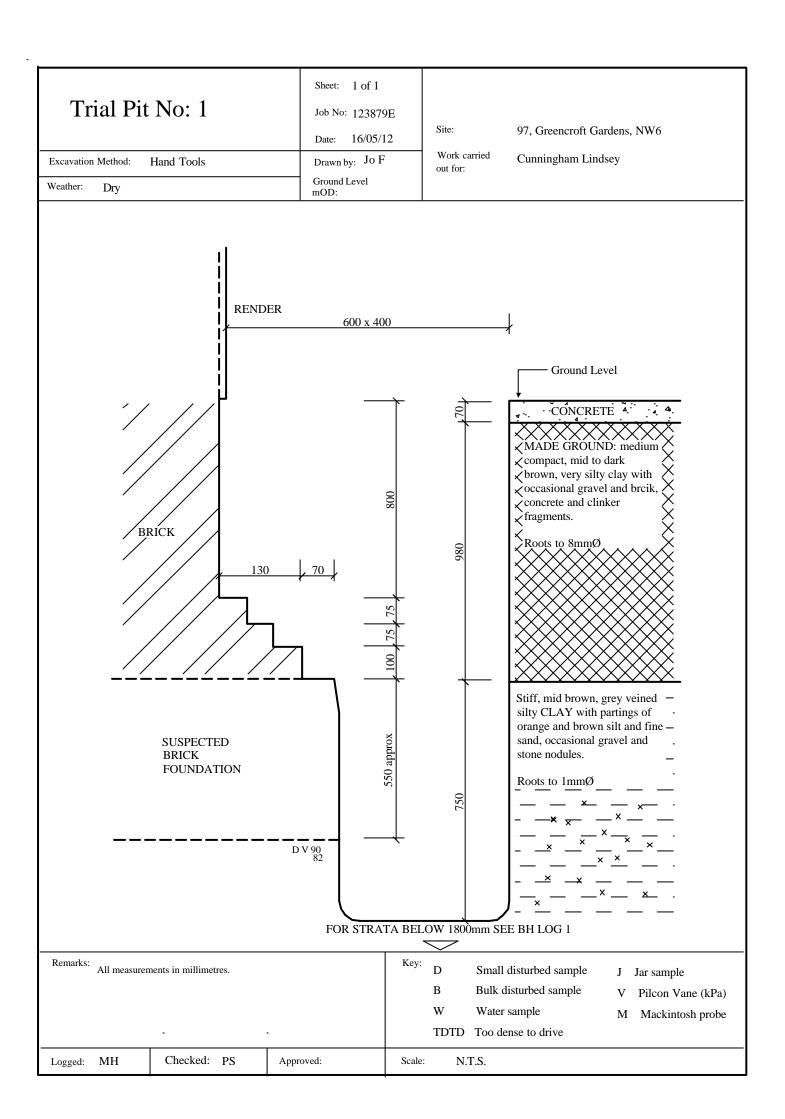
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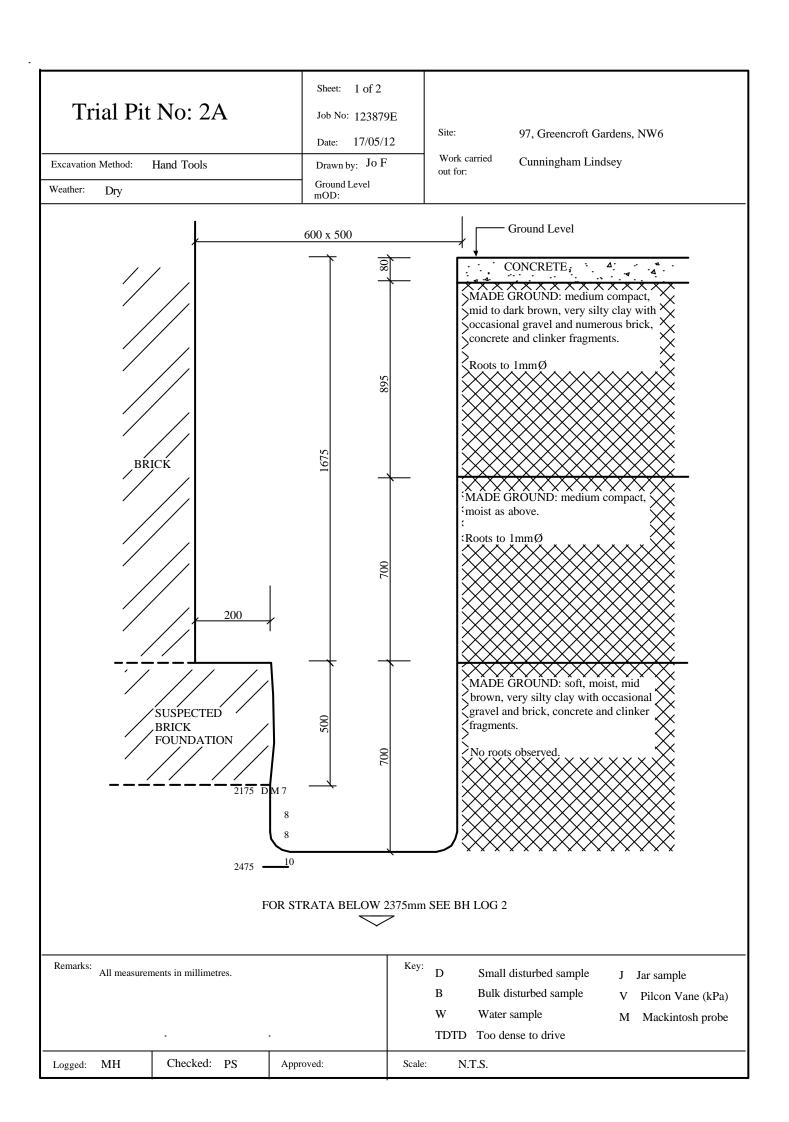
WWW.CETSAFEHOUSE.COM

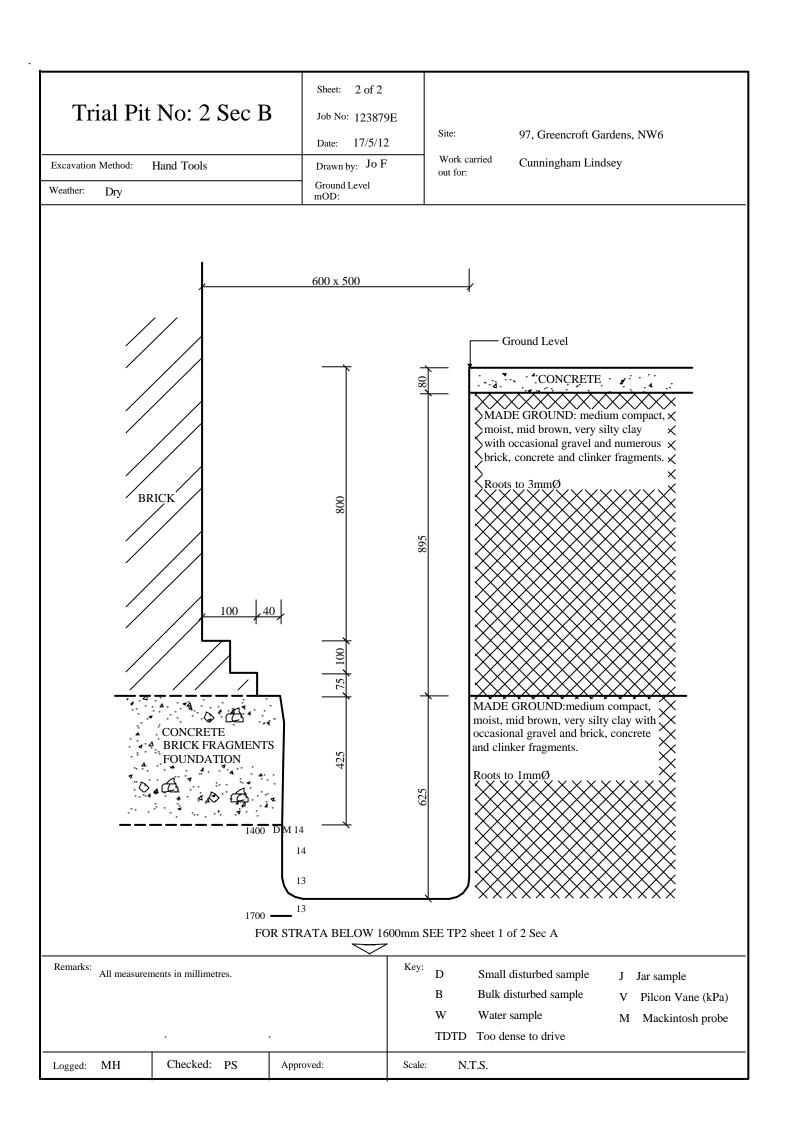
Tel: 01277 655377 Fax: 01277 655977

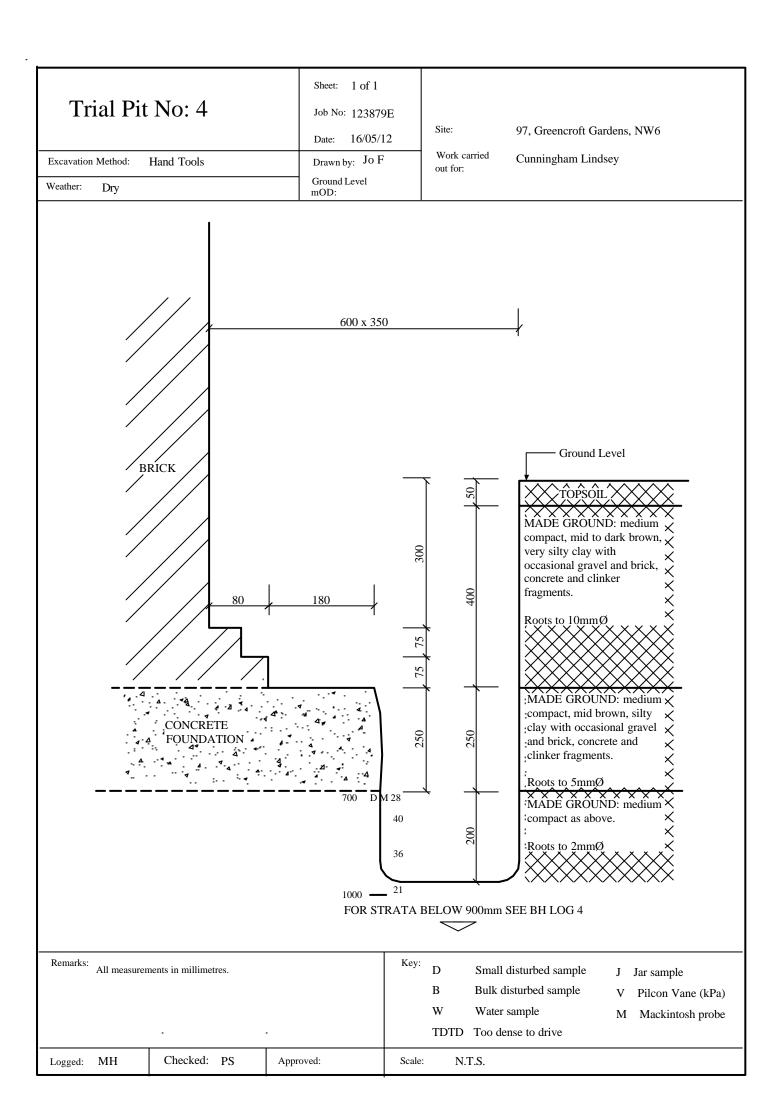
Investigation Layout Plan  MH PS (SI) (Checked)	Date:	&17/05/12		, Greencroft Gardens, NW ingham Lindsey	6
TREE - X1  X4  NO 99 X4		SMALL  TRUE  DSV  /WP	G D XI  TPE B A DSWC WC SVP		X1  NO 95 X4
	WG WG MH HEDGE TREE FOOTPA	HEDO TH	RWG TPI DA	BH1	EDGE TREES
	GREENCROFT  ON SITE TREE IDENTIFICA		ONLY NOT AUTHENT	ICATED	
Remarks:  Coal shute / basement in area of Scale:  N.T.S.		Key: Combined Gulley Manhole Rain Water Pipe Rain Water Gulle Soil Vent Pipe Waste Gulley Waste Pipe	RWWG MH RWP	Surface Water Drain Foul Water Drain Tree / Bush (approx. ht in m) Trial Pit Borehole	











Bor	ehole No:	1		Sheet:	1 of 2					
		& Datum		Job No:	123879E	i.	Site:			97, Greencroft Gardens, NW6
Boring	Method:	C.F.A		Date:	16/05/20	12				
Diame	ter: 100mm	Coordinates:		Ground 1	Level			Carried		Cunningham Lindsey
	T			mOD:			out for			
Depth (m)	I	Description of Strata		Thick- ness (m)	Legend	Sample	Туре	Гest Result	Depth (m)	Field Records/Comments Depth to water (m)
	As trial pit 1			1.80						
	with partings of	n, grey veined silty ( Forange and brown sional claystone nod	silt and	1.70	x x x. x. x	D D	v	98 104 112 114	2.50	Hair and fibrous roots to 3.0m with dead and decomposing root fragments to 5.0m
	Very stiff, mid CLAY with par silt and fine san nodules and cry	brown	4.50	x  x 	D D	V	150+ 150+	<ul><li>5.00</li><li>4.50</li></ul>		
Remar	l ks:				l —	D Sn B Bu	nall dis ılk distu	D. Too I turbed san	mple	J Jar sample V Pilcon Vane (kPa)
Logged	: MH	Checked: PS	Drawn By	Jo F		W W		nple NTS		M Mackintosh Probe  Weather: Dry

Bor	ehole No:	1	Sheet:	2 of 2						
		& Datum	Job No:	123879E	i.	Site:		9	7, Greencroft Gardens, NW6	
Boring	Method:	C.F.A	Date:	16/05/20	12					
Diame	ter: 100mm	Coordinates:	Ground i	Level		Work out for	Carried	C	Cunningham Lindsey	
Depth (m)	Ι	Description of Strata	Thick- ness (m)	Legend	Sample	-	Геst Result	Depth (m)	Field Records/Comments	Depth to water (m)
	CLAY with par	brown, grey veined silty tings of orange and brown d, occasional claystone estals		x	D	V	150+ 150+	5.00		
8.00	Borehole	e ends at 8.0m								
Datun	ole dry and oper n installed at 8.0	m. No soil samples taken o	r insitu strei	ngth	D Sr B Bu	nall dis ılk distı	turbed sa urbed san	nple V	Jar sample Pilcon Vane (kPa)	
ests c	arried out below : MH	Checked: PS Drawn b	y Jo F		W W	ater sar	nple NTS	N W	Mackintosh Probe Veather: Dry	

Bore	ehole No:	2	Sheet:	1 of 1						
			Job No:	123879E	Ξ	Site:			97, Greencroft Gardens, NW6	
Boring I	Method:	Hand Auger	Date:	17/05/20	)12					
Diamete	er: 75mm	Coordinates:	Ground I	Level		Work out fo	Carried		Cunningham Lindsey	
Depth			Thick-				Test		1	Depth
(m)		Description of Strata	ness (m)	Legend	Sample	Туре	Result	Depth (m)		(m)
A	As trial pit 2		2.375							
2.375									Dead and decomposing root	
		ND: soft, moist, mid brown with occasional gravel and	0.225		D	M	10	2.50	fragments to 5.0m	
		and clinker fragments		v			34 34			
				x			34 36			
v a	with partings o	orn, grey veined, silty CLAY of orange and brown silt occasional claystone	1.20	 x	D	V	118 110	3.00		
	iodules and ci	ystais		x.	D	V	128 132	3.50		
	Very stiff as ab	ove		x x 	D	V	150+ 150+	4.00		
			1.20	x  x.	D	V	150+ 150+	4.50		
5.00	Borehol	e ends at 5.0m		x	D	V	150+ 150+	5.00		
Remarks				I	Key:		.D. Too I			
Boreho.	le dry and ope	n on completion			В В		urbed san		<ul><li>J Jar sample</li><li>V Pilcon Vane (kPa)</li><li>M Mackintosh Probe</li></ul>	
5.00 _	Borehol		1.20		D D Key: D Si B Bi	V V V T.D.T mall disulk dist	150+ 150+ 150+ 150+ 150+ 150+ 150+ 150+	4.00 4.50 5.00 Dense to mple	Drive J Jar sample V Pilcon Vane (kPa)	

Bor	ehole No:	3	Sheet:	1 of 1					
			Job No:	123879E	Ε	Site:			97, Greencroft Gardens, NW6
Boring	Method:	Hand Auger	Date:	16/05/20	)12				
Diame	ter: 75mm	Coordinates:	Ground I	Level		Work out for	Carried		Cunningham Lindsey
Depth			Thick-			-	Test		Depth
(m)	:	Description of Strata	ness (m)	Legend	Sample	_	Result	Depth (m)	Field Records/Comments to water (m)
	As trial pit 3		1.00						
1.00	brown, silty cla	ND: medium compact, mid ay with occasional gravel crete and clinker fragments	0.10	X	D	М	12 14 16	1.00	Roots to 1mm diameter to 2.1m with dead and decomposing root fragments to 5.0m
1.80	with partings o	Firm, mid brown, grey veined silty CLAY with partings of orange and brown silt and tine sand, occasional gravel and stone		x  x	D	V	15 56 60	1.50	
			1.70	x  x 	D D	v	84 94 100 100	2.00	
	nodules and cry	ystals		x.	D	V	118 126	3.00	
3.50				x x	D	V	150 150+	3.50	
	Very stiff as ab	ove	1.50	 	D	V	150+ 150+	4.00	
				x.	D	V	150+ 150+	4.50	
5.00	Borehol	e ends at 5.0m		X	D	V	150+ 150+	5.00	
Remar Boreh	ole dry and ope	n on completion  Checked: PS Drawn By	Jo F	1	В В	nall dis	.D. Too I sturbed san urbed san mple	mple	Drive J Jar sample V Pilcon Vane (kPa) M Mackintosh Probe Weather: Dry

Boreho	le No:	4		1 of 1 123879I	7	Site:			07. Caramanafa Candana NWG
Boring Metho	od:	Hand Auger	Date:	16/05/20		site:			97, Greencroft Gardens, NW6
Diameter:	75mm	Coordinates:	Ground I	Level		Work out for	Carried r:		Cunningham Lindsey
Depth (m)	:	Description of Strata	Thick- ness (m)	Legend	Sample		Test Result	Depth (m)	Field Records/Comments to water (m)
As tri	al pit 4		1.00		D	V	60	1.00	Roots to 1mm diameter to 2.0m with dead and decomposing
Firm, silty, orang	gravelly e and bro ccasional	on / orange, grey veined CLAY with partings of wn silt and fine sand stone nodules	0.20	x o			60		root fragments to 5.0m
with p	Stiff as above		0.30	X 	D	V	84 94	1.50	
Stiff a			0.30			**		2.00	
with p	partings o	n, grey veined silty CLAY f orange and brown silt occasional claystone	1.20	x   x	D	V	98 104 102	2.00	
nodul	es and cr	ystals					92		
3.00				X.	D	V	150+ 150+	3.00	
					D	V	150 150+	3.50	
Very	stiff as ab	ove	2.00		D	V	150+ 150+	4.00	
				x	D	V	150+ 150+	4.50	
5.00	Borehol	e ends at 5.0m			D	V	150+ 150+	5.00	
Remarks: Borehole dry	y and ope	n on completion		1	В В	nall dis ılk dist	.D. Too I turbed sar urbed san	mple	J Jar sample V Pilcon Vane (kPa)
Logged:	MH	Checked: PS Drawn By	Jo F		W W Scale:	ater saı	mple NTS		M Mackintosh Probe Weather: Dry

# **Laboratory Testing Results**

Location: 97. Greencroft Gardens

Work carried

out for:

Cunningham Lindsey - Solent Date Tested: 18/05/2012 Date of Report: 29/05/2012

TP/BH	Sample Ref	Tyma	Moisture Content	Soil Fraction	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Modified Plasticity	Soil Class	Filter Paper Contact	Soil Sample	In situ Shear Vane	Organic Content	pH Value	Sulphate ( g		Class
No No	Depth (m)	Туре	(%) [1]	> 0.425mm (%) [2]	(%)[3]	(%)[4]		[5]	Index (%)[6]	[7]	Time	Suction (kPa)	Strength (kPa) [9]	( % )[10]	[11]	so <sub>3</sub>	so <sub>4</sub> [13]	[14]
1	1.60(U/S)	D	29	<5	77	23	53	0.11	53	CV	168	99	86					
	2.0	D	27	<5	75	22	53	0.11	53	CV	168	140	101					
	2.5	D	27	<5														
	3.0	D	29	<5	66	24	43	0.12	43	СН	168	155	113					
	3.5	D	31	<5														
	4.0	D	31	<5							168	183	> 150					
	4.5 5.0	D D	30 32	<5 <5							168	208	> 150					
	3.0	ט	32	<)							108	208	> 150					

#### Test Methods / Notes

- [1] BS 1377 : Part 2 : 1990, Test No 3.2
- [2] Estimated if <5%, otherwise measured
- [3] BS 1377: Part 2: 1990, Test No 4.4
- [4] BS 1377: Part 2: 1990, Test No 5.3
- [5] BS 1377 : Part 2 : 1990, Test No 5.4
- [6] BRE Digest 240: 1993
- [7] BS 5930: 1981: Figure 31 Plasticity Chart for the classification
- [8] In-house method S9a adapted from BRE IP 4/93

- [9] Values of shear strength were determined in situ by CET Safehouse using
- a Pilcon hand vane or Geonor vane (GV).
- [10] BS 1377: Part 3: 1990, Test No 4
- [11] BS 1377: Part 3: 1990, Test No 9
- [12] BS 1377: Part 3: 1990, Test No 5.6
- [13]  $SO_4 = 1.2 \times SO_3$
- [14] BRE Special Digest One (Concrete in Aggressive Ground) 2005

Note that if the SO<sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class respectively unless water soluble magnesium testing is undertaken to prove otherwise

### Key

D Disturbed sample ( small ) В Disturbed sample ( bulk ) U Undisturbed sample

W Groundwater sample

ENP Essentially Non-Plastic by inspection

Date Sampled:

Date Received:

16+17/05/2012

18/05/2012

U/S Underside of Foundation

# **Laboratory Testing Results**

97, Greencroft Gardens Location:

Cunningham Lindsey - Solent Work carried

out for:

Date Received: 18/05/2012 Date Tested: 18/05/2012 Date of Report: 29/05/2012

16+17/05/2012

Date Sampled:

S	ample Ref.		Moisture	Soil	Liquid	Plastic	Plasticity	Liquidity	Modified	Soil	Filter Paper	Soil	In situ	Organic	pН	Sulphate	Content	
TP/BH	Depth	Type	Content	Fraction	Limit	Limit	Index	Index	Plasticity	Class	Contact	Sample	Shear Vane	Content	Value	( g		Class
No.	( m )			> 0.425mm					Index		Time	Suction	Strength			so <sub>3</sub>	so <sub>4</sub>	
			(%)[1]	(%)[2]	(%)[3]	(%)[4]	(%)[5]	[5]	(%)[6]	[7]	(h) [8]	(kPa)	(kPa) [9]	(%)[10]	[11]	[12]	[13]	[14]
2	1.40(U/S)	D	39	<5	66	24	42	0.36	42	СН	168							
	2.18(U/S)	D	38	<5	63	23	40	0.37	40	СН	168							
	2.5	D	37	10														
	3.0	D	30	<5	72	26	47	0.10	47	CV	168	112	114					
	3.5	D	32	<5									130					
	4.0	D	32	<5							168	127	> 150					
	4.5	D	33	<5									> 150					
	5.0	D	31	<5							168	170	> 150					

Test Methods / Notes

[9] Values of shear strength were determined in situ by CET Safehouse using

a Pilcon hand vane or Geonor vane (GV).

Note that if the SO<sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class respectively unless water soluble magnesium testing is undertaken to prove otherwise

Key D

Disturbed sample ( small ) В Disturbed sample ( bulk )

U Undisturbed sample

Groundwater sample

Essentially Non-Plastic by inspection ENP

Underside of Foundation U/S

<sup>[1]</sup> BS 1377 : Part 2 : 1990, Test No 3.2

<sup>[2]</sup> Estimated if <5%, otherwise measured

<sup>[3]</sup> BS 1377: Part 2: 1990, Test No 4.4

<sup>[4]</sup> BS 1377: Part 2: 1990, Test No 5.3

<sup>[5]</sup> BS 1377: Part 2: 1990, Test No 5.4

<sup>[6]</sup> BRE Digest 240: 1993

<sup>[7]</sup> BS 5930: 1981: Figure 31 - Plasticity Chart for the classification of fine soils

<sup>[8]</sup> In-house method S9a adapted from BRE IP 4/93

<sup>[10]</sup> BS 1377: Part 3: 1990, Test No 4 [11] BS 1377 : Part 3 : 1990, Test No 9

<sup>[12]</sup> BS 1377: Part 3: 1990, Test No 5.6

<sup>[13]</sup> SO<sub>4</sub> = 1.2 x SO<sub>3</sub>

<sup>[14]</sup> BRE Special Digest One (Concrete in Aggressive Ground) 2005

# **Laboratory Testing Results**

97, Greencroft Gardens Location:

Work carried Cunningham Lindsey - Solent

out for:

Date Received: 18/05/2012 Date Tested: 18/05/2012 Date of Report: 29/05/2012

Date Sampled:

16+17/05/2012

	ample Ref.	m	Moisture	Soil	Liquid	Plastic	Plasticity	Liquidity	Modified	Soil	Filter Paper	Soil	In situ	Organic	pН		Content	CI.
TP/BH No.	Depth ( m )	Type	Content	Fraction > 0.425mm	Limit	Limit	Index	Index	Plasticity Index	Class	Contact Time	Sample Suction	Shear Vane Strength	Content	Value	so <sub>3</sub>	/1) so <sub>4</sub>	Class
110.	(m)		(%)[1]	(%) [2]	(%)[3]	(%)[4]	(%)[5]	[5]	(%)[6]	[7]		(kPa)	(kPa) [9]	(%)[10]	[11]	[12]	[13]	[14]
3	0.80(U/S)	D	32	6	66	25	41	0.16	39	СН								
	1.0	D	31	14														
	1.5	D	32	<5							168	86	58					
	2.0	D	29	<5	71	24	47	0.11	47	CV	168	166	89					
	2.5	D	31	<5									100					
	3.0	D	32	<5	72	25	47	0.15	47	CV	168	161	122					
	3.5	D	33	<5									> 150					
	4.0	D	32	<5							168	217	> 150					
	4.5	D	32	<5									> 150					
	5.0	D	32	<5							168	258	> 150					

#### Test Methods / Notes

- [1] BS 1377 : Part 2 : 1990, Test No 3.2
- [2] Estimated if <5%, otherwise measured
- [3] BS 1377: Part 2: 1990, Test No 4.4
- [4] BS 1377: Part 2: 1990, Test No 5.3
- [5] BS 1377: Part 2: 1990, Test No 5.4
- [6] BRE Digest 240: 1993
- [7] BS 5930: 1981: Figure 31 Plasticity Chart for the classification
- [8] In-house method S9a adapted from BRE IP 4/93

- [9] Values of shear strength were determined in situ by CET Safehouse using
- a Pilcon hand vane or Geonor vane (GV). [10] BS 1377: Part 3: 1990, Test No 4
- [11] BS 1377: Part 3: 1990, Test No 9
- [12] BS 1377: Part 3: 1990, Test No 5.6
- [14] BRE Special Digest One (Concrete in Aggressive Ground) 2005

Note that if the SO<sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class respectively unless water soluble magnesium testing is undertaken to prove otherwise

#### Key

D Disturbed sample ( small ) В Disturbed sample (bulk) Undisturbed sample

U Groundwater sample

ENP Essentially Non-Plastic by inspection Underside of Foundation U/S

# **Laboratory Testing Results**

Location: 97, Greencroft Gardens

Work carried

123879

out for:

Our Ref:

Cunningham Lindsey - Solent Date Tested: Date of Report:

	ample Ref.		Moisture	Soil	Liquid	Plastic	Plasticity	Liquidity	Modified	Soil	Filter Paper	Soil	In situ	Organic	pН	Sulphate		
TP/BH	Depth	Type	Content	Fraction	Limit	Limit	Index	Index	Plasticity	Class	Contact	Sample	Shear Vane	Content	Value	( g SO3	/1) so <sub>4</sub>	Class
No.	( m )		(%)[1]	> 0.425mm (%) [2]	(%)[3]	(%)[4]	(%)[5]	[5]	Index (%)[6]	[7]	Time (h) [8]	Suction (kPa)	Strength (kPa) [9]	(%)[10]	[11]	[12]	[13]	[14]
			, , ,	, , , , ,	, ,, ,	, , , , , ,	, ,, , , ,		( / 0 / [ 0 ]		, , , , ,	( 1)	, , ,	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				
4	0.70(U/S)	D	27	11	63	23	41	0.09	36	СН								
	1.0	D	25	26	61	23	38	0.05	28	СН	168	83	60					
	1.5	D	29	<5									89					
	2.0	D	29	<5	64	21	43	0.18	43	СН	168	204	101					
	2.5	D	32	<5									97					
	3.0	D	32	<5	72	26	47	0.12	47	CV	168	157	> 150					
	3.5	D	32	<5									> 150					
	4.0	D	31	<5							168	232	> 150					
	4.5	D	33	<5									> 150					
	5.0	D	32	<5							168	237	> 150					
	Orada / Nista																	

#### Test Methods / Notes

- [1] BS 1377 : Part 2 : 1990, Test No 3.2
- [2] Estimated if <5%, otherwise measured
- [3] BS 1377: Part 2: 1990, Test No 4.4
- [4] BS 1377: Part 2: 1990, Test No 5.3
- [5] BS 1377: Part 2: 1990, Test No 5.4
- [6] BRE Digest 240: 1993
- [7] BS 5930: 1981: Figure 31 Plasticity Chart for the classification
- [8] In-house method S9a adapted from BRE IP 4/93

- [9] Values of shear strength were determined in situ by CET Safehouse using
- a Pilcon hand vane or Geonor vane (GV).
- [10] BS 1377: Part 3: 1990, Test No 4 [11] BS 1377: Part 3: 1990, Test No 9
- [12] BS 1377: Part 3: 1990, Test No 5.6

- [14] BRE Special Digest One (Concrete in Aggressive Ground) 2005

Note that if the SO<sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4m or DS-5m class respectively unless water soluble magnesium testing is undertaken to prove otherwise

#### Key

D Disturbed sample ( small ) В Disturbed sample (bulk) U Undisturbed sample Groundwater sample

W ENP Essentially Non-Plastic by inspection

Underside of Foundation U/S

Date Sampled:

Date Received:

16+17/05/2012

18/05/2012

18/05/2012

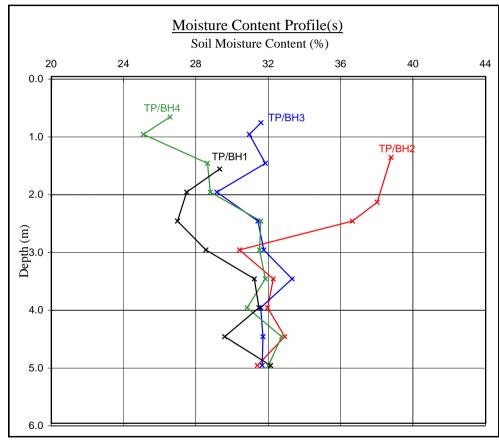
29/05/2012

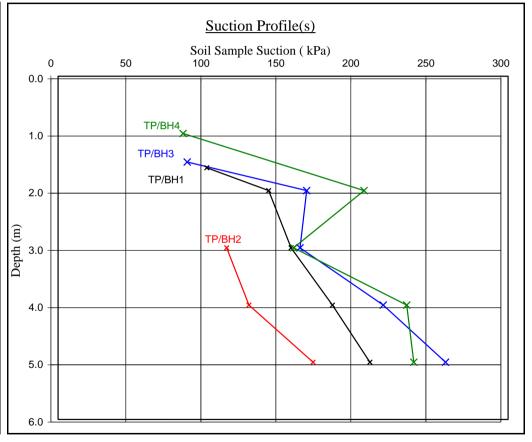
# Moisture Content and Suction Profiles

Location: 97, Greencroft Gardens Date Received: 18/05/2012

Work carried Cunningham Lindsey - Solent Note: Unless specifically noted the profiles have not been Date Tested: 18/05/2012

out for: Pate of Report : 29/05/2012





Date Sampled:

16+17/05/2012

#### Note

Our Ref:

123879

If plotted, 0.4 LL and PL+2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clays) at shallow depths.

#### Not

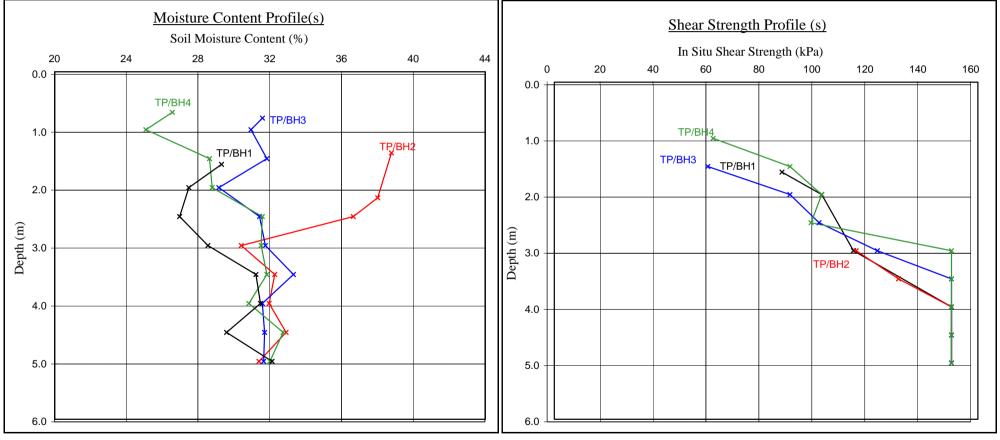
When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clays) at shallow depths. Note that the sample disturbance component is dependant on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompacted samples. This may or may not be appropriate in this instance and judgement should be exercised.

# Moisture Content and Shear Strength Profiles Date Sampled:

Location: 97, Greencroft Gardens Date Received: 18/05/2012

Work carried Cunningham Lindsey - Solent Note: Unless specifically noted the profiles have not been Date Tested: 18/05/2012

out for: Pate of Report : 29/05/2012



#### Note

If plotted, 0.4 LL and PL+2 ( after Driscoll, 1983 ) should only be applied to London Clay ( and similarly overconsolidated clays ) at shallow depths.

#### Note

Unless otherwise stated, values of Shear Strength were determined in situ by CET Safehouse using a Pilcon Hand Vane the calibration of which is limited to a maximum reading of 150 kPa.

16+17/05/2012

	Sheet:	1 of 1		
<b>EPSL</b> European Plant Science Laboratory	Job No: Date: Order No:	123879 22/05/2012 401524	Site:  Work carried out for:	97 Greencroft Gardens, London  Cunningham Lindsey
	EPSL Ref:	220512114412		·

## Certificate of Analysis

The following work was commissioned by CET Safehouse Limited on behalf of their client. Root samples were obtained in sealed packets from the above site with no reference given as to the types of tree or shrub from which they may have originated.

The results were as follows -

Trial pit/ Borehole <u>number</u>	Root diameter ( <u>mm</u> )	Tree, shrub or climber from which root originates	Result of starch test
TP1 (U/S)	1.5	Tilia spp.	Negative
BH1 (to 3m)	<1	(3 roots) broadleaved species, too juvenile for positive identification * (2 roots)	Negative
TP2 (U/S)	<1	(2 roots)  Acer spp. **  (2 roots)	Positive
TP3 (U/S)	1	Tilia spp.	Positive
TP3 (U/S)	<1	broadleaved species, too juvenile for positive identification (2 roots)	Negative
BH3 (to 2.1m)	<1	probably Tilia spp. (2 roots)	Negative
TP4 (U/S)	1.5	Rosa spp. (3 roots)	Positive
BH4 (to 2m)	1.5	Tilia spp. (2 roots)	Positive

<sup>\*</sup> Also in a state of decay.

Tilia spp. are limes.

 $Acer\ spp.\ are\ maples, including\ sycamore,\ Norway\ maple,\ and\ Japanese\ maples.$ 

Rosa spp. are roses.

Heltwheel

Address for correspondence: EPSL, Intec, Parc Menai, Bangor, Gwynedd, North Wales, LL57 4FG

**Telephone:** 01248 672 652

 $\textbf{\textit{e-mail:}}\ \, lab@marishalthompson.co.uk$ 

Head of laboratory Services: M D Mitchell B.Sc., M.Phil.

Plant Anatomist: Dr G S Turner B.Sc., M.Sc., Ph.D

Plant Anatomist: Dr F M Cardias Williams B.Sc., M.Sc., Ph.D.

Consultant: Dr M P Denne B.Sc., M.Sc., Ph.D

Registered in England. No 295427, Registered Office: Bank Chambers, 29 High Street, Ewell, Epsom, Surrey, KT17 1SB

<sup>\*\*</sup> Both rather juvenile.



# **Level Monitoring - Relative Movement Over Time**

Our Ref: 00/27297 Date of Issue 18/01/13

## **Provider Details**

Name: CET Safehouse

## **Monitoring Details**

Instruction Date:	
First Reading Date:	24/05/12
Maximum No Visits:	5
Anticipated Expiry Date:	Mar-13
Monitoring Int (Wks):	8

# **Client Details**

Insurance Co.:	
Client Name:	Cunningham Lindsey
Technical Mgr:	Raymond Borrow
Email:	
Client Ref:	4291445
Address:	
Tel	
Other Email:	

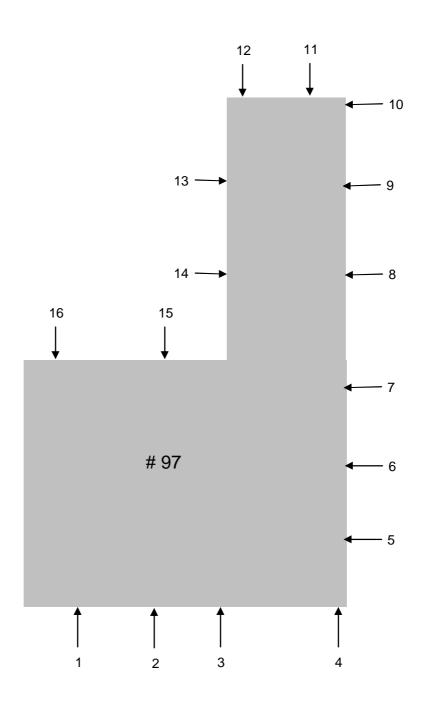
## **Risk Address**

Occupier:	
Address:	97 Greencroft Gardens
Address:	
Town:	London
County:	
Post Code:	NW6 3PG
Tel Day:	
Tel Eve:	
Mobile:	
Other:	
Other:	



# **Level Monitoring - Site Sketch**

Our Ref: 00/27297 Date of Issue 18/01/13





# **Level Monitoring - Comments**

Our Ref: 00/27297 Date of Issue 18/01/13

Using deep datum at front of property.



# **Level Monitoring - Reduced Levels**

Our Ref: 00/27297 Date of Issue 18/01/13

Our Not. 00/21/231						Date 01 13340 10/01/13						
Reading Date	24/05/12	26/07/12	19/09/12	12/11/12	10/01/13							
Point / Reading	1	2	3	4	5	6	7	8	9	10	11	12
1	11.220	11.219	11.217	11.219	11.221							
2	11.295	11.293	11.290	11.293	11.295							
3	11.278	11.276	11.271	11.274	11.277							
4	11.308	11.307	11.302	11.305	11.308							
5	10.607	10.606	10.602	10.603	10.607							
6	10.645	10.645	10.642	10.643	10.646							
7	10.679	10.679	10.678	10.677	10.680							
8	10.524	10.524	10.523	10.522	10.525							
9	10.507	10.507	10.507	10.505	10.508							
10	10.179	10.180	10.179	10.178	10.181							
11	10.311	10.310	10.310	10.308	10.311							
12	10.283	10.282	10.279	10.278	10.283							
13	10.079	10.079	10.076	10.075	10.080							
14	10.252	10.252	10.248	10.246	10.251							
15	10.749	10.749	10.749	10.746	10.749							
16	10.761	10.761	10.761	10.759	10.761							

Highest reduced level is displayed green, lowest reduced level is displayed red



# **Level Monitoring - Relative Movement Over Time**

Our Ref: 00/27297 Date of Issue 18/01/13

Our Not. 00/21/201							Date 01 13346 10/01/13					
Reading Date	24/05/12	26/07/12	19/09/12	12/11/12	10/01/13							
Point / Reading	1	2	3	4	5	6	7	8	9	10	11	12
1	0	-1	-3	-2	1							
2	0	-2	-5	-2	0							
3	0	-2	-7	-3	0							
4	0	-2	-6	-4	0							
5	0	-1	-6	-4	-1							
6	0	-1	-3	-3	0							
7	0	0	-1	-2	1							
8	0	0	-1	-2	1							
9	0	0	0	-1	2							
10	0	1	0	-1	2							
11	0	-1	-1	-3	1							
12	0	-1	-4	-5	0							
13	0	0	-3	-4	0							
14	0	-1	-4	-6	-2							
15	0	0	0	-3	0							
16	0	0	0	-2	0							



# Level Monitoring - Movement relative to datum against Date

