



Wildwood Lodge 9 North End Hampstead London NW3 7HH

# DESIGN CONSTRUCTION STATEMENT ADDENDUM

GB/8250 – Version 2.0 2<sup>nd</sup> Occtober 2013

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GB/8250 – Version 2.0 2<sup>nd</sup> Occtober 2013

#### 1.0 INTRODUCTION

- 1.1. This Design Construction Statement Addendum has been prepared by Taylor Whalley Spyra as requested by Canaway Fleming Architects as part of the proposed planning to extend the basement beneath the existing house and steps to access the basement from the rear garden.
- 1.2. The existing Design Construction Statement undertaken by Taylor Whalley Spyra in June 2012 has been reviewed and remains valid for the existing consented basement and the purpose of this addendum is to review any affects that the extended basement space may have on the existing Grade II listed structure, including the consented basement and the surrounding area, with regard to structural stability, ground / surface water flow and flooding.
- 1.3. The existing consented proposal is to refurbish the existing Victorian property, replace the existing 1950's extensions to the main building and form the basement areas under the house and to either side.
- 1.4. The information contained within this Construction Design Statement Addendum has been reviewed against Camden Planning Guidance Basements and Lightwells (CPG4) and Camden Development Policy DP27 and has indicated the site location on the Camden Geological Hydrogeological & Hydrological Study Extracts Figures 11, 12,14, 15 & 16 which confirm the site is outside the influence of these areas (refer to Appendix F).

#### 2.0 EXISTING SITE AND CONSENTED SCHEME

- 2.1. The site is on the corner of North End and Parfitt Close with a large rear garden approximately orientated North to South, existing level at the front is 111.000 and at the rear 109.800 (refer to Appendix A & H).
- 2.2. The existing Victorian building has been retained and a number of 1950's additions have been demolished. There was an existing basement under the main part of the building which has been demolished and the consented basement scheme is being constructed at the moment.
- 2.3. The consented scheme consisted of two basements either side of the existing house linked by a basement under the front of the existing house.
- 2.4. The underpinning to support the above load bearing walls to the existing house has been installed, with all sheet piling and temporary support works in place and the west side basement and basement under the existing house have been constructed. The works on site are ongoing with the remaining basement slab for the west of the house having been cast and the RC walls now being installed.

#### 3.0 PROPOSED WORKS

- 3.1. The consented scheme included retaining some of the ground under the rear of the existing house between the underpinning and a newly constructed RC basement wall. The proposed basement adjustment requires the retained ground to be removed and the basement extended fully under the existing house.
- 3.2. From a structural aspect not having to retain the ground under the house between the newly constructed underpinning and an additional RC wall simplifies the construction and allows for the underpinning to be tied back to the extended RC Basement raft slab. This will further reduce any risk of differential settlement for the existing house.
- 3.3. The basement area level under the existing house which is to be extended is the highest of the basement levels at 107.110 and the west basement area level of 106.100 with the

lowest point of the actual swimming pool level of 104.850. The east basement level is 106.840.

- 3.4. The existing ground floors are to remain to provide additional horizontal propping.
- 3.5. The removal of the soil and the extending of the basement structure under the house will not affect the structural stability of the existing house or affect any adjoining properties which are not in the zone of influence of the extended basement.

#### 4.0 GROUND WATER AND SURFACE WATER FLOW

- 4.1. The extension of the basement towards the rear under the existing house will not affect the ground water flow which flows in a northwest direction and the existing ground water level is below the basement slab level. There is no ground water flow within the volume of soil which was to be retained under the house. The below ground granular drainage being installed, which is to be extended under the extra basement space (refer to Appendix D), will allow any build-up of ground water to pass under the basement and flow towards the northwest maintaining the existing site condition.
- 4.2. The proposed surface water drainage design is designed to keep to the existing site conditions with SUDS to manage the hard and soft surface water runoff landscaped areas.

#### **5.0 SOIL INVESTIGATION**

- 5.1. In 2012 a full soil investigation was undertaken by MRH Geotechnical comprising 3 boreholes between 13 & 15m deep which confirms the ground conditions as Bagshot Beds approximately 4m deep overlaying Claygate Beds.
- 5.2. Monitoring of the groundwater has been undertaken with the ground water level of 105.950 which is below the basement slab level under the existing house (refer to Appendix E). The highest ground water level recorded was when the boreholes were originally installed at 107.150 which is approximately at structural slab level.
- 5.3. The ground water levels encountered during excavations on site have been low and dealt with by localised dewatering where encountered whilst underpinning along the west basement. This will not affect the adjoining properties as dewatering is controlled and restricted local only to this area.

#### **6.0 ADJACENT PROPERTIES**

- 6.1. None of the adjacent properties are affected by the additional basement space under the existing house. Hogarth House to the west is set back approximately 11.5m from the closest point of the proposed basement with Parfitt Close between.
- 6.2. North End, the road outside the site, is adjacent to the South boundary approximately 11m away from the closest point of the proposed additional basement space.
- 6.3. The Wildwood Property is adjacent to the East boundary with an existing boundary wall between. The House has a basement and is set back a considerable amount from North End. The front left corner of the house is 13m way from the closest point of the proposed additional basement space.
- 6.4. London Underground has a section of the Northern Line Tunnel running under the east of the site with a crown level of 72.400 above Ordnance Datum (refer to Appendix G). They have previously confirmed that the existing consented scheme which put the tunnel crown 33m below the lowest point of the basement was of sufficient depth not to be affected by the proposed development. The additional basement space is 35m above and on plan set back 6m from the tunnel crown so will not influence the tunnels. The

- proposed additional basement space does not encroach any further, but extends parallel with the tunnel line.
- 6.5. All properties that are adjacent to the proposed developments have been condition surveyed and signed Party Wall Agreements with regard to The Party Wall Act 1996 are in place as part of the consented scheme.
- 6.6. The design of the basement and temporary support works is undertaken so as to minimise any structural disturbance to the adjoining properties, but as the nearest 2 buildings have basements and are not directly adjacent to the proposed basement works it is not envisaged that any structural disturbance to them will occur.
- 6.7. The existing building, adjacent buildings and the adjacent ground is being monitored with regard to movement. The results of the monitoring confirm that the limits set are within the allowable designed tolerances.

#### 7.0 ASSESSMENT OF THE EFFECTS OF MOVEMENT ON ADJACENT STRUCTURES

- 7.1. There are two possible causes of ground movement which can be associated with the proposed additional basement space; the excavation of the basement and the adjustment of the ground under the net load changes. The only structure that this may affect is Wildwood Lodge itself.
- 7.2. In the long term the Claygate within which the basement is constructed will adjust to the changes that have taken place as a result of the net load changes and water pressure will build up on the underside of the slab. In this case, there will be a net load reduction and there will be a tendency for the structure to rise a small amount. This re-adjustment may result in small upward movement of the surrounding ground, but this is unlikely to result in any significant effect on the existing structure.

#### 8.0 BASEMENT CONSTRUCTION SEQUENCE OF WORKS

- 8.1. Proposed Sequence of Works.
  - Install within the site area and to the existing house and surrounding area a number of fixed monitoring nodes to monitor possible movement during the works.
  - The existing house is to be retained and adjoining buildings and add-ons demolished as shown on the demolition drawing, all foundations are to be grubbed out.
  - The sheet piling is to be installed with a Silent Piler similar to a Giken Rig which
    presses the sheet piles into the ground using the resistance of the adjoining installed
    sheet piles.
  - The existing building is then to be underpinned with RC pins and associated support steel work.
  - The ground is to be locally excavated 1m to allow installation of the steel wailers and props at high level.
  - The ground is then to be locally excavated to allow installation of the steel wailers and props at low level.
  - The ground is then to be excavated to formation level.
  - The drainage pipes and granular drainage channels are to be installed.
  - The basement RC slab build-up is to be installed and then the basement slab cast with 150mm high kickers for all the RC basement walls and internal columns.
  - Install and cast all internal RC walls and columns to underside of ground floor slab.
  - Once the RC basement slab has gained the required design strength the sheet piling walling beams at low level are to be removed.
  - The basement RC walls are then to be cast to 600mm below the underside of the high level walling beams.
  - Once the basement RC walls have gained the required design strength, install
    diagonal props fixed to the RC wall and RC basement slab and then remove the high
    level sheet piling walling beams and shoring.

- The RC ground floor slab and RC wall down stands can then be cast.
- Once the RC ground floor slab has gained the required design strength the additional diagonal props can be removed.
- During the construction period the sheet piling and surrounding ground will be monitored at regular intervals to confirm the construction tolerances are within the agreed design parameters.
- The existing structure is now supported on the new basement raft slab under, with the adjoining basement boxes either side forming a solid box construction to the whole basement structure.
- Continue with construction of remainder of the structure over.

#### 9.0 CONCLUSIONS

- 9.1. The selection of the main contractor and sheet piling sub-contractor and designer of temporary works has been based on having previous experience constructing similar projects and the same contractor is used to install the proposed additional basement space as works are progressing.
- 9.2. Site personnel have been selected based on experience of similar projects. Selection of plant and machinery has been based on minimising noise and vibration.
- 9.3. Detailed analysis of the various aspects of construction has been reviewed to demonstrate that the level of sequencing will enable the proposed additional basement space to be constructed safely and with ground movements kept within acceptable tolerances.
- 9.4. The stability of the existing house should not be affected by the proposed additional basement space works. The underpinning has already taken place as part of the consented scheme and the ground movement and settlement locally are within the design parameters.
- 9.5. The temporary dewatering of the area is being designed and monitored to reduce the water level locally to the area of works for the construction of the swimming pool and is only needed for a short period of time. Water levels are being monitored.
- 9.6. The construction of the basement will not affect the ground water flow and incorporates perforated ground water pipes within a granular drainage channel installed under the basement slab and this is to be extended under the proposed additional basement space. This will allow the existing ground water regime to be maintained and restrict ground water building up behind the basement walls and also restrict any change in ground water flow affecting adjoining structures (refer to Appendix D).
- 9.7. The above ground surface water drainage design incorporates SUDS and two soakaways within the rear garden area which will reduce the existing surface water discharging into the existing public sewer and minimise any risk of localised flooding.
- 9.8. The project as currently being undertaken on site is being monitored as further detailed design is undertaken and as works progress on site and to keep in terms of the general construction process, structural stability, the long term integrity of adjacent buildings and the existing property and surrounding infrastructure.

## Appendix A

TWS - 8250\_SK01 - Site Location Plan indicating Adjoining Properties

















Existing Borehole









No. 10 No. 6 No. 12

Title	SITE LOCATION PLAN ALSO INDICATING ADJACENT PROPERTIES	

Drawn GB

Scale NTS

Rev

Drawing No. SK01

Job No. 8250

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WILDWOOD LODGE, 9 NORTH END LONDON NW3 7HH, Contract

## Appendix B

Architects Basement and Ground Floor Layouts



Do not scale from this drawing. All dimensions should be decked on site for accuracy.

 $\bigcirc_{z}$ 

Variation reference, document 025-A-SH-01-001 rev 8

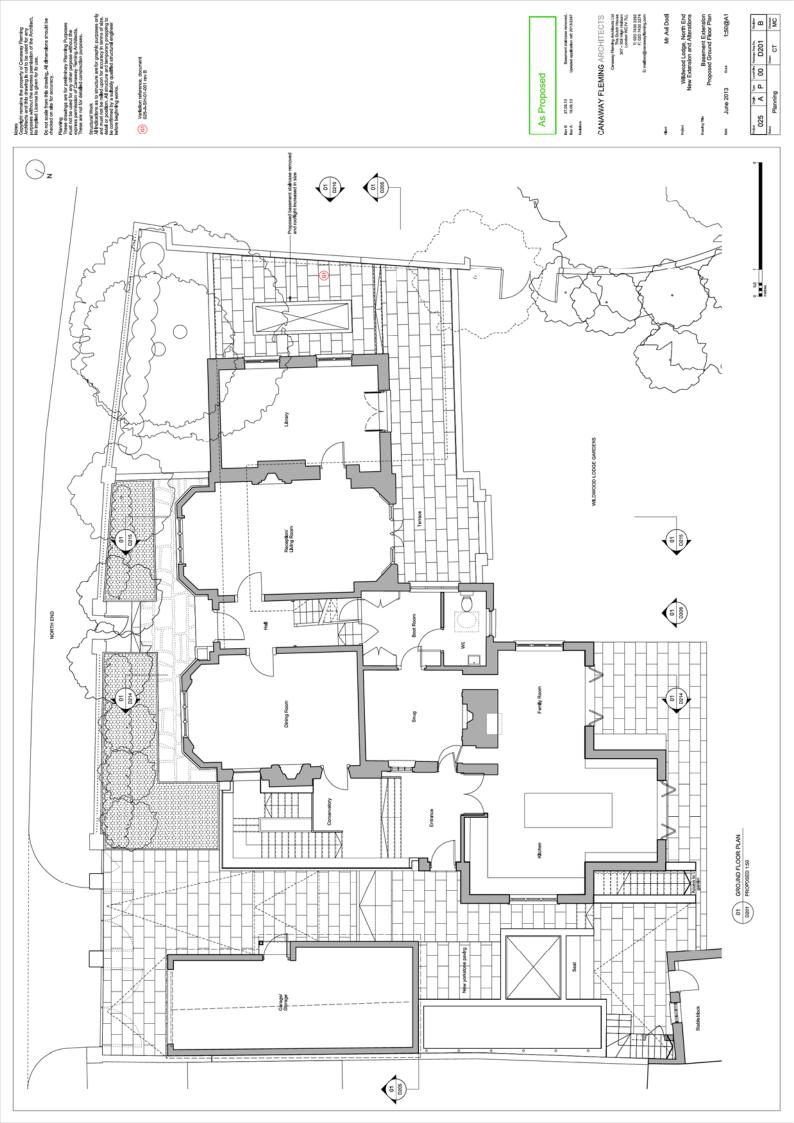
Basement layout amended. Updated application ref: 2013/2247 Basement layout amended,

CANAWAY FLEMING ARCHITECTS

T: 020 7430 2262 F: 020 7430 2274 E: nalbox@canawayfleming.com

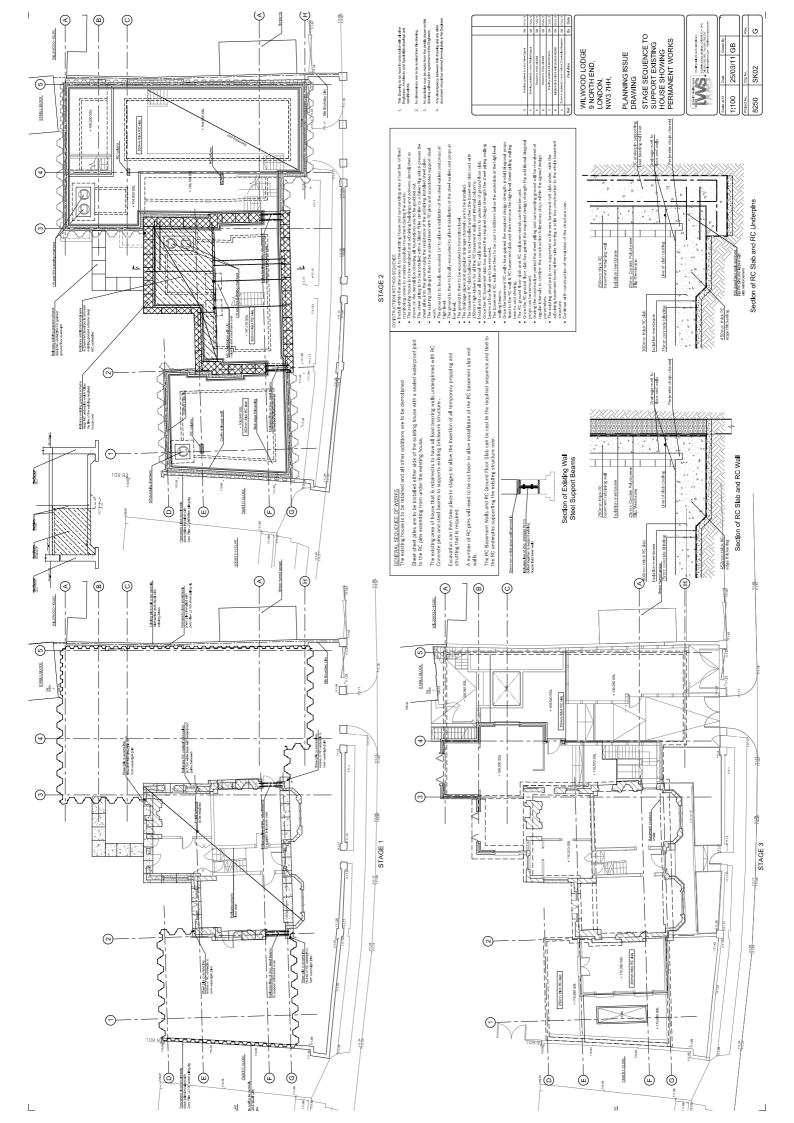
Mr Avi Dodi Wildwood Lodge, North End New Extension and Alterations Basement Extension Proposed Basement Plan 1:50@A1 88





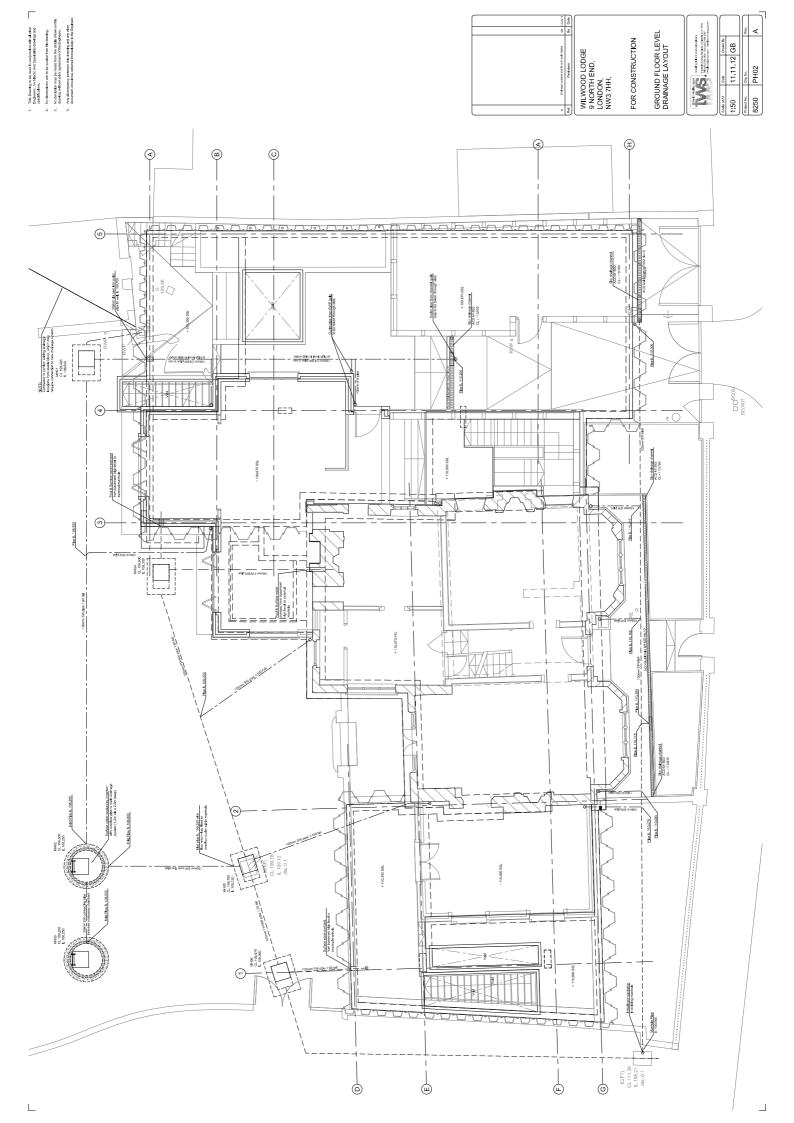
## Appendix C

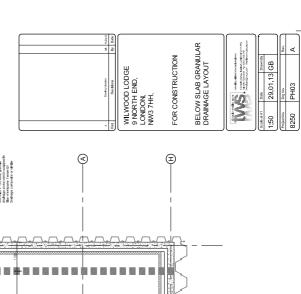
TWS - 8250\_SK02 rev G - Stages Sequence to Support Existing House

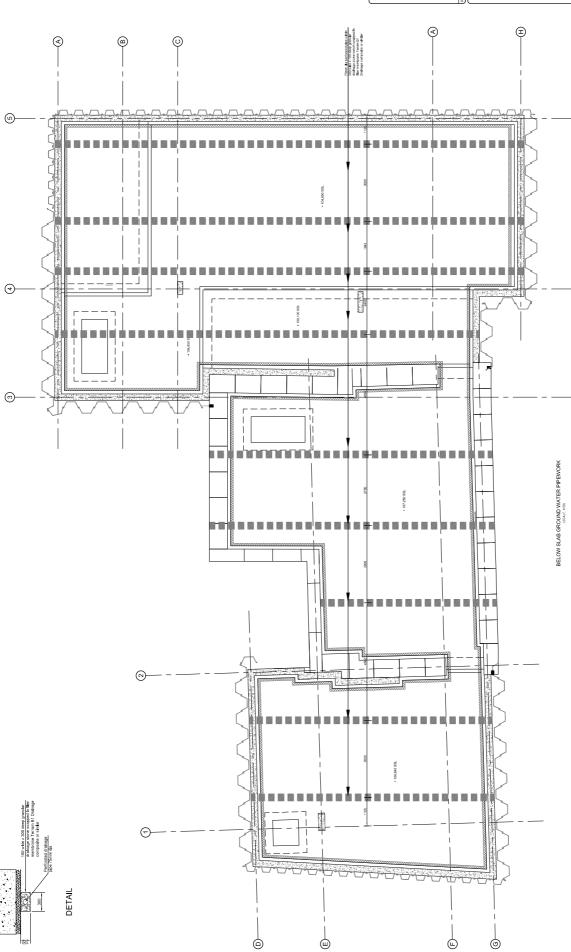


### Appendix D

TWS - 8250\_PH02 rev A - Ground Floor Level Drainage Layout TWS - 8250\_PH03 rev A - Below Slab Granular Drainage Layout







## Appendix E

MRH Ground Investigation Report Ref 121311-January 2012

CONSULTANCY, SITE INVESTIGATION CONSTRUCTION MATERIALS TESTING, CONTAMINATED LAND SURVEYS, DESK STUDIES, RISK ASSESSMENT.



## GROUND INVESTIGATION FOR

9 NORTH END LONDON NW3 7HH

Job No:

121311

Date

January 2012







# REPORT ON A GROUND INVESTIGATION AT WILDWOOD LODGE, 9 NORTH END, HAMPSTEAD, LONDON NW3 7HH

#### 1 INTRODUCTION

- 1.1 This report has been prepared for Taylor Whalley Spyra, who are acting on behalf of Allenton Ltd.
- 1.2 Our brief for the investigation was to:
  - a) Construct three boreholes with associated soil sampling and in situ testing
  - b) Laboratory testing of soil samples for classification

#### 2 DETAILS OF FIELD WORK

- 2.1 The fieldwork comprised the construction of three boreholes at the positions indicated in appendix A.
- 2.2 Soil samples were recovered at regular intervals during the drilling operations, sealed in inert, airtight containers and transported to the laboratory for testing and detailed descriptions.
- 2.3 Water level observations were made during the drilling works and noted on the borehole logs.
- 2.4 The fieldwork was carried out on the 23rd and 24th January 2012.

#### 3 GENERAL GEOLOGY AND REVEALED STRATA

- 3.1 The boreholes proved Made Ground to depths varying from 1.20m 1.60m.
- 3.2 The boreholes then penetrated firm silty Clay with clayey Sands and Gravels from 1.80m 3.60m (BH 2) and 2.30m 3.80m (BH 2).
- 3.3 The boreholes were extended and encountered firm to stiff, becoming stiff silty Clay with laminations of silt.
- 3.4 Details of the boreholes, sample depths, in situ test results and revealed stratum are given in appendix B.
- 3.5 The 1:50,000 scale geological map indicates the natural deposits of area to be near a boundary of The Bagshot Formation and Claygate Beds with London Clay at depth.

#### 4 GROUNDWATER

- 4.1 Water seepage's were noted at depths of 3.30m (BH 1), 2.10m (BH 2) and 3.40m (BH 3).
- 4.2 In order to allow long term monitoring, piezometers were installed in each borehole. On completion of the drilling works, water levels of 2.80m (BH's 1 and 3), and 2.75m (BH 2) were recorded.

#### Wildwood Lodge, 9 North End, Hampstead, London NW3 7HH

#### 5 LABORATORY TESTING

- 5.1 The recovered soil samples were tested for moisture levels, together with ten Atterberg Limit determinations.
- 5.2 The results and detailed sample descriptions are tabulated in appendix C, categorising the Clay elements to be of medium to high plasticity (Plasticity Index 28% 42%).
- 5.3 Although this is indicative of a moderately high susceptibility to moisture related cyclic volume change there were no indications of desiccation within the samples tested.

#### 6 CONCLUSIONS

- 6.1 The findings of the boreholes indicate natural ground at depths of 1.20m 1.60m.
- 6.2 We understand that the proposed development will comprise the construction of a new structure incorporating basements
- 6.3 The in situ tests carried out in the boreholes indicate the following bearing capacities for foundation design purposes within the natural ground. However, any open excavations would require shoring and the facility of pumping groundwater, due to the instability of the ground and high water table.:

BH No	Depth (m)	Allowable Bearing Capacity (KN/m²)				
1	2.00	105				
1	2.50	110				
2	1,50	110				
2	2.00-2.45	160 (SPT)				
3	1.50	120				
3	2.00	110				
3	2.50-2.95	120 SPT)				

- 6.4 Plots of the Shear Strengths versus depth profiles are presented in appendix B (Sheet 7), while the SPT (N) values are noted on the borehole logs.
- The soluble sulphate contents of the samples tested from boreholes 1 and 3 at a depth of 2.00m were 340mg/l and 360mg/l with corresponding pH values of 7.4 and 7.5 respectively.
- 6.6 The site can therefore be categorised as DS1 in accordance with BRE guidelines, thus not requiring any special precautions for concrete in contact with the ground.

#### Wildwood Lodge, 9 North End, Hampstead, London NW3 7HH

#### 7 REFERENCES

1) British Standard EN ISO 14688-1:2002

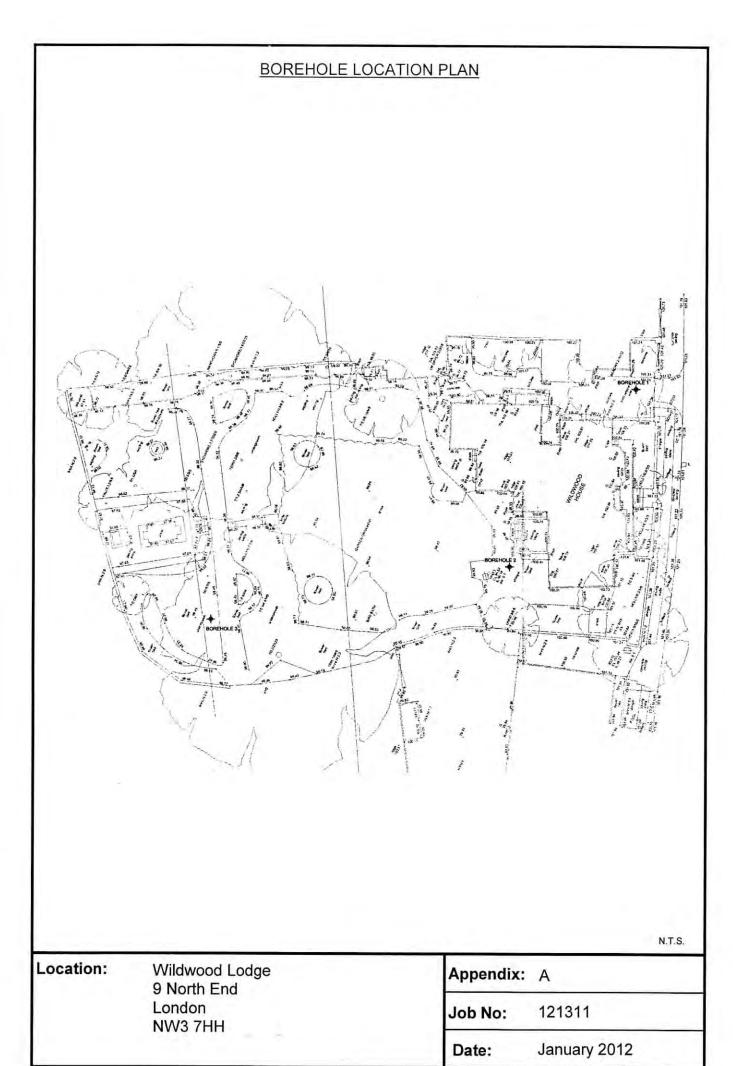
Stephen Hewher.

- 2) British Standard 5930: 1999
- 3) British Standard 1377: Parts 1-9
- 4) British Geological Survey Sheet 256 (1:50,000 scale) North London
- 5) BRE Special Digest 1: Concrete in aggressive ground (2005)
- 6) NHBC Standards, Chapter 4.2
- 7) Foundation Design and Construction (M.J. Tomlinson, Fifth Edition)

Stephen J. Hudson

mail@mrhgeotechnical.com

# APPENDIX A BOREHOLE LOCATION PLAN



# APPENDIX B BOREHOLE LOGS

BORE	OREHOLE LOG - M R H GEOTECHNICAL  HOLE NO. Sheet 1 of									1		
CLIENT			Aller	nton Ltd		SITE	wood Lodge, 9 No					
DATE OF FI 23/01/1	ELDWORK 12 - 23/01		sc	CALE 1:50	LEVEL/POSITION GROUND / AS	APPENDIX A	OPERATOR PA/SA	LOGGED BY	JOB NO	). 1311		
SAMPLE DEPTH	RECORD TYPE		T N S	Standp/ Piezo	DESC	RIPTION OF S	TRATUM (thicknes	ss)	DEPTH	LEGENI		
				77 77	A C	Cobble paving Sand (0.09	(0.10)		0.10			
0.50	D1				Soft grey very sil brick fragments. M	lty, sandy cla	ay with rounded	stones and	0.15			
1.00	D2	(38)										
1.50	D3	(40)			Firm pale brown la	minated orang	ge brown very si	lty slightly	1.60			
2.00	D4	(52)			Sandy CLAT WITH LI	sandy CLAY with traces of gravel (2.20)						
2.50	D5	(54)			Water standing at :	2.80m				* x x x x x x x x x x x x x x x x x x x		
3.00	D6	(54)			Water seepage at 3				Σ	* * * * * * * * Z * * *		
3.50	D7	(56)							3.80	× × × × × × × × × ×		
_4.00 - - - - - - 4.50	D8	(60)			Firm dark greenish silty CLAY (0.90)	brown with t	races of orange	brown very		× × × × × ×		
- 4.50 - - - -5,00	D10	(58)		-	Firm grey very sil	Firm grey very silty CLAY with laminations of silt (2.40)						
- 5.50	D10									x x x x x x x x x x x x x x x x x x x		
- - - - - - - -	D12	(64)	KIIIIX		Piezometer installe	ed				* * * * * * * * * * * * * * * * * * *		
6,50	D13									* x x x x x x x x x x x x x x x x x x x		
7,00	D14	(74)			Firm to stiff grey	very silty C	TLAY (3.70)		7,10	* * * * * *		
7.50	D15					300	Rich Assessor			* * * * * *		
8.00	D16	(78)								× × × × × × ×		
9.00	D17	(76)								* * * * * * * * * * * * * * * * * * *		
					Bore	ehole continu	es on Sheet 2			x x x x x x x x x x x x x x x x x x x		
_10.00	D18	(88)	);;;50 DI		TA-0	11010 00111111	To on once a		*	× × ·		
DEPTH DEPTH STRUCK CASE		WATER	DEPTH		EMARKS ON GROUNDWATER A	AND CAPING	The state of the s	D AND REMARKS				
STRUCK CASE	D TIME 1HOUR	2.80	SEALED	) NEI	seepage at 3,30m, pi	Y	Mechanical auge Piezometer inst					
							KEY: D = Disturbed U = Undisturb All dimensions		Bulk Sample Water Sample erwise stated			

BORE	HOLE	LOG	- M F		HOLE NO.	BH 2	1		
CLIENT		A	Allenton I	i d	SITE Wild	wood Lodge, 9 No	orth End, London	NW3 7HH	
DATE OF F 23/01/1	ELDWORK 2 - 23/01		SCALE 1:50	LEVEL/POSITION GROUND / A	S APPENDIX A	OPERATOR PA/SA	LOGGED BY	JOB NO	
SAMPLE DEPTH	RECORD TYPE	SPT N (Cu-kN/r	Stand	DES	CRIPTION OF S	TRATUM (thicknes	ss)	DEPTH	LEGENI
				Pirm to stiff gre	ey very silty	CLAY			* * * * * * * * * * * * * * * * * * *
11.00	D19	(110)		Stiff grey very s	silty CLAY (2.	40)		10.80	x x x x x x x x x x x x x x x x x x x
12.00	D20	(114)							x x x x x x x x x x x x x x x x x x x
_13.00	D21	(122)		Borehole e	ends, unable to	o penetrate obst	ruction	13.20	* * * * * * * * * * * *
GROUNDWAT DEPTH DEPTH TRUCK CASE		WATER D	EALED Wat	N REMARKS ON GROUNDWATER or seepage at 3.30m, postalled		BORING METHO Mechanical auge Piezometer inst KEY: D = Disturbe U = Undistur	d Sample B =	Bulk Sample Water Sample	

#### BOREHOLE LOG - M R H GEOTECHNICAL HOLE NO. BH 2 Sheet 1 of 2 CLIENT SITE Allenton Ltd Wildwood Lodge, 9 North End, London NW3 7HH DATE OF FIELDWORK LEVEL/POSITION SCALE **OPERATOR** LOGGED BY JOB NO. 23/01/12 ~ 24/01/12 1:50 GROUND / AS APPENDIX A PA/SA SH 121311 SAMPLE RECORD SPT N Standp/ DESCRIPTION OF STRATUM (thickness) DEPTH DEPTH (Cu-kN/m<sup>2</sup> LEGEND Piezo Turf over topsoil (0.20) 0.20 Firm greyish brown laminated dark orange brown silty, sandy clay with occasional gravel and brick fragments. MADE GROUND (1.10)0.50 DI 1.00 D2 (58) 1.30 Firm yellowish brown laminated pale bluish grey and dark orange brown very silty CLAY (0.50)1 50 D3 (52) 1.80 2.00 - 2.45 D4 N=16 Medium dense orange brown silty, clayey SAND and GRAVEL (0.30) 2.10 V Firm orange brown very silty, slightly sandy CLAY with traces 2.50 D5 (50) Water seepage at 2.10m Water standing at 2.75m 3.00 D6 (58) 3.50 D7 1721 3.40 Firm to stiff dark grey very silty CLAY (2.90) 4.00 D8 (78) 4.50 D9 (76) 5.00 DIO (75) 5 50 D11 6.00 D12 (80) Piezometer installed 5 30 Firm to stiff dark bluish grey very silty CLAY with laminations of silt $\{4.50\}$ 6.50 D13 -7.00 (76) 7.50 D15 8.00 D16 (76) 9.00 D17 (B2) Borehole continues on Sheet 2 -10.00 D18 (94) GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS DEPTH WATER ELAPSED TIME REMARKS ON GROUNDWATER AND CASING Mechanical auger Piezometer installed 2.10 1HOUR 2.75 Water seepage at 2.10m. Piezometer installed KEY: D = Disturbed Sample B = Bulk Sample U = Undisturbed Sample W = Water Sample All dimensions are in metres unless otherwise stated

	TOLE	LUG	- IVI K	H GEOTECHN			Sheet 2 of	2	
CLIENT		A	llenton Ltd		SITE Wild	wood Lodge, 9 N	orth End, London	NW3 7HH	
DATE OF FIE 23/01/12	LDWORK - 24/01/		SCALE 1:50	LEVEL/POSITION GROUND / AS	APPENDIX A	OPERATOR PA/SA	LOGGED BY	JOB NO	1311
SAMPLE F	RECORD	SPT N (Cu-kN/r	Standp/ Piezo	DESCR	RIPTION OF ST	TRATUM (thickne	ss)	DEPTH	LEGEN
				Firm to stiff dark laminations of silt			Y with	10.80	× × × × × × × × × × × × × × × × × × ×
11.00	D19	(118)		Stiff dark grey ver	ry silty CLA	Y (4.20)			* * * * * * * * * * * * * * * * * * *
_12.00	D20	(114)							* * * * * * * * * * * * * * * * * * *
13,00	D21								*
14 00	D22	(128)							* * * * * * * * * * * * * * * * * * *
_15,00	D23	(130)			Borehole	ends		15.00	* * * * * * * * * * * * * * * * * * *
GROUNDWA	TER AND	CASING IN	FORMATION	I-		BORING METU	OD AND REMARKS		
DEPTH DEPTH STRUCK CASE	ELAPSED		DEPTH SEALED Wate	REMARKS ON GROUNDWATER A r seepage at 2.10m. P alled		Mechanical au Piezometer in	ger		
								Bulk Sample Water Sample	ė

#### HOLE NO. BH 3 BOREHOLE LOG - M R H GEOTECHNICAL Sheet 1 of 2 CLIENT SITE Allenton Ltd Wildwood Lodge, 9 North End, London NW3 7HH DATE OF FIELDWORK SCALE LEVEL/POSITION **OPERATOR** LOGGED BY JOB NO. 24/01/12 - 24/01/12 1:50 GROUND / AS APPENDIX A PA/SA SH 121311 SAMPLE RECORD SPT N Standp/ DESCRIPTION OF STRATUM (thickness) DEPTH LEGEND DEPTH TYPE Cu-kN/m2) Piezo Black sandy topsoil with traces of fine gravel and brick fragments. MADE GROUND (0.60)0.50 D1 0.60 Friable pale brown silty clay with traces of brick fragments. MADE GROUND (0.60)1.00 D2 1.20 Firm pale brown laminated bluish grey and yellowish brown silty CLAY (1.10)1.50 D3 (60) 2.00 D4 (54)2.30 2.50 - 2.95 D5 N = 12Medium dense yellowish brown silty clayey SAND with some gravel and black rounded pebbles $(1.50)\,$ Water standing at 2,80m 3.00 D6 3.50 + 3.95 N=19 Water seepage at 3.40m 3.80 4.00 (66) Firm to stiff dark grey laminated dark bluish grey very silty CLAY with laminations of silt (3.30)D8 4.50 (58) D9 5.00 D10 (64) 5.50 D11 6.00 D12 (76) Piezometer installed 6.50 D13 7.00 D14 (94) 7.10 Stiff dark grey very silty CLAY with laminations of silt 7.50 D15 8.00 D16 (104) 9.00 D17 (112)Borehole continues on Sheet 2 \_10.00 (106) GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS ELAPSED TIME WATER LEVEL DEPTH SEALED Mechanical auger REMARKS ON GROUNDWATER AND CASING Piezometer installed 3.40 1HOUR 2.80 Water seepage at 3.40m. Piezometer installed KEY: D = Disturbed Sample B = Bulk Sample U = Undisturbed Sample W = Water Sample All dimensions are in metres unless otherwise stated

	HOLE	LOG	- M R	H GEOTECHI	1		HOLE NO. Sheet 2 of	BH 2	3
CLIENT		7	Allenton L	£ď	SITE Wildw	wood Lodge, 9 N	North End, London	NW3 7HH	
	12 - 24/01,	/12	SCALE 1:50	LEVEL/POSITION GROUND / AS	APPENDIX A	OPERATOR PA/SA	LOGGED BY	JOB NO	). 1311
SAMPLE DEPTH	RECORD TYPE	SPT N (Cu-kN/r	N Standp m <sup>2</sup> ) Piezo	DESC	RIPTION OF S	TRATUM (thickne	ess)	DEPTH	LEGEN
11.00	D19	(116)		Stiff dark grey ve	ry silty CLAY	Y with laminati	ons of silt		X X X X X X X X X X X X X X X X X X X
13.00	D21	(130)		Boreholé en	ds, unable to	o penetrate obst	cruction	13,20	* * * * * * * * * * * * * * * * * * *
BROUNDWAT DEPTH DEPTH STRUCK CASED 3.40 -	H ELAPSED TIME	WATER DI	DEPTH EALED Wate	N  REMARKS ON GROUNDWATER AN er seepage at 3.40m. Picalled	ND CASING	BORING METHO Mechanical auge Piezometer inst			
						KEY: D = Disturber U = Undisturb		Bulk Sample Water Sample	

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Appendix B

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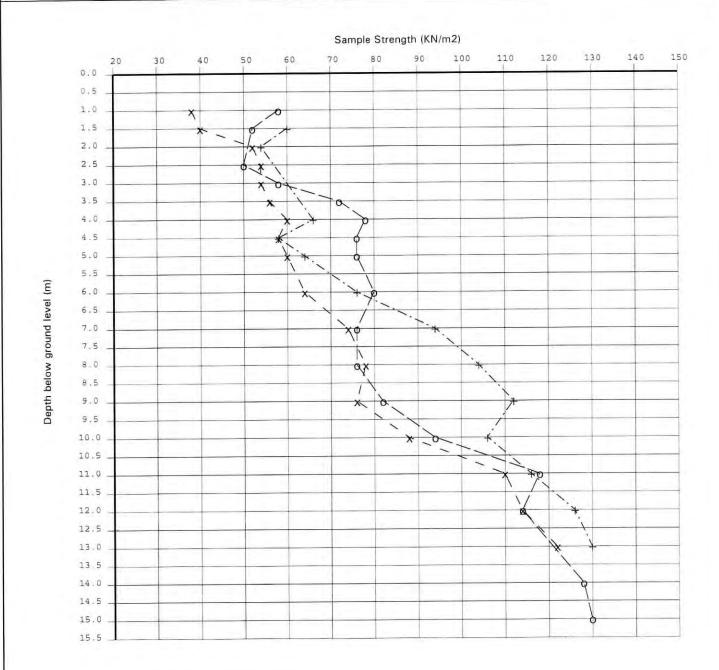
Job No.

Wildwood Lodge, 9 North End,

121311

London NW3 7HH

Sample Strength (KN/m2) vs Depth below ground level (m)



	x: BH 1	o: BH 2	+ : BH 3			
Key to						7
Data Points						
			8			

## APPENDIX C

MOISTURE CONTENT TEST RESULTS
AND
ATTERBERG LIMIT DETERMINATIONS

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Appendix C

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## SUMMARY OF MOISTURE CONTENT, LIQUID LIMIT, PLASTIC LIMIT,

### PLASTICITY INDEX AND LIQUIDITY INDEX

Borehole/ Pit No.	Depth m.	Sample	Moisture Content (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)	Description (BS 5930:1981:41)
BH 1	0.50	D1	32		=	37		Soft grey very silty, sandy clay with rounded stones and brick fragments. MADE GROUND
BH I	1.00	D2	30	\$	9	181		Soft grey very silty, sandy clay with rounded stones and brick fragments. MADE GROUND
BH 1	1.50	D3	30	-		- e		Soft grey very silty, sandy clay with rounded stones and brick fragments. MADE GROUND
BH 1	2.00	D4	20	46	16	30	0.13	Firm pale brown laminated orange brown very silty slightly sandy CLAY with traces of gravel. CI: CLAY of medium plasticity. (97% passing 425um)
BH 1	2.50	D5	21	.3	95	12.		Firm pale brown laminated orange brown very silty slightly sandy CLAY with traces of gravel
BH 1	3.00	D6	23	48	19	29	0.14	Firm pale brown laminated orange brown very silty slightly sandy CLAY with traces of gravel. CI: CLAY of medium plasticity. (94% passing 425um)
вн 1	3.50	D7	27	2	-	4.5		Firm pale orange brown very silty CLAY
BH 1	4.00	D8	27	59	24	35	0.09	Firm dark greenish brown with traces of orange brown very silty CLAY with laminations of silt. CH: CLAY of high plasticity. (100% passing 425um)
BH 1	4.50	D9	29		2	1,2		Firm dark greenish brown with traces of orange brown very silty CLAY with laminations of silt
BH 1	5.00	D10	30	66	26	40	0.10	Firm grey very silty CLAY with laminations of silt. CH: CLAY of high plasticity. (100% passing 425um)
BH 1	5.50	D11	30	1	+	100		Firm grey very silty CLAY with laminations of silt
BH 1	6.00	D12	29	₹1				Firm grey very silty, slightly sandy CLAY
BH 1	6.50	D13	32	8	7			Firm grey very silty CLAY
BH 1	7.00	'D14	32	-	-	10.5		Firm grey very silty CLAY with laminations of silt
BH 1	7.50	D15	31	91	1-	115		Firm to stiff grey very silty CLAY
BH 1	8.00	D16	31	-	i e s			Firm to stiff grey very silty CLAY
		. —						

METHOD OF PREPARATION : BS 1377:PART 1:1990:7.4 & PART 2:1990:4.2

BS 1377:PART 2:1990:3.2, 4.4, 5.3, 5.4 METHOD OF TEST

: U = Undisturbed, B = Bulk, D = Disturbed, J = Jar, W = Water, SPT = Split Spoon Sample, TYPE OF SAMPLE KEY

C = Core Cutter

COMMENTS

: Sample disturbance, loss of moisture, variation from test procedure, location and origin REMARKS TO INCLUDE of test specimen within original sample. Oven drying temperature if not 105-110 deg C.

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## SUMMARY OF MOISTURE CONTENT, LIQUID LIMIT, PLASTIC LIMIT,

### PLASTICITY INDEX AND LIQUIDITY INDEX

Borehole/ Pit No:	Depth m.	Sample	Moisture Content (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)	Description (BS 5930:1981:41)
BH 1	9.00	D17	36	7	1.5	×		Firm to stiff grey very silty CLAY with numerous laminations of silt
BH 1	10.00	D18	32	-	13	4/		Firm to stiff grey very silty CLAY with laminations of silt
BH 1	11.00	D19	30	- 2	- 5	-		Stiff grey very silty CLAY
BH 1	12.00	D20	29	-2	-	2		Stiff grey very silty CLAY
BH 1	13.00	D21	29	3	4	20		Stiff grey very silty CLAY
BH 2	0.50	DI	23	+	(-)			Firm greyish brown silty, sandy clay with occasional gravel, topsoil and brick fragments. MADE GROUND
BH 2	1.00	D2	22		-			Firm dark brown laminated dark orange brown very silty, sandy clay with occasional gravel and brick fragments. MADE GROUND
BH 2	1.50	D3	23	48	19	29	0.14	Firm yellowish brown laminated pale bluish grey and dark orange brown very silty CLAY, CI: CLAY of medium plasticity. (98% passing 425um)
BH 2	2.00	D4	15	100	(le)	12		Medium dense orange brown silty, clayey SAND and GRAVEL
BH 2	2.50	D5	23	7		=		Firm orange brown very silty, slightly sandy CLAY with traces of gravel
BH 2	3.00	D6	27	115	4	-		Firm orange brown very silty, slightly sandy CLAY with traces of gravel
BH 2	3.50	D7	28	66	24	42	0.10	Firm to stiff dark grey laminated brown very silty CLAY. CH: CLAY of high plasticity.  100% passing 425um)
BH 2	4,00	D8	28		-			Firm to stiff dark grey very silty CLAY
BH 2	4,50	. D9	28	0	-	3		Firm to stiff dark grey very silty CLAY with laminations of silt
BH 2	5.00	D10	29	~	-	-		Firm to stiff dark grey very silty CLAY
BH 2	5.50	D11	29	-	2	4		Firm to stiff dark bluish grey very silty CLAY

METHOD OF PREPARATION : BS 1377:PART 1:1990:7.4 & PART 2:1990:4.2

METHOD OF TEST : BS 1377: PART 2:1990:3.2, 4.4, 5.3, 5.4

: U = Undisturbed, B = Bulk, D = Disturbed, J = Jar, W = Water, SPT = Split Spoon Sample, TYPE OF SAMPLE KEY

C = Core Cutter

COMMENTS

REMARKS TO INCLUDE : Sample disturbance, loss of moisture, variation from test procedure, location and origin of test specimen within original sample. Oven drying temperature if not 105-110 deg C.

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## SUMMARY OF MOISTURE CONTENT, LIQUID LIMIT, PLASTIC LIMIT, PLASTICITY INDEX AND LIQUIDITY INDEX

Borehole/ Pit No	Depth m.	Sample	Moisture Content	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)	Description (BS 5930:1981:41)
BH 2	6.00	D12	31	68	28	40	0,08	Firm to stiff dark grey very silty CLAY. CH: CLAY of high plasticity, (100% passing 425um)
BH 2	6.50	D13	34	- 2	3	-		Firm to stiff dark bluish grey very silty CLAY with numerous laminations of silt
BH 2	7.00	D14	31	- 1	=	2		Firm to stiff dark bluish grey very silty CLAY with laminations of silt
BH 2	7.50	D15	30	-		1+0		Firm to stiff dark bluish grey very silty CLAY
BH 2	8.00	D16	31	-	-			Firm to stiff dark bluish grey very silty CLAY with laminations of silt
BH Z	9.00	D17	31	7	18	31		Firm to stiff dark bluish grey very silty CLAY with laminations of silt
BH 2	10.00	D18	29	-	100	1.60		Firm to stiff dark bluish grey very sllty CLAY
BH 2	11.00	D19	28		G <del>E</del> o	-		Stiff dark grey very silty CLAY
BH 2	12.00	D20	29	(2.5)	9	T Bell		Stiff dark grey very silty CLAY with laminations of silt
BH 2	13.00	D21	28	G	3	5.0		Stiff dark grey very silty CLAY
BH 2	14.00	D22	27	1.0	8	4		Stiff dark grey with traces of orange brown very silty CLAY, occasional laminations of silt
BH 2	15.00	D23	27	1451	÷	-		Stiff dark bluish grey very silty CLAY
BH 3	0.50	D1	30		- 3-	=		Black sandy topsoil with traces of fine gravel and brick fragments. MADE GROUND
BH 3	1.00	D2	15	-2.1	.=	121		Friable pale brown silty clay with traces of brick fragments. MADE GROUND
BH 3	1.50	D3	23	47	19	28	0.14	Firm pale brown laminated bluish grey and yellowish brown silty CLAY. CI: CLAY of medium plasticity, (99% passing 425um)
BH 3	2,00	D4	26	1.5	-	-		Firm pale brown laminated bluish grey and yellowish brown silty CLAY

METHOD OF PREPARATION : BS 1377:PART 1:1990:7.4 & PART 2:1990:4.2

: BS 1377:PART 2:1990:3.2, 4.4, 5.3, 5.4 METHOD OF TEST

TYPE OF SAMPLE KEY : U = Undisturbed, B = Bulk, D = Disturbed, J = Jar, W = Water, SPT = Split Spoon Sample,

C = Core Cutter

COMMENTS

REMARKS TO INCLUDE : Sample disturbance, loss of moisture, variation from test procedure, location and origin of test specimen within original sample. Oven drying temperature if not 105-110 deg C.

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121311

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# SUMMARY OF MOISTURE CONTENT, LIQUID LIMIT, PLASTIC LIMIT, PLASTICITY INDEX AND LIQUIDITY INDEX

Borehole/ Pit No,	Depth m.	Sample	Moisture Content	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index	Description (BS 5930:1981:41)
BH 3	2.50	D5	14		-	7-8-		Medium dense yellowish brown silty clayey SAND with some gravel
BH 3	3.00	D6	15	۵	-	- :		Medium dense yellowish brown silty clayey SAND and GRAVEL with some black rounded pebbles
BH 3	3.50 -3.95	D7	11	*	-	-		Medium dense yellowish brown silty, clayey, sandy GRAVEL
BH 3	4.00	D8	29	66	25	41	0.10	Firm to stiff dark bluish grey very silty CLAY. CH: CLAY of high plasticity. (100% passing 425um)
BH 3	4.50	D9	29	~	11.	2		Firm to stiff dark grey very silty CLAY with numerou laminations of silt
BH 3	5,00	D10	31	*	-	-		Firm to stiff dark grey laminated dark bluish grey very silty CLAY with laminations of silt
BH 3	5.50	D11	30	68	27	41	0.07	Firm to stiff dark grey laminated dark bluish grey very silty CLAY. CH: CLAY of high plasticity. (100% passing 425um)
ВН 3	6.00	D12	28	15	· · · ·			Firm to stiff dark grey silty CLAY
BH 3	6.50	D13	28	×-	~	>€×		Firm to stiff dark grey laminated dark bluish grey very silty CLAY
BH 3	7.00	D14	28	1=1	-	×		Firm to stiff dark grey silty CLAY with occasional white shell debris
BH 3	7.50	D15	28		-	-		Stiff dark grey silty CLAY with occasional white shell debris
BH 3	8.00	D16	29	0	( <del>-</del> 4.)	100	11	Stiff dark bluish grey very silty CLAY
BH 3	9.00	D17	29	-		-5		Stiff dark bluish grey very silty CLAY
ВН 3	10.00	DIB	33		3	-		Stiff dark grey very silty CLAY with laminations of silt
BH 3	11.00	Di9	32	-	3	-		Stiff dark grey very silty CLAY with laminations of silt
BH 3	12.00	D20	29	5		>-		Stiff dark grey very silty CLAY
BH 3	13.00	D21	30			-	1.0	Stiff dark grey silty CLAY

METHOD OF PREPARATION : BS 1377:PART 1:1990:7.4 & PART 2:1990:4.2

METHOD OF TEST ; BS 1377:PART 2:1990:3.2, 4.4, 5.3, 5.4

: U = Undisturbed, B = Bulk, D = Disturbed, J = Jar, W = Water, SPT = Split Spoon Sample, TYPE OF SAMPLE KEY

C = Core Cutter

COMMENTS

REMARKS TO INCLUDE : Sample disturbance, loss of moisture, variation from test procedure, location and origin of test specimen within original sample. Oven drying temperature if not 105-110 deg C.

## Appendix F

Camden Geological-Hydrogeological & Hydrological Study Extracts Figures 11, 12, 14, 15 & 16



## 9 NORTH END HAMPSTEAD LONDON, NW3 7HH

#### CAMDEN GEOLOGICAL, HYDROGEOLOCICAL AND HYDROLOGICAL STUDY EXTRACTS

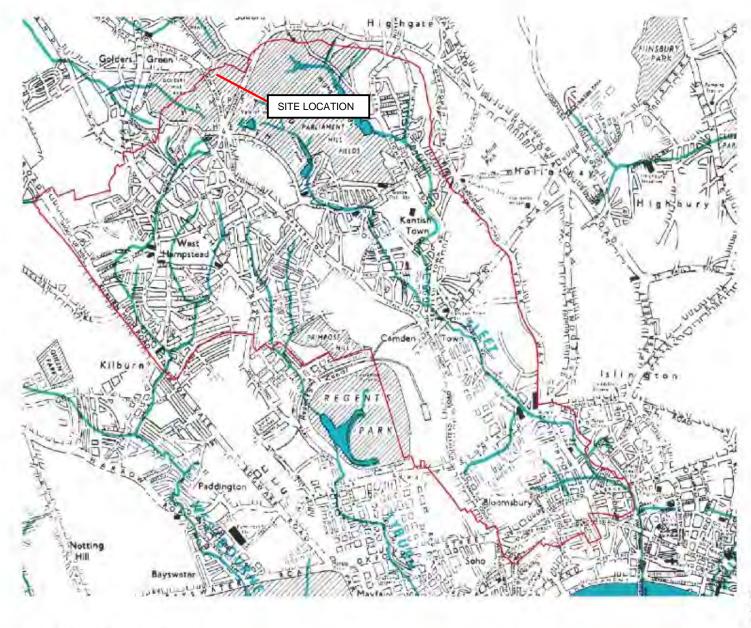
FIGURES 11 - WATERCOURSES

FIGURES 12 – CAMDEN SURFACE WATER FEATURES

FIGURES 14 - HAMPSTEAD HEATH SURFACE WATER CATCHMENTS AND DRAINAGE

FIGURES 15 - FLOOD MAP

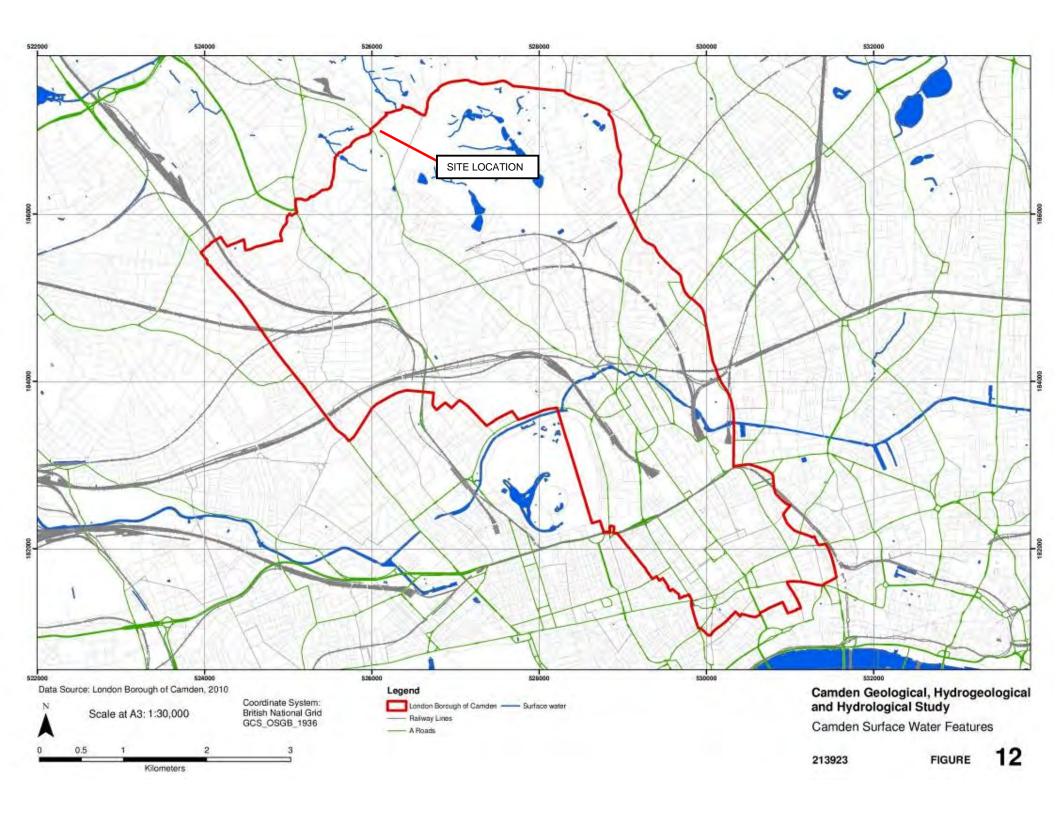
FIGURES 16 - SLOPE ANGLE MAP

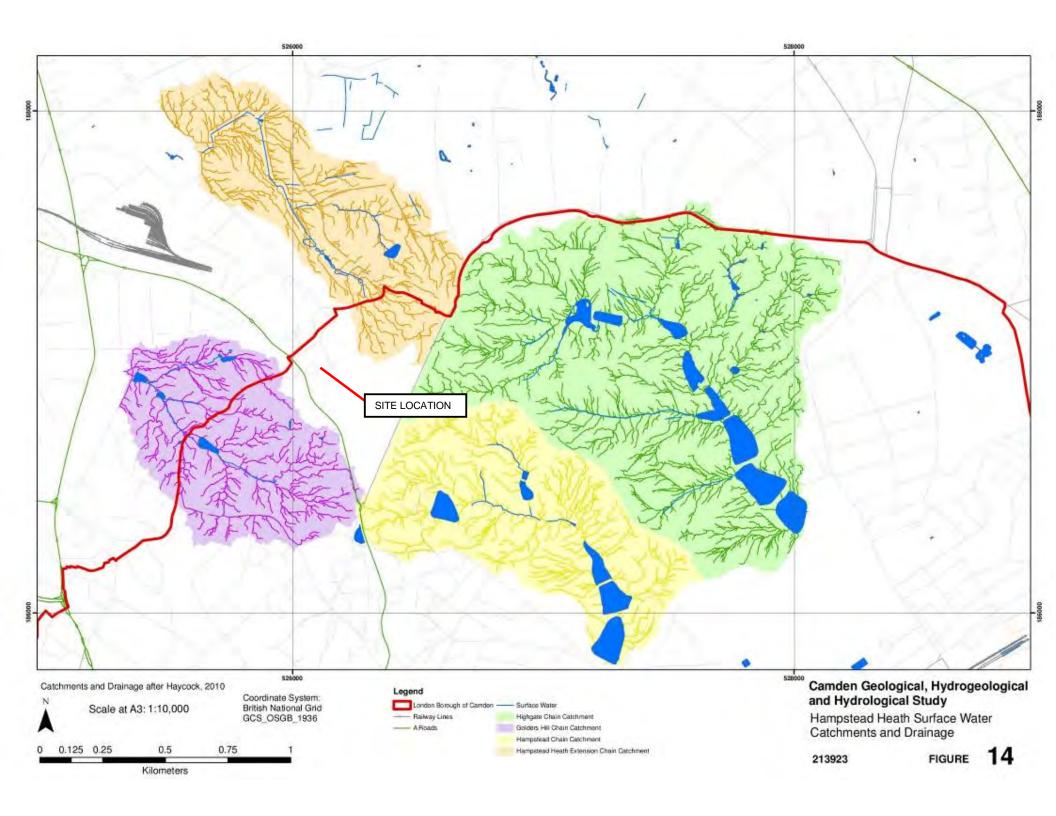


Camden Geological, Hydrogeological and Hydrological Study Watercourses

Source - Barton, Lost Rivers of London

213923 FIGURE 11





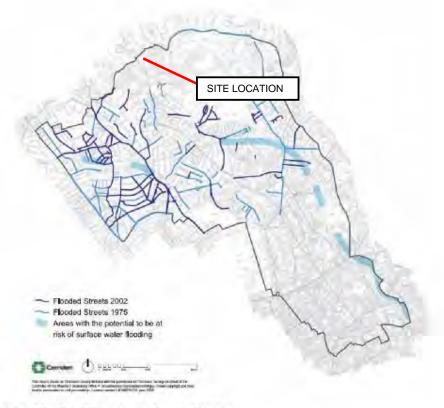
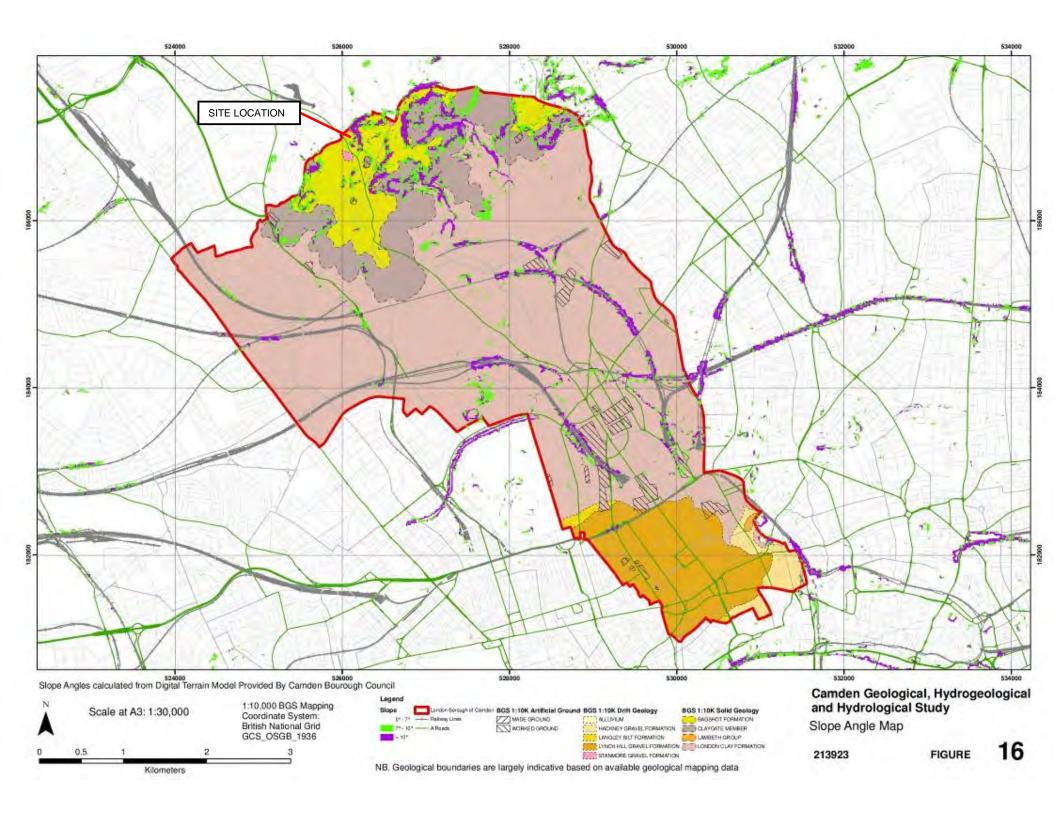


Figure 5 from Core Strategy, London Borough of Camden

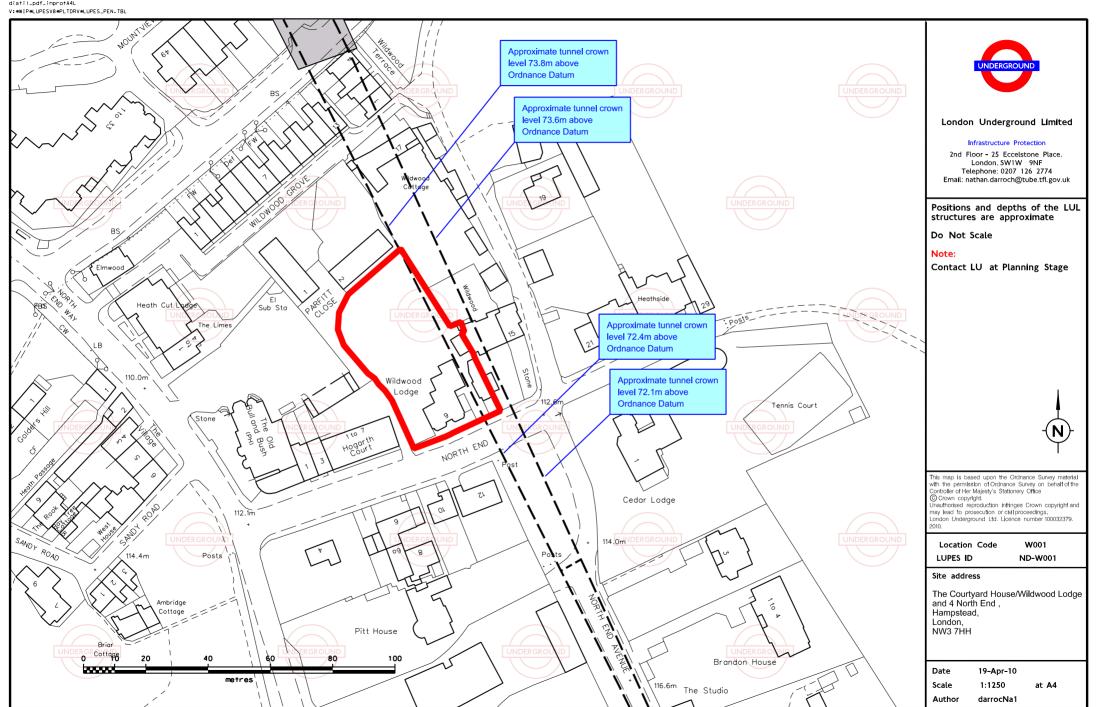
Camden Geological, Hydrogeological and Hydrological Study Flood Map

FIGURE 15



### Appendix G

London Underground Limited Tunnel Location Drawing ND-W001 TWS - 8250\_SK07 rev A – Basement Layout Showing LUL Tunnel Crowns



BASEMENT LAYOUT SHOWING LOCATIONS OF LUL TUNNEL CROWNS Uniformity and interest production in a production of the producti PLANNING ISSUE DRAWING

	Approximate furnel crown level of 73.1m above Ordnance Dafum
	Approximate tunel crown level of 72.4m above Ortherice Datum
	SHOWING C OF CROWN OF D TUNNELS
The first in the international proof is considered to the consider	BASEMENT FLOOR PLAN SHOWING APPROXIMATE CENTRE LINE OF CROWN OF LONDON UNDERGROUND TUNNELS

| Coope at A1 | Debe | Depart | Depart

# Appendix H

Topographical Survey Drawing 11025-P-SI-rev 2

