BASIC GEOTECHNICAL GROUND INVESTIGATION REPORT

AT

15 SOUTH HILL PARK LONDON, NW3



Specialists in the investigation & reclamation of brownfield sites



Report Title: Basic Geotechnical Ground Investigation Report at 15 South Hill Park, London,
NW3Job No: P8525J292

Report Status : Final v1.1

Date: September 2013

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1 INTRODUCTION

1.1 Terms of Reference

- 1.1.1 David Zobel ("The Client") has commissioned Jomas Associates Ltd ('JAL'), to obtain ground parameters at 15 South Hill Park, London NW3, to enable preliminary foundation recommendations to be offered, prior to redevelopment of the site for residential use.
- 1.1.2 To this end a basic intrusive investigation of the site, has been undertaken in accordance with JAL's email proposal dated 08 August 2013.

1.2 Objectives

- 1.2.1 The objectives of JAL's investigation were as follows:
 - To assess ground conditions and obtain geotechnical parameters to inform foundation design, which is to be undertaken by the structural engineer.

1.3 Scope of Works

- 1.3.1 The following tasks were undertaken to achieve the objectives listed above:
 - Basic intrusive ground investigation to determine shallow ground conditions;
 - Laboratory geotechnical and chemical testing on soil samples collected from the site; and,
 - The compilation of this report, which provides data above, and indicative recommendations for foundation design.

1.4 Limitations

- 1.4.1 Jomas Associates Ltd ('JAL') has prepared this report for the sole use of David Zobel, in accordance with the generally accepted consulting practices and for the intended purposes as stated in the agreement under which this work was completed. This report may not be relied upon by any other party without the explicit written agreement of JAL. No other third party warranty, expressed or implied, is made as to the professional advice included in this report. This report must be used in its entirety.
- 1.4.2 The records search was limited to information available from public sources; this information is changing continually and frequently incomplete. Unless JAL has actual knowledge to the contrary, information obtained from public sources or provided to JAL by site personnel and other information sources, have been assumed to be correct. JAL does not assume any liability for the misinterpretation of information or for items not visible, accessible or present on the subject property at the time of this study.
- 1.4.3 Whilst every effort has been made to ensure the accuracy of the data supplied, and any analysis derived from it, there may be conditions at the site that have not been disclosed by the investigation, and could not therefore be taken into account. As with any site, there may be differences in soil conditions between exploratory hole positions. Furthermore, it should be noted that groundwater conditions may vary due to seasonal and other effects and may at times be significantly different from those measured by the investigation. No liability can be accepted for any such variations in these conditions.



1.4.4 This report is not an engineering design and the figures and calculations contained in the report should be used by the Structural Engineer, taking note that variations may apply, depending on variations in design loading, in techniques used, and in site conditions. Our recommendations should therefore not supersede the Engineer's design.



2 SITE SETTING

2.1 Site Information

2.1.1 The site location plan is appended to this report in Appendix 1.

2.2 Desk Study

- 2.2.1 A desk study was not commissioned as part of this investigation.
- 2.2.2 Published data indicates that the site is directly underlain by solid deposits of the London Clay Formation.
- 2.2.3 The proposed development area is situated at the rear of 15 South Hill Park. South Hill Park rises fairly steeply north and eastwards away from Hampstead high street and comprises mostly 3 storey Victorian houses. The rear garden is approximately 15 x 10m, laid to lawn and is bordered by hedgerows to the neighbouring properties along with several fruit trees. A decking area is present outside the rear doors with a summerhouse at the top of the garden. Access to the rear is either through the house on the upper ground floor level or via a narrow steep side pathway.

2.3 Previous Investigation

2.3.1 JAL are not aware of any previous investigations at the site

2.4 Proposed Development

It is understood that the proposed development is to comprise an extension to the lower ground floor of the existing property. The floor level is anticipated to be approximately 4m below existing garden level.



3 **GROUND INVESTIGATION**

3.1 Rationale for Ground Investigation

3.1.1 The ground investigation was designed in order to gather data representative of the ground conditions within the vicinity of the proposed building. The exploratory hole locations were as specified by the Structural Engineer and Building Contractor

3.2 Scope of Ground Investigation

- 3.2.1 The ground investigation was undertaken on 02-03 September 2013, and comprised;
 - Drilling of 4 No. boreholes via windowless and handheld window sampling to a depth of up to 8.45m below ground level (mbgl) with in-situ testing and sampling.
- 3.2.2 2 No. boreholes (BH3 and BH4) were drilled in the lawned area, BH2 was drilled through the decking and BH1 was drilled inside the kitchen downstairs.
- 3.2.3 The exploratory hole position was measured in using tape and reel, as shown in Appendix 1. The exploratory borehole records are included in Appendix 2.
- 3.2.4 The exploratory holes were backfilled with the arisings (in the reverse order in which they were drilled) and the ground surface was reinstated with soft-standing so that no depression was left.

3.3 Sampling Rationale

3.3.1 Soil samples were taken at various depths as shown in the borehole log. Undisturbed liner samples were collected from the borehole where possible.

3.4 Standard Penetration Tests (SPTs)

- 3.4.1 In-situ standard/cone penetration tests were undertaken in the windowless sampling boreholes in accordance with BS EN ISO 22476-2 'Methods of Test on Soils for Engineering Purposes (Part 9)'; to determine the relative density of the underlying , and therefore give an indication of soil 'strength'.
- 3.4.2 The results are presented on the individual exploratory hole records in Appendix 2.

3.5 Groundwater Monitoring

3.5.1 Groundwater strikes noted during drilling (if any), are recorded within the exploratory hole records in Appendix 2.

3.6 Laboratory Analysis

- 3.6.1 A number of soil samples obtained from the exploratory holes were submitted to UKAS accredited The Environmental Laboratory Ltd and Professional Soils Laboratory Ltd;
 - 6 No. samples for Water Soluble Sulphate and pH Analyses
 - 2 No. Waste Acceptance Criteria analyses
 - 8 No. samples for Plasticity Indices and Moisture Content Analyses



- 3 No. samples for Undrained Shear Strength in Triaxial Compression
- 3.6.2 Where required, testing was undertaken in accordance with BS1377:1990 Method of Tests for Soils for Civil Engineering Purposes.
- 3.6.3 Results are presented in Appendix 3.



4 GROUND CONDITIONS

4.1 Soil

4.1.1 Ground conditions were logged in accordance with the requirements of BS5930:1999. Detailed borehole logs are provided in Appendix 2. The ground conditions encountered are summarised in Table 4.1 below, based on the strata observed during the investigation.

Stratum and Description	Encountered from (m bgl)	Base of strata (m bgl)	Thickness range (m)
MADE GROUND (clay/silt/gravel with flint, brick, mortar, concrete)	0.0	0.3 – 0.8	0.3 – 0.8
Grey brown/orange veined blue grey silty CLAY	0.3 – 0.8	>4.0 - >8.45 (terminal depth of all boreholes)	>3.7 - >7.9

4.2 Hydrogeology

4.2.1 Boreholes were reported to be dry during the drilling operations.



5 GEOTECHNICAL ENGINEERING RECOMMENDATIONS

5.1 Ground Investigation Summary

- 5.1.1 No detailed structural engineering design information, with respect to the type of construction and associated structural loadings, was provided at the time of preparing this report. Consequently, a detailed discussion of all the problems that may arise during the proposed redevelopment scheme is beyond the scope of this report.
- 5.1.2 Practical solutions to the difficulties encountered, both prior to, and during construction, are frequently decided by structural constraints or economical factors. For these reasons, this discussion is predominantly confined to remarks of a general nature, which are based on site conditions encountered during the intrusive investigations.
- 5.1.3 It is understood that the proposed development is to comprise an extension to the rear of the existing property.
- 5.1.4 The results of JAL's ground investigation revealed a ground profile comprising Made Ground (up to 0.8m thick) over deposits of CLAY (revealed to terminal depth of boreholes to a maximum of 8.45mbgl).
- 5.1.5 A summary of ground conditions is provided in table 4.1, with results of laboratory geotechnical testing summarised in table 5.1 below.

SECTION 5 GEOTECHNICAL ENGINEERING RECOMMENDATIONS



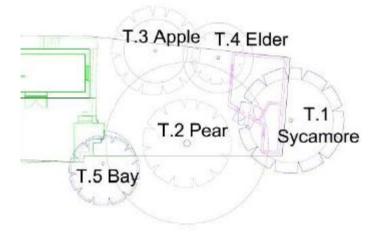
Table 5.1 – Preliminary Geotechnical Parameters

Strata	SPT 'N' Value	Shear Strength Cu (kPa)	Moisture content (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (Plasticity Term)	Particle Size Distribution (% passing 0.425mm)	Modified Plasticity Index	NHBC Volume Change Classification
MADE GROUND (clay/silt/gravel with flint, brick, mortar, concrete)	-		-	-	-	-	-	-	-
Grey brown/orange veined blue grey silty CLAY	15-32	111-146	20-30	67-79	23-27	44-53	100%	44-53	High



5.2 Trees near Building

5.2.1 JAL were provided with a copy of a Tree Survey Report for the site, produced by Phelps Associates (08 August 2013), and attached in Appendix 5 of this report.



- 5.2.2 The extract above shows five trees in the vicinity of the proposed extension. It is understood that Trees T2 T4 will be removed as part of the proposed development operations. All trees including those that will be removed or retained, must however be taken into account in the design of foundations in accordance with NHBC guidance.
- 5.2.3 The Tree Survey Report provides individual tree survey data, however their distance from the building footprint is not provided, and has thus been estimated by Jomas as presented in table 5.2 below.

	Table 5.2 –	Tree Data	a	
Tree Species	Estimated distance to building footprint – Lower GF (m)	Current Height (m)	Mature Height	Water Demand Category
T1 - Sycamore	8m	9	22	Moderate
T2 - Pear	2m	6	12	Moderate
T3 - Apple	2m	5	10	Moderate
T4 - Elder	5m	4	10	Low
T5 – Bay [laurel?]	1m	5	10	Moderate



5.3 Foundations

- 5.3.1 Structural loads for the proposed development are currently unavailable. The ground investigation has shown that ground conditions comprise Made Ground over clayey sand deposits, overlying clay.
- 5.3.2 The clay deposits have been reported to be of high volume change potential. Given that a number of trees have been identified within close proximity to the proposed building footprint, consideration should be give to the effects of their removal, in particular the change in groundwater profile due to the removal of these trees which will likely result in a rebound in the groundwater table. Therefore heave precaution will be required where existing trees have been or are to be removed. This should also be considered during design of the site drainage.
- 5.3.3 Shrinkable soils are subject to changes in volume as their moisture content is altered. The resulting shrinkage or swelling of the soil can cause subsidence or heave damage to foundations, the structures they support and services.
- 5.3.4 The results of standard penetration testing undertaken in the CLAY deposits at between 2m and 8m bgl indicated SPT values of 15 32, indicating a firm to stiff material. It is considered that conventional strip or spread foundations extending into the CLAY, can be designed with allowable bearing pressure in the region of 140kPa.
- 5.3.5 However, assuming high shrinkage soil and moderate water demand tree of approximately 22m height (mature height has been used as tree is to be retained), approximately 8m from building footprint, NHBC recommend foundations must extend to 1.75mbgl. With respect to a moderate water demand tree of 10m mature height (T5 Bay, which is also to be retained), approximately 1m from the building footprint, the NHBC guidance recommends that foundations extend to 2.25mbgl.
- 5.3.6 It is noted that the lower ground floor is proposed to be constructed approximately 4m below existing ground level. Based upon the calculations presented above, the floor level will be outside the zone of influence of the trees identified above. On this basis, it is considered that conventional foundations may be used with an allowable bearing pressure of 140kPa.
- 5.3.7 Reference should be made to guidance provided within the NHBC Standards in the design of foundations for the proposed new house. Design should be undertaken by a Structural Engineer.

5.4 Ground Floor Slabs

- 5.4.1 Where clay sub-soils persist at formation level within the influence of trees or other significant vegetation, it is recommended that suspended ground floor slabs are adopted, with a 75mm void to accommodate any potential heave.
- 5.4.2 Formations of the structures should be inspected by a competent person. Any loose or soft material should be removed and replaced with well-graded, properly compacted granular fill or lean mix concrete. The formation should be blinded if left exposed for more than a few hours or if inclement weather is experienced.



5.5 Excavations

- 5.5.1 Excavations during the intrusive works although open for a relatively short period of time remained reasonably stable. However it is recommended that the stability of all excavations should be assessed during construction.
- 5.5.2 Any vertically sided excavations require support to provide safe man access and to support the sides of the excavation. Supports should be installed as excavation proceeds. For service excavations, overlapping trench sheets could be used as close support in the Made Ground and Sand deposits to minimise ground loss. Alternatively consideration could be given to the use of trench boxes provided excavations take place within the boxes.

5.6 Groundwater Control

5.6.1 Groundwater was not encountered during the ground investigation. Depending upon the impact of seasonal variations, groundwater is not anticipated during construction works.

5.7 Ground Movement

5.7.1 The effects of settlement and settlement due to recoverable heave should be considered upon receipt of the structural loads for the proposed development.

5.8 Concrete in the Ground

- 5.8.1 Sulphate attack on building foundations occurs where sulphate solutions react with the various products of hydration in Ordinary Portland Cement (OPC) or converted High-Alumina Cement (HAC). The reaction is expansive, and therefore disruptive, not only due to the formation of minute cracks, but also due to loss of cohesion in the matrix.
- 5.8.2 Characteristic sulphate values for soil and groundwater have been devised based upon the recommendations set out in the BRE Special Digest 1: 2005.
- 5.8.3 This indicates that in a data set where there are five to nine results available, the mean of the highest two of the sulphate test results should be taken as the characteristic value for water-soluble sulphate (mg/l SO4).
- 5.8.4 Consideration of pH (of soils and groundwater) is also necessary in selecting a Design Sulphate Class. The Design Sulphate Class should subsequently be selected based upon the lower pH determination resulting from these reviews.
- 5.8.5 Six soil samples collected from the site were analysed for water soluble sulphate (SO₄). Results reported were between 329mg/l and 3531mg/l, with associated pH concentrations of 7.5 to 8.0. The full analytical results are presented in Appendix 4.
- 5.8.6 Based on the results described above, the mean of the highest two concentrations has been calculated as 3070mg/l. Consequently, the required concrete class for the site is DS-4 with ACEC class of AC-3s (assuming static groundwater) in accordance with the procedures outlined in BRE Special Digest 1.



6 **REFERENCES**

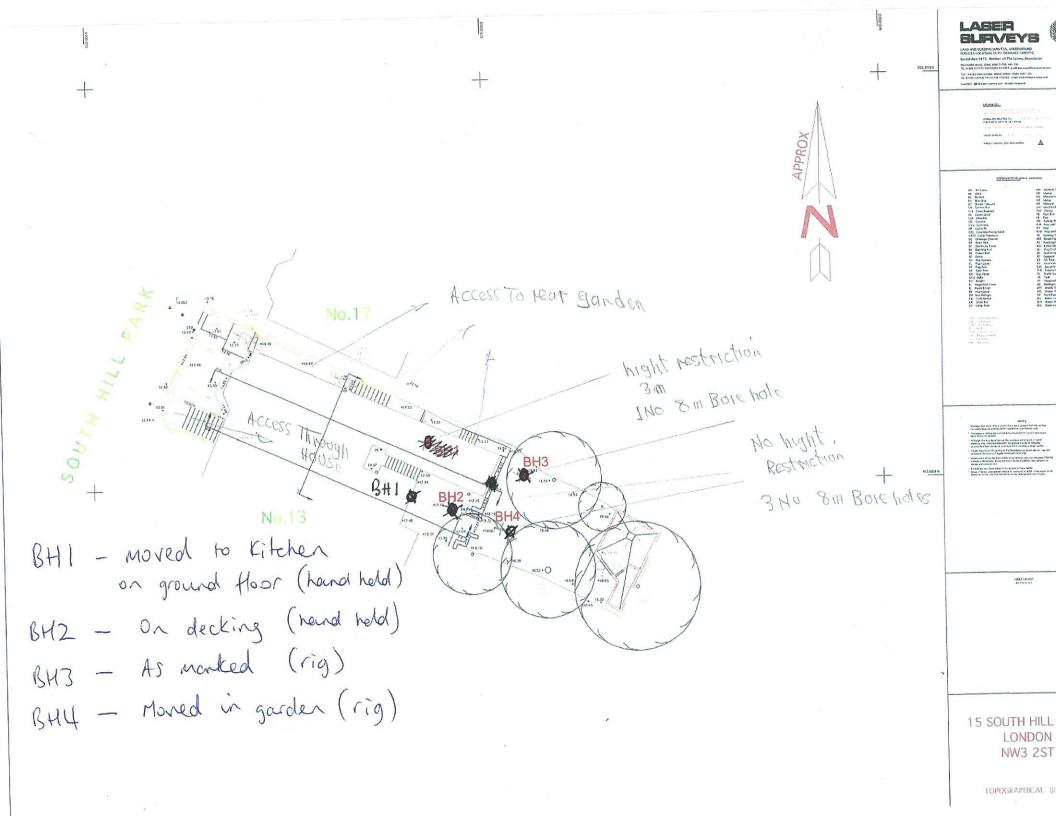
Arboricultural Tree Report Phelps Associates Ltd ref PA.S781: 8 August 2013 Code of Practice for Site Investigations BS5930: 1999 NHBC Standards Chapter 4: 2011



APPENDICES



APPENDIX 1 – FIGURES





APPENDIX 2 – EXPLORATORY HOLE RECORDS

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												patches, occasional re	pot trace to 3.5m.	
D	4.0	_	-				_							
S	4.0	2	2	3	4	4	5	16				[Darker grey brown fi	rom 6.0m with fine c	rystals }
D	4.5													
D	5.0													
S	5.0	3	3	4	4	5	5	18						
_														
D	5.5													
D S	6.0 6.0	3	4	4	4	5	6	19						
	0.0	5	-7	-		5		1.0		⊢				
D	6.5													
D	7.0													
S	7.0	4	5	5	5	6	5	21		L				
D	7.5									-				
D	7.5 8.0									-				
S	8.0 8.0	4	4	5	6	6	8	25	8.00					
										-				
										-				
	Sampli	ing Co	de: U	Undi							D - Small Disturb		(U*) Non recovery	of Sample
					Jo							ay, Stockley Park, UB		
						T: 01	895 7	7 218	7 E: info@joma	asas	sociates.com W: wy	ww.jomasassociates.o	com	



APPENDIX 3 – GEOTECHNICAL LABORATORY TEST RESULTS



LABORATORY REPORT



4043

Contract Number: PSL13/3242

Client's Reference:

Report Date: 17 September 2013

Client Name: Jomas Jomas Associates Ltd 1 Furzeground Way Lakeside House Stockley Park UB11 1BD

For the attention of: Roni Savage

Contract Title: South Hill Park

Date Received:9/9/2013Date Commenced:9/9/2013Date Completed:17/9/2013

Notes: Observations and Interpretations are outside the UKAS Accreditation

A copy of the Laboratory Schedule of accredited tests as issued by UKAS is attached to this report. This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced in full, without the prior written approval of the laboratory.

A Watkins

(Director)

Checked and Approved Signatories:

M.bur

M Beastall (Laboratory Manager)

5 – 7 Hexthorpe Road, Hexthorpe, Doncaster DN4 0AR tel: +44 (0)844 815 6641 fax: +44 (0)844 815 6642 e-mail: rgunson@prosoils.co.uk awatkins@prosoils.co.uk

R Gunson

(Director)

Page 1 of

SUMMARY OF LABORATORY SOIL DESCRIPTIONS

Hole Number	Sample Number	Sample Type	Depth m	Description of Sample
BH1		D	1.00	Brown slightly sandy CLAY.
BH1		U	1.00-2.00	Stiff brown slightly sandy CLAY.
BH1		U	4.00-5.00	Stiff brown slightly gravelly slightly sandy CLAY.
BH2		D	1.00	Reddish brown slightly sandy CLAY.
BH2		U	2.00-3.00	Stiff brown slightly gravelly sandy silty CLAY.
BH3		D	1.00	Reddish brown slightly sandy CLAY.
BH3		D	2.00	Brown slightly sandy CLAY.
BH3		D	5.00	Reddish brown slightly sandy CLAY.
BH4		D	1.00	Reddish brown slightly sandy CLAY.
BH4		D	2.00	Reddish brown slightly sandy CLAY.
BH4		D	4.00	Reddish brown slightly sandy CLAY.

	Compiled by	Date	Checked by	Date	Approved by	Date
PSL	SUDER	17/09/13	M.S.	17/09/13	M.but	17/09/13
Professional Soils Laboratory		SOUTH H		Contract No: PSL13/324		
		50011111		Client Ref:		

SUMMARY OF SOIL CLASSIFICATION TESTS

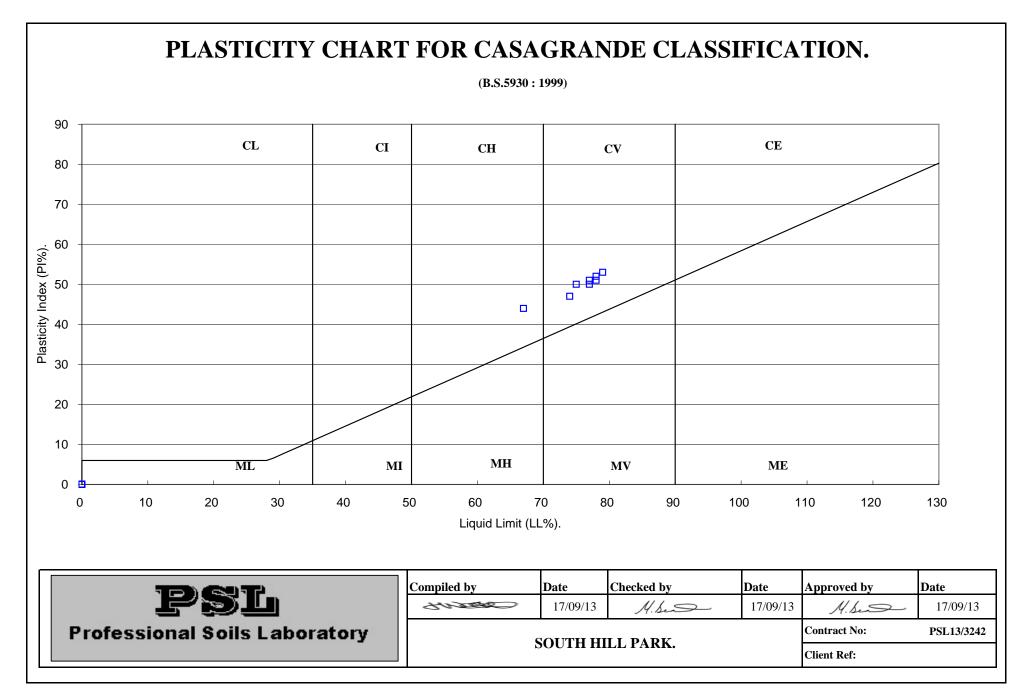
				Moisture	Bulk	Dry	Particle	Liquid	Plastic	Plasticity	%	
Hole		Sample	Depth	Content	Density	Density	Density	Limit	Limit	Index	Passing	Remarks
Number	Number	Туре	m	%	Mg/m ³	Mg/m ³	Mg/m ³	%	%	%	.425mm	
				Clause 3.2	Clause 7.2	Clause 7.2	Clause 8.	Clause 4.3/4.4	Clause 5.	Clause 6.		
BH1			1.00	28				78	27	51	100	Very high plasticity CV.
BH2			1.00	20				67	23	44	100	High plasticity CH.
BH3			1.00	24				74	27	47	100	Very high plasticity CV.
BH3			2.00	24				77	27	50	100	Very high plasticity CV.
BH3			5.00	30				77	26	51	100	Very high plasticity CV.
BH4			1.00	24				75	25	50	100	Very high plasticity CV.
BH4			2.00	27				78	26	52	100	Very high plasticity CV.
BH4			4.00	29				79	26	53	100	Very high plasticity CV.

(B.S. 1377 : PART 2 : 1990)

SYMBOLS: NP: Non Plastic

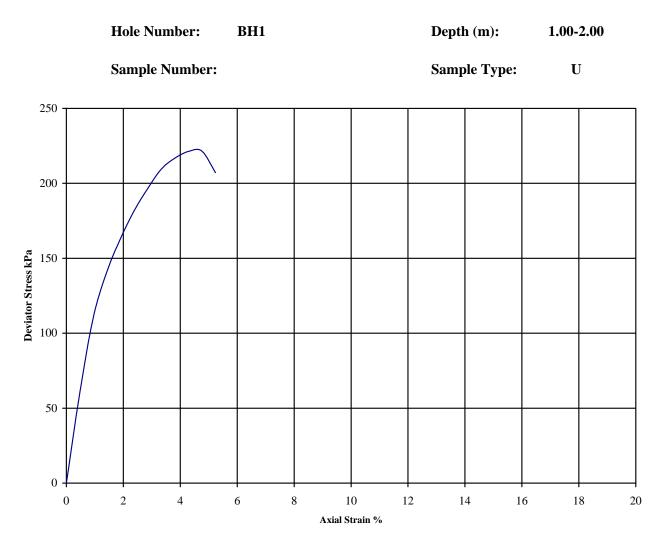
*: Liquid Limit and Plastic Limit Wet Sieved.

	Compiled by	Date	Checked by	Date	Approved by	Date
e par	SW250	17/09/13	M. Sur	17/09/13	M. Sen	17/09/13
Professional Soils Laboratory		SOUTH HI		Contract No:	PSL13/3242	
		5001H HI	Client Ref:			



Undrained Shear Strength in Triaxial Compression

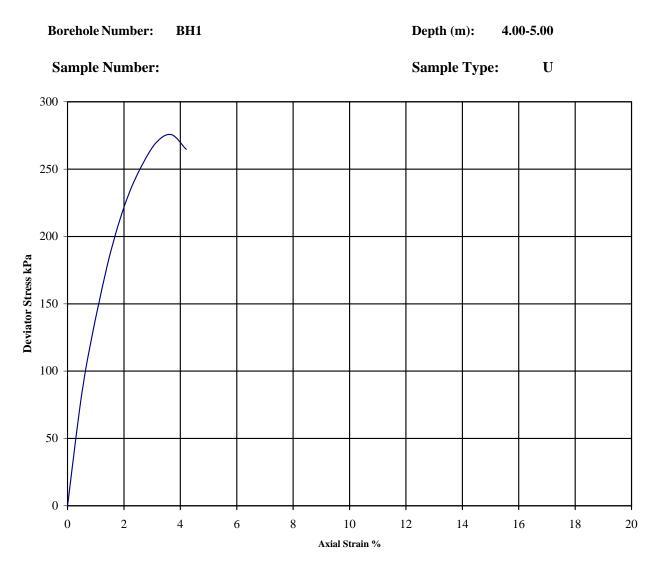
without measurement of Pore Pressure B.S. 1377 : Part7 : Clause 8 : 1990



Diamete	Diameter (mm): 102.0 Height		Height ((mm): 210.0 Test: 100 mm Single S			Stage. Undisturbed							
Specimen	Moisture	Bulk	Dry	Cell	Corr. Max.	Shear	Failure	Mode		Remarks				
	Content	Density	Density	Pressure	Deviator	Strength	Strain	of	Sample taken from top of tube					
	(%)	(Mg/m3)	(Mg/m3)	(kPa)	Stress	Cu	(%)	Failure	Rate of strain = 1.9 %/min					
					(kPa)	(kPa)			Latex Membrane used 0.2 mm thickness,					
				θ_3	$(\theta_1 - \theta_3)_f$	$^{1}/_{2}(\theta_{1}-\theta_{3})_{f}$			Correction applied 0.36 kPa					
А	25	1.80	1.43	20	222	111	4.8	Brittle	See summary of soil descriptions.					
									Checked	Date	Approved	Date		
									M.S.S.	17/09/13	M.S.S.	17/09/13		
Profe	PSL Professional Soils Laboratory				SOUTH HILL PARK.						Contract No: PSL13/3242			

Undrained Shear Strength in Triaxial Compression

without measurement of Pore Pressure B.S. 1377 : Part7 : Clause 8 : 1990



Diameter (mm):		38 Height (mm):		76	76 Test: 38 mm Single Stage				
	Moisture	Bulk	Dry	Cell	Deviator	Shear	Failure	Mode	Remarks
Specimen	Content	Density	Density	Pressure	Stress	Strength	Strain	of	See summary of soil descriptions.
	(%)	(Mg/m3)	(Mg/m3)	(kPa)	(kPa)	(kPa)	(%)	Failure	
А	29	1.94	1.51	80	276	138	3.7	Brittle	

Checked and Approved By Date 17/09/13 M.S.S.



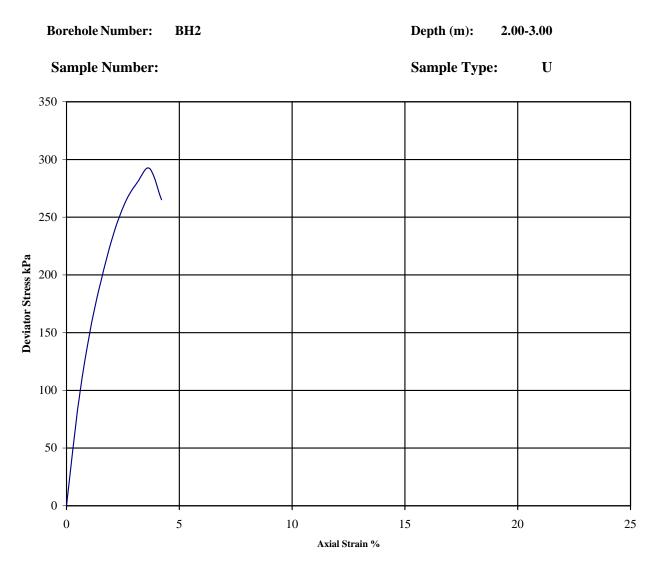
SOUTH HILL PARK.

Contract No:

PSL13/3242

Undrained Shear Strength in Triaxial Compression

without measurement of Pore Pressure B.S. 1377 : Part7 : Clause 8 : 1990



Diameter (mm):		38 Height (mm):		76 Test: 38 mm Single Stage.			ngle Stage		
	Moisture	Bulk	Dry	Cell	Deviator	Shear	Failure	Mode	Remarks
Specimen	Content	Density	Density	Pressure	Stress	Strength	Strain	of	See summary of soil descriptions.
	(%)	(Mg/m3)	(Mg/m3)	(kPa)	(kPa)	(kPa)	(%)	Failure	
А	27	1.96	1.54	40	292	146	3.7	Brittle	

Checked and Approved By Date *M.b.* 17/09/13



SOUTH HILL PARK.

Contract No: PSL13/3242



APPENDIX 4 – CHEMICAL LABORATORY TEST RESULTS







Reporting Date: 12 September 2013

2683

THE ENVIRONMENTAL LABORATORY LTD

F.A.O. Roni Savage Jomas Associates Limited Lakeside House 1 Furzeground Way Stockley Park, UB11 1BD

ANALYTICAL REPORT No. AR50605

Samples Received By: Courier Sample Receipt Date: 06/09/13 Your Job No: P8574J326 Your Order No: ---Site Location: South Hill Park No Samples Received: 6 Date of Sampling: 02/09/13 - 03/09/13

This report was written by: Stuart Ballard

Authorised By;

ς, 1

Steve Knight Reporting Manager

Any comments, opinions or interpretations expressed herein are outside the scope of UKAS accreditation (Accreditation Number 2683)



THE ENVIRONMENTAL LABORATORY LTD

Unit A2, Windmill Road, Ponswood Industrial Estate, St Leonards On Sea, East Sussex, TN38 9BY

Tel: 01424 718618 Fax: 01424 729911

ANALYTICAL REPORT No. AR50605

Location: South Hill Park

F.A.O. Roni Savage Jomas Associates Limited Lakeside House 1 Furzeground Way Stockley Park, UB11 1BD Your Job No: P8574J326 Your Order No: ---Reporting Date: 12/09/13

ELAB

	Characteristic	Clay loam	Clay loam S	ilty Clay loam	Silt loam	Clay loam	Clay loam
<u>Soils</u>	Date Sampled	02/09/13 -	02/09/13 -	02/09/13 -	02/09/13 -	02/09/13 -	02/09/13 -
	TP/BH	BH1	BH2	BH3	BH4	BH4	BH4
	Depth (m)	0.50	1.50	2.50	0.25	4.00	8.00
	Our ref	82895	82903	82915	82923	82929	82937
pH Value**	(Units)	7.8	8.0	7.7	7.5	7.8	7.7
Water Soluble 2:1 Sulphate	(mg/l as SO ₄)	1419.0	329.0	2323.0	199.0	2609.0	3531.0

All results expressed on dry weight basis

** - MCERTS accredited test

* = UKAS accredited test





THE ENVIRONMENTAL LABORATORY LTD

Report No:	1A	NALYTICAL REPORT NO	o. AR50605		Page 3 of 7				
				CLIENT:	Jomas Associate	es Limited			
Project Name:		Location: South Hill	Park		1				
				Landfill	Waste Acceptance	e Critena			
Lab Reference		82897		LandfIII Waste Acceptance Critena Limits					
Sampling Date		02/09/13 - 03/09/	13		Stable Non-				
Sample ID		BH1		Inert Waste Landfill	HAZARDOUS waste in non-	Hazardous Waste Landfill			
Depth		1.50		hazardous Landfill					
Solid Waste Analysis									
TOC (%)	0.2			3%	5%	6%			
Loss on Ignition (%)**	3.9					10%			
BTEX (mg/kg)**	<0.01			6					
Sum of PCBs (mg/kg)**	<0.01			1					
Mineral Oil (mg/kg)**	<5			500					
Total PAH (mg/kg)**	<0.5	l i		100					
pH (Units)**	7.8								
Acid Neutralisation Capacity (mol/kg)	<0.1				To be	To be evaluated			
Eluate Analysis	2:1	8:1	Cumulative 10:1		s for compliance				
	mg/l	mg/l	mg/kg	using BS EN	12457-3 at L/S 1	0 l/kg (mg/kg)			
Arsenic*	<0.005	<0.005	<0.1	0.5	2	25			
Barium*	0.027	0.021	<0.1	20	100	300			
Cadmium*	<0.001	<0.001	<0.01	0.04	100	5			
Chromium*	<0.005	<0.005	<0.1	0.5	10	70			
Copper*	0.007	<0.005	<0.1	2	50	100			
Mercury*	<0.0001	<0.0001	<0.001	0.01	0.2	2			
Molybdenum*	<0.0001	<0.005	<0.0	0.5	10	30			
Nickel*	<0.005	<0.005	<0.1	0.4	10	40			
Lead*	<0.005	<0.005	<0.1	0.4	10	50			
Antimony	<0.005	<0.005	<0.1	0.06	0.7	5			
Selenium*	<0.005	<0.005	<0.01	0.00	0.7	7			
Zinc*	0.009	<0.005	<0.1	4	50	200			
Chloride*	42	3	41	800	15000	25000			
Fluoride*	<1	<1	<1	10	150	500			
Sulphate*	2021	680	3899	1000	20000	50000			
TDS	3530	1210	6888	4000	60000	100000			
Phenol Index	<0.5	<0.5	<0.5	1	-	-			
DOC	11.6	5.1	26	500	800	1000			
Leach Test Information									
pH *	7.9	7.7							
EC*	3370	1384							
Sample Mass (kg)	0.221								
Dry Matter (%)	79			ł					
Moisture (%)	27	<u>├</u> ──┤──		<u> </u>					
· ·	21	<u>├</u> ──┤──		<u> </u>					
Stage 1 Volume Eluate L2 (litres)	0.202			ł					
Filtered Eluate L2 (litres)	0.302								
I MOIOU EIUALO VEI (IMIOS)	0.132								

*= UKAS accredited

** - MCERTS accredited test





THE ENVIRONMENTAL LABORATORY LTD

Waste Acceptance Criteria ANAI Report No:		NALYTICAL REPOR	T No. AR50605			Page 4 of	
				CLIENT:	Jomas Associate		
Project Name:		Location: South	Hill Park				
.,		Location: Court				0.1	
Lab Reference		82912		Landflll Waste Acceptance Critena Limits			
Sampling Date		02/09/13 - 03	/09/13		Stable Non- reactive		
Sample ID		BH3		Inert Waste Landfill	HAZARDOUS waste in non-	Hazardous Waste Landfill	
Depth		0.30			hazardous Landfill		
Solid Waste Analysis							
TOC (%)	2.7			3%	5%	6%	
Loss on Ignition (%)**	6.0					10%	
BTEX (mg/kg)**	<0.01			6			
Sum of PCBs (mg/kg)**	<0.01			1			
Mineral Oil (mg/kg)**	19			500			
Total PAH (mg/kg)**	35.2			100			
pH (Units)**	7.6						
Acid Neutralisation Capacity (mol/kg)	<0.1				To be	To be evaluated	
Eluate Analysis	2:1	8:1	Cumulative 10:1	Limit values for compliance leaching		Ģ	
,	mg/i	mg/l	mg/kg	USING BS EN	12457-3 at L/S 1	0 i/kg (mg/kg)	
Arsenic*	0.011	0.011	<0.1	0.5	2	25	
Barium*	0.013	0.010	<0.1	20	100	300	
Cadmium*	<0.001	<0.001	<0.01	0.04	1	5	
Chromium*	< 0.005	<0.005	<0.1	0.5	10	70	
Copper*	0.027	0.008	<0.1	2	50	100	
Mercury*	<0.0001	<0.0001	<0.001	0.01	0.2	2	
Molybdenum*	0.031	0.014	<0.1	0.5	10	30	
Nickel*	<0.005	< 0.005	<0.1	0.4	10	40	
Lead*	<0.005	0.010	<0.1	0.5	10	50	
Antimony	0.015	0.008	0.05	0.06	0.7	5	
Selenium*	<0.005	<0.005	<0.01	0.1	0.5	7	
Zinc*	0.006	0.007	<0.1	4	50	200	
Chloride*	4	<1	2	800	15000	25000	
Fluoride*	<1	<1	<1	10	150	500	
Sulphate* TDS	30	8	57	1000	20000	50000	
	350	150	895	4000	60000	100000	
Phenol Index DOC	< 0.5	<0.5	<0.5	1 500	-	-	
	28.0	13.7	79	500	800	1000	
Leach Test Information							
pH *	8.3 278	8.1 146					
EC*	278	146					
Sample Mass (kg)	0.206						
Dry Matter (%)	85	├		+			
Moisture (%)	18	<u>├</u>		+			
	18	├		+			
Stage 1 Volume Eluate L2 (litres)	0.318	<u>├</u>		+		-	
Filtered Eluate L2 (litres)	0.318	├		+			
Thered Eluale VET (IIIIes)	0.112						

*= UKAS accredited

** - MCERTS accredited test





THE ENVIRONMENTAL LABORATORY LTD

SAMPLE RECEIPT AND TEST DATES

Our Analytical Report Number	AR50605
Your Job No:	P8574J326
Sample Receipt Date:	06/09/13
Reporting Date:	12/09/13
Registered:	06/09/13
Prepared:	07/09/13
Analysis complete:	12/09/13

TEST METHOD SUMMARY

PARAMETER	Analysis Undertaken on	Date Tested	Method Number	Technique
pH Value**	Air dried sample	10/09/13	113	Probe
Water Soluble 2:1 Sulphate	Air dried sample	09/09/13	209	Colorimetry

* = UKAS Accredited test ** - MCERTS Accredited test Determinands not marked with * or ** are not accredited MCERTS accreditation covers samples which are predominantly sand, clay, loam or combinations of these three soil types

Any comments, opinions, or interpretations expressed herein are outside the scope of UKAS accreditation (Accreditation Number 2683)





THE ENVIRONMENTAL LABORATORY LTD

SAMPLE RECEIPT AND TEST DATES

Our Analytical Report Number	AR50605
Your Job No:	P8574J326
Sample Receipt Date:	06/09/13
Reporting Date:	12/09/13
Desistant	00/00/40
Registered:	06/09/13
Prepared:	07/09/13
Analysis complete:	12/09/13

TEST METHOD SUMMARY

PARAMETER		Date Tested	Method	Technique
	Undertaken on		Number	
pH Value**	Air dried sample	09/09/13	113	Electrometric
Total Organic Carbon	Air dried sample	10/09/13	210	Automated IR Absorption
Loss on Ignition**	Air dried sample	09/09/13	129	Gravimetric
Neutralization Capacity to pH 7	Air dried sample	09/09/13	-	EA
Benzene**	As submitted sample	09/09/13	181	GCMS
Toluene**	As submitted sample	09/09/13	181	GCMS
Ethyl Benzene**	As submitted sample	09/09/13	181	GCMS
Xylenes**	As submitted sample	09/09/13	181	GCMS
Mineral Oil**	As submitted sample	09/09/13	117	GCFID
PCB 28**	Air dried sample	10/09/13	120	GCMS
PCB 52**	Air dried sample	10/09/13	120	GCMS
PCB 101**	Air dried sample	10/09/13	120	GCMS
PCB 118**	Air dried sample	10/09/13	120	GCMS
PCB 138**	Air dried sample	10/09/13	120	GCMS
PCB 153**	Air dried sample	10/09/13	120	GCMS
PCB 180**	Air dried sample	10/09/13	120	GCMS
Speciated PAH**	As submitted sample	09/09/13	133	GCFID

The analysts' guide for sampling, analysis and clearance procedures

* = UKAS Accredited test

** - MCERTS Accredited test

Determinands not marked with a * or ** are not accredited

MCERTS accreditation covers samples which are predominantly sand, clay, loam or combinations of these three soil types

Any comments, opinions, or interpretations expressed herein are outside the scope of UKAS accreditation (Accreditation Number 2683)





THE ENVIRONMENTAL LABORATORY LTD

LEACHATE SAMPLE RECEIPT AND TEST DATES

Our Analytical Report Number	AR50605				
Your Job No:	P8574J326				
Sample Receipt Date:	06/09/13				
Reporting Date:	12/09/13				
Registered:	06/09/13				
Prepared:	07/09/13				
Analysis complete:	12/09/13				

LEACHATE TEST METHOD SUMMARY

PARAMETER	Method Number	Technique
Arsenic*	101	ICPMS
Cadmium*	101	ICPMS
Chromium*	101	ICPMS
Lead*	101	ICPMS
Nickel*	101	ICPMS
Copper*	101	ICPMS
Zinc*	101	ICPMS
Mercury*	101	ICPMS
Selenium*	101	ICPMS
Antimony	101	ICPMS
Barium*	101	ICPMS
Molybdenum*	101	ICPMS
pH Value*	113	Electrometric
Electrical Conductivity*	136	Probe
Dissolved Organic Carbon	102	TOC analyser
Chloride*	131	Ion Chromatography
Fluoride*	131	Ion Chromatography
Sulphate*	131	Ion Chromatography
Total Dissolved Solids	144	Gravimetric
Phenol Index	121	HPLC

* = UKAS Accredited test Determinands not marked with * are not accredited

Any comments, opinions, or interpretations expressed herein are outside the scope of UKAS accreditation (Accreditation Number 2683)



APPENDIX 5 – TREE REPORT

4.1 Tree Survey Schedule

Site:	15 South Hill Par	k, London,	Surv	/eyor:	Andrew Phelps		
Date of Survey:	15 th July 2013				R	lef:	PA S781
			 -		 		

Tree No	English Name	Height (m)	Crown Spread	Ground Clearance	Age Class	Stem Diameter (mm)	No of Stems	Protection Radius (Metres)	Vigour (Growth Vitality)	Structural Condition	Amenity Landscape Contribution	B.S Cat Ret Value	Sub Cat	Useful Life	Structural Condition/Observations
T.1	Sycamore	9	3	3	Mature	200	1	2.4	Normal	Good	Medium	В	2	40	No visible defects
T.2	Pear	6	3	3	Over Mature	380	1	4.5	Moderate	Good	Low	С	-	<20	Life expectancy less than 20 years in my view
Т.3	Apple	5	2.5	3	Over Mature	150	1	1.8	Moderate	Good	Low	С	-	<20	Life expectancy less than 20 years in my view
T.4	Elder	4	1.5	2	Mature	100	1	1.2	Moderate	Good	Low	С	-	20	No visual defects
T.5	Bay	5	2.5	2	Mature	160	1	1.9	Normal	Good	Low	В	-	30	No visual defects
Т.6	Hornbeam	8	5	5	Mature	350	1	4.2	Normal	Good	High	A	1/2	>40	Council owned tree

Notes:

- 1. Height describes the approx. height of the tree in metres from ground level.
- 2. Crown spread refers to the crown radius in metres from the stem centre and is expressed as an average of NESW if symmetrical
- 3. Ground Clearance is the height in metres of crown clearance above adjacent ground level.
- 4. Diameter Breast Height (DBH) is the diameter of the stem measured in mm at 1.5m from ground level for single stemmed trees or at ground level for multistemmed trees. DBH may be estimated where access is restricted.
- 5. Age Class is the tree's relative age to its species and is expressed as Newly planted (NP) Young (Y), Middle Mature (MM), Mature (M) and Over Mature (OM).
- 6. Protection Multiplier is 12 for single stemmed trees and for trees with more than one stem diameter(s) should be measured in accordance with Annex C, and the RPA should be determined from Annex D of the BS5837:2012.

- 7. Protection Radius is a radial distance in metres measured from the trunk centre.
- 8. Growth Vitality Normal ; Moderate (below normal); Poor (sparse, weak); Dead (dead or dying tree)
- Structural/Arboricultural Condition Good (no or only minor defects); Moderate (remediable defects); Poor (major defects present). See Condition Key (4.1) for detail
- 10. Landscape Contribution High (prominent landscape feature); Medium (visible in landscape); Low (secluded/among other trees)
- 11. B.S Cat refers to (BS 5837:2012 Table 1) and refers to tree/group quality and value; 'A' High; 'B' Moderate; 'C' Low; 'U' Remove. See Table 1 Cascade chart for tree quality assessment
- 12. Sub Cat refers to the retention criteria values where 1 is arboricultural, 2 is landscape and 3 mainly cultural values, including conservation.
- 13. Useful Life is the tree's estimated remaining contribution in years.

