D Site Investigation and Basement Impact Assessment (GEA)



12 Pilgrims Lane London NW3 1SN

Site Investigation & Basement Impact Assessment Report

December 2024

J21282 Rev 3



Ground investigation | Geotechnical consultancy | Contaminated land assessment

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Rev No	Status	Revision Details	Date	Approved for Issue
0	Draft		24 June 2022	
1	Final	Final review	28 June 2022	
2	Final	Amendments following audit	14 September 2022	
3	Final	Amendments to proposals	11 December 2024	81

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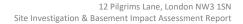
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Executive summary

This executive summary contains an overview of the key findings and conclusions. No reliance should be placed on any part of the executive summary until the whole of the report has been read. Other sections of the report may contain information that puts into context the findings that are summarised in the executive summary.

Brief

This report describes the findings of a ground investigation, basement impact assessment and ground movement analysis carried out by Geotechnical and Environmental Associates Limited (GEA) on the instructions of Elliott Wood Partnership, on behalf of Mr Alex Shamash, with respect to the extension of the existing lower ground floor level, as part of significant alterations to the house. Formation level for the proposed lower ground floor level is understood to be approximately 3.20 m below the existing ground floor level. The purpose of the investigation has been to determine the ground conditions and hydrogeology, to assess the extent of any contamination and to provide information to assist with the design of suitable foundations. The report also includes information required to comply with London Borough of Camden Planning Guidance, with respect to the requirement for a Basement Impact Assessment (BIA).

Desk Study & BIA Findings

The immediate area remained undeveloped until between 1915 and 1935, when the site was developed with the majority of the existing property, and the neighbouring sites to the northeast were also developed with residential properties. The next available map, dated 1952, shows that the infill extension that forms the northeast end of the existing property had been constructed. Few significant changes have subsequently been made within the site and immediately surrounding area. A search of public registers and databases has been made via the Envirocheck database, which indicates that there are no historic or existing landfill sites within 1km of the site, and the NRPB radon atlas lists the site as being in a non-affected area.

Ground conditions

The investigation generally confirmed the expected ground conditions in that, below a nominal to locally moderate thickness of made ground, London Clay was encountered to the maximum depth of the investigation, of 8.00 m. The made ground generally comprised brown sandy silty clay with frequent fine to coarse gravel sized fragments of brick flint and rare charcoal, and extended to depths of between 0.60 m and 2.00 m. Beneath the front garden, the London Clay comprised an initial horizon of stiff becoming firm with depth, slightly desiccated brown sandy silty clay, to a depth of 1.50 m in both Borehole Nos 1 and 2, whereupon firm becoming stiff with depth, fissured slightly sandy silty clay with frequent partings of sand and silt was encountered to a depth of 6.30 m in both boreholes. Below a depth of 6.30 m, Borehole Nos 1 and 2 both encountered stiff becoming very stiff with depth fissured brownish grey silty clay to the maximum depth investigated. Within the rear garden, directly beneath the made ground, both Borehole Nos 3 and 4 generally encountered firm becoming stiff with depth fissured brown silty clay with partings of sand and silt and occasional selenite, which was proved to the base of the boreholes at depths of 4.00 m.

Groundwater was not generally encountered during the investigation, but perched water was encountered within the made ground in one of the trial pits. Contamination testing has revealed the presence of elevated concentrations of arsenic, lead, PAH compounds and asbestos contamination within the made ground.

Recommendations

Formation level of the proposed basement will be within the stiff silty clay of the London Clay. Excavations for the proposed basement structure will require temporary support to maintain stability and to prevent any excessive ground movements. The proposed use of reinforced concrete underpinning to form the proposed lower ground floor and support the existing building should be suitable. Significant groundwater flows are not anticipated within the excavation, although localised inflows are likely to be encountered within the made ground, as well as from partings of silt and sand in the London Clay. Site workers should adopt suitable precautions when handling soil and areas of new soft landscaping will need to be formed with a clean cover system.

Basement Impact Assessment

The BIA has not indicated any concerns with regard to the effects of the proposed basement on the site and surrounding area. It has been concluded that the impacts identified can be mitigated by appropriate design and standard construction practice.

Ground Movement Analysis Conclusions

The analysis has concluded that the predicted damage to the neighbouring properties from the construction of the proposed basements would be 'Negligible'. On this basis, the damage that has been predicted to occur as a result of the construction the proposed basement falls within the limits acceptable to the London Borough of Camden, assuming that the careful control is taken during construction of the proposed excavations, and monitoring will be required to ensure that no excessive movements occur that would lead to damage in excess of these limits. The basement floor slabs will need to be suspended, unless they can be suitably reinforced to cope with anticipated heave movements.







Part 1: Investigation Report

This section of the report details the objectives of the investigation, the work that has been carried out to meet these objectives and the results of the investigation. Interpretation of the findings is presented in Part 2.

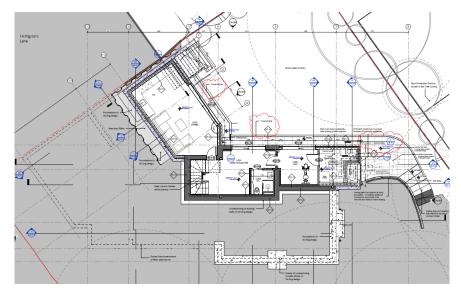
1.0 Introduction

Geotechnical and Environmental Associates Limited (GEA) has been commissioned by Elliott Wood Partnership, on behalf of Mr Alex Shamash, to carry out a desk study, ground investigation and ground movement analysis at 12 Pilgrims Lane, London NW13 1SN.

This report also forms part of a Basement Impact Assessment (BIA), which has been carried out in accordance with guidelines from the London Borough of Camden (LBC) in support of a planning application.

1.1 **Proposed Development**

It is understood that it is proposed to extend the existing lower ground floor level, as part of significant alterations to the house. Formation level for the proposed lower ground floor level is understood to be approximately 3.20 m below the existing ground floor level.



This report is specific to the proposed development and the advice herein should be reviewed if the development proposals are amended.

1.2 Purpose of Work

The principal technical objectives of the work carried out were as follows:

- **G** to check the history of the site with respect to previous contaminative uses;
- **G** to provide an assessment of the risk of encountering unexploded ordnance (UXO);
- **G** to determine the ground conditions and their engineering properties;
- to use the above information to provide recommendations with respect to the design of suitable foundations and retaining walls;
- to assess the impact of the proposed basement on the local hydrogeology, hydrology and stability of the surrounding natural and build environment;
- **G** to provide an indication of the degree of soil contamination present; and
- G to assess the risk that any such contamination may pose to the proposed development, its users or the wider environment.

1.3 Scope of Work

In order to meet the above objectives, a desk study was carried out, followed by a ground investigation. The desk study comprised:

- a review of historical Ordnance Survey (OS) maps and environmental searches sourced from the Envirocheck database;
- **G** a review of readily available geology maps;
- a preliminary UXO risk assessment; and
- **G** a walkover survey of the site carried out in conjunction with the fieldwork.

In the light of this desk study an intrusive ground investigation was carried out which comprised, in summary, the following activities:



- two boreholes advanced to a depth of 8.00 m using a combination of rotary augering and percussive sampling techniques, with standard penetration tests (SPTs) carried out at regular intervals to provide quantitative data on the strength of the soils;
- **G** two window sampler boreholes advanced to a depth of 4.00 m;
- a total of ten foundation inspection trial pits, hand excavated to depths of between 0.75 m and 1.40 m, over two separate phases;
- installation of three gas and groundwater monitoring standpipes and one subsequent monitoring visit carried out to date;
- **G** testing of selected soil samples for contamination and geotechnical purposes; and
- provision of a report presenting and interpreting the above data, together with our advice and recommendations with respect to the proposed development.

This report includes a contaminated land assessment which has been undertaken by a suitably qualified and competent professional in accordance with the methodology presented by the Environment Agency in their Land contamination risk assessment (LCRM)¹ published 8 October 2020. This involves identifying, making decisions on, and taking appropriate action to deal with, land contamination in a way that is consistent with government policies and legislation within the United Kingdom. Risk management is divided into three stages; Risk Assessment, Options Appraisal and Remediation, and each stage comprises three tiers. The Risk Assessment stage includes preliminary risk assessment (PRA), generic quantitative risk assessment (GQRA) and detailed quantitative risk assessment (DQRA)and this report includes the PRA and GQRA.

The exploratory methods adopted in this investigation have been selected on the basis of the constraints of the site including but not limited to access and space limitations, together with any budgetary or timing constraints. Where it has not been possible to reasonably use an EC7 compliant investigation technique a practical alternative has been adopted to obtain indicative soil parameters and any interpretation is based upon engineering experience, local precedent where applicable and relevant published information.

1.3.1 Basement Impact Assessment

The work carried out includes a Hydrological and Hydrogeological Assessment and Land Stability Assessment (also referred to as Slope Stability Assessment). These assessments

form part of the BIA procedure specified in the London Borough of Camden (LBC) Planning Guidance CPG² and their Guidance for Subterranean Development³ prepared by Arup (the "Arup report") in accordance with Policy A5 of the Camden Local Plan 2017. The aim of the work is to provide information on surface water, groundwater and land stability and in particular to assess whether the development will affect neighbouring properties or groundwater movements and whether any identified impacts can be appropriately mitigated by the design of the development.

1.3.2 Qualifications

The land stability element of the BIA has been carried out by Martin Cooper, a BEng in Civil Engineering, a chartered engineer (CEng), member of the Institution of Civil Engineers (MICE), and Fellow of the Geological Society (FGS) who has over 20 years' specialist experience in ground engineering. The subterranean (groundwater) flow assessment has been carried out by John Evans, MSc in Hydrogeology, Chartered Geologist (CGeol) and Fellow of the Geological Society of London (FGS). The surface water and flooding assessment has been carried out by Rupert Evans, a hydrologist with more than ten years consultancy experience in flood risk assessment, surface water drainage schemes and hydrology / hydraulic modelling. Rupert Evans is a Chartered Environmentalist, Chartered Water and Environmental Manager and a Member of CIWEM.

The assessments have been made in conjunction with Steve Branch, a BSc in Engineering Geology and Geotechnics, MSc in Geotechnical Engineering, a CGeol and FGS with some 30 years' experience in geotechnical engineering and engineering geology. All assessors meet the qualification requirements of the Council guidance.

1.4 Limitations

The conclusions and recommendations made in this report are limited to those that can be made on the basis of the investigation. The results of the work should be viewed in the context of the range of data sources consulted, the number of locations where the ground was sampled and the number of soil or ground water samples tested. No liability can be accepted for information in other data sources or conditions not revealed by the sampling or testing. Any comments made on the basis of information obtained from the client or third parties are given in good faith on the assumption that the information is accurate; no independent validation of such information has been made by GEA.

³ Ove Arup & Partners (2010) Camden geological, hydrogeological and hydrological study. Guidance for Subterranean Development. For London Borough of Camden November 2010





¹ https://www.gov.uk/government/publications/land-contamination-risk-management-lcrm 2 London Borough of Camden Planning Guidance CPG (January 2021) Basements

² London Borough of C Ref J21282 Rev 3



2.0 The Site

2.1 Site Description

The site is located in Hampstead within the London Borough of Camden, approximately 450 m to the east of Hampstead London Underground station and 400 m west of Hampstead Heath railway station. It fronts onto Pilgrims Lane to the northwest and southwest, is bounded to the southeast by the rear gardens of properties on the northwest side of Downshire Hill, and by the adjoining 14 Pilgrims Lane to the northeast. The site may additionally be located by National Grid Reference 526850, 185660 and is shown on the map extract below.



A walkover of the site was carried out by a geotechnical engineer from GEA during the site work. It is approximately triangular in shape, measuring roughly 25 m northwest to southeast by 30 m northeast to southwest at its maximum extent. The site is occupied by 12 Pilgrims Lane, a split level two/three-storey house, with a paved front garden and soft landscaped rear garden, both of which include mature and semi-mature trees.

2.2 Site History

The earliest available Ordnance Survey (OS) map studied, dated 1875, shows the site within an area of landscaped gardens. The southwestern end of Pilgrims Lane had already been established by this time, along with much of the surrounding road network and residential properties to the south and east of the site, but the area to the north is shown as undeveloped, comprising open fields.

By 1896, the area to the north of the site had been developed with the existing road network and residential properties, and the rest of Pilgrims Lane had been established, though at this time the north-eastern end of the road was named Worsley Road. The 1896 map also shows that the majority of the houses fronting onto what is now named Pilgrims Lane had already been constructed, with the exception of No 12 and some of the neighbouring properties to the northeast. This area remained undeveloped until between 1915 and 1935, when the site was developed with the majority of the existing property, and the neighbouring sites to the northeast were also similarly developed.

The next available map, dated 1952, shows that the infill extension that forms the northeast end of the existing property had been constructed. Few significant changes have subsequently been made within the site and immediately surrounding area.

2.3 Other Information

A search of public registers and databases has been made via the Envirocheck database and relevant extracts from the search are appended. Full results of the search can be provided if required.

There are no historic or existing landfill sites within 1km of the site, and no records of potentially infilled land within 250 m of the site.

Reference to records compiled by the Health Protection Agency (formerly the National Radiological Protection Board) indicates that the site falls within an area where less than 1% of homes are affected by radon emissions and therefore basic radon protective measures will not be necessary.

There are no pollution incidents to controlled waters or local authority pollution prevention and control authorisations within 250 m of the site. The site is not located within a nitrate vulnerable zone or any other area of sensitive land use.

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The nearest contemporary trade directory entry is located 83 m to the southwest of the site, and pertains to pet foods and animal feeds. The status of this entry is recorded as inactive.

The site is not located in an area liable to flooding from rivers or the sea without defences, as defined by the Environment Agency.

2.4 Preliminary UXO Risk Assessment

A Preliminary UXO Risk Assessment has been completed by 1st Line Defence (report ref PA14246-00, dated September 2021), and a copy of the report is included in the appendix. The risk assessment has been carried out in accordance with the guidelines provided by CIRIA⁴, which state that the likelihood of encountering and detonating UXO below a site should be assessed along with establishing the consequences that may arise. The first phase comprises a preliminary risk assessment, which should be undertaken at an early stage of the development planning. If such an assessment identifies a high level of risk then a detailed risk assessment should be carried out by a UXO specialist, which will identify an appropriate course of action with regard to risk mitigation.

The report indicates that, during World War II (WWII), the site was within the Metropolitan Borough of Hampstead, which sustained an overall very high density of bombing. London bomb census mapping does not record any bomb strikes within the site boundary or within the immediate surrounding area. Additionally, London County Council bomb damage mapping indicates that the structure did not sustain any damage.

It is considered likely that the site would have been subject to regular access during the war, thus a low / minimal risk of encountering unexploded ordnance has been identified for the site and no further action is recommend in this respect.

2.5 Geology

The Geological Survey map of the area (BGS sheet 256) indicates that the site is located at the boundary between the Claygate Member, and the underlying London Clay Formation. The Bagshot Formation, which overlies the Claygate Member, is recorded at surface further upslope to the northwest of the site.

The geology in this area is generally horizontally bedded such that the boundary between the geological formations roughly follows the ground surface contour lines. A borehole drilled by the BGS on Hampstead Lane to the north of the site, generally referred to as the 'Hampstead Heath borehole', was advanced to a depth of 66.74 m (61.97 m OD) at National Grid Reference 526455, 186890. The borehole record indicates that the Bagshot Formation extended to a depth of 19.00 m (109.71 m OD) and penetrated the full thickness of the Claygate Member, which was found to extend to a depth of 35.00 m (93.71 m OD), below which the London Clay Formation was encountered.

According to the BGS memoir, the Claygate Member comprises alternating beds of clayey silt, very silty clay, sandy silt and glauconitic silty fine sand. The lower part of the Claygate Member is generally more bioturbated. A bed of calcareous concretions is present near the base in many places. The London Clay Formation is homogenous, slightly calcareous silty clay to very silty clay, with some beds of clayey silt grading to silty fine-grained sand. The boundary between the Claygate Member and London Clay is transitional, and often difficult to distinguish.

GEA has previously carried out a ground investigation at 8 Pilgrims Lane in 2011, located approximately 20 m to the south of the site. The investigation generally encountered the expected ground conditions in that below a nominal thickness of made ground, the Claygate Member was encountered over the London Clay. The Claygate Member generally comprised firm light brown mottled grey fissured clay with orange-brown silt partings and selenite crystals and was found to extend to a depth of 6.9 m. The London Clay initially comprised firm becoming stiff grey fissured clay with rare pyrite nodules and was proved to the full depth investigated of 19.0 m.

A supplementary investigation was also carried out at 8 Pilgrims Lane by Listers, in 2012. The investigation encountered made ground to depths of between 0.15 m and 3.20 m, below which the Claygate Member was encountered and recorded to depths of between 4.50 m and 6.80 m. Beneath the Claygate Member, the London Clay was encountered, and proved to the maximum depth investigated, of 15.0 m. The clays of the Claygate Member generally comprised firm to stiff becoming stiff slightly fissured brown silty clay with



⁴ CIRIA C681 (2009) Unexploded ordnance (UXO) A guide for the construction industry
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occasional claystone, and the London Clay comprised stiff becoming very stiff grey locally fissured silty clay with occasional claystone bands.

2.6 Hydrology and Hydrogeology

The Claygate Member is classified by the Environment Agency as a Secondary 'A' Aquifer, defined as permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. The Claygate Member is predominantly cohesive in nature and therefore groundwater flow is likely to be relatively slow, although horizons of sandier soils do occur in this stratum, resulting in the permeability ranging from "very low" to "high". The Claygate Member is only designated as a Secondary Aquifer because it contains sand horizons, which provide more permeable layers for the storage of groundwater. Where saturated and continuous sand beds are not present, the Claygate Member behaves hydraulically more like the underlying London Clay, exhibiting poor aquifer properties which accounts for the variable permeability described above. In this case it is not capable of storing and transmitting water in usable amounts and receives very low levels of annual recharge due to very low permeability.

Under the same classification system, the London Clay is designated as Unproductive Strata, which refers to deposits that have low permeability and negligible significance for water supply or river base flow. The London Clay is not capable of supporting a groundwater table, although pockets of perched groundwater do occur within fissures and silt and sand partings. Published data for the permeability of the London Clay indicates the horizontal permeability to generally range between 1×10^{-10} m/s and 1×10^{-8} m/s, with an even lower vertical permeability.

To the northeast of the site (up topographic and hydraulic gradient), in Hampstead Heath and Parliament Hill, is a series of spring lines and ponds, which drain in a southerly direction, down the valley, towards both the Highgate and Hampstead Ponds. The positions of these springs are likely to mark the boundary between the Bagshot Formation and the Claygate Member. The springs have been the source of a number of London's "lost" rivers, notably the Fleet, Westbourne and Tyburn. A tributary of the former River Fleet flowed around 350 m to the east of the site at its closest point, flowing southwards away from the site.

The nearest surface water feature is the Hampstead No 1 Pond, located 374 m to the northeast, and the site does not lie within any of the catchment areas of the Hampstead Heath pond chains. The site is not within an area shown by the Environment Agency to be at risk from flooding from rivers or the sea, and it is not shown on the EA surface water

flood maps as being in an area with a potential risk from surface water flooding. It is also not within a Groundwater Source Protection Zone as defined by the Environment Agency.

Apart from a small area of soft landscaping, the site is entirely covered by the existing building and hardstanding. Infiltration of rainwater is therefore generally restricted to surface water drains, with the majority of surface runoff draining into combined sewers.

The proposed basement will be entirely beneath the footprint of the existing property, or within areas of existing hardstanding, such that there will be no change to the present conditions, for example through the loss of any permeable areas, and there will not be an increase in runoff rate or volume into the existing sewer system, or that could have a potentially adverse impact on the surrounding area. There should not, therefore, be any requirement for any mitigation measures.

2.7 Preliminary Risk Assessment

Part IIA of the Environmental Protection Act 1990, which was inserted into that Act by Section 57 of the Environment Act 1995, provides the main regulatory regime for the identification and remediation of contaminated land. As part of the new regime local authorities are required to carry out inspections of their area to identify sites that may be contaminated. The determination of contaminated sites is based on a "suitable for use" approach which involves managing the risks posed by contaminated land by making risk-based decisions. This risk assessment is carried out on the basis of establishing one or more "pollution linkages"; a pollution linkage requires a source of contamination, a sensitive target or receptor that is at risk from the contamination and a pathway by which the contamination can travel from the source to the target.

A risk assessment should be carried out for consideration by the Local Planning Authority (LPA) before the planning application is determined. Where unacceptable risks are identified proposals will need to be made to address these risks as part of the development process. The guidance recognises the benefits of a phased approach, and the desk study is the first phase in the process of investigating and identifying contamination to assist in the determination of a planning application.

Source

The desk study findings indicate that the site does not have a potentially contaminative history as it has apparently been developed with the existing house since some time between 1915 and 1935. No potential sources of ground gas have been identified.

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Receptor

The site will continue to have a residential end use, such that end users will represent high sensitivity receptors, as at present. Buried services are likely to come into contact with any contaminants present within the soils through which they pass, and site workers are likely to come into contact with any contaminants present during construction works. The presence of the London Clay beneath the site means that the chalk aquifer at depth represents a relatively low sensitivity receptor.

The Claygate Member is classified as a Secondary 'A' Aquifer, and as a result, groundwater is considered to be a potentially moderately sensitive target.

Pathway

Within the site, end users will be isolated from direct contact with any contaminants present within the made ground by the new building and surrounding hard surfacing, such that a pathway whereby end users could come into direct contact with potentially contaminated soils will not exist in these areas. Only in areas of soft landscaping will end users potentially come into contact with contaminants, though such pathways are already in existence.

Any soluble contaminants within the made ground could potentially migrate onto adjacent sites as a result of infiltration of surface run-off, although this pathway is also already in existence. If the Claygate Member contains sufficient sand horizons then it could potentially allow the migration of contaminated groundwater through the shallow soils to surrounding sites, but the presence of the London Clay will prevent a pathway existing to the Principal Aquifer at depth.

Buried services are likely to come into contact with any contaminants present within the soils through which they pass, and site workers are likely to come into contact with any contaminants present during construction works.

There is thus considered to be a low potential for a contaminant pathway to be present between any potential contaminant source and a target for the particular contaminant.

Preliminary Risk Appraisal

On the basis of the above it is considered that there is a LOW risk of there being a significant contaminant linkage at this site, which would result in a requirement for major remediation work.

3.0 Screening and Scoping Assessment

The Camden guidance suggests that any development proposal that includes a basement should be screened to determine whether a full BIA is required. A number of screening tools are included in the Arup report and for the purposes of this report reference has been made to Appendices E1, E2 and E3 which include a series of questions within screening flowcharts for surface flow and flooding, subterranean (groundwater) flow and land stability. The flowchart questions and responses to these questions are tabulated below.

3.1 Subterranean (Groundwater) Screening Assessment

Question	Response for 12 Pilgrims Lane
1a. Is the site located directly above an aquifer?	Yes. At least part of the site is mapped as underlain by the Claygate Member, which is designated as a Secondary 'A' Aquifer.
1b. Will the proposed basement extend beneath the water table surface?	It is possible that the proposed basement could extend beneath a water table if continuous and saturated sand layers are present in the Claygate Member. In the absence of these sand layers the Claygate Member clay and underlying London Clay strata cannot support a continuous water table.
2. Is the site within 100 m of a watercourse, well (used/ disused) or potential spring line?	No. There are no local ponds, wells, watercourses or spring lines within 100 m
3. Is the site within the catchment of the pond chains on Hampstead Heath?	No. Figure 14 of the Arup report confirms that the site is not located within these catchment areas.
4. Will the proposed basement development result in a change in the proportion of hard surfaced / paved areas?	Yes. The proposed extension to the southwest of the existing building will be within an area that currently comprises soft landscaping, thus there will be a slight increase in the proportion of hardstanding.
5. As part of the site drainage, will more surface water (e.g. rainfall and run-off) than at present be discharged to the ground (e.g. via soakaways and/or SUDS)?	No. The majority of run-off from hardstanding will drain to the sewer system, as it does currently. Low permeability ground conditions would not allow any meaningful discharge to the ground.
6. Is the lowest point of the proposed excavation (allowing for any drainage and foundation space under the basement floor) close to or lower than, the mean water level in any local pond or spring line?	No. There are no local ponds or spring lines.

The above assessment has identified the following potential issues that need to be assessed:





- Q1a At least part of the site is located above the Claygate Member, which is a Secondary 'A' Aquifer.
- Q1b There is a possibility that the proposed basement may extend beneath the water table.
- Q4 The proposed development will result in a slight increase in the proportion of hard surfaced areas

3.2 Land Stability Screening Assessment

Question	Response for 12 Pilgrims Lane
1. Does the existing site include slopes, natural or manmade, greater than $7^\circ ?$	No.
2. Will the proposed re-profiling of landscaping at the site change slopes at the property boundary to more than 7°?	No. The site will not be significantly re-profiled as part of the development.
3. Does the development neighbour land, including railway cuttings and the like, with a slope greater than 7°?	No. As indicated on the Slope Angle Map Fig 16 of the Arup report.
4. Is the site within a wider hillside setting in which the general slope is greater than 7°?	No. Reference to Figure 16 of the Arup report indicates that the site is not in an area where slopes are generally greater than 7°.
5. Is the London Clay the shallowest strata at the site?	Yes. As indicated on the geological map and Figures 3 and 5 of the Arup report, at least part of the site is underlain by the London Clay.
6. Will any trees be felled as part of the proposed development and / or are any works proposed within any tree protection zones where trees are to be retained?	No. The felling of trees is not understood to form part of the proposed development, and the proposed development is not understood to extend into the protection zones of any of the nearby trees.
7. Is there a history of seasonal shrink-swell subsidence in the local area and / or evidence of such effects at the site?	Yes. The area is prone to these effects as a result of the presence of shrinkable London Clay.
8. Is the site within 100 m of a watercourse or potential spring line?	No. There are no watercourses or potential spring lines within 100 m of the site.
9. Is the site within an area of previously worked ground?	No. Not according to BGS mapping and Figure 3 of the Arup report.
10a. Is the site within an aquifer?	Yes. At least part of the site is mapped as underlain by the Claygate Member, which is designated as a Secondary 'A' Aquifer.

Question	Response for 12 Pilgrims Lane
10b. Will the proposed basement extend beneath the water table such that dewatering may be required during construction?	It is possible that the proposed basement could extend beneath a water table if permeable sand layers are present in the Claygate Member. In the absence of these sand layers the Claygate Member clay and underlying London Clay strata cannot support a continuous water table. As a result, it is considered that significant inflows are unlikely to be encountered, although this does not eliminate a requirement for potential mitigation measures during basement construction.
11. Is the site within 50 m of Hampstead Heath ponds?	No. Figure 14 of the Arup report confirms that the site is not located within this catchment area.
12. Is the site within 5 m of a highway or pedestrian right of way?	Yes. The development is bounded to the northwest and southwest by Pilgrims Lane
13. Will the proposed basement significantly increase the differential depth of foundations relative to neighbouring properties?	Yes. The proposed basement will require deeper foundations, such that the development will increase the foundation depths relative to the neighbouring properties.
14. Is the site over (or within the exclusion zone of) any tunnels, e.g. railway lines?	No. An online search for London Underground Tunnels and railway tunnels did not indicate any in the proximity of the site. This is confirmed with reference to ARUPs Transport Infrastructure map, Figure 18. Thames Water has been contacted and their plans indicate no deep sewers or tunnels under or in close proximity of the site.

The above assessment has identified the following potential issues that need to be assessed:

- Q5. The London Clay is the shallowest strata across much of the site.
- Q7. The site is in an area likely to be affected by seasonal shrink-swell.
- Q10a. At least part of the site is located above the Claygate Member, which is a Secondary 'A' Aquifer
- Q10b There is a possibility that the proposed basement may extend beneath the water table.
- Q12. The development is within 5 m of Pilgrims Lane.
- Q13. The basement will increase the foundation depths relative to the neighbouring properties.





3.3 Surface Flow and Flooding Screening Assessment

Question	Response for 12 Pilgrims Lane
1. Is the site within the catchment of the pond chains on Hampstead Heath?	No. Figure 14 of Arup report confirms that the site is not located within this catchment area.
2. As part of the proposed site drainage, will surface water flows (e.g. volume of rainfall and peak run-off) be materially changed from the existing route?	No. Any additional surface water from the increase in hardstanding area will be attenuated and discharged into the Thames Water sewers to ensure the surface water flow regime will be unchanged. The basement will be beneath the footprint of the building, therefore the 1 m distance between the roof of the basement and ground surface as recommended by the Arup report and para 3.2 of the CPG (2021) does not apply across these areas.
3. Will the proposed basement development result in a change in the proportion of hard surfaced / paved areas?	Yes. The proposed extension to the southwest of the existing building will be within an area that currently comprises soft landscaping, thus there will be a slight increase in the proportion of hardstanding.
4. Will the proposed basement development result in changes to the profile of the inflows (instantaneous and long term) of surface water being received by adjacent properties or downstream watercourses?	No. Any additional surface water from the increase in hardstanding area will be attenuated and discharged into the Thames Water sewers to ensure the surface water flow regime will be unchanged. The basement will be beneath the footprint of the building, therefore the 1 m distance between the roof of the basement and ground surface as recommended by the Arup report and para 3.2 of the CPG (2021) does not apply across these areas.
5. Will the proposed basement result in changes to the quality of surface water being received by adjacent properties or downstream watercourses?	No. The proposed basement is very unlikely to result in any changes to the quality of surface water being received by adjacent properties or downstream watercourses as the surface water drainage regime will be unchanged and the land uses will remain the same.

Question	Response for 12 Pilgrims Lane
6. Is the site in an area identified to have surface water flood risk according to either the Local Flood Risk Management Strategy or the Strategic Flood Risk Assessment or is it at risk of flooding, for example because the proposed basement is below the static water level of nearby surface water feature?	No. The findings of this BIA together with the Camden Flood Risk Management Strategy dated 2013 and Figures 3iii, 4e, 5a and 5b of the SFRA dated 2014, in addition to the Environment Agency online flood maps show that the site has a very low flooding risk from surface water, sewers, reservoirs (and other artificial sources), groundwater and fluvial/tidal watercourses. It is possible that the basement will be constructed within pockets of perched water and the recommendations outlined in the BIA with regards to water-proofing and tanking of the basement will reduce the risk to acceptable levels. In accordance with paragraph 5.11 of the CPG, a positive pumped device will be installed in the basement in order to further protect the site from sewer flooding.

The above assessment has identified the following potential issues that need to be assessed.

Q3 The proposed development will result in a slight increase in the proportion of hard surfaced areas



4.0 Scoping and Site Investigation

The purpose of scoping is to assess in more detail the factors to be investigated in the impact assessment. Potential impacts are assessed for each of the identified potential impact factors.

4.1 **Potential Impacts**

The following potential impacts have been identified by the screening process.

Potential Impact	Consequence
The site is located directly above an aquifer	The east part of the site is underlain by the Claygate Member, which is classified as a Secondary 'A' Aquifer. This may have the potential of being able to support local water supplies as well as forming an important source of base flow for local rivers. There is the potential for the hydrogeological setting to be affected by a basement development.
The proposed basement extends beneath the water table surface.	It is possible the basement excavation could extend below the water table. Should this happen, the basement structure is capable of diverting groundwater flow such that groundwater level is affected on both the up slope and down slope side of the basement structure. This in turn has the potential to affect the local hydrogeology and any adjacent structures.
London Clay is the shallowest strata at the site. There is a moderate potential of seasonal shrink-swell subsidence in the local area	The London Clay is prone to seasonal shrink-swell (subsidence and heave). If a new basement is not dug to below the depth likely to be affected by tree roots this could lead to damaging differential movement between the subject site and adjoining properties, however new trees do not form part of the proposed development
The site is within 5 m of a highway or pedestrian right of way.	Excavation of a basement may result in structural damage to the road or footway.
The proposed development will result in an increase in differential depth relative to neighbouring properties.	The stability of all surrounding structures will need to be ensured at all times. An analysis of the predicted ground movements will be completed once the scheme is finalised, to assess the impact on neighbouring buildings.

Potential Impact	Consequence
A change in the in proportion of hard surfaced or paved areas of a property will affect the way in which rainfall and surface water are transmitted away from a	Changes to the surface water received by the adjacent properties and nearby watercourses.
property.	The sealing off of the ground surface by pavements and buildings to rainfall may result in decreased recharge to the underlying ground.

These potential impacts have been investigated through the site investigation, as detailed in Section 13.0.

4.2 Exploratory Work

In order to meet the objectives described in Section 1.2, two boreholes were advanced to a depth of 8.00 m using a combination of rotary augering and percussive sampling techniques, and two window sampler boreholes were advanced to a depth of 4.00 m. A total of ten trial pits were also manually excavated to provide access to the foundations of the existing structures on the site.

During boring, undisturbed and disturbed samples were obtained from the boreholes for subsequent laboratory examination and testing. Standard Penetration Tests (SPTs) were also carried out at regular intervals in the rotary augered boreholes to provide additional quantitative data on the strength of soils encountered.

Gas and groundwater monitoring standpipes were installed in three of the boreholes, to a maximum depth of 8.00 m, which have been subsequently monitored on one occasion to date.

A selection of disturbed and undisturbed samples recovered from the boreholes was submitted to a soil mechanics laboratory for a programme of geotechnical testing and an analytical laboratory for a programme of contamination testing.

The above work was carried out under the supervision of a geotechnical engineer from GEA. The borehole and trial pit records are appended, together with the results of the laboratory testing and a site plan indicating the borehole and trial pit locations. The levels on the borehole and trial pit records have been interpolated from spot heights shown on a drawing provided by the consulting engineers (drawing ref 4757/1, dated September 2021), which are understood to be relative to an assumed site datum 100.00 m below ground floor level.





4.3 Sampling Strategy

The general borehole and trial pit positions were agreed with the client and consulting engineers and positioned on site by GEA with due regard to the proposed development, whilst avoiding areas of known services.

A total of six samples of the shallow soil were subjected for analysis for a range of common industrial contaminants and contamination indicative parameters. For this investigation, the analytical suite for the soil included a range of metals, total petroleum hydrocarbons (TPH), polycyclic aromatic hydrocarbons (PAH), total cyanide and monohydric phenols. The same six samples were also screened for asbestos. The contamination analyses were carried out at an MCERTs accredited laboratory with the majority of the testing suite accredited to MCERTS standards. A summary of the MCERTs accreditation and test methods are included with the attached results and further details are available upon request.

5.0 Ground Conditions

The investigation generally encountered the expected ground conditions, in that beneath a limited to moderate thickness of made ground, the London Clay Formation was encountered, and proved to the full depth of the investigation, of 8.00 m (91.80m ASD).

5.1 Made Ground

The made ground typically comprised brown sandy silty clay with frequent fine to coarse gravel sized fragments of brick flint and rare charcoal, and extended to depths of between 0.60 m and 2.00 m.

Apart from the presence of fragments of extraneous material noted above, including fragments of suspected cement asbestos in Trial Pit No 6, no other visual or olfactory evidence of contamination was observed during the fieldwork. Six samples of the made ground have however been analysed for a range of contaminants as a precautionary measure, and the same six samples of the made ground have also been screened for the presence of asbestos, the results of which are detailed within Section 5.4.

5.2 London Clay

Beneath the front garden within the northwest of the site, the London Clay comprised an initial horizon of stiff becoming firm with depth, slightly desiccated brown sandy silty clay, to a depth of 1.50 m in both Borehole Nos 1 and 2, whereupon firm becoming stiff with depth, fissured slightly sandy silty clay with frequent partings of sand and silt was encountered to a depth of 6.30 m in both boreholes. Below a depth of 6.30 m, Borehole Nos 1 and 2 both encountered stiff becoming very stiff with depth fissured brownish grey silty clay to the maximum depth investigated.

Beneath the rear garden in the southeast and southwest of the site, directly beneath the made ground, both Borehole Nos 3 and 4 generally encountered firm becoming stiff with depth fissured brown silty clay with partings of sand and silt and occasional selenite, which was proved to the base of the boreholes at depths of 4.00 m.

Laboratory plasticity index tests indicate the clay to generally be of high to very high plasticity and high volume change potential.





5.3 Groundwater

Groundwater was not generally encountered during the investigation, but perched water was encountered within the made ground in one of the trial pits. A single return monitoring visit has been carried out to date, the results of which are presented in the table below.

Date	Borehole No	Depth to water (m) [Level (m OD)]
14/06/2022	1	2.80
	2	2.60
	3	1.86

5.4 Soil Contamination

The table below sets out the values measured within the six samples analysed and the generic risk based screening values for a residential end use without plant uptake; all concentrations are in mg/kg unless otherwise stated.

Determinant	BH1 0.25 m	BH1 0.75 m	BH2 0.20 m	BH3 0.50 m	TP1 0.15 m	TP2 0.50 m
рН	8.7	8.5	8.5	8.0	6.5	8.2
Arsenic	40	30	21	29	26	29
Cadmium	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2
Chromium	33	31	34	35	34	44
Lead	220	190	230	600	350	430
Mercury	0.8	< 0.3	< 0.3	1.0	0.9	1.2
Selenium	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Copper	44	57	42	88	75	65
Nickel	26	19	23	23	21	35
Zinc	130	91	130	200	290	120
Total Cyanide	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Phenols	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0

Determinant	BH1 0.25 m	BH1 0.75 m	BH2 0.20 m	BH3 0.50 m	TP1 0.15 m	TP2 0.50 m
Total PAH	14.7	2.10	37.9	13.9	14.2	5.34
Sulphide	< 1.0	2.3	< 1.0	< 1.0	< 1.0	< 1.0
Benzo(b)fluoranthene	1.3	0.24	4.2	1.4	1.5	0.63
Benzo(a)pyrene	1.4	0.21	3.9	1.4	1.5	0.53
Dibenz(a,h)anthracene	< 0.05	< 0.05	0.52	< 0.05	< 0.05	< 0.05
Naphthalene	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
ТРН	57	< 10	100	37	43	160
Total Organic Carbon %	0.8	0.8	1.5	1.6	6.5	1.8
Figure in held indicates concentration in success of side based call guideling values, as discussed in Datt D of this report						

Figure in bold indicates concentration in excess of risk-based soil guideline values, as discussed in Part 2 of this report

In addition, the same six sample of the made ground, have been screened for the presence of asbestos and the results are shown in the table below. Two fragments of solid material picked from the soil in Trial Pit No 6 are also in the process of being screened for the presence of asbestos.

BH/TP ref	Sample depth (m)	Asbestos detected	Quantification; total asbestos in sample (%)
BH1	0.25	None	-
BH1	0.75	None	-
BH2	0.20	None	-
BH3	0.50	Chrysotile – Loose fibres	< 0.001
TP1	0.15	None	-
TP2	0.50	None	-

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5.4.1 Generic Quantitative Risk Assessment

The use of a risk-based approach has been adopted to provide an initial screening of the test results to assess the need for subsequent site-specific risk assessments. Contaminants of concern are those that have values in excess of generic human health risk-based guideline values, which are either the CLEA⁵ Soil Guideline Values where available, the Suitable 4 Use Values⁶ (S4UL) produced by LQM/CIEH calculated using the CLEA UK Version 1.07⁷ software, or the DEFRA Category 4 Screening values⁸, assuming a residential end use with plant uptake. The key generic assumptions for this end use are as follows:

- **G** that groundwater will not be a critical risk receptor;
- C that the critical receptor for human health will be young female children aged less than six years old;
- **G** that the exposure duration will be six years;
- that the critical exposure pathways will be direct soil and indoor dust ingestion, consumption of homegrown produce, consumption of soil adhering to home grown produce, skin contact with soils and dust, and inhalation of dust and vapours; and
- **G** that the building type equates to a two-storey terraced house.

It is considered that these assumptions are suitable for this generic first assessment of this site. The tables of generic screening values derived by GEA and an explanation of how each value has been derived are included in the Appendix.

Where contaminant concentrations are measured at concentrations below the generic screening value it is considered that they pose an acceptable level of risk and thus further consideration of these contaminant concentrations is not required. However, where concentrations are measured in excess of these generic screening values there is considered to be a potential that they could pose an unacceptable risk and thus further action will be required which could include;

 additional testing to zone the extent of the contaminated material and thus reduce the uncertainty with regard to its potential risk;

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- site specific risk assessment to refine the assessment criteria and allow an assessment to be made as to whether the concentration present would pose an unacceptable risk at this site; or
- soil remediation or risk management to mitigate the risk posed by the contaminant to a degree that it poses an acceptable risk.

The table below indicates contaminants of concern and the locations of the samples where the elevated concentrations were measured.

Contaminant of Concern	Maximum concentration recorded (mg/kg)	Generic Risk-Based Screening Value	Location of elevated concentrations
Arsenic	40	37	BH1
Lead	600	200	BH's 1-3 & TP's 1 & 2
Benzo(b)fluoranthene	4.2	2.6	BH2
Dibenz(a,h)anthracene	0.52	0.24	BH2

In addition, all samples of the made ground have been screened for the presence of asbestos. Only one of the samples was found to contain asbestos, in the form of loose fibres of Chrysotile, at a concentration of < 0.001 %. Two fragments of solid material picked from the soil in Trial Pit No 6 are also in the process of being screened for the presence of asbestos.

The significance of these results is considered further in Part 2 of the report.

5.5 **Existing Foundations**

The findings of the trial pits are summarised in the table below. Sketches of each pit are included in the Appendix.

Trial Pit No	Structure	Foundation detail	Bearing Stratum
1	Party Wall with No 14	Brickwork over concrete strip / trench fill foundation	MADE GROUND

CL:AIRE (2013) Development of Category 4 Screening Levels for Assessment of Land Affected by Contamination Final Project Report SP1010 and DEFRA (2014) Development of Category 4 Screening Levels for Assessment of Land Affected by Contamination Policy Companion Document SP1010



⁵ Updated Technical Background to the CLEA Model (Science Report SC050021/SR3) Jan 2009 and Soil Guideline Value reports for specific contaminants; all DEFRA and Environment Agency.

⁶ The LQM/CIEH S4UIs for Human Health Risk Assessment S4UL3065 November 2014

⁷ Contaminated Land Exposure Assessment (CL|EA) Software Version 1.071 Environment Agency 2015

Trial Pit No	Structure	Foundation detail	Bearing Stratum
		Top: 0.55 m Base: 0.90 m Lateral projection: 240 mm	
2	Front Wall	Brickwork over concrete strip / trench fill foundation Top: 0.68 m Base: 0.78 m Lateral projection: 320 mm	Stiff brown mottled orange-brown fissure silty clay with parting of clayey sand and sill Rare blue-grey veining
3	Front Wall	Brickwork over concrete strip / trench fill foundation Top: 0.55 m Base: 0.75 m Lateral projection: 280 mm	Stiff brown mottled orange-brown fissured silty clay with partings of clayey sand and silt. Rare blue-grey veining
4	Front Wall	Brickwork over concrete strip / trench fill foundation Top: 0.56 m Base: 0.82 m Lateral projection: 250 mm	Stiff brown mottled orange-brown fissured silty clay with partings of clayey sand and silt. Rare blue-grey veining
5	Front Wall	Brickwork over concrete strip / trench fill foundation Top: 0.60 m Base: 0.90 m Lateral projection: 210 mm	MADE GROUND
6	S Flank Wall	Brickwork over concrete strip / trench fill foundation Top: 0.40 m Base: 0.75 m Lateral projection: 210 mm	MADE GROUND
7	Rear Wall	Brickwork over concrete strip / trench fill foundation Top: 0.50 m Base: 0.70 m Lateral projection: 240 mm	MADE GROUND
8	Rear Wall	Not determined	N/A
9	Party Wall with No 14	Brickwork over concrete strip / trench fill foundation Top: 0.25 m Base: 0.55 m Lateral projection: 250 mm	MADE GROUND
10	Rear Wall	Brickwork over concrete strip / trench fill foundation Top: 0.48 m Base: 0.85 m Lateral projection: 300 mm	Firm brown mottled orange-brown fissured silty clay with partings of clayey sand and silt Rare blue-grey veining



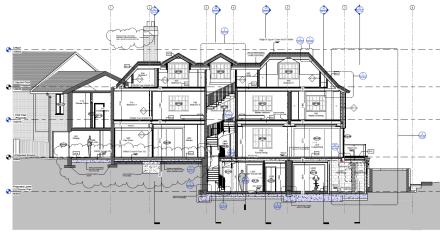


Part 2: Design Basis Report

This section of the report provides an interpretation of the findings detailed in Part 1, in the form of a ground model, and then provides advice and recommendations with respect to the proposed development.

6.0 Introduction

It is understood that it is proposed to extend the existing lower ground floor level, as part of significant alterations to the house. Formation level for the proposed lower ground floor level is understood to be approximately 3.20 m below the existing ground floor level.



Section AA - Proposed

7.0 Ground Model

The desk study findings indicate that the site does not have a potentially contaminative history as it has apparently been developed with the existing house since some time between 1915 and 1935. On the basis of the fieldwork, the ground conditions at this site can be characterised as follows:

- beneath a limited to moderate thickness of made ground, the London Clay Formation is present, and extends to the maximum depth investigated, of 8.00 m (91.80 m ASD);
- C the made ground typically comprises brown sandy silty clay with frequent fine to coarse gravel sized fragments of brick flint and rare charcoal and extends to depths of between 0.60 m and 2.00 m;
- within the northern part of the site, the London Clay initially consists of stiff becoming firm with depth, slightly desiccated brown sandy silty clay, to a depth of 1.50 m, whereupon firm becoming stiff with depth, fissured slightly sandy silty clay with frequent partings of sand and silt is present, and extends to a depth of 6.30 m. Below a depth of 6.30 m, stiff becoming very stiff with depth fissured brownish grey silty clay is present, to the maximum depth investigated, of 8.00 m (91.80 m ASD);
- within the southern part of the site, the London Clay consists of firm becoming stiff with depth fissured brown silty clay with partings of sand and silt and occasional selenite, to the maximum depth investigated in this area, of 4.00 m (93.50 m ASD);
- G groundwater was not generally encountered during the investigation, but perched water was encountered within the made ground in one of the trial pits, and has been measured at a shallowest depth of 1.86 m during the single groundwater monitoring visit completed to date;
- contamination testing has revealed the presence of elevated concentrations of arsenic, lead, PAH compounds and asbestos contamination within the made ground; and







8.0 Advice & Recommendations

Excavations for the proposed lower ground floor will require temporary support to maintain stability and to prevent any excessive ground movements.

Formation level for the proposed lower ground floor will be within the London Clay at a depth of around 3.20 m below the existing ground floor level, at roughly 96.50 m ASD.

The proposed use of reinforced concrete underpinning to form the proposed lower ground floor and support the existing building should be suitable. Perched water is likely be encountered within the made ground, but significant groundwater inflows during the excavation are not anticipated.

8.1 Basement Construction

The formation level of the proposed lower ground floor level is anticipated to extend to a depth of approximately 3.20 m below the existing ground floor level, at roughly 96.50 m ASD. Formation level is therefore expected to be within the stiff clays of the London Clay.

Groundwater inflows were not noted during drilling and prolonged inflows are not generally encountered within the London Clay, as they are generally attributable to localised partings of silt and sand and are therefore of limited volume. Shallow inflows of perched water may be encountered from within the made ground, particularly within the vicinity of existing foundations, and were encountered within one of the trial pits during the investigation.

Due to the cohesive nature of the London Clay, it is likely that the rate of any inflow will be relatively slow, such that any potential inflows are unlikely to be significant and should be adequately dealt with through sump pumping. The selected contractor should have a contingency plan in place to deal with more significant or prolonger inflows as a precautionary measure if a watertight retention scheme is not adopted.

The design of basement support in the temporary and permanent conditions needs to take account of the necessity to maintain the stability of the excavation and the surrounding structures and to protect against potential groundwater inflows. There are a number of methods by which the sides of the basement excavation could be supported in the temporary and permanent conditions. The choice of wall may be governed to a large extent by whether it is to be incorporated into the permanent works and have a load bearing function.

Provided that groundwater inflows can be controlled it should be possible to form the retaining walls by means of concrete underpinning using a traditional 'hit and miss' approach. Careful workmanship will be required to ensure that movement of the surrounding structures is restricted but this method will have the benefit of minimising the plant required and maximising usable space in the new basement. Consideration should be given to the stability of excavations to form the underpins and the contractor should have measures in place to deal with groundwater inflows such as sump pumping or localised grouting.

The ground movements associated with the basement excavation will depend on the method of excavation and support and the overall stiffness of the basement structure in the temporary condition. Thus, a suitable amount of propping will be required to provide the necessary rigidity. In this respect the timing of the provision of support to the wall will have an important effect on movements. Consideration will also need to be given to a retention system that maintains the stability at all times of the existing building, neighbouring properties and structures. The existing foundations will need to be underpinned prior to excavation of the basement or will need to be supported by new retaining walls. A Ground Movement Analysis has been carried out and is presented in Part 3 below.

8.1.1 Basement Retaining Walls

The following parameters are suggested for the design of the permanent basement retaining walls.

Stratum	Bulk Density (kg/m³)	Effective Cohesion (c' – kN/m²)	Effective Friction Angle $(\varphi' - degrees)$
Made ground	1700	Zero	20
London Clay	1950	Zero	23

On the basis of the groundwater observations made to date and low permeability of the London Clay, significant groundwater inflows are unlikely to be encountered within the proposed excavations. However, consideration will need to be given to the risk of surface water building up behind the retaining walls and unless adequate drainage can be incorporated to prevent such build-up, it is recommended that the basement is designed with a water level assumed to be 1.0 m below ground level.

The advice in BS8102:2009⁹ should be followed in this respect and with regard to the provision of suitable waterproofing.

8.1.2 Basement Heave

Due to variations in the existing ground levels, the excavation depths required to form the proposed lower ground floor level are anticipated to vary between around 1.20 m to 3.20 m. This will result in differential net unloading of between around 15 kN/m² and 60 kN/m², which will result in differential heave of the underlying London Clay. This will comprise immediate elastic movement, which will account for approximately 40 % of the total movement and be expected to be complete during the construction period, and long-term movements, which will theoretically take many years to complete. These movements will, to some extent, be mitigated by the loads applied by the proposed development, and are considered in more detail in Part 3 of this report.

8.2 Spread Foundations

Spread foundations bearing beneath the proposed lower ground floor level in the stiff silty clay of the London Clay may be designed to apply a net allowable bearing pressure of 125 kN/m². This value is based on the Standard Penetration Test (SPT) results, and the results of the unconsolidated undrained triaxial tests, both of which indicate that an average cohesion value of around 60 kN/ m² to 65 kN/m² can be adopted at the proposed formation level, based on a relationship of Cu = $4.5 \times N60$, as shown on the appended SPT & Cohesion / depth graph. This average value has then been used in the standard bearing capacity equation to give a net allowable bearing pressure of around 125 kN/m², which is based on a factor of safety of three against bearing capacity failure

The intrusive investigation found evidence of desiccation on site, but the depth of the lower ground floor excavation is such that foundations are anticipated to be placed below the likely depth of any desiccation. However, it would be prudent to have the formation level inspected for signs of tree root growth.

8.3 Shallow Excavations

On the basis of the borehole findings, it is considered that it will be generally feasible to form relatively shallow excavations terminating within the made ground or London Clay without the requirement for lateral support, although localised instabilities are likely to occur if more granular material or groundwater is encountered.

Significant inflows of groundwater into shallow excavations are not generally anticipated, although seepages are likely be encountered from perched water tables within the made ground, particularly within the vicinity of existing foundations, although such inflows should be suitably controlled by sump pumping. Rising head tests could be carried out on the existing standpipes to provide an indication of the rates of inflow that could be expected. Ideally, trial excavations extending to as close to proposed formation level as possible should be carried out to provide an indication of the likely stability and presence of inflows.

If deeper excavations are considered or if excavations are to remain open for prolonged periods it is recommended that provision be made for battered side slopes or lateral support. Where personnel are required to enter excavations, a risk assessment should be carried out and temporary lateral support or battering of the excavation sides considered in order to comply with normal safety requirements.

8.4 Lower Ground Floor Slab

The lower ground floor slab will need to be suspended over a void or a layer of compressible material to accommodate the anticipated heave and any potential uplift forces from groundwater pressures, unless the slab can be suitably reinforced to cope with these movements.

8.5 Hydrogeological Assessment

The desk study and ground investigation have confirmed that the site is underlain by Unproductive Strata of the London Clay Formation and as such a continuous groundwater table is not present below the site. The proposed development will not therefore have an impact on the local hydrogeological setting.

9 BS8102 (2009) Code of practice for protection of below ground structures against water from the ground Ref J21282 Rev 3 11 December 2024





8.6 Effect of Sulphates

Chemical analyses of samples from the made ground and natural soils have revealed relatively moderate to high concentrations of soluble sulphate and near-neutral pH in accordance with Class DS-2 to DS-3 conditions of Table C2 of BRE Special Digest 1:SD Third Edition (2005). The measured pH values of the samples show that an ACEC class of AC-2s would be appropriate for the site. This assumes a static water condition at the site.

87 Contamination Risk Assessment

The desk study findings indicate that the site does not have a potentially contaminative history as it has apparently been developed with the existing house since some time between 1915 and 1935. No significant on-site or off-site potential sources of contamination have been identified, including existing or historical landfill sites, thus the site is not considered to have had a particularly contaminative history. However, the results of the chemical analyses carried out on samples of the made ground have indicated elevated concentrations of arsenic within one of the samples tested, lead within five of the samples tested, and PAH species within one of the samples tested, although total PAH is not elevated. Additionally, loose fibres of chrysotile asbestos have been identified in one of the samples of made ground analysed, at a concentration of < 0.001 %, and fragments of solid material were picked from the soil in Trial Pit No 6, which are in the process of being screened for the presence of asbestos.

The source of the arsenic and lead contamination is not known but the made ground was noted as containing variable amounts of extraneous material, and it is therefore likely that a fragment of such material was present within the samples tested, accounting for the elevated concentrations. Information on Urban Soil Chemistry provided by the BGS also indicates that background concentrations for arsenic in the vicinity of the site are between 35 mg/kg and 40 mg/kg, and that background concentrations for lead in the vicinity of the site are between 600 mg/kg and 900 mg/kg, such that a significant proportion of the measured concentrations could be the result of residual airborne sources, particularly from historical use of leaded petrol.

Lead compounds are relatively immobile, unlikely to be in a soluble form and are considered to be non-volatile or of a low volatility. The lead contamination does not therefore present a significant vapour risk or a significant risk of leaching and migration within any perched groundwater within the made ground. As the site is underlain by

HSE (1992) HS(G)66 Protection of workers and the general public during the development of contaminated land 11 Ref J21282

London Clay, which cannot support a continuous groundwater table, a risk to groundwater is also not identified.

Statistical analysis of the specific PAH species identified in the elevated sample recovered from Borehole No 2 has found the concentration to be of a petrogenic origin, i.e. it originates form a geological source such as coal or crude, although the specific source is unknown. Fragments of extraneous material were noted within the made ground, so it is likely that this has resulted in the elevated concentration of the specific PAH species Benzo(b)fluoranthene, and Dibenz(ah)anthracene. As such, the contamination is not considered likely to be soluble and should not, therefore pose a risk of vapours or to adjacent sites or groundwater.

As asbestos is insoluble, it is not considered to pose any meaningful risk to groundwater, the development, or to neighbouring sites through migration in the ground. It is however potentially hazardous to human health as airborne fibres and could thus pose a risk through inhalation, According to the HSE¹⁰, there is now significant monitoring evidence available within the ground investigation and remediation industry to suggest that significant visible quantities of bound asbestos-containing materials (ACMs) will need to be present to give rise to exposures above 0.01 f/ml, equivalent to one tenth of the control limit, unless they are being subject to highly energetic processes (e.g. crushing, power screening and grading of demolition waste and made ground or soil).

No new areas of soft landscaping will be created as part of the proposals, but areas of soft landscaping will remain in the front and rear garden areas such that the risk to end users will be no greater than currently present. The measured contaminants in the made ground will nevertheless pose a risk to site workers during the ground works. These risks are further assessed below.

8.7.1 Protection of Site Workers

Site workers should be made aware of the potential contamination and a programme of working should be identified to protect workers handling any soil. The method of site working should be in accordance with guidelines set out by HSE¹¹ and CIRIA¹² and the requirements of the Local Authority Environmental Health Officer.

A watching brief should be maintained during the site works and if any suspicious soil is encountered, it should be inspected by a suitably qualified engineer and further testing carried out if required.

HMSO

CIRIA (1996) A guide for safe working on contaminated sites. Report 132, Construction Industry. Research and Information Association



¹⁰ HSE (2016) Asbestos: The analysts' guide for sampling, analysis and clearance procedures HSG248. Second Edition 2016. In preparation.

¹²



Due to the asbestos contamination recorded in parts of the made ground, the construction works fall under the Control of Asbestos Regulations 2012, but are unlikely to comprise notifiable or licensed work, provided that the control limit of 0.1 fibres per cubic centimetre is not exceeded during the construction works¹³.

8.7.2 Services

It is unlikely that services are at risk from the contamination noted in the made ground. However, details of any proposed protection measures for buried plastic services will in any case need to be approved by the EHO and the relevant service authority prior to the adoption of any scheme.

88 Waste Disposal

Under the European Waste Directive, waste is classified as being either Hazardous or Non-Hazardous and landfills receiving waste are classified as accepting hazardous or nonhazardous wastes or the non-hazardous sub-category of inert waste in accordance with the Waste Directive. Waste classification is a staged process, and this investigation represents the preliminary sampling exercise of that process. Once the extent and location of the waste that is to be removed has been defined, further sampling and testing may be necessary. The results from this ground investigation should be used to help define the sampling plan for such further testing, which could include WAC leaching tests where the totals analysis indicates the soil to be a hazardous waste or inert waste from a contaminated site. It should however be noted that the Environment Agency guidance WM3¹⁴ states that landfill WAC analysis, specifically leaching test results, must not be used for waste classification purposes.

Any spoil arising from excavations or landscaping works, which is not to be re-used in accordance with the CL:AIRE¹⁵ guidance, will need to be disposed of to a licensed tip. Waste going to landfill is subject to landfill tax at either the standard rate of £98.60 per tonne (about £185 per m³) or at the lower rate of £3.15 per tonne (roughly £5.85 per m³). However, the classifications for tax purposes and disposal purposes differ and currently all made ground and topsoil is taxable at the 'standard' rate and only naturally occurring soil and stones, which are accurately described as such in terms of the 2011 Order, would qualify for the 'lower rate' of landfill tax.

Based on the technical guidance provided by the EA it is considered likely that the soils encountered during this ground investigation, as represented by the chemical analyses carried out, would be generally classified as follows.

Soil Type	Waste Classification (Waste Code)	WAC Testing Required Prior to Landfill Disposal?	Current applicable rate of Landfill Tax
Made ground	Non-hazardous (17 05 04)	Should not be required but confirm with receiving landfill	£98.60/tonne (Standard rate)
London Clay	Assumed to be Inert (17 05 04)	Should not be required but confirm with receiving landfill	£3.15 / tonne (Reduced rate for uncontaminated naturally occurring rocks and soils)

Under the requirements of the European Waste Directive all waste needs to be pre-treated prior to disposal. The pre-treatment process must be physical, thermal, chemical or biological, including sorting. It must change the characteristics of the waste in order to reduce its volume, hazardous nature, facilitate handling or enhance recovery. The waste producer can carry out the treatment, but they will need to provide documentation to prove that this has been carried out. Alternatively, the treatment can be carried out by an approved contractor. The Environment Agency has issued a position paper¹⁶ which states that in certain circumstances, segregation at source may be considered as pre-treatment and thus excavated material may not have to be treated prior to landfilling if the soils can be segregated onsite prior to excavation by sufficiently characterising the soils in-situ prior to excavation.

The above opinion with regard to the classification of the excavated soils is provided for guidance only and should be confirmed by the receiving landfill once the soils to be discarded have been identified.

The local waste regulation department of the Environment Agency (EA) should be contacted to obtain details of tips that are licensed to accept the soil represented by the test results. The tips will be able to provide costs for disposing of this material but may require further testing.

CL:AIRE March 2011. The Definition of Waste: Development Industry Code of Practice Version 2

Environment Agency 23 Oct 2007 Regulatory Position Statement Treating non-hazardous waste for landfill - Enforcing the new

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requirement

¹³ HSE (2016) Asbestos: The analysts' guide for sampling, analysis and clearance procedures HSG248. Second Edition 2016. In preparation

Environment Agency 2015. Guidance on the classification and assessment of waste. Technical Guidance WM3 First Edition 14 Ref J21282



Part 3: Ground Movement Analysis

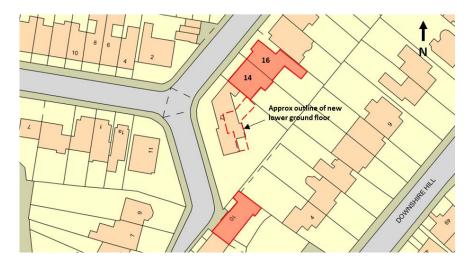
This section of the report comprises an analysis of the ground movements arising from the proposed basement and foundation scheme discussed in Part 2 and the information obtained from the investigation, presented in Part 1 of the report.

9.0 Introduction

The sides of an excavation will move to some extent regardless of how they are supported. The movement will typically be both horizontal and vertical and will be influenced by the engineering properties of the ground, groundwater level and flow, the efficiency of the various support systems employed and the efficiency or stiffness of any support structures used. An analysis has been carried out of the likely movements arising from the proposed basement excavation and the results of this analysis have been used to predict the effect of these movements on surrounding structures.

9.1 Nearby Sensitive Structures

Nearby sensitive structures comprise the adjoining 14 and 16 Pilgrims Lane to the northeast and 10 Pilgrims Lane to the south, as shown on the plan below.



The heights of the buildings have been estimated from observation, and from drawings provided by the consulting engineers. The underside of the foundations of 14 Pilgrims Lane have been determined based on the trial pitting described in the previous section, and the foundations of 16 Pilgrims Lane, which adjoins 14, have been assumed to be the same. The foundations of 10 Pilgrims Lane have been assumed. The heights and underside of foundations are summarised on the table below.

Sensitive structure	Height of building above foundation level (m)	Underside of foundation, depth (m) [m ASD)
14 Pilgrims Lane	3.7 to 6.5	0.70 [99.30]
16 Pilgrims Lane	6.5	0.70 [99.30]
10 Pilgrims Lane	7.1	0.50 [97.10]

During the site walkover, no evidence was seen to indicate that 14 or 16 Pilgrims Lane have existing basement levels and no planning applications were found relating to proposed basements beneath these properties during a search of planning records held by Camden Council. To the south, 10 Pilgrims Lane is located on a sloping site, such that it has a lower ground floor level similar to the existing lower ground floor level at 12 Pilgrims Lane. The locations of the neighbouring buildings have been input into the model based on dimensions calculated from scaled drawings.

9.2 Construction Sequence

It is currently proposed to form the new lower ground floor level by means of reinforced concrete underpinning. The pressure applied by the lower ground floor underpins has been conservatively modelled at the allowable bearing pressure provided in Part 2 of this report, of 125 kN/m². When the unloading due to the proposed excavation is taken into account, which is anticipated to range from between 15 kN/m² and 60 kN/m² due to the changes in existing ground level across the site, the net pressure beneath the underpins is estimated to range from around 65 kN/m².

The following sequence of operations has been derived to enable analysis of the ground movements around the basement, both during and after construction.

1. construction of underpins to the existing perimeter walls in hit and miss sequence;





- 2. install props and excavation of lower ground floor to formation level;
- 3. cast basement slabs; and
- 4. casting of ground floor slab and removal of temporary props once concrete has sufficiently cured.

It is understood that underpinning of the existing perimeter walls will take place in a 'hit and miss' sequence, in stages to be agreed with the temporary works engineer and under party wall agreement. Underpinning should generally be undertaken in short sections not exceeding 1.00 m to 1.20 m in length, with no adjacent pin to be excavated until a minimum of 48 hours after the adjacent pin has been cast and dry-packed placed, with the sides of the excavation adequately shored and propped.

The detail of the support provided to adjacent walls is beyond the scope of this report at this stage and the structural engineer will be best placed to agree a methodology with the basement contractors once appointed.

The individual panel widths of the liner wall will need to be adequately laterally propped and sufficiently dowelled together, and the concrete will need to be cast and adequately cured prior to excavation of the basement and removal of the formwork and supports. It is assumed that the corners of the excavation will be locally stiffened by cross-bracing or similar and that the new retaining walls will not be cantilevered at any stage during the construction process. It is assumed that adequate temporary propping of the new retaining walls, particularly at the top level, will occur at all times prior to the construction of permanent concrete floor slabs.

10.0 Ground Movements

An assessment of ground movements within and surrounding the excavation has been undertaken using the P-Disp and X-Disp computer programs licensed from the OASYS suite of geotechnical modelling software from Arup. These programs are commonly used within the ground engineering industry and are considered to be appropriate tools for this analysis.

The X-Disp and P-Disp programs have been used to predict ground movements likely to arise from the excavation and construction of the proposed basement. This includes the heave / settlement of the ground (vertical movement) and the lateral movement of soil behind the proposed retaining walls (horizontal movement). Both the P-Disp and X-Disp programs are commonly used within the ground engineering industry and are considered to be appropriate tools for the purpose of this analysis.

The basement has been modelled based on scaled dimensions from plan drawings provided by the consulting engineers and the corners of the basement are defined by x and y coordinates. As such, for the purpose of these analyses, the x direction is orientated approximately east-west, and the y direction is orientated approximately north-south, roughly parallel with the front elevation of the building. Vertical movement is in the zdirection.

The lower ground floor structure has been modelled as a simplified polygon with maximum dimensions of around 18.80 m by 12.00 m, and a founding depth of 96.50 m ASD. The proposed lower ground floor will be formed through underpinning of the existing foundations, and a new lower ground floor slab.

It is assumed that suitable propping will be provided during the construction of the basement and in the permanent condition, such that the walls can be considered to be stiff for the purpose of the ground movement modelling.

The full outputs of all the analyses can be provided on request but samples of the output movement contour plots are included within the appendix.





10.1 Ground Movements – Surrounding the Basement

10.1.1 Model Used

For the X-Disp analysis, the soil movement relationships used for the embedded retaining walls are the default values within CIRIA report C760¹⁷, which were derived from a number of historic case studies.

Installation of underpins:

Predictions of the vertical and horizontal ground movements behind the walls, as a result of underpinning, can be based on case study information within CIRIA C760¹⁸ for a planar diaphragm wall installed into stiff clay, which is considered to be a conservative approach.

Underpinned walls are unlikely to move horizontally to any significant degree as they are subject to a continued vertical loading from the structure above. The use of datasets derived from case studies of embedded retaining walls will therefore be expected to overestimate horizontal movements, but will provide an indication of the pattern of possible horizontal and vertical movements. The magnitude of the any movement will also be controlled to a large extent by the quality of workmanship

Excavation Phase:

Suitable propping will be provided during the construction of the lower ground floor and in the permanent condition, such that the walls can be considered to be stiff for the purpose of the ground movement modelling. Thus, the horizontal ground movement curve for 'excavations in front of high stiffness wall in stiff clay' has been adopted for the subsequent excavation required to form the proposed lower ground floor.

For the XDisp analysis, a maximum depth of 3.17 m has been adopted for the excavation based on a ground level of 99.67 m ASD and a formation level of 96.50 m ASD. Within the footprint of the existing lower ground floor, an excavation depth of 1.20 m is anticipated, based on a ground level of 97.70 m ASD, and a formation level of 96.50 m ASD. For the X-Disp analysis, the maximum excavation depth of 3.18 m has been conservatively adopted throughout.

10.1.2 **Results**

The movements predicted by X-Disp surrounding the lower ground floor are summarised in the table below; the results are presented below and in subsequent tables to the degree of accuracy required to allow predicted variations in ground movements around the structure to be illustrated but may not reflect the anticipated accuracy of the predictions

	Wall Movement (mm)		
Phase of Works	Vertical Settlement	Horizontal Movement	
Installation of underpinned wall	<2	<2	
Combined Installation and Excavation Movements	2 to 6	4 to 12	

The analysis has indicated that the vertical settlements that will result from wall installation are anticipated to be no more than 2 mm, with movements increasing to between 2 mm and 6 mm of vertical settlement, from the combined wall installation and excavation phases, with maximum movements concentrated at the re-entrant corners.

The analysis also indicates that maximum horizontal movements that will result from wall installation are also anticipated to be up to around 2 mm, with movements increasing to around 4 mm to 12 mm as a result of the combined wall installation and excavation phases.

The movements set out in the table and discussed above are the maximum movements and the analysis has indicated that they occur immediately or just outside the line of the retaining walls, and also account for the likely overprediction of movements within reentrant corners included within the model.

10.2 Ground Movements – Resulting from Excavation

10.2.1 Model Used

Unloading of the London Clay will take place as a result of the excavation of the proposed basement and the reduction in vertical stress will cause heave to take place. Undrained soil parameters have been used to estimate the potential short-term movements, which include the "immediate" or elastic movements as a result of the basement excavation. Drained parameters have been used to provide an estimate of the total long-term movement.

The elastic analysis requires values of soil stiffness at various levels to calculate displacements. Values of stiffness for the soils at this site are readily available from



¹⁷ Gaba, A, Hardy, S, Powrie, W, Doughty, L and Selemetas, D (2017) Embedded retaining walls – guidance for economic design CIRIA Report C760

¹⁸ Gaba, A, Hardy, S, Powrie, W, Doughty, L and Selemetas, D (2017) Embedded retaining walls – guidance for economic design CIRIA Report C760.



published data and we have used a well-established method to provide our estimates. This relates values of Eu and E', the undrained and drained stiffness respectively, to values of undrained cohesion, as described by Padfield and Sharrock¹⁹ and Butler²⁰ and more recently by O'Brien and Sharp²¹. Whilst values of Eu can be taken as 750 x Cu for the London Clay and a ratio of E' to Cu of 0.75 considered appropriate and in line with more recent published data, more conservative values of 500 x Cu for Eu and a ratio of E' to Cu of 0.60 have been adopted at this stage.

The pressure applied by the lower ground floor underpins has been conservatively modelled at the allowable bearing pressure provided in Part 2 of this report, of 125 kN/m². When the unloading due to the proposed excavation is taken into account, which is anticipated to range from between 20 kN/m² and 60 kN/m² due to the changes in existing ground level across the site, the net pressure beneath the underpins is estimated to range from around 65 kN/m².

The soil parameters used in this analysis and tabulated below have been primarily derived from the onsite investigation and extrapolation of data for the London Clay at depth. A rigid boundary for the analysis has been set at around 46.50 m below formation level (50 m ASD) within the London Clay, as below this depth the clay soils of the London Clay are considered to be essentially incompressible. An initial strength of 30 kN/m² and a subsequent increase in cohesion of 7.5 kN/m² for each metre of depth has been adopted to provide a conservative estimate of the likely strength profile within the London Clay beneath the site.

Stratum	Depth Range (m) (m OD)	Bulk Density (Kg/m³)	Eu (KN/m²)	E'(KN/m²)	
Made Ground	GL to 1.00 (100.00 to 99.00)	1700	10,000	10,000	
London Clay	1.00 to 46.50 (99.00 to -50.00)	1950	15,000 to 189,500	9,000 to 113,700	

10.2.2 Results

The predicted movements are summarised in the table below; the results are presented below and in subsequent tables to the degree of accuracy required to allow predicted variations in ground movements around the structure(s) to be illustrated, but may not reflect the anticipated accuracy of the predictions. In the table below, heave movements are shown as negative, and settlement movements are shown as positive.

Location	Short term movement (mm)	Total movement (mm)				
Maximum movement - towards central areas of proposed lower ground floor	-4 to 6	-6 to 8				
Along basement walls	0 to 12	2 to 14				
Note: -ve values denote heave, and +ve values denote settlement						

The P-Disp analysis indicates that, by the time construction is complete, between 4 mm of heave and 6 mm of settlement is likely to have taken place beneath the central parts of the lower ground floor. The analysis using drained parameters has indicated that total movements within these areas can be expected to increase by around 2 mm to a maximum of between 6 mm heave and 8 mm settlement. In general, heave movements are anticipated within the northern part of the lower ground floor, where the excavation depth is greater, and settlement movements are expected within the footprint of the existing lower ground floor, where the proposed excavation depths are much less.

Beneath the perimeter walls, between 0 mm and 12 mm of settlement is anticipated, with the smallest movements within the northern part of the lower ground floor. The analysis using drained parameters has indicated that total movements beneath the perimeter walls can be expected to increase by around 2 mm to a maximum of between 2 mm and 14 mm settlement.

The analysis has indicated that immediately outside of the excavation, settlement movements of less than 8 mm can be expected, with movements of less than 2 mm predicated immediately outside of the northern part of the proposed excavation.

O'Brien AS and Sharp P (2001) Settlement and heave of overconsolidated clays - a simplified non-linear method. Part Two, Ground Engineering, Nov 2001, 48-53



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Ref J21282 Rev 3 11 December 2024

¹⁹ Padfield, CJ and Sharrock, MJ (1983) Settlement of structures on clay soils. CIRIA Special Publication 27

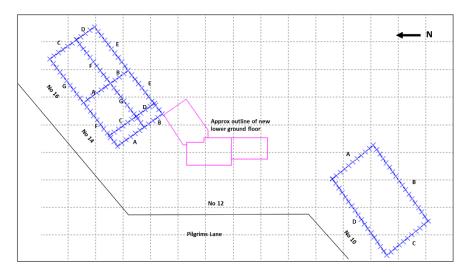
²⁰ Butler FG (1974) *Heavily overconsolidated clays: a state of the art review.* Proc Conf Settlement of Structures, Cambridge, 531-578, Pentech Press, Lond



11.0 Damage Assessment

In addition to the above assessment of the likely movements that will result from the proposed development, any neighbouring buildings within the zone of influence of the excavations are considered to be sensitive structures, requiring Building Damage Assessments, on the basis of the classification given in Table 6.4 of CIRIA report C760²².

The sensitive structures outlined previously have been modelled as displacement lines in the analysis along which the damage assessment has been undertaken. The labelling adopted is shown on the diagram below.



Plan view of neighbouring structures and the labelling of each wall

11.1 Damage to Neighbouring Structures

The ground movements resulting from the underpinning and lower ground floor excavation phases have been calculated using X-Disp modelling software to carry out an assessment of the likely damage to adjacent properties and the results are discussed below. The building damage reports for sensitive structures highlighted above are included in the appendix and indicate that predominantly the damage to the adjoining and nearby structures due to the construction of the proposed lower ground floor would be either 'Negligible (0)' or 'Very Slight (1)'. A summary of the structures indicated as affected is included below.

Structure	Elevation Reference	Category of Damage*
14 Pilgrims Lane	Wall A	Very Slight (1)
14 Filgriffis Lane	Walls B to G	Negligible (0)
16 Pilgrims Lane	Walls B and E	Negligible (0)
to Piignins Lane	Walls A, C, D, F and G	Below limit of detection
10 Pilgrims Lane	Walls A to D	Below limit of detection

*From Table 6.4 of C760: Classification of visible damage to walls.

Building damage categories Negligible to Very Slight are widely considered to cause only aesthetic damage to buildings such that structural instability is highly unlikely to occur. All sensitive structures have been assessed to have a damage category of 'Negligible' to 'Very Slight', such that all sensitive structures are considered to fall within acceptable limits.

The results discussed above are based on individual building lines, or walls, that in some instances, have been further divided up within the analysis into a series of segments that are assumed to be able to move independently of one another, with the most critical segment determining the result for the entire wall. In reality, this is unlikely to be the case as the walls will behave as single stiff elements that are also joined continuously with the rest of the structure.

The results therefore provide a conservative estimate of the behaviour of each of the sensitive structures and overestimate the degree of damage, although they provide a useful

22 Gaba, A, Hardy, S, Powrie, W, Doughty, L and Selemetas, D (2017) Embedded retaining walls – guidance for economic design CIRIA Report C760





indication of the most critical structures within the adjoining properties that may require further assessment, as detailed below.

11.2 Monitoring of Ground Movements

The predictions of ground movement based on the ground movement analysis should be checked by monitoring of the adjacent properties and structures. The structures to be monitored during the construction stages should include the existing property and the neighbouring structure assessed above. Condition surveys of the above existing structures should be carried out before and after the proposed works.

The precise monitoring strategy will be developed at a later stage, and it will be subject to discussions and agreements with the owners of the adjacent properties and structures. Contingency measures will be implemented if movements of the adjacent structures exceed predefined trigger levels. Both contingency measures and trigger levels will need to be developed within a future monitoring specification for the works.

12.0 GMA Conclusions

The analysis has concluded that the predicted damage to the neighbouring properties from the construction of the proposed basements would be 'Negligible' to 'Very Slight'.

On this basis, the damage that has been predicted to occur as a result of the construction the proposed basement falls within the limits acceptable to the London Borough of Camden assuming that the careful control is taken during construction of the proposed excavations, and monitoring will be required to ensure that no excessive movements occur that would lead to damage in excess of these limits.

The separate phases of work, including piling and subsequent excavation of the proposed basement, will in practice be separated by a number of weeks. This will provide an opportunity for the ground movements during and immediately after installation of the retaining walls to be measured and the data acquired can be fed back into the design and compared with the predicted values. Such a comparison will allow the ground model to be reviewed and the predicted wall movements to be reassessed prior to the main excavation taking place so that propping arrangements can be adjusted if required.



Part 4: Basement Impact Assessment

This section of the report evaluates the direct and indirect implications of the proposed project, based on the findings of the previous screening and scoping, site investigation and ground movement assessment.

13.0 Introduction

The screening identified a number of potential impacts. The desk study and ground investigation information has been used below to review the potential impacts, to assess the likelihood of them occurring and the scope for reasonable engineering mitigation.

13.1 Potential Impacts

The table below summarises the previously identified potential impacts and the additional information that is now available from the ground investigation in consideration of each impact.

Potential Impact	Consequence
The site is located directly above an aquifer	The east part of the site is mapped by the BGS as underlain by the Claygate Member, which is classified as a Secondary 'A' Aquifer, but the ground investigation encountered the London Clay, which is classified as an unproductive stratum and cannot support a continuous water table. A continuous groundwater table was not recorded during the investigation.
The proposed basement extends beneath the water table surface.	The ground investigation encountered the London Clay, which is classified as an unproductive stratum and cannot support a continuous water table. Permeable sand layers of the Claygate Member were not encountered, and a continuous groundwater table was not recorded during the investigation. Isolated pockets of perched groundwater may be encountered within the made ground, and within fissures and sand partings in the London Clay. but these are unlikely to form a continuous water table beneath the site. It is therefore considered that significant inflows are unlikely to be encountered, although this does not eliminate a requirement for potential mitigation measures during basement construction.
London Clay is the shallowest strata at the site.	Multiple potential impacts depending on the specific setting of the basement development. For example, in

Potential Impact	Consequence
There is a moderate potential of seasonal shrink-swell subsidence in the local area.	terraced properties, the implications of a deepened basement/foundation system on neighbouring properties should be considered.
The development is located within 5 m of a highway or pedestrian right of way.	Should the design of retaining walls and foundations not take into account the presence of nearby infrastructure, it may lead to the structural damage of footways, highways and associated buried services.
The proposed development will result in an increase in differential depth relative to neighbouring properties	The stability of neighbouring structures will need to be ensured throughout the development. A ground movement analysis and building damage assessment has been carried out and reported in Part 3 of this report.
A change in the in proportion of hard surfaced or paved areas of a property will affect the way in which rainfall and surface water are transmitted away from a property.	Changes to the surface water received by the adjacent properties and nearby watercourses. The sealing off of the ground surface by pavements and buildings to rainfall may result in decreased recharge to the underlying ground.

The results of the site investigation have therefore been used below to review the remaining potential impacts, to assess the likelihood of them occurring and the scope for reasonable engineering mitigation.

The site is located above an aquifer, and the proposed basement may extend beneath the water table surface

The London Clay is present directly beneath the made ground, which is classified as an unproductive stratum. A continuous groundwater table is therefore unlikely to be present beneath the site, although perched groundwater may be present within the made ground and within fissures and sand partings in the London Clay.

Although an eastern portion of the site is mapped by the BGS as underlain by the Claygate Member, which is classified as a Secondary 'A' Aquifer, soils of the Claygate Member were not encountered during the ground investigation.

Given the above and the fact that there will be space around and beneath the proposed basement construction, it is not considered that it will have any significant influence on the local hydrogeology and will not therefore have any potential impact on any adjoining sites.

However, groundwater protection measures will be required as part of the proposed basement construction due to the potential for isolated inflows from the made ground and sand partings in the London Clay. It is anticipated that a provision for sump pumping will be



adequate with respect to this development although it would be prudent, as with any site, for the chosen contractor to have a contingency plan in place to deal with any short or longterm inflows that are more significant than expected.

The site is underlain by London Clay which would be subject to seasonal shrink-swell

Shrinkable clay is present within a depth that can be affected by tree roots. Numerous trees are present on the site, and desiccation was noted in the boreholes located in close proximity to existing trees. However, the proposed lower ground floor will extend to a depth such that new foundations should bypass any desiccated soils.

Subject to inspection of excavations in the normal way to ensure that there is not significant unexpectedly deep root growth, it is not considered that the occurrence of shrink-swell issues in the local area has any bearing on the proposed development.

Location of public highway

The lower ground floor excavation will extend to within 5.00 m from Pilgrims Lane and therefore the excavation may affect the highway. The proposed development will include retaining walls that will be designed to maintain the stability of the surrounding ground, thus protecting the adjacent road and associated infrastructure beyond.

There is nothing unusual or exceptional in the proposed development or the findings of the previous investigation that give rise to any concerns with regard to stability over and above any development of this nature, although this will be confirmed through further site investigation.

Differential founding depths / Neighbouring structures

The proposed lower ground floor is expected to extend to a maximum depth of approximately 3.5 m, such that ground movements as a result of the proposed excavations would be expected to reduce to zero at a distance of approximately 14 m, corresponding to four times the retained height, based on the CIRIA ground movement curve for an 'excavation in front of a stiff wall in stiff clay' (Fig 6.15a of CIRIA C760). As the adjoining 14 & 16 Pilgrims Lane to the northeast, as well as 10 Pilgrim Lane to the southwest of the site are within this zone, the stability of the structures has been considered as part of a ground movement analysis and building damage assessment reported in Part 3. The analysis found that movements are anticipated to be negligible (Category 0) to very slight (Category 1) and as such, the proposed basement construction and excavation are unlikely to impact the adjacent structures.

Increase in proportion of hardstanding across the site

The proposed extension to the southwest of the existing building will be within an area that currently comprises soft landscaping, thus there will be a slight increase in the proportion of hardstanding.

The sealing of the ground surface to rainfall, by increasing the building area, would result in decreased recharge to the underlying ground, although the low permeability of the underlying London Clay would result in a low recharge in any case and consequently there would be little or no effect on groundwater.

It is anticipated that all surface water will be discharged to the sewer network through existing connections and the volumes of surface water run-off from the site are not anticipated in increase significantly. The effects of attenuation due to the construction of the basement are likely to be minimal given the low permeability of the shallow soils and that surface water will be discharged to the sewer network through existing connections. On this basis of this impact assessment, the proposed development will not have a significant detrimental effect on surface water flow

13.2 BIA Conclusions

A Basement Impact Assessment has been carried out following the information and guidance published by the London Borough of Camden. It is concluded that the proposed development is unlikely to result in any specific land or slope stability issues.

13.3 Non-Technical Summary of Evidence

This section provides a short summary of the evidence acquired and used to form the conclusions made within the BIA.

13.3.1 Screening

The following table provides the evidence used to answer the subterranean (groundwater flow) screening questions.

The following table provides the evidence used to answer the slope stability screening questions.





Question	Evidence
1. Does the existing site include slopes, natural or manmade, greater than 7°?	Topographical maps and Figures 16 and 17 of the Arup report and confirmed during a site walkover
2. Will the proposed re-profiling of landscaping at the site change slopes at the property boundary to more than 7°?	The details of the proposed development provided do not include the re-profiling of the site to create new slopes
3. Does the development neighbour land, including railway cuttings and the like, with a slope greater than 7° ?	Topographical maps and Figures 16 and 17 of the Arup report
4. Is the site within a wider hillside setting in which the general slope is greater than 7°?	
5. Is the London Clay the shallowest strata at the site?	Geological maps and Figures 3, 5 and 8 of the Arup report
6. Will any trees be felled as part of the proposed development and / or are any works proposed within any tree protection zones where trees are to be retained?	The details of the proposed development including architectural drawings.
7. Is there a history of seasonal shrink-swell subsidence in the local area and / or evidence of such effects at the site?	Knowledge on the ground conditions of the area and reference to NHBC guidelines were used to make an assessment of this, in addition to a visual inspection of the buildings carried out during the site walkover.
8. Is the site within 100 m of a watercourse or potential spring line?	Topographical maps acquired as part of the desk study and Figures 11 and 12 of the Arup report
9. Is the site within an area of previously worked ground?	Geological maps and Figures 3, 5 and 8 of the Arup report
10. Is the site within an aquifer?	Aquifer designation maps acquired from the Environment Agency as part of the desk study and Figures 3, 5 and 8 of the Arup report.
11. Is the site within 50 m of Hampstead Heath ponds?	Topographical maps acquired as part of the desk study and Figures 12 and 14 of the Arup report
12. Is the site within 5 m of a highway or pedestrian right of way?	Site plans and the site walkover.
13. Will the proposed basement significantly increase the differential depth of foundations relative to neighbouring properties?	Camden planning portal and the site walkover confirmed the position of the proposed lower ground floor relative to the neighbouring properties.
14. Is the site over (or within the exclusion zone of) any tunnels, e.g. railway lines?	Maps and plans of infrastructure tunnels were reviewed.

The following table provides the evidence used to answer the surface water flow and flooding screening questions.

Question	Evidence						
1. Is the site within the catchment of the pond chains on Hampstead Heath?	Topographical maps acquired as part of the desk study and Figures 12 and 14 of the Arup report						
2. As part of the proposed site drainage, will surface water flows (e.g. volume of rainfall and peak run-off) be materially changed from the existing route?							
3. Will the proposed basement development result in a change in the proportion of hard surfaced / paved areas?							
4. Will the proposed basement development result in changes to the profile of the inflows (instantaneous and long term) of surface water being received by adjacent properties or downstream watercourses?	A site walkover confirmed the current site conditions. Details provided on the proposed development.						
5. Will the proposed basement result in changes to the quantity of surface water being received by adjacent properties or downstream watercourses?							
6. Is the site in an area known to be at risk from surface water flooding such as South Hampstead, West Hampstead, Gospel Oak and Kings Cross, or is it at risk of flooding because the proposed basement is below the static water level of a nearby surface water feature?	Flood risk maps acquired from the Environment Agency as part of the desk study, Figure 15 of the Arup report, the Camden Flood Risk Management Strategy dated 2013 and the North London Strategic Flood Risk Assessment dated 2008.						
2 Scoping and Site Investigation The questions in the screening stage that there were answered 'yes', were taken forward to a scoping stage and the potential impacts discussed in Section 4.0 of this report, with reference to the possible impacts outlined in the Arup report.							
A ground investigation has been carried out, which has allowed an assessment of the							

13.3.2

l Bi potential impacts of the basement development on the various receptors identified from the screening and scoping stages. Principally the investigation aimed to establish the ground conditions, including the groundwater level, the engineering properties of the underlying soils to enable suitable design of the basement development and the configuration of existing party wall foundations. The findings of the investigation are discussed in Section 5.0 of this report and summarized in both Section 7.0 and the Executive Summary.







13.3.3 Impact Assessment

Section 13.0 of this report summarises whether, on the basis of the findings of the investigation, the potential impacts still need to be given consideration and identifies ongoing risks that will require suitable engineering mitigation. Section 9.0 of this report also provides recommendations for the design of the proposed development.

A ground movement analysis and building damage assessment has been carried out and its findings are presented in Part 3.

14.0 Outstanding Risks & Issues

This section of the report aims to highlight areas where further work is required as a result of limitations on the scope of this investigation, or where issues have been identified by this investigation that warrant further consideration. The scope of risks and issues discussed in this section is by no means exhaustive, but covers the main areas where additional work may be required.

The ground is a heterogeneous natural material and variations will inevitably arise between the locations at which it is investigated. This report provides an assessment of the ground conditions based on the discrete points at which the ground was sampled, but the ground conditions should be subject to review as the work proceeds to ensure that any variations from the Ground Model are properly assessed by a suitably qualified person.

As discussed throughout the report, perched water is likely to be encountered during the lower ground floor excavation, although the findings of the investigation indicate that potential inflows are unlikely to be significant and should be adequately dealt with through sump pumping. However, groundwater monitoring should be continued for as long as possible prior to construction, and trial excavations should be considered to assess the extent of inflows to be expected within the proposed basement excavations.

If during ground works any visual or olfactory evidence of contamination is identified it is recommended that further investigation be carried out and that the risk assessment is reviewed.

These areas of doubt should be drawn to the attention of prospective contractors and further investigation will be required or sufficient contingency should be provided to cover the outstanding risk.





Appendix

a. Field Work

Site Plan Borehole Records Trial Pit Records

b. Lab Testing

Geotechnical Test Results SPT & Cohesion/Depth Graph Chemical Test Results Generic Risk Based Screening Values

c. Desk Study

Envirocheck Extracts Historical Maps Risk Assessment Tables UXO Preliminary Risk Assessment

d. Ground Movement Analysis

PDisp Analysis – Tabular input - Short Term Movements PDisp Analysis – Tabular input - Total Movements PDisp Analysis – Contour output - Short Term Movements PDisp Analysis – Contour output - Total Movements PDisp Analysis – Tabular output - Short Term Movements PDisp Analysis – Tabular output - Total Movements

XDisp Analysis – Tabular input - Installation XDisp Analysis – Tabular input - Installation and Excavation XDisp Analysis – Vertical movements - Installation XDisp Analysis – Horizontal movements - Installation XDisp Analysis – Vertical movements - Installation and Excavation XDisp Analysis – Horizontal movements - Installation and Excavation XDisp Analysis – Tabular output - Installation XDisp Analysis – Tabular output - Installation



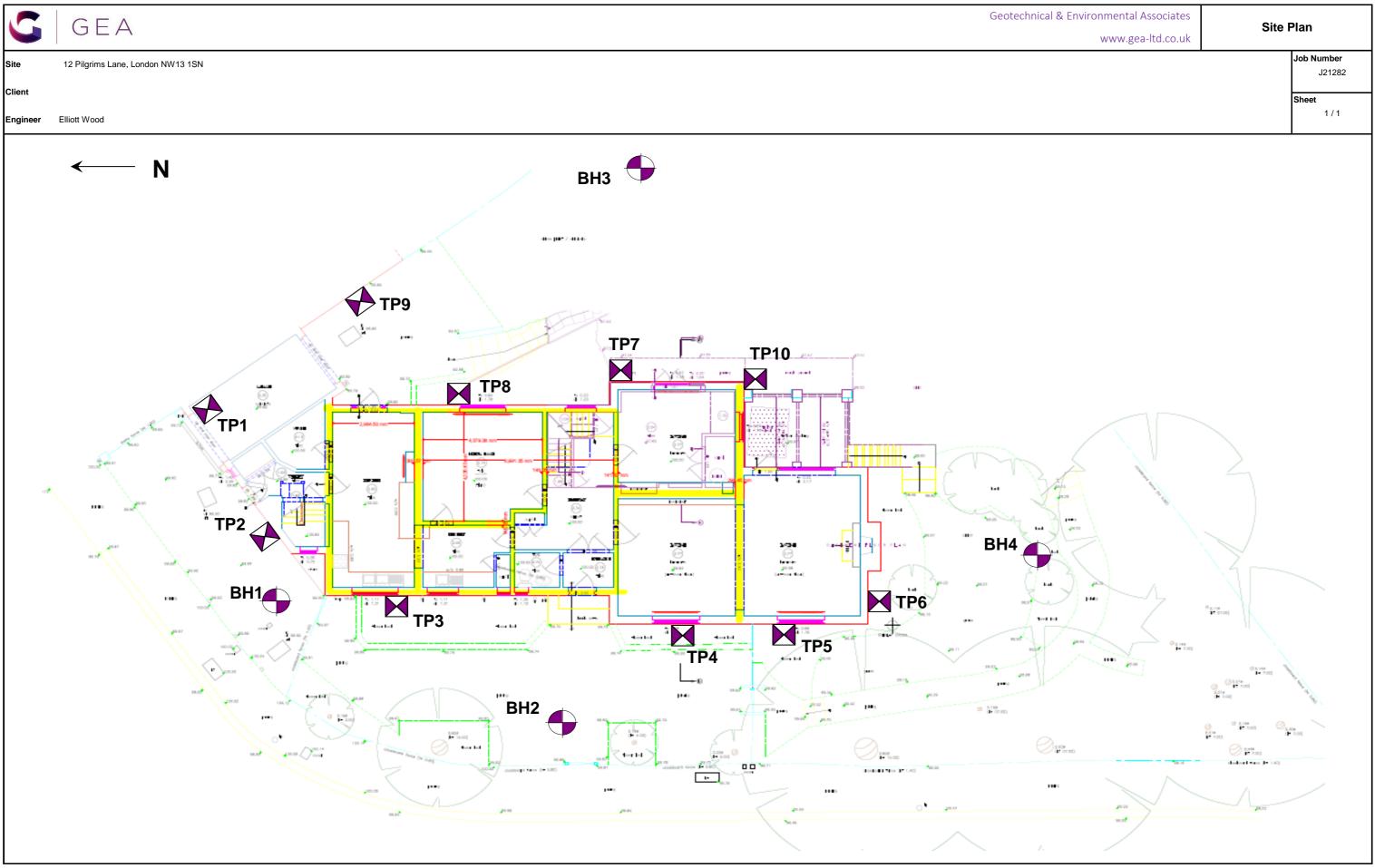


Field Work

Site Plan Borehole Records Trial Pit Records



S	GEA	Geotechnical &
te	12 Pilgrims Lane, London NW13 1SN	





	Project						BOREHOLE No			
12 Pilgrims Lane, London, NW3 1SN									ВШ	
	Job No		Date 21-09-21	Ground Level (m OD) Co-Ordinates ()			ВП	BH1		
	J212	282	21-09-21							
	Client				•	En	gineer		Sheet	
							Elliott We	bod	1 of	1
	SAN	/IPLES & ⁻	TESTS					STRATA		ent
		Туре	Test	Water	Reducer	4	Depth			ume
	Depth	No	Result	≥	Level	Legend	Depth d (Thick- ness)	DESCRIPTION		Instrument / Backfill
Date: 27 June 2022	0.25 0.50 0.75 1.00 1.20-1.65 1.85 2.00-2.45 2.75 3.00-3.45 4.00-4.45 4.75 5.00-5.45 6.00	D D D D U100 D U100 D U100 D D D D D D D D	2,3/3,3,3,4 N60 = 18 2,3/2,3,3,3 N60 = 15 2,2/2,4,3,4 N60 = 18				(4.80)	PAVING SLABS over MADE GROUND (stif desiccated sandy silty clay, with frequen gravel sized fragments of brick and flint. roots.) Stiff becoming firm slightly desiccated be clay with frequent partings of orange bro- sand and grey silt Firm becoming stiff with depth fissured s silty clay with frequent partings of orang sand and grey silt. Occasional blue-grey to Occasional roots. 3.00 Occasional fine gravel sized selenite 3.00 m 4.00 Sand partings are only occasional and below 4.00 m	t fine to coarse Occasional	
=	F	U100 D D	2,3/4,4,4,5 N60 = 23				(2.15) 8.45	Stiff becoming very stiff with depth fissu silty clay with occasional partings of brow and silt. Frequent fine crystals (mica) and white shell fragments.	wn-grey sand	
Report ID: CABLE PERCUSSION Project: J21282 - 12 PILGRIMS LANE.GPJ Library: GEA LIBRARY.GLB	Boring Depth	Date Ti	and Water Ok	g	V	IS Vater Jepth	No ground	GENERAL REMARKS services pit to 1.20 m dwater encountered installed to depth of 8.0 m		
Report	All dimensic Scale	ons in metre 1:62.5	es Method/ Plant Used CF	A,	/ Percu	ssive s	ampler		Logged By AG	



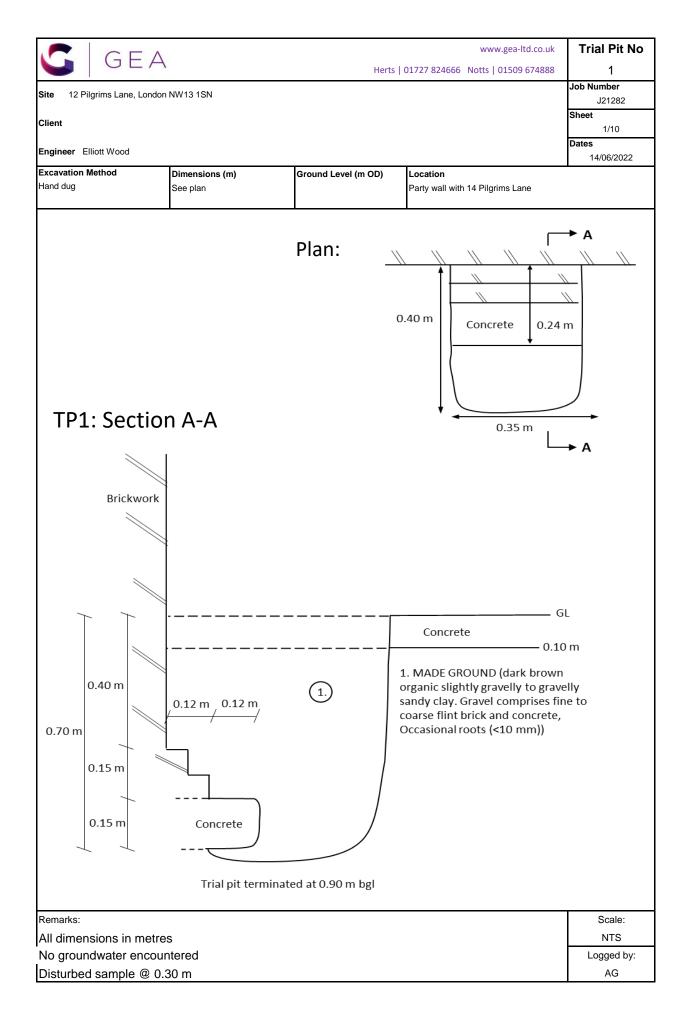
	Project									BOREHO	E N	0
	12 P	ilgrims La	ne, London, NV	V3	1SN					ВШ	•	
Ī	Job No		Date 21-09-21		Gro	ound Le	vel (m OD)	Co-Ordinates ()		BH2	2	
	J212	282	21-09-21									
	Client					En	ngineer			Sheet		
							Elliott Wo	bod		1 of	1	
Ì	SAN	VPLES & -	TESTS					STRATA			nt	=
ł			Test	ater	Poduco	4	Depth d (Thick-				- un	ackfi
	Depth	Type No	Result	Š	Level	Legend	d (Thick- ness)	DESCRIPTION			Instr	/ Backfill
-	0.25 0.50 0.75 1.00	D D D D					(0.60) 0.60 (0.90)	PAVING SLABS over MADE GROUND desiccated sandy silty clay, with freq gravel sized fragments of brick and fl roots.) Stiff becoming firm slightly desiccate clay with frequent partings of orange	ient int. (d br	: fine to coarse Occasional		
	1.20-1.65 1.85 2.00-2.45 2.75	D D U100 D	1,2/2,2,2,1 N60 = 9				* 1.50 	sand and grey silt Firm becoming stiff with depth fissur silty clay with frequent partings of or sand and grey silt. Occasional blue-gr Occasional roots. 2.50 Occasional coarse sand to fine g crystals below 2.50 m	ange ey v	e brown clayey reining.		
	3.00-3.45 3.75 4.00-4.45	D D U100	1,2/3,4,3,3 N60 = 18					3.50 Sand partings are only occasion below 3.50 m	al an	id no roots		
27 June 2022	4.75 5.00-5.45 6.00	D D D	1,2/2,3,4,4 N60 = 18									
Date:	6.50-6.95 7.50	U100 D					(2.15)	Stiff becoming very stiff with depth f silty clay with occasional partings of and silt. Frequent fine crystals (mica) white shell fragments.	orov	vn-grey sand		
Library: GE	8.00-8.45	D	2,3/3,4,4,6 N60 = 23				 					
12 PILGRIMS LANE.GPJ												
: J21282 -	Boring Depth		and Water Ok ime Casin Depth [1S Vater Depth		GENERAL REMARKS				
Report ID: CABLE PERCUSSION Project: J21282 - 12 PILGRIMS LANE.GPJ Library: GEA LIBRARY.GLB							No ground	services pit to 1.20 m dwater encountered installed to depth of 8.0 m				
Report ID	All dimensic Scale	ons in metre 1:62.5	es Method/ Plant Used CF	A,	Percu	ssive s	ampler		ľ	Logged By AG		

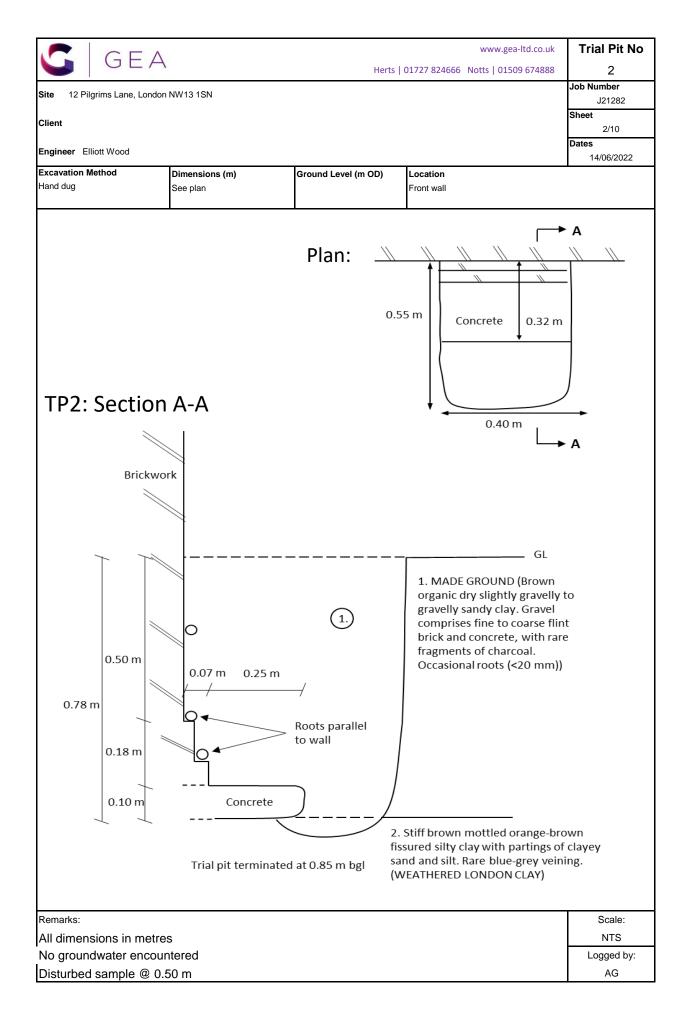


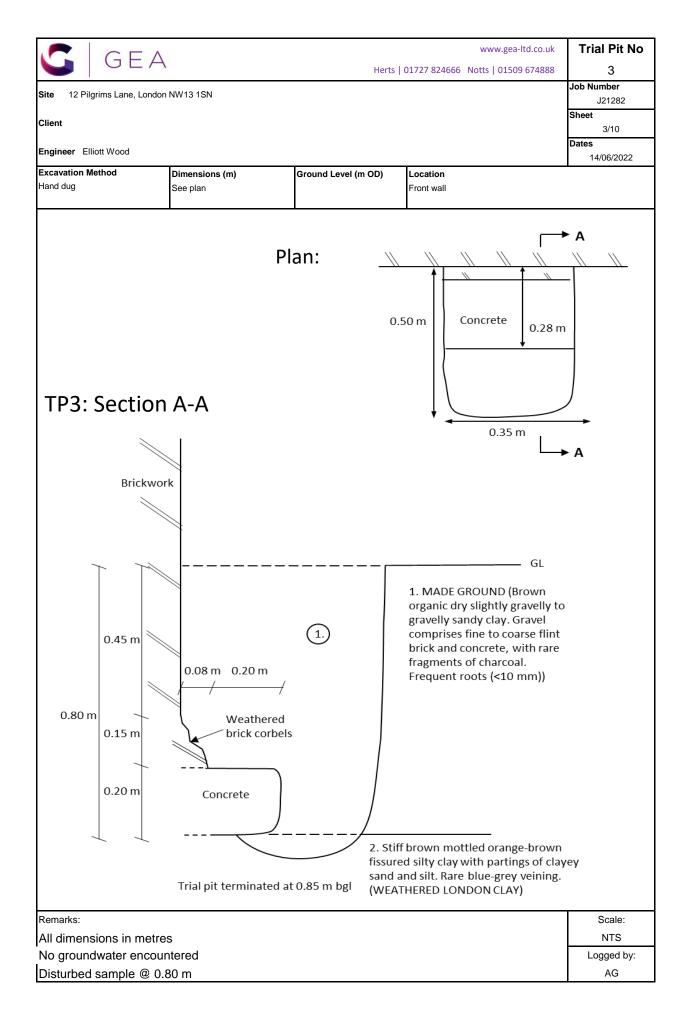
	Project										BOREHO	LE No
	12	Pilgrims		London, N	<i>N</i> 3						BH3	2
	Job No		Da	24-09-2	1	Gr	ound Le	evel (m OD)	Co-Ordinates ()			2
		1282		24-09-2	1							
	Client						Er	ngineer			Sheet	
								Elliott W	ood		1 of	
	SA	MPLES	& TES	TS	5				STRATA			fill
	Depth	Type No		Test Result	Wate	Reduce Level	dLegen	Depth d (Thick- ness)	DESCRIPTION			Instrument / Backfill
	0.50	D						(1.00)	TURF over MADE GROUND (brow frequent fine to coarse gravel size flint and rare charcoal. Occasiona	ed fragr I tree r	ments of brick, oots)	
	1.30	D						<u>, 14, 1 51, 14, 1</u>	Firm becoming stiff with depth br orange-brown fissured silty clay w sand and silt. Rare blue-grey veini	vith par	ottled rtings of clayey	
	2.00	D						 				
	_2.80 3.20	D						╤┤╌┽╌┝╌╌┾╴	3.00 Occasional coarse sand sized below 3.00 m	seleni	te crystals	
	4.00	D					××	<u>-</u> , 4.00				
e 2022	-											
iLB Date: 27 June 2022	-											
y: GEA LIBRARY.G	-											
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12 PILGRIMS LA												
t: J21282 -	Borir Depth	ng Progre Date	ess and Time	d Water O Casi Depth	ose ng Dia.	rvatio	ns Vater Depth		GENERAL REMARKS			
Report ID: CABLE PERCUSSION Project: J21282 - 12 PILGRIMS LANE.GPJ Library: GEA LIBRARY.GLB					- 101			No groun	services pit to 1.20 m dwater encountered installed to depth of 4.0 m			
Report ID:		sions in me le 1:62.5	etres N F	Method/ Plant Used H	anc	d-held	windo	w sample	r		Logged By AG	

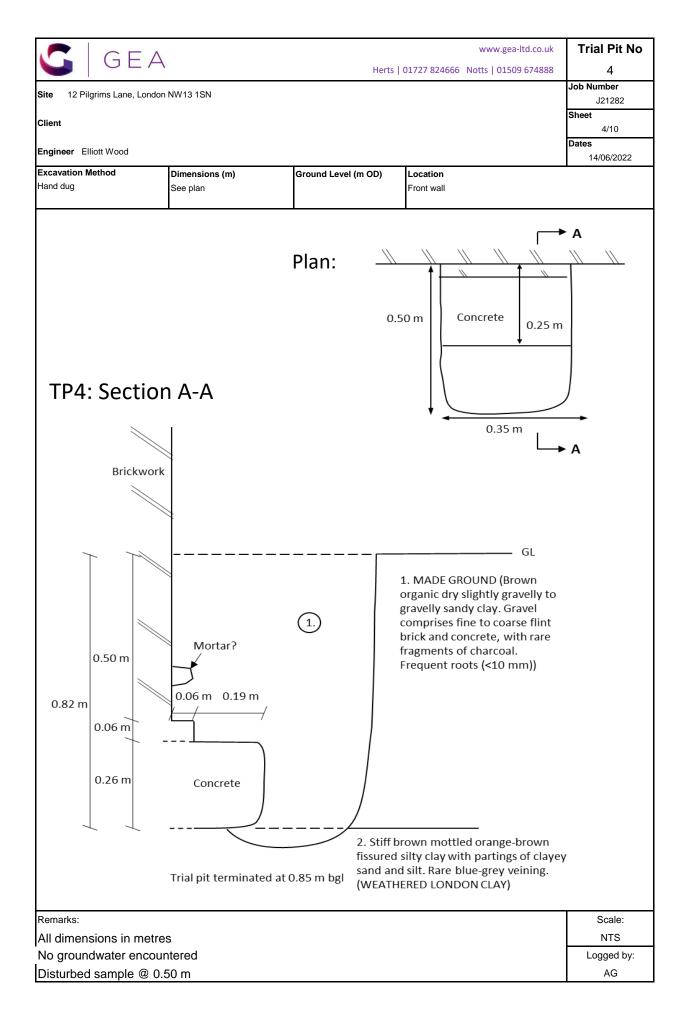


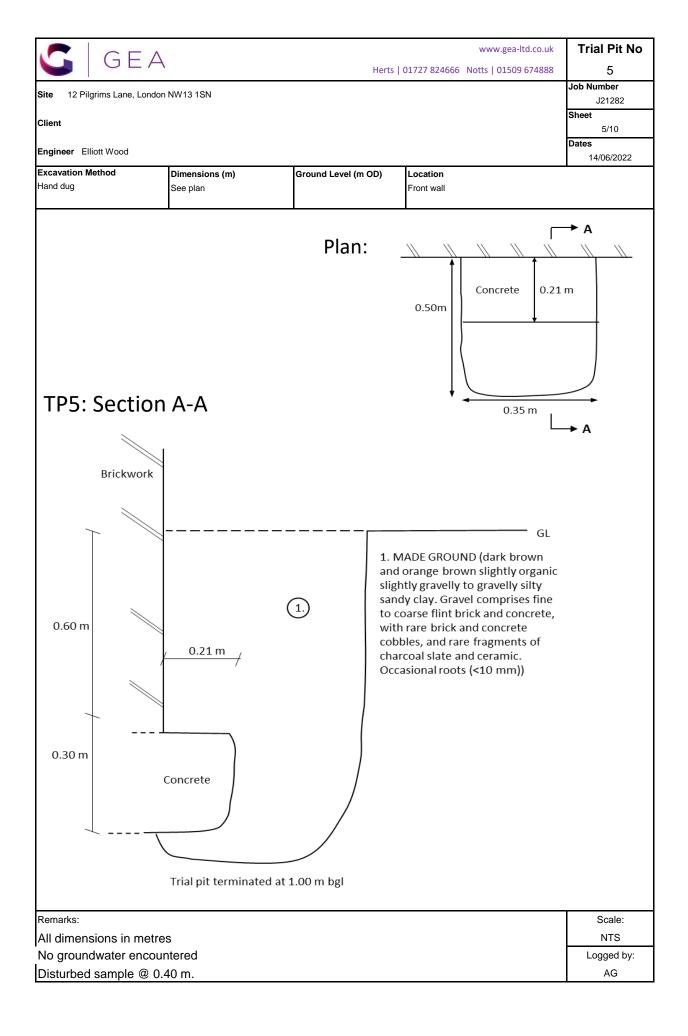
	Project									BOREHOL	E No
	12	Pilgrims I		ondon, N\		1SN				рц/	
	Job No		Date	^e 24-09-22	1	Gr	ound Lev	vel (m OD)	Co-Ordinates ()	BH4	÷
		.282		24-09-22	1						
	Client							gineer		Sheet	
								Elliott Wo	ood	1 of	
	SA	SAMPLES & TESTS							STRATA		fill
	Depth	Type No		Test Result	Wate	Reduce Level	d Legend	Depth (Thick- ness)	DESCRIPTION		Instrument / Backfill
	1.00	D						(2.00)	TURF over MADE GROUND (stiff desiccat organic brown sandy silty clay with frequ coarse gravel sized fragments of brick, fli charcoal. Occasional roots)	lent fine to	
	1.60	D						< < ≪ ≪ 2.00			
	2.10	D						2.00	Stiff locally firm brown mottled orange-b silty clay with partings of clayey sand and blue-grey veining.	orown fissured d silt. Rare	
	_2.80	D						(2.00)	Side Brey venning.		
	3.20	D						× 			
	- 4.00	D					* <u>×</u>	4.00			
Report ID: CABLE PERCUSSION Project: J21282 - 12 PILGRIMS LANE.GPJ Library: GEA LIBRARY.GLB Date: 27 June 2022		g Progre	ss and	Water OI	DSe	rvatio	ns		GENERAL		
CABLE PERCUSSION Project: J2128	Depth		Time	Casir Depth I	ng Dia.	mm	Vater Depth	No ground	SERVICES pit to 1.20 m dwater encountered backfilled with arisings on completion		
Report ID	All dimens Scal	ions in met e 1:62.5	tres M Pla	ethod/ ant Used H	anc	d-held	window	v sampleı	r	Logged By AG	

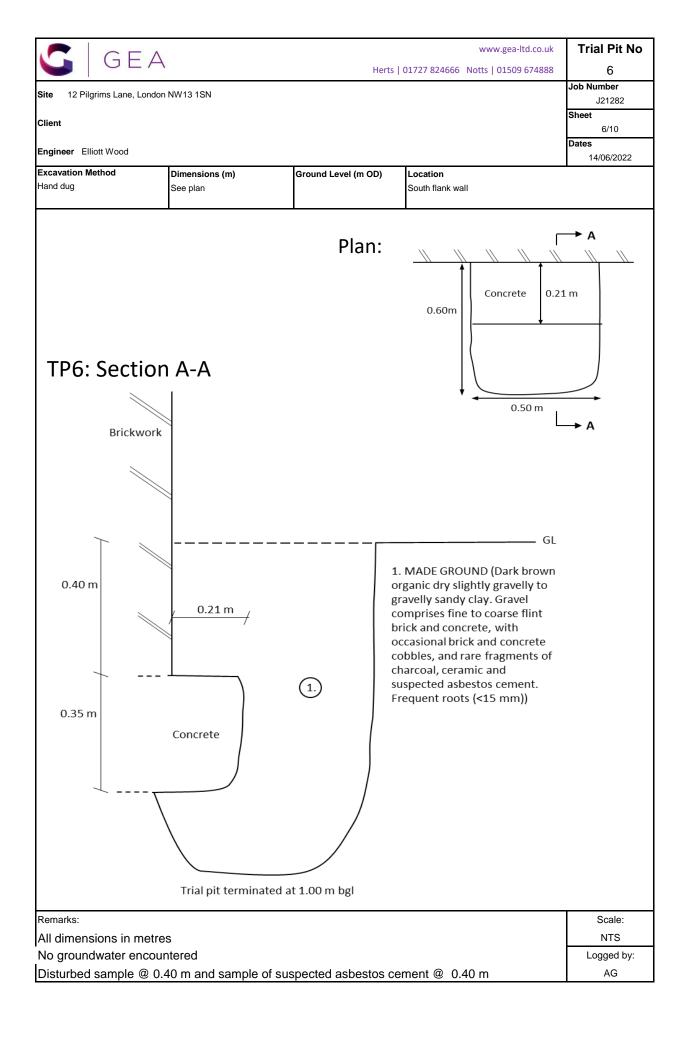


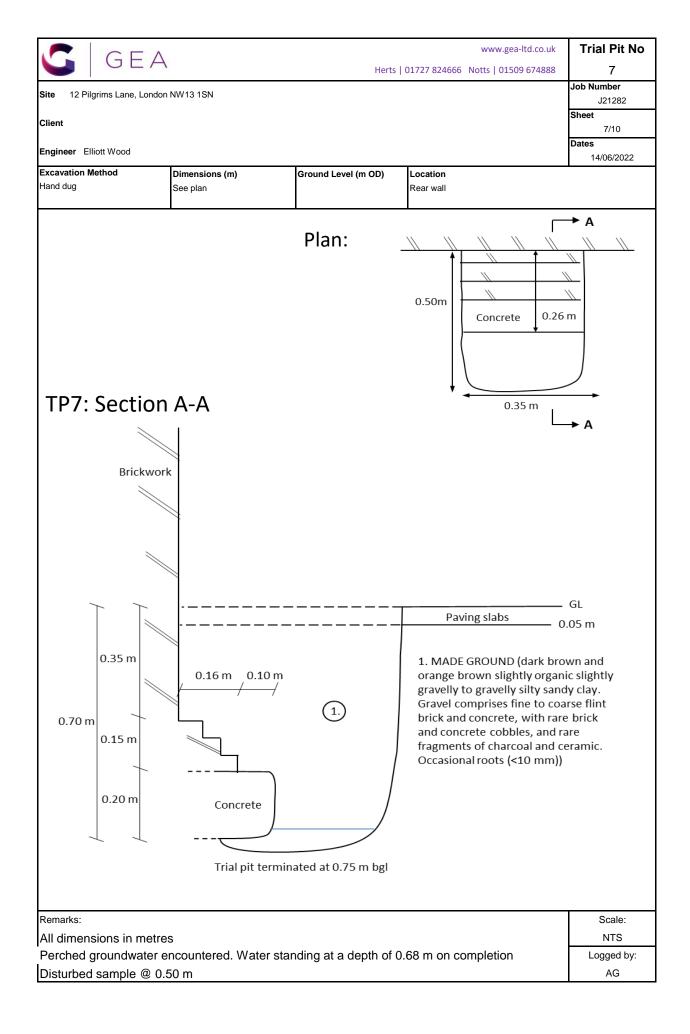


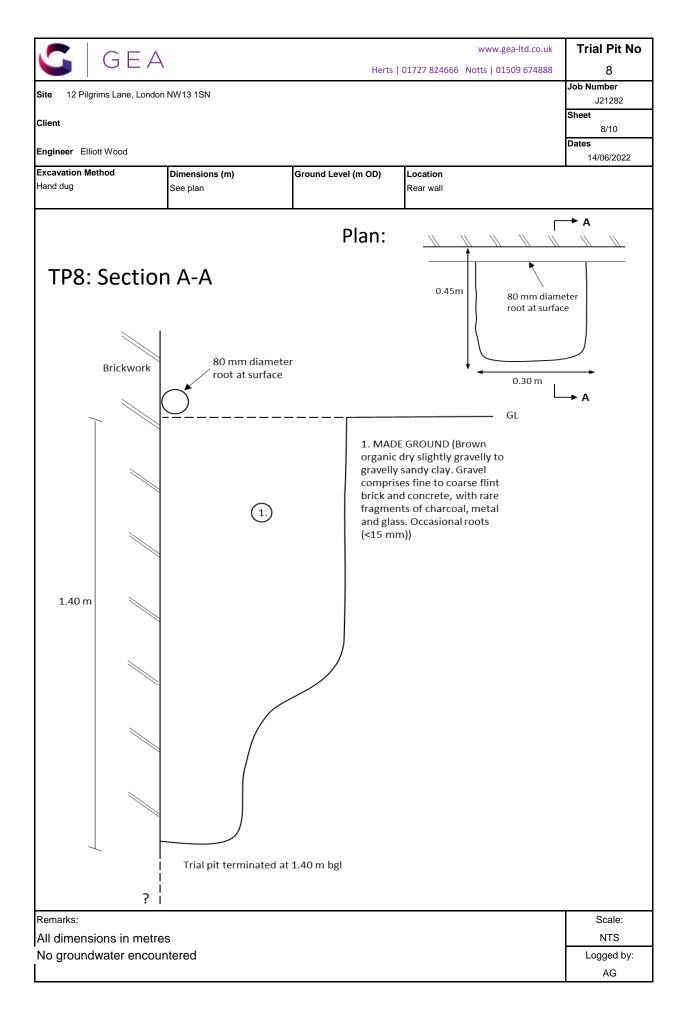


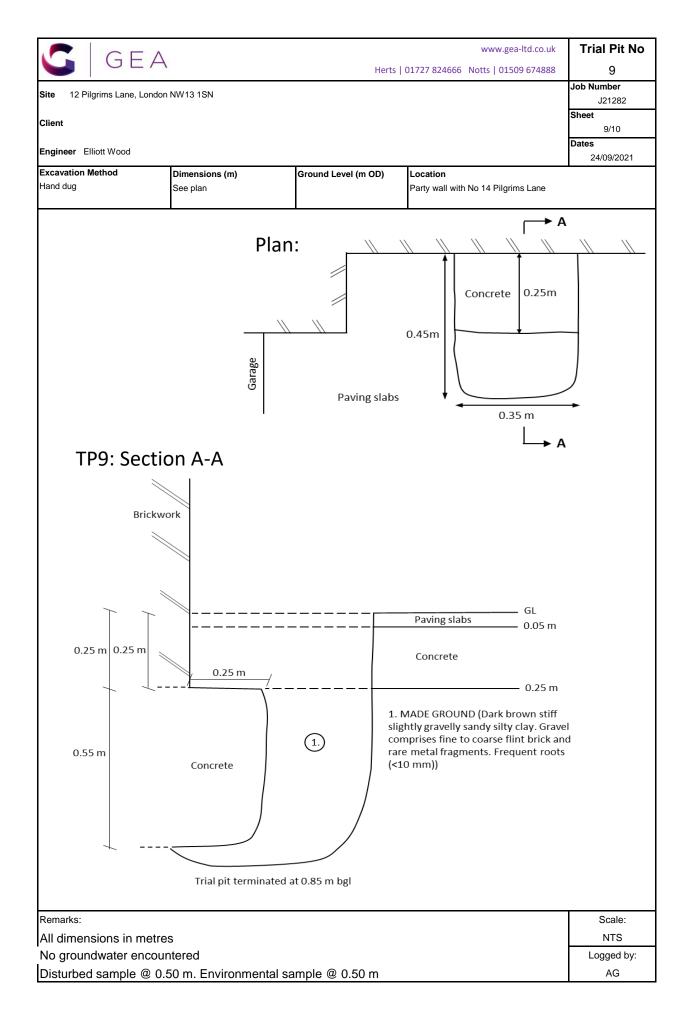


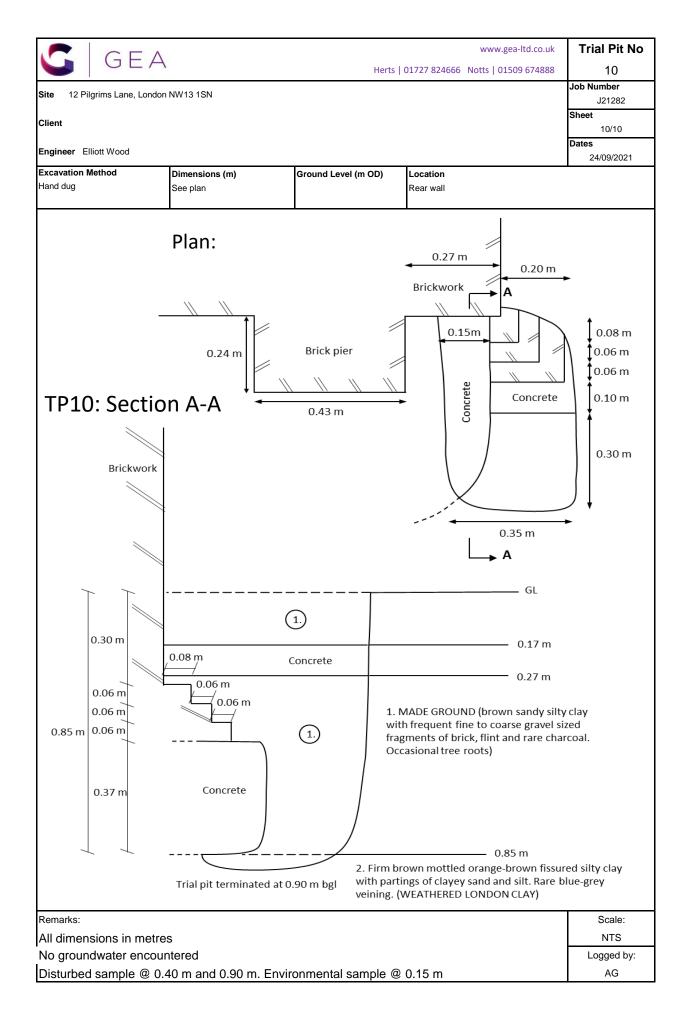














Lab Testing

Geotechnical Test Results SPT & Cohesion/Depth Graph Chemical Test Results Generic Risk Based Screening Values



SUMMARY OF GEOTECHNICAL TESTING

Sample details							ficatio	n Test	ts	Density	Tests	U	ndrained Tr	riaxial Com	pression	Ch	emical Te	ests	
Location	Depth (m)	Sample Ref	Туре	Description	WC %	LL %	PL %	PI %	<425 μm %	Bulk Mg/m ³	Dry Mg/m³	Condition	Cell Pressure kPa	Deviator Stress kPa	Shear Stress kPa	рН	2:1 W/S SO4 g/L	W/S Mg mg/L	Other tests and comments
BH1	1.20-1.65		D	Orangish brown and brown gravelly sandy silty CLAY. Sand is fine. Gravel is fine to medium.	23.6	60	21	39	99										
BH1	1.85		D	Brown sandy silty CLAY with rare gypsum crystals. Sand is fine.	32.3														
BH1	2.00-2.45		U	Stiff fissured brown mottled dark brown CLAY.	33.7					1.97	1.47	Undisturbed	40	133	67				
BH1	2.75		D	Brown sandy silty CLAY with rare gypsum crystals. Sand is fine.	26.7	72	27	45	100										
BH1	3.75		D													8.8	2.2		
BH1	4.00-4.45		U	Stiff fissured brown mottled grey CLAY with rare gypsum crystals.	30.2					1.90	1.46	Undisturbed	80	184	92				
BH1	5.00-5.45		D	Brown sandy silty CLAY with rare gypsum crystals. Sand is fine.	27.1	68	28	40	100										
BH1	6.50-6.75		U	Stiff fissured dark grey mottled dark brown CLAY.	25.5					2.02	1.61	Undisturbed	130	204	102				
BH2	0.50		D													8.7	1.3		
BH2	0.75		D	Orangish brown and brownish grey gravelly sandy silty CLAY. Sand is fine. Gravel is fine to medium and includes brick fragments.	21.2	46	20	26	98										

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

Checked and Approved by	Project Number:	
	GEO / 34021	(®
LEH -	Project Name:	GEOLABS
J Sturges - Operations Manager	12 PILGRIMS LANE, LONDON, NW3 1SN	
22/10/2021	J21282	

Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

SUMMARY OF GEOTECHNICAL TESTING

										P									
Sample details							Classification Tests			Density	Tests	U	ndrained T	riaxial Com	pression	Ch	emical Te	ests	
Location	Depth (m)	Sample Ref	Туре	Description	WC %	LL %	PL %	PI %	<425 μm %	Bulk Mg/m³	Dry Mg/m³	Condition	Cell Pressure kPa	Deviator Stress kPa	Shear Stress kPa	рН	2:1 W/S SO4 g/L	W/S Mg mg/L	Other tests and comments
BH2	1.20-1.65		D	Dark grey and dark brown mottled gravelly sandy silty CLAY. Sand is fine to medium.	26.0														
BH2	2.00-2.45		U	Stiff fissured brown CLAY with rare gypsum crystals.	29.0					1.96	1.52	Undisturbed	40	132	66				
BH2	2.75		D	Brown sandy silty CLAY with rare gypsum crystals. Sand is fine.	27.1														
BH2	3.75		D	Brown and grey sandy silty CLAY. Sand is fine.	31.5	76	28	48	100										
BH2	4.00-4.45		U	Stiff fissured dark brown CLAY.	30.5					1.90	1.45	Undisturbed	80	176	88				
BH2	4.75		D	Brown sandy silty CLAY with rare gypsum crystals. Sand is fine.	31.2	76	29	47	100										
BH2	6.50-6.95		U	Stiff fissured dark brown CLAY.	24.7					1.99	1.59	Undisturbed	130	296	148				
BH2	8.00-8.45		D	Dark grey sandy silty CLAY. Sand is fine.	27.2	68	25	43	100										
BH3	1.30		D	Orangish brown and greyish brown sandy CLAY.	32.4														
TP1	0.90		D	Orangish brown and greyish brown slightly gravelly sandy silty CLAY. Sand is fine. Gravel is fine to medium.	35.0	75	29	46	99										

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

Checked and Approved by	Project Number:	
101	GEO / 34021	(®
HH I	Project Name:	GEOLABS
J Sturges - Operations Manager	12 PILGRIMS LANE, LONDON, NW3 1SN	
22/10/2021	J21282	

Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

Location Depth (m) Sample Type

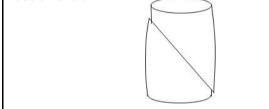
BH1 2.00-2.45 U Description:

Stiff fissured brown mottled dark brown CLAY.

Specimen Details

-		
Specimen conditions		Undisturbed
Length	(mm)	202.5
Diameter	(mm)	102.4
Moisture content	(%)	33.7
Bulk density	(Mg/m³)	1.97
Dry density	(Mg/m³)	1.47
Test Details		
Latex membrane thickness	(mm)	0.3
Specimen height prior to shearing	(mm)	202.5
Membrane correction	(kPa)	0.5
Mean rate of shear	(%/min)	2.0
Cell pressure	(kPa)	40
Strain at failure	(%)	6.4
Maximum deviator stress	(kPa)	133
Shear Stress Cu	(kPa)	67

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	110



Tested by FA

tions Manager

J Sturges - Operations 22/10/2021

Checked and Approved by Project Number:

Project Name:

GEO / 34021



12 PILGRIMS LANE, LONDON, NW3 1SN J21282

 Test Report By GEOLABS Limited
 Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

 Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

Page 1 of 1 (Ref 1634904258)

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

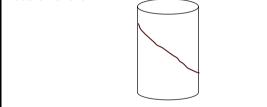
Location Depth (m) Sample Type

BH1 4.00-4.45 U Description: Stiff fissured brown mottled grey CLAY with rare gypsum crystals.

Specimen Details

opeoimen Detailo		
Specimen conditions		Undisturbed
Length	(mm)	202.2
Diameter	(mm)	102.8
Moisture content	(%)	30.2
Bulk density	(Mg/m³)	1.90
Dry density	(Mg/m³)	1.46
Test Details		
Latex membrane thickness	(mm)	0.3
Specimen height prior to shearing	(mm)	202.2
Membrane correction	(kPa)	0.3
Mean rate of shear	(%/min)	2.0
Cell pressure	(kPa)	80
Strain at failure	(%)	4.0
Maximum deviator stress	(kPa)	184
Shear Stress Cu	(kPa)	92

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	65

Version 93.210726

Tested by FA

ons Manager

J Sturges - Operations 22/10/2021

Checked and Approved by Project Number:

Project Name:

ibei.

GEO / 34021



12 PILGRIMS LANE, LONDON, NW3 1SN J21282

 Test Report By GEOLABS Limited
 Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

 Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

Page 1 of 1 (Ref 1634904262)

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

Location Depth (m) Sample Type

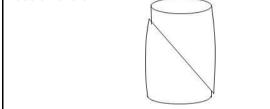
BH1 6.50-6.75 U Description:

Stiff fissured dark grey mottled dark brown CLAY.

Specimen Details

-		
Specimen conditions		Undisturbed
Length	(mm)	202.3
Diameter	(mm)	102.3
Moisture content	(%)	25.5
Bulk density	(Mg/m³)	2.02
Dry density	(Mg/m³)	1.61
Test Details		
Latex membrane thickness	(mm)	0.3
Specimen height prior to shearing	(mm)	202.2
Membrane correction	(kPa)	0.4
Mean rate of shear	(%/min)	2.0
Cell pressure	(kPa)	130
Strain at failure	(%)	5.9
Maximum deviator stress	(kPa)	204
Shear Stress Cu	(kPa)	102

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	90

Version 93.210726

Tested by FA

ons Manager

J Sturges - Operations 22/10/2021

Checked and Approved by Project Number:

Project Name:

GEO / 34021



12 PILGRIMS LANE, LONDON, NW3 1SN J21282

 Test Report By GEOLABS Limited
 Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

 Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

Page 1 of 1 (Ref 1634904266)

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

Location Depth (m) Sample Type

BH2 2.00-2.45 U Description:

Stiff fissured brown CLAY with rare gypsum crystals.

Specimen Details

	Undisturbed
(mm)	201.9
(mm)	103.6
(%)	29.0
(Mg/m³)	1.96
(Mg/m³)	1.52
(mm)	0.3
(mm)	201.8
(kPa)	0.4
(%/min)	2.0
(kPa)	40
(%)	5.9
(kPa)	132
(kPa)	66
	(mm) (%) (Mg/m ³) (Mg/m ³) (mm) (mm) (kPa) (%/min) (kPa) (%) (kPa)

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	150

Version 93.210726

Tested by FA

tions Manager

J Sturges - Operations 22/10/2021

Checked and Approved by Project Number:

Project Name:

GEO / 34021



12 PILGRIMS LANE, LONDON, NW3 1SN J21282

 Test Report By GEOLABS Limited
 Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

 Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

Page 1 of 1 (Ref 1634904269)

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

Location Depth (m) Sample Type

BH2 4.00-4.45 U Description:

Stiff fissured dark brown CLAY.

Specimen Details

-		
Specimen conditions		Undisturbed
Length	(mm)	201.6
Diameter	(mm)	102.5
Moisture content	(%)	30.5
Bulk density	(Mg/m³)	1.90
Dry density	(Mg/m³)	1.45
Test Details		
Latex membrane thickness	(mm)	0.3
Specimen height prior to shearing	(mm)	201.5
Membrane correction	(kPa)	0.4
Mean rate of shear	(%/min)	2.0
Cell pressure	(kPa)	80
Strain at failure	(%)	6.0
Maximum deviator stress	(kPa)	176
Shear Stress Cu	(kPa)	88

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	80

Version 93.210726

Tested by FA

Checked and Approved by Project Number:

Project Name:

GEO / 34021

12 PILGRIMS LANE, LONDON, NW3 1SN

J Sturges - Operations Manager 22/10/2021 J21282 Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION

Location Depth (m) Sample Type

BH2 6.50-6.95 U Description:

Stiff fissured dark brown CLAY.

Specimen Details

	Undisturbed
(mm)	202.1
(mm)	102.8
(%)	24.7
(Mg/m³)	1.99
(Mg/m³)	1.59
(mm)	0.3
(mm)	202.1
(kPa)	0.7
(%/min)	2.0
(kPa)	130
(%)	10.4
(kPa)	296
(kPa)	148
	(mm) (%) (Mg/m ³) (Mg/m ³) (mm) (mm) (kPa) (%/min) (kPa) (%) (kPa)

Mode of failure



Orientation of the sample	Vertical
Distance from top of tube mm	70

Version 93.210726

Checked and Approved by Project Number:

Project Name:

Tested by FA

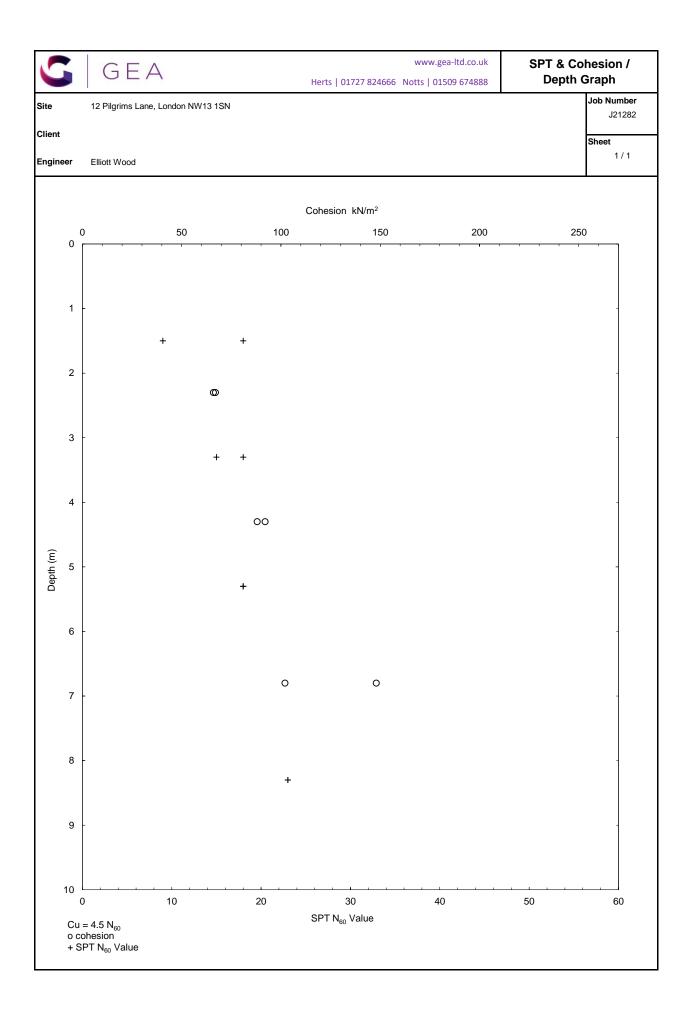
J Sturges - Operations Manager 22/10/2021 GEO / 34021

12 PILGRIMS LANE, LONDON, NW3 1SN J21282

 Test Report By GEOLABS Limited
 Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

 Client : Geotechnical & Environmental Associates Limited, Widbury Barn, Widbury Hill, Ware, Hertfordshire, SG12 7QE

Page 1 of 1 (Ref 1634904277)





Alexander Goodsell Geotechnical & Environmental Associates Widbury Barn Widbury Hill Ware Hertfordshire SG127QE



i2 Analytical Ltd. 7 Woodshots Meadow, Croxley Green Business Park, Watford, Herts, WD18 8YS

t: 01923 225404 f: 01923 237404 e: reception@i2analytical.com

e: AlexGoodsell@gea-ltd.co.uk

Analytical Report Number : 21-12515

Project / Site name:	12 Pilgrams Lane	Samples received on:	27/09/2021
Your job number:	J21282	Samples instructed on/ Analysis started on:	27/09/2021
Your order number:		Analysis completed by:	07/10/2021
Report Issue Number:	1	Report issued on:	07/10/2021
Samples Analysed:	6 soil samples		

Durrado Signed:

Joanna Wawrzeczko Technical Reviewer (Reporting Team) For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils	- 4 weeks from reporting
leachates	- 2 weeks from reporting
waters	- 2 weeks from reporting
asbestos	- 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies. An estimate of measurement uncertainty can be provided on request.





Analytical Report Number: 21-12515 Project / Site name: 12 Pilgrams Lane

Lab Sample Number				2025073	2025074	2025075	2025076	2025077
Sample Reference				BH1	BH1	BH2	BH3	TP1
Sample Number				None Supplied				
Depth (m)				0.25	0.75	0.20	0.50	0.15
Date Sampled				24/09/2021	24/09/2021	24/09/2021	24/09/2021	24/09/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	11	11	9.8	5.7	13
Total mass of sample received	kg	0.001	NONE	0.60	0.70	0.60	0.60	0.60
Asbestos in Soil Screen / Identification Name	Туре	N/A	ISO 17025	-	-	-	Chrysotile	-
Asbestos in Soil	Туре	N/A	ISO 17025	Not-detected	Not-detected	Not-detected	Detected	Not-detected
Asbestos Quantification (Stage 2)	%	0.001	ISO 17025	-	-	-	< 0.001	-
Asbestos Quantification Total	%	0.001	ISO 17025	-	-	-	< 0.001	-
General Inorganics				0.7	0.5	0.5		
pH - Automated	pH Units	N/A	MCERTS	8.7	8.5	8.5	8.0	6.5
Total Cyanide	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Sulphate as SO4 Water Soluble SO4 16hr extraction (2:1 Leachate	mg/kg	50	MCERTS	630	810	710	800	1200
Equivalent)	g/l	0.00125	MCERTS	0.025	0.016	0.022	0.042	0.033
Sulphide	mg/kg	1	MCERTS	< 1.0	2.3	< 1.0	< 1.0	< 1.0
Water Soluble Chloride (2:1)	mg/kg	1	MCERTS	19	7.0	9.9	9.5	25
Total Organic Carbon (TOC)	%	0.1	MCERTS	0.8	0.8	1.5	1.6	6.5
Total Phenols Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Speciated PAHs								
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	1.4	0.25	2.6	1.1	1.1
Anthracene	mg/kg	0.05	MCERTS	0.28	< 0.05	0.51	0.27	0.23
Fluoranthene	mg/kg	0.05	MCERTS	2.6	0.42	6.9	2.4	2.5
Pyrene	mg/kg	0.05	MCERTS	2.6	0.39	6.4	2.3	2.3
Benzo(a)anthracene	mg/kg	0.05	MCERTS	1.6	0.26	3.9	1.6	1.5
Chrysene	mg/kg	0.05	MCERTS	1.2	0.23	3.2	1.3	1.2
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	1.3	0.24	4.2	1.4	1.5
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	0.87	0.10	1.8	0.94	0.93
Benzo(a)pyrene	mg/kg	0.05	MCERTS	1.4	0.21	3.9	1.4	1.5
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	0.63	< 0.05	1.9	0.62	0.79
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	0.52	< 0.05	< 0.05
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	0.82	< 0.05	2.1	0.69	0.91
Total PAH		1						
Speciated Total EPA-16 PAHs	mg/kg	0.8	MCERTS	14.7	2.10	37.9	13.9	14.2



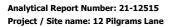


Analytical Report Number: 21-12515 Project / Site name: 12 Pilgrams Lane

Lab Sample Number	2025073	2025074	2025075	2025076	2025077			
Sample Reference				BH1	BH1	BH2	BH3	TP1
Sample Number			None Supplied	None Supplied	None Supplied	None Supplied	None Supplied	
Depth (m)				0.25	0.75	0.20	0.50	0.15
Date Sampled				24/09/2021	24/09/2021	24/09/2021	24/09/2021	24/09/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Heavy Metals / Metalloids	•							
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	40	30	21	29	26
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0	< 4.0	< 4.0	< 4.0	< 4.0
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	33	31	34	35	34
Copper (aqua regia extractable)	mg/kg	1	MCERTS	44	57	42	88	75
Lead (aqua regia extractable)	mg/kg	1	MCERTS	220	190	230	600	350
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	0.8	< 0.3	< 0.3	1.0	0.9
Nickel (aqua regia extractable)	mg/kg	1	MCERTS	26	19	23	23	21
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	130	91	130	200	290
Petroleum Hydrocarbons								
TPH C10 - C40	mg/kg	10	MCERTS	57	< 10	100	37	43
TPH (C8 - C10)	mg/kg	0.1	MCERTS	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
TPH (C10 - C12)	mg/kg	2	MCERTS	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0
TPH (C12 - C16)	mg/kg	4	MCERTS	< 4.0	< 4.0	4.2	< 4.0	< 4.0
TPH (C16 - C21)	mg/kg	1	MCERTS	15	< 1.0	23	8.4	11
TPH (C21 - C35)	mg/kg	1	MCERTS	36	< 1.0	66	27	30

U/S = Unsuitable Sample I/S = Insufficient Sample





Lab Sample Number	2025078			
Sample Reference	TP2			
Sample Number		None Supplied		
Depth (m)				0.50
Date Sampled				24/09/2021
Time Taken	None Supplied			
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status	
Stone Content	%	0.1	NONE	< 0.1
Moisture Content	%	0.01	NONE	15
Total mass of sample received	kg	0.001	NONE	0.60

Asbestos in Soil Screen / Identification Name	Туре	N/A	ISO 17025	-
Asbestos in Soil	Туре	N/A	ISO 17025	Not-detected
Asbestos Quantification (Stage 2)	%	0.001	ISO 17025	-
Asbestos Quantification Total	%	0.001	ISO 17025	-

General Inorganics

pH - Automated	pH Units	N/A	MCERTS	8.2
Total Cyanide	mg/kg	1	MCERTS	< 1.0
Total Sulphate as SO4	mg/kg	50	MCERTS	800
Water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.091
Sulphide	mg/kg	1	MCERTS	< 1.0
Water Soluble Chloride (2:1)	mg/kg	1	MCERTS	8.7
Total Organic Carbon (TOC)	%	0.1	MCERTS	1.8

Total Phenols

Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0

Speciated PAHs

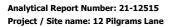
Naphthalene	mg/kg	0.05	MCERTS	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	0.49
Anthracene	mg/kg	0.05	MCERTS	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	0.82
Pyrene	mg/kg	0.05	MCERTS	0.71
Benzo(a)anthracene	mg/kg	0.05	MCERTS	0.65
Chrysene	mg/kg	0.05	MCERTS	0.52
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	0.63
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	0.37
Benzo(a)pyrene	mg/kg	0.05	MCERTS	0.53
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	0.29
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	0.33

Total PAH

Speciated Total EPA-16 PAHs mg,	mg/kg	0.8	MCERTS	5.34
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Lab Sample Number				2025078
Sample Reference	TP2			
Sample Number	None Supplied			
Depth (m)	0.50			
Date Sampled				24/09/2021
Time Taken				None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status	
Heavy Metals / Metalloids				
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	29
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	< 0.2
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	44
Copper (aqua regia extractable)	mg/kg	1	MCERTS	65
Lead (aqua regia extractable)	mg/kg	1	MCERTS	430
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	1.2
Nickel (aqua regia extractable)	mg/kg	1	MCERTS	35
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	< 1.0
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	120

Petroleum Hydrocarbons

TPH C10 - C40	mg/kg	10	MCERTS	160
TPH (C8 - C10)	mg/kg	0.1	MCERTS	< 0.1
TPH (C10 - C12)	mg/kg	2	MCERTS	< 2.0
TPH (C12 - C16)	mg/kg	4	MCERTS	56
TPH (C16 - C21)	mg/kg	1	MCERTS	91
TPH (C21 - C35)	mg/kg	1	MCERTS	15

U/S = Unsuitable Sample I/S = Insufficient Sample







Analytical Report Number:21-12515Project / Site name:12 Pilgrams LaneYour Order No:12

Certificate of Analysis - Asbestos Quantification

Methods:

Qualitative Analysis

The samples were analysed qualitatively for asbestos by polarising light and dispersion staining as described by the Health and Safety Executive in HSG 248.

Quantitative Analysis

The analysis was carried out using our documented in-house method A006-PL based on HSE Contract Research Report No: 83/1996: Development and Validation of an analytical method to determine the amount of asbestos in soils and loose aggregates (Davies et al, 1996) and HSG 248. Our method includes initial examination of the entire representative sample, then fractionation and detailed analysis of each fraction, with quantification by hand picking and weighing.

The limit of detection (reporting limit) of this method is 0.001 %.

The method has been validated using samples of at least 100 g, results for samples smaller than this should be interpreted with caution.

Sample Number	Sample ID	Sample Depth (m)	Sample Weight (g)	Asbestos Containing Material Types Detected (ACM)	PLM Results	Asbestos by hand picking/weighing (%)	Total % Asbestos in Sample
2025076	BH3	0.50	170	Loose Fibres	Chrysotile	< 0.001	< 0.001

Both Qualitative and Quantitative Analyses are UKAS accredited.

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.





Analytical Report Number : 21-12515 Project / Site name: 12 Pilgrams Lane

* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
2025073	BH1	None Supplied	0.25	Brown clay and loam with gravel and brick.
2025074	BH1	None Supplied	0.75	Brown clay and loam with gravel and brick.
2025075	BH2	None Supplied	0.2	Brown loam and clay with gravel and vegetation.
2025076	BH3	None Supplied	0.5	Brown loam and clay with gravel and vegetation.
2025077	TP1	None Supplied	0.15	Brown clay and loam with gravel and vegetation.
2025078	TP2	None Supplied	0.5	Brown clay and loam with gravel.





Analytical Report Number : 21-12515 Project / Site name: 12 Pilgrams Lane

Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Metals in soil by ICP-OES	Determination of metals in soil by aqua-regia digestion followed by ICP-OES.	In-house method based on MEWAM 2006 Methods for the Determination of Metals in Soil.	L038-PL	D	MCERTS
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS
Asbestos identification in soil	Asbestos Identification with the use of polarised light microscopy in conjunction with disperion staining techniques.	In house method based on HSG 248	A001-PL	D	ISO 17025
Chloride, water soluble, in soil	Determination of Chloride colorimetrically by discrete analyser.	In house method.	L082-PL	D	MCERTS
Hexavalent chromium in soil	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry.	In-house method	L080-PL	W	MCERTS
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	W	NONE
Monohydric phenols in soil	Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by colorimetry.		L080-PL	W	MCERTS
Speciated EPA-16 PAHs in soil	Determination of PAH compounds in soil by extraction in dichloromethane and hexane followed by GC-MS with the use of surrogate and internal standards.		L064-PL	D	MCERTS
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS
Sulphide in soil	Determination of sulphide in soil by acidification and heating to liberate hydrogen sulphide, trapped in an alkaline solution then assayed by ion selective electrode.	In-house method	L010-PL	D	MCERTS
Total sulphate (as SO4 in soil)	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Total cyanide in soil	Determination of total cyanide by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (Skalar)	L080-PL	w	MCERTS
Total organic carbon (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.	In house method.	L009-PL	D	MCERTS
TPH Banding in Soil by FID	Determination of hexane extractable hydrocarbons in soil by GC-FID.	In-house method, TPH with carbon banding and silica gel split/cleanup.	L076-PL	w	MCERTS
Asbestos Quantification - Gravimetric	Asbestos quantification by gravimetric method - in house method based on references.	HSE Report No: 83/1996, HSG 248, HSG 264 & SCA Blue Book (draft).	A006-PL	D	ISO 17025
D.O. for Gravimetric Quant if Screen/ID positive	Dependent option for Gravimetric Quant if Screen/ID positive scheduled.	In house asbestos methods A001 & A006.	A006-PL	D	NONE





Analytical Report Number : 21-12515 Project / Site name: 12 Pilgrams Lane

Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
TPH in (Soil)		In-house method, TPH with carbon banding and silica gel split/cleanup.	L076-PL	D	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland. Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.



Generic Risk-Based Soil Screening Values

Job Number

J21282

Sheet 1 / 1

Site Client

Engineer

Elliott Wood

Proposed End Use Residential with plant uptake

12 Pilgrims Lane, London NW13 1SN

Soil Organic Matter content % 1.0

GEA

Contaminant	Screening Value mg/kg	Data Source	Contaminant	Screening Value mg/kg	Data Source			
	Metals		Hydrocarbons					
Arsenic	37	C4SL	Banded TPH (8-10)	52	Calc1			
Cadmium	22	C4SL	Banded TPH (10-12)	114	Calc1			
Chromium (III)	910	S4UL	Banded TPH (12-16)	215	Calc1			
Chromium (VI)	21	C4SL	Banded TPH (16-21)	400	Calc1			
Copper	2,400	S4UL	Banded TPH (21-35)	1692	Calc1			
Lead	200	C4SL	Benzene	0.2	C4SL			
Elemental Mercury	1.2	S4UL	Toluene	120	SGV			
Inorganic Mercury	40	S4UL	Ethyl Benzene	65	SGV			
Nickel	130	S4UL	Xylene	42	SGV			
Selenium	350	SGV	Aliphatic C5-C6	42	S4UL			
Zinc	3,700	S4UL	Aliphatic C6-C8	100	S4UL			
	Anions		Aliphatic C8-C10	27	S4UL			
Soluble Sulphate	500 mg/l	Structures	Aliphatic C10-C12	130	S4UL			
Sulphide	50	Structures	Aliphatic C12-C16	1100	S4UL			
Chloride	400	Structures	Aliphatic C16-C35	65,000	S4UL			
	Others		Aromatic C6-C7	See Benzene	S4UL			
Organic Carbon (%)	6	Methanogenic potential	Aromatic C7-C8	See Toluene	S4UL			
Total Cyanide	140	WRAS	Aromatic C8-C10	34	S4UL			
Total Mono Phenols	184	SGV	Aromatic C10-C12	74	S4UL			
	PAH		Aromatic C12-C16	140	S4UL			
Naphthalene	2.30	S4UL	Aromatic C16-C21	260	S4UL			
Acenaphthylene	170	S4UL	Aromatic C21-C35	1100	S4UL			
Acenaphthene	210	S4UL	PRO (C ₅ –C ₁₀)	323	Calc2			
Fluorene	170	S4UL	DRO (C ₁₂ –C ₂₈)	66,500	Calc2			
Phenanthrene	95	S4UL	Lube Oil (C ₂₈ –C ₄₄)	66,100	Calc2			
Anthracene	2,400	S4UL	ТРН	750	Trigger to consi			
Fluoranthene	280	S4UL			speciated testi			
Pyrene	620	S4UL	Chlorina	ted Solven	ts			
Benzo(a)anthracene	7.2	S4UL	1,1,1 trichloroethane (TCA)	8.8	S4UL			
Chrysene	15	S4UL	tetrachloroethane (PCA)	1.2	S4UL			
Benzo(b)fluoranthene	2.6	S4UL	tetrachloroethene (PCE)	0.18	S4UL			
Benzo(k)fluoranthene	77.0	S4UL	trichloroethene (TCE)	0.016	S4UL			
Benzo(a)pyrene	4.35	C4SL	1,2-dichloroethane (DCA)	0.0071	S4UL			
Indeno(1 2 3 cd)pyrene	27.0	S4UL	vinyl chloride (Chloroethene)	0.00064	S4UL			
Dibenz(a h)anthracene	0.24	S4UL	tetrachloromethane (Carbon tetra	0.026	S4UL			
Benzo (g h i)perylene	320	S4UL	trichloromethane (Chloroform)	0.91	S4UL			
Total PAH Screen	62.1	B(a)P / 0.15						

Notes : Concentrations measured below these screening values may be considered to represent 'uncontaminated conditions' which pose a 'LOW' risk to human

health. Concentrations measured in excess of these values indicate a potential risk which require further, site specific risk assessment.

C4SL - Defra Category 4 Screening value based on Low Level of Toxicological Risk

SGV - Soil Guideline Value, derived from the CLEA model and published by Environment Agency 2009 - where not superseded by C4SL

S4UL - LQM/CIEH Suitable for use Level (2015) based on 'minimal' level of risk

Calc1 - sum of thresholds for Ali & Aro fractions - assuming a 35% Aro:65% Ali ratio as is commonly encountered in the soil

Calc2 - sum of nearest available carbon range specified including BTEX for PRO fraction

Total PAH based on B(a)P / 0.15 - GEA experience indicates that Benzo(a) pyrene rarely exceeds 15% of the total PAH concentration

S	GEA Widbury Barn Widbury Hill Ware SG12 7QE	Generic Risk-Based Soil Screening Values
Site	12 Pilgrims Lane, London NW13 1SN	Job Number J21282
Client		Sheet
Engineer	Elliott Wood	2/2
Proposed E	nd Use Residential with plant uptake	
The key generic assumptions for this end use are as follows;		
	that groundwater will not be a critical risk receptor;	
	that the critical receptor for human health will be a young female aged 0 to 6 years old;	
	that the exposure duration will be six years;	
	that the building type equates to a terraced house.	
	that the critical exposure pathways will be direct soil and indoor dust ingestion, consumption of h of soil adhering to home grown produce, skin contact with soils and dust, and inhalation of dust a	• • •
Where contaminant concentrations are measured at concentrations below the generic screening value it is considered that they pose an acceptable level of risk and thus further consideration of these contaminant concentrations is not required. However, where concentrations are measured in excess of the generic screening value there is considered to be a potential that they could pose an unacceptable risk and thus further action will be required which could include:		
•	additional testing to zone the extent of the contaminated material and thus reduce the uncertaint	y with regard to its potential risk;
	site specific risk assessment to refine the assessment criteria and allow an assessment to be maconcentration present would pose an unacceptable risk at this site; or	ade as to whether the
•	soil remediation or risk management to mitigate the risk posed by the contaminant to a degree the	nat it poses an acceptable risk.



Desk Study

Envirocheck Extracts Historical Maps Risk Assessment Tables UXO Preliminary Risk Assessment





Envirocheck[®] Report:

Datasheet

Order Details:

Order Number: 287612059_1_1

Customer Reference: J21282

National Grid Reference: 526850, 185660

Slice:

1

Site Area (Ha): 0.08

Search Buffer (m): 1000

Site Details:

12, Pilgrims Lane LONDON NW3 1SN

Client Details:

Mr S Branch GEA Ltd Widbury Barn Widbury Hill Ware Herts SG12 7QE



Contents

Report Section	Page Number
Summary	-
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Introduction

GEA

The Environment Act 1995 has made site sensitivity a key issue, as the legislation pays as much attention to the pathways by which contamination could spread, and to the vulnerable targets of contamination, as it does the potential sources of contamination.

Tor this reason, Landmark's Site Sensitivity maps and Datasheet(s) place great emphasis on statutory data provided by the Environment Agency/Natural Resources Wales and the Scottish Environment Protection Agency; it also incorporates data from Natural England (and the Scottish and Welsh equivalents) and Local Authorities; and highlights hydrogeological features required by environmental and geotechnical consultants. It does not include any information concerning past uses of land. The datasheet is produced by querying the Landmark database to a distance defined by the client from a site boundary provided by the client. In this datasheet the National Grid References (NGRs) are rounded to the nearest 10m in accordance with Landmark's agreements with a number of Data Suppliers.

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Report Version v53.0

Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Agency & Hydrological					
BGS Groundwater Flooding Susceptibility	pg 1	Yes			n/a
Contaminated Land Register Entries and Notices					
Discharge Consents	pg 1				1
Prosecutions Relating to Controlled Waters			n/a	n/a	n/a
Enforcement and Prohibition Notices					
Integrated Pollution Controls					
Integrated Pollution Prevention And Control					
Local Authority Integrated Pollution Prevention And Control					
Local Authority Pollution Prevention and Controls	pg 1			2	7
Local Authority Pollution Prevention and Control Enforcements					
Nearest Surface Water Feature	pg 2			Yes	
Pollution Incidents to Controlled Waters					
Prosecutions Relating to Authorised Processes					
Registered Radioactive Substances	pg 2			27	13
River Quality					
River Quality Biology Sampling Points					
River Quality Chemistry Sampling Points					
Substantiated Pollution Incident Register	pg 9				1
Water Abstractions	pg 9				(*4)
Water Industry Act Referrals					
Groundwater Vulnerability Map	pg 10	Yes	n/a	n/a	n/a
Groundwater Vulnerability - Soluble Rock Risk			n/a	n/a	n/a
Groundwater Vulnerability - Local Information			n/a	n/a	n/a
Bedrock Aquifer Designations	pg 10	Yes	n/a	n/a	n/a
Superficial Aquifer Designations			n/a	n/a	n/a
Source Protection Zones	pg 11				1
Extreme Flooding from Rivers or Sea without Defences				n/a	n/a
Flooding from Rivers or Sea without Defences				n/a	n/a
Areas Benefiting from Flood Defences				n/a	n/a
Flood Water Storage Areas				n/a	n/a
Flood Defences				n/a	n/a
OS Water Network Lines	pg 11			3	19

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Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Waste					
BGS Recorded Landfill Sites					
Historical Landfill Sites					
Integrated Pollution Control Registered Waste Sites					
Licensed Waste Management Facilities (Landfill Boundaries)					
Licensed Waste Management Facilities (Locations)					
Local Authority Landfill Coverage	pg 14	1	n/a	n/a	n/a
Local Authority Recorded Landfill Sites					
Potentially Infilled Land (Non-Water)	pg 14			1	6
Potentially Infilled Land (Water)	pg 14			2	
Registered Landfill Sites					
Registered Waste Transfer Sites					
Registered Waste Treatment or Disposal Sites					
Hazardous Substances					
Control of Major Accident Hazards Sites (COMAH)					
Explosive Sites					
Notification of Installations Handling Hazardous Substances (NIHHS)					
Planning Hazardous Substance Consents					
Planning Hazardous Substance Enforcements					

GEA Summary Page 501 to 1000m Data Type On Site 0 to 250m 251 to 500m Number (*up to 2000m) Geological pg 15 Yes n/a n/a n/a BGS 1:625,000 Solid Geology **BGS Estimated Soil Chemistry BGS Recorded Mineral Sites** BGS Urban Soil Chemistry pg 15 Yes Yes Yes BGS Urban Soil Chemistry Averages Yes pg 18 **CBSCB** Compensation District n/a n/a n/a **Coal Mining Affected Areas** n/a n/a n/a Mining Instability n/a n/a n/a Man-Made Mining Cavities Natural Cavities Non Coal Mining Areas of Great Britain n/a n/a Potential for Collapsible Ground Stability Hazards pg 18 Yes n/a n/a Potential for Compressible Ground Stability Hazards n/a n/a Potential for Ground Dissolution Stability Hazards n/a n/a Potential for Landslide Ground Stability Hazards pg 18 Yes Yes n/a n/a pg 18 Yes Potential for Running Sand Ground Stability Hazards n/a n/a Potential for Shrinking or Swelling Clay Ground Stability Hazards Yes pg 18 n/a n/a Radon Potential - Radon Affected Areas n/a n/a n/a Radon Potential - Radon Protection Measures n/a n/a n/a Industrial Land Use Contemporary Trade Directory Entries 14 35 95 pg 19 **Fuel Station Entries** pg 31 1 Points of Interest - Commercial Services 1 pg 31 17 Points of Interest - Education and Health pg 32 5 4 Points of Interest - Manufacturing and Production pg 33 2 12 Points of Interest - Public Infrastructure 4 2 pg 34 11

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Points of Interest - Recreational and Environmental

Gas Pipelines

Underground Electrical Cables

GEA

Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m (*up to 2000m)
Sensitive Land Use					
Ancient Woodland					
Areas of Adopted Green Belt					
Areas of Unadopted Green Belt					
Areas of Outstanding Natural Beauty					
Environmentally Sensitive Areas					
Forest Parks					
Local Nature Reserves	pg 38				1
Marine Nature Reserves					
National Nature Reserves					
National Parks					
Nitrate Sensitive Areas					
Nitrate Vulnerable Zones					
Ramsar Sites					
Sites of Special Scientific Interest					
Special Areas of Conservation					
Special Protection Areas					
World Heritage Sites					

GEA

Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	BGS Groundwater	Flooding Susceptibility				
	Flooding Type:	Limited Potential for Groundwater Flooding to Occur	A13NW (W)	0	1	526850 185664
	Discharge Consents	S				
1	Operator: Property Type: Location: Authority: Catchment Area: Reference: Permit Version: Effective Date: Issued Date: Revocation Date: Discharge Type: Discharge Environment: Receiving Water: Status: Positional Accuracy:	Thames Water Utilities Ltd WTW/WATER COLLECTION/TREATMENT/SUPPLY Hampstead Environment Agency, Thames Region Not Supplied Temp.0140 1 15th September 1989 15th September 1989 15th September 1989 5th October 2000 Trade Effluent Freshwater Stream/River River Thames Authorisation revoked Located by supplier to within 100m	A17SE (NW)	765	2	526200 186100
	Local Authority Pol	lution Prevention and Controls				
2	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Perkins Dry Cleaners 40 Heath Street, London, Nw3 6te London Borough of Camden, Pollution Projects Team PPC/DC9 12th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A12NE (W)	466	3	526374 185724
	Local Authority Pol	lution Prevention and Controls				
3	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	The Royal Free Hospital Pond Street, LONDON, NW3 2QG London Borough of Camden, Pollution Projects Team Not Given 24th July 1992 Local Authority Air Pollution Control PG5/1Clinical waste incineration processes under 1 tonne an hour Authorisation revoked Manually positioned to the address or location	A14SW (SE)	499	3	527296 185410
	Local Authority Pol	Iution Prevention and Controls				
4	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Belsize Park Service Station 215 Haverstock Hill, LONDON, NW3 4RE London Borough of Camden, Pollution Projects Team PPC21 2nd January 1999 Local Authority Pollution Prevention and Control PG1/14 Petrol filling station Permitted Automatically positioned to the address	A8NE (SE)	535	3	527187 185227
	Local Authority Pol	lution Prevention and Controls				
5	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Pyramid Cleaners 52 Besize Lane, London, Nw3 5ar London Borough of Camden, Pollution Projects Team PPC/DC8 1st January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A8SE (S)	656	3	526872 184985
	Local Authority Pol	Iution Prevention and Controls				
6	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Top Choice Dry Cleaners 96 Fleet Road, London, Nw3 2qx London Borough of Camden, Pollution Projects Team PPC/DC13 12th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A14SE (E)	689	3	527529 185471



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Local Authority Pol	lution Prevention and Controls				
7	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Perkins Dry Cleaners 171 Haverstock Hill, London, Nw3 4qs London Borough of Camden, Pollution Projects Team PPC/DC7 12th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A9NW (SE)	767	3	527342 185055
	Local Authority Pol	lution Prevention and Controls				
7	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Swan Dry Cleaners 163 Haverstock Hill, London, Nw3 4qt London Borough of Camden, Pollution Projects Team PPC/DC42 24th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A9NW (SE)	803	3	527371 185032
	Local Authority Pol	lution Prevention and Controls				
8	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Janet'S Hand Laundry Ltd 281a Finchley Road, London, Nw3 6nd London Borough of Camden, Pollution Projects Team PPC/DC14 12th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A7SW (SW)	988	3	526167 184924
	Local Authority Pol	lution Prevention and Controls				
8	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Hampstead Express Dry Cleaning 279a Finchley Road, London, Nw3 6lt London Borough of Camden, Pollution Projects Team PPC/DC6 12th January 2007 Local Authority Pollution Prevention and Control PG6/46 Dry cleaning Permitted Located by supplier to within 10m	A7SW (SW)	996	3	526178 184902
	Nearest Surface Wa	ter Feature				
			A14NW	374	-	527195
	D	1 • 0 • • • • • •	(NE)			185851
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Pacifional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, Hampstead, LONDON, Greater London, NW3 2QG Environment Agency, Thames Region AR0446 12th July 1995 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation	A14SW (SE)	495	2	527292 185410
	-	Automatically positioned to the address				
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type: Description:	tive Substances Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, Hampstead, LONDON, Greater London, NW3 2QG Environment Agency, Thames Region AT8398 17th January 1996 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA	A14SW (SE)	498	2	527292 185405
	Description: Status: Positional Accuracy:	Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address				



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Anthony Nolan Trust (Ant) Royal Free Hospital, Pond Street, Hampstead, London, NW3 2QG Environment Agency, Thames Region Bz0777 14th July 2005 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	499	2	527297 185411
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Anthony Nolan Trust (Ant) Royal Free Hospital, Pond Street, Hampstead, London, NW3 2QG Environment Agency, Thames Region Bw7643 1st December 2003 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	499	2	527297 185411
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Anthony Nolan Trust (Ant) Royal Free Hospital, Pond Street, Hampstead, London, NW3 2QG Environment Agency, Thames Region Bj5716 14th February 2001 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	499	2	527297 185411
	Registered Radioac					
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region CD3170 13th July 2009 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Application has been authorised and any conditions apply to the	A14SW (SE)	500	2	527297 185410
	Positional Accuracy:	operator Automatically positioned to the address				
<u> </u>	Registered Radioac					
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region CB2954 20th July 2007 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to an authorisation under S13 or S14 RSA in respect of a registration under S7 when Technetium 99M is used being =< 10 gigabecquerels Authorisation either revoked or cancelled Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region Ca2592 13th April 2006 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation	A14SW (SE)	500	2	527297 185410
		Automatically positioned to the address				



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioad	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, LONDON, NW3 2QG Environment Agency, Thames Region Bz9162 9th December 2005 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioad	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region Bz1617 9th September 2005 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioad	tive Substances				
9	Name: Location:	Anthony Nolan Trust (Ant) Medical Physics Department Royal Free Hospital, Pond Street, Hampstead, London, NW3 2QG	A14SW (SE)	500	2	527297 185410
	Authority: Permit Reference: Dated: Process Type:	Environment Agency, Thames Region Bz0831 14th July 2005 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1)				
	Description: Status: Positional Accuracy:	Minor variation to a registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation Manually positioned to the address or location				
	Registered Radioad					
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region By5714 6th December 2004 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA	A14SW (SE)	500	2	527297 185410
	Status:	Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address				
	Registered Radioad	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region By5706 22nd November 2004 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1)	A14SW (SE)	500	2	527297 185410
	Description:	Discretionary registration under the Act of an open source which is also the subject of an authorisation				
	Status:	Application has been authorised and any conditions apply to the operator				
	-	Automatically positioned to the address				
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region Bw6841 1st December 2003 Authorisation under S13 RSA for the disposal of Radioactive waste (was	A14SW (SE)	500	2	527297 185410
	Description: Status: Positional Accuracy:	RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address				



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region Bt8759 12th May 2003 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region Bs4863 25th July 2002 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Minor variation to a registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation	A14SW (SE)	500	2	527297 185410
		Automatically positioned to the address				
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type:	tive Substances Anthony Nolan Trust (Ant) Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region Br6392 29th April 2002 Registration under S7 RSA for the keeping and use of Radioactive materials	A14SW (SE)	500	2	527297 185410
	Description:	(was RSA60 S1) Registration under the Act of an open source which is also the subject of an				
	Status:	authorisation Authorisation superseded by a substantial or non substantial variation				
		Automatically positioned to the address				
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type: Description:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, HAMPSTEAD, LONDON, NW3 2QG Environment Agency, Thames Region Br6406 29th April 2002 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Substantial variation to a registration under the Act of an open source which is also the subject of an authorisation	A14SW (SE)	500	2	527297 185410
	Status: Positional Accuracy:	Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address				
	Registered Radioac			<u> </u>		
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free And University College Medical School Of University College London Medical Physics Department, Royal Free Hospital, Pond Street, London, Greater London, NW3 2PF Environment Agency, Thames Region Bm0214 28th November 2001 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Substantial variation to a registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
_	Registered Radioac					
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, London, NW3 2QG Environment Agency, Thames Region Bj5708 14th February 2001 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation	A14SW (SE)	500	2	527297 185410
		Automatically positioned to the address				



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Polymasc Pharmaceuticals Plc Royal Free Hospital, Pond Street, Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region Bj5678 14th February 2001 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Authorisation under RSA Authorisation either revoked or cancelled Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac					
9	Name:	Royal Free And University College Medical School Of University College	A14SW	500	2	527297
	Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	London Medical Physics Department, Royal Free Hospital, Pond Street, London, Greater London, NW3 2PF Environment Agency, Thames Region BB6254 27th October 1998 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Minor variation to a registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	(SE)			185410
	-					
9	Registered Radioac Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region AV1327 11th August 1997 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Royal Free And University College Medical School Of University College London Medical Physics Department, Royal Free Hospital, Pond Street, London, Greater London, NW3 2PF Environment Agency, Thames Region AR0403 12th July 1995 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac					
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital,Pond Street,Hampstead, LONDON, NW3 2QG Environment Agency, Thames Region AH9987 21st June 1994 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, LONDON, NW3 2QG Environment Agency, Thames Region AB4095 31st March 1991 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Authorisation under RSA Authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	500	2	527297 185410



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free London Nhs Foundation Trust The Royal Free Hospital, Pond Street, Hampstead, Nw3 2qg Environment Agency, Thames Region UB3935DG Not Supplied Not Supplied Application has been determined by the EA Automatically positioned to the address	A14SW (SE)	500	2	527297 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead NHS Trust Royal Free Hospital, Pond Street, Hampstead, LONDON, Greater London, NW3 2QG Environment Agency, Thames Region AV8011 25th October 1996 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	501	2	527292 185400
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead NHS Trust Royal Free Hospital, Pond Street, Hampstead, LONDON, Greater London, NW3 2QG Environment Agency, Thames Region AR0373 11th July 1995 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Minor variation to a registration under the Act of an open source which is also the subject of an authorisation Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	504	2	527302 185410
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free Hampstead Nhs Trust Royal Free Hospital, Pond Street, Hampstead, LONDON, Greater London, NW3 2QG Environment Agency, Thames Region AE8658 24th March 1992 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Registration under the Act of multiple open sources which are also the subject of authorisations Authorisation superseded by a substantial or non substantial variation Automatically positioned to the address	A14SW (SE)	506	2	527302 185405
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	University College London Royal Free Campus, Rowland Hill Street, London, Nw3 2pf Environment Agency, Thames Region By6001 7th May 2015 Not Supplied Not Supplied Replaced Located by supplier to within 100m	A14SW (SE)	507	2	527300 185400
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	University College London Royal Free Campus, Rowland Hill Street, London, Nw3 2pf Environment Agency, Thames Region Bz9758 7th May 2015 Not Supplied Not Supplied Replaced Located by supplier to within 100m	A14SW (SE)	507	2	527300 185400



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free And University College Medical School Of University College London Royal Free Hospital, Pond Street, London, NW3 2QG Environment Agency, Thames Region By6010 3rd August 2005 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Substantial variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Manually positioned to the address or location	A14SW (SE)	507	2	527299 185399
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free And University College Medical School Of University College London Royal Free Hospital, Pond Street, London, NW3 2QG Environment Agency, Thames Region Bw7635 1st December 2003 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Minor variation to authorisation under RSA Authorisation superseded by a substantial or non substantial variation Manually positioned to the address or location	A14SW (SE)	507	2	527299 185399
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Royal Free And University College Medical School Of University College London Royal Free Hospital, Pond Street, London, NW3 2QG Environment Agency, Thames Region Bj5694 14th February 2001 Authorisation under S13 RSA for the disposal of Radioactive waste (was RSA60 S7) Authorisation under RSA Authorisation superseded by a substantial or non substantial variation Manually positioned to the address or location	A14SW (SE)	507	2	527299 185399
	Registered Radioac	tive Substances				
9	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	University College London Royal Free Campus, Rowland Hill Street, London, Nw3 2pf Environment Agency, Thames Region SB3598DT Not Supplied Not Supplied Application has been determined by the EA Located by supplier to within 100m	A14SW (SE)	507	2	527300 185400
	Registered Radioac	tive Substances				
10	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Anthony Nolan Trust Anthony Nolan Histocompatibility Laboratories, 77b Fleet Road, Hampstead, London, Nw3 2qr Environment Agency, Thames Region CB1915 21st January 2016 Not Supplied Not Supplied Replaced Automatically positioned to the address	A14SW (SE)	631	2	527442 185404
	Registered Radioac	tive Substances				7
10	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Anthony Nolan Trust Anthony Nolan Histocompatibility Laboratories, 77b Fleet Road, Hampstead, London, Nw3 2qr Environment Agency, Thames Region CB5171 21st January 2016 Not Supplied Not Supplied Replaced Automatically positioned to the address	A14SW (SE)	631	2	527442 185404



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Registered Radioad	tive Substances				
10	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status: Positional Accuracy:	Anthony Nolan Trust Anthony Nolan Histocompatibility Laboratories, 77b Fleet Road, Hampstead, London, Nw3 2qr Environment Agency, Thames Region AB3298DT Not Supplied Not Supplied Not Supplied Application has been determined by the EA Automatically positioned to the address	A14SW (SE)	631	2	527442 185404
	Registered Radioad	tive Substances				
11	Name: Location: Authority: Permit Reference: Dated: Process Type: Description: Status:	Polymasc Pharmaceuticals Plc Anthony Nolan Building, Royal Free Hospital Site, Fleet Road; Hampstead, LONDON, Greater London, NW3 2EZ Environment Agency, Thames Region AU4924 20th February 1996 Registration under S7 RSA for the keeping and use of Radioactive materials (was RSA60 S1) Registration under the Act of an open source which is also the subject of an authorisation Authorisation either revoked or cancelled Manually positioned to the address or location	A14SW (E)	654	2	527500 185495
	Substantiated Pollu	ition Incident Register				
12	Authority: Incident Date: Incident Reference: Water Impact: Air Impact: Land Impact: Positional Accuracy: Pollutant:	Environment Agency - Thames Region, North East Area 23rd September 2003 191922 Category 2 - Significant Incident Category 4 - No Impact Category 4 - No Impact Located by supplier to within 10m Pollutant Not Identified: Not Identified	A19SW (NE)	580	2	527254 186101
	Water Abstractions					
	Operator: Licence Number: Permit Version: Location: Authority: Abstraction Type: Source: Daily Rate (m3): Yearly Rate (m3): Details: Authorised Start: Authorised Start: Authorised End: Permit Start Date: Permit End Date: Positional Accuracy:	London Borough Of Camden 28/39/39/0219 1 Swiss Cottage Open Space- Borehole Environment Agency, Thames Region Municipal Grounds: Spray Irrigation - Direct Water may be abstracted from a single point Groundwater Not Supplied Not Supplied Swiss Cottage Open Space, Winchester Road, London. 01 January 31 December 1st April 2008 Not Supplied Located by supplier to within 10m	A3SW (S)	1361	2	526800 184280
	Water Abstractions		A2014/	1000	_	E00750
	Operator: Licence Number: Permit Version: Location: Authority: Abstraction Type: Source: Daily Rate (m3): Yearly Rate (m3): Details: Authorised Start: Authorised Start: Authorised End: Permit Start Date: Positional Accuracy:	London Borough Of Camden Th/039/0039/087 1 Swiss Cottage Open Space- Borehole Environment Agency, Thames Region Municipal Grounds: Spray Irrigation - Direct Water may be abstracted from a single point Groundwater Not Supplied Not Supplied Swiss Cottage Open Space, Winchester Road, London 01 April 31 March 5th December 2013 Not Supplied Located by supplier to within 10m	A3SW (S)	1383	2	526750 184261

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Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Water Abstractions					
	Operator: Licence Number: Permit Version:	London Borough Of Camden Th/039/0039/087 1	A3SW (S)	1383	2	526750 184261
	Location: Authority: Abstraction: Abstraction Type: Source: Daily Rate (m3): Yearly Rate (m3): Dataile:	Swiss Cottage Open Space- Borehole Environment Agency, Thames Region Municipal Grounds: General Washing/Process Washing Water may be abstracted from a single point Groundwater Not Supplied Not Supplied				
	Details: Authorised Start: Authorised End: Permit Start Date: Permit End Date: Positional Accuracy:	Swiss Cottage Open Space, Winchester Road, London 01 April 31 March 5th December 2013 Not Supplied Located by supplier to within 10m				
	Water Abstractions					
	Operator: Licence Number: Permit Version: Location: Authority:	London Borough Of Camden Th/039/0039/087 1 Swiss Cottage Open Space- Borehole Environment Agency, Thames Region	A3SW (S)	1383	2	526750 184261
	Abstraction: Abstraction Type: Source: Daily Rate (m3): Yearly Rate (m3): Details:	Municipal Grounds: Lake And Pond Throughflow Water may be abstracted from a single point Groundwater Not Supplied Not Supplied Swiss Cottage Open Space, Winchester Road, London				
	Authorised Start: Authorised End: Permit Start Date: Permit End Date: Positional Accuracy:	01 April 31 March 5th December 2013 Not Supplied Located by supplier to within 10m				
	Groundwater Vulne					
	Combined	Secondary Bedrock Aquifer - High Vulnerability	A13NW	0	4	526851
	Classification: Combined	High	(NE)			185664
	Vulnerability: Combined Aquifer: Pollutant Speed: Bedrock Flow: Dilution: Baseflow Index:	Productive Bedrock Aquifer, No Superficial Aquifer Intermediate Mixed 300-550 mm/year 40-70%				
	Superficial Patchiness: Superficial	<90% <3m				
	Thickness: Superficial Recharge:	No Data				
	Groundwater Vulne	rability Map				
	Combined Classification:	Unproductive Aquifer (may have productive aquifer beneath)	A13NW (E)	0	4	526851 185664
	Combined Vulnerability:	Unproductive Unproductive Bedrock Aguifer, No Superficial Aguifer				
	Combined Aquifer: Pollutant Speed: Bedrock Flow: Dilution: Baseflow Index:	Intermediate Mixed 300-550 mm/year 40-70%				
	Superficial Patchiness: Superficial	<90% <3m				
	Thickness: Superficial Recharge:	No Data				
	-	rability - Soluble Rock Risk				
		-i				
	Bedrock Aquifer De Aquifer Designation:	signations Secondary Aquifer - A	A13NW (NE)	0	4	526851 185664
	Bedrock Aquifer De	signations	(11)			100004
	Aquifer Designation:	Unproductive Strata	A13NW (E)	0	4	526851 185664
	Superficial Aquifer No Data Available	Designations				



Map ID	Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
13	Source Protection Zones Name: Not Supplied Source: Environment Agency, Head Office Reference: Not Supplied Type: Zone II (Outer Protection Zone): Either 25% of the source area or a 400 day travel time whichever is greater.	A3NE (S)	999	2	526966 184648
	Extreme Flooding from Rivers or Sea without Defences None				
	Flooding from Rivers or Sea without Defences None Areas Benefiting from Flood Defences None Flood Water Storage Areas				
	None Flood Defences				
	None				
14	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 5204.1 Watercourse Level: Underground Permanent: True Watercourse Name: The Fountains Catchment Name: Thames Primacy: 1	A14SW (E)	358	5	527227 185651
15	OS Water Network Lines Watercourse Form: Lake Watercourse Length: 172.6 Watercourse Level: On ground surface Permanent: True Watercourse Name: Hampstead Ponds Catchment Name: Thames Primacy: 1	A14NW (E)	395	5	527233 185821
16	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 13.5 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A14SW (E)	446	5	527315 185663
17	OS Water Network Lines Watercourse Form: Inland river Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A14NW (NE)	526	5	527289 185984
18	OS Water Network Lines Watercourse Form: Lake Watercourse Length: 118.5 Watercourse Level: On ground surface Permanent: True Watercourse Name: Hampstead Ponds Catchment Name: Thames Primacy: 1	A19SW (NE)	534	5	527285 186003
19	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 11.9 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A19SW (NE)	588	5	527249 186116
20	OS Water Network Lines Watercourse Form: Lake Watercourse Length: 178.1 Watercourse Level: On ground surface Permanent: True Watercourse Name: Hampstead Ponds Catchment Name: Thames Primacy: 1	A19SW (NE)	593	5	527245 186127

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Map ID	Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
21	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 71.1 Watercourse Level: Underground Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18SE (NE)	677	5	527163 186285
22	OS Water Network Lines Watercourse Form: Inland river Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	697	5	526987 186369
23	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 214.5 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	705	5	526930 186387
24	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 68.4 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	706	5	526954 186384
25	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 10.1 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	715	5	527125 186345
26	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 40.8 Watercourse Level: Underground Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	716	5	527116 186349
27	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 62.7 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NW (N)	752	5	526715 186428
28	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 124.3 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NW (N)	762	5	526771 186446
29	OS Water Network Lines Watercourse Form: Lake Watercourse Length: 117.6 Watercourse Level: On ground surface Permanent: True Watercourse Name: Hampstead Ponds Catchment Name: Thames Primacy: 1	A18NE (N)	769	5	526937 186451



Map ID	Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
30	OS Water Network Lines Watercourse Form: Inland river Watercourse Length: 164.2 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NE (N)	882	5	526922 186565
31	OS Water Network Lines Watercourse Form: Inland river	A19NW	948	5	527476
31	Watercourse Length: 17.4 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	(NE)	940	5	527476 186396
	OS Water Network Lines				
32	Watercourse Form: Inland river Watercourse Length: 5.9 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A19NW (NE)	964	5	527483 186411
	OS Water Network Lines				
33	Watercourse Form: Inland river Watercourse Length: 184.0 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A19NW (NE)	965	5	527488 186408
	OS Water Network Lines				
34	Watercourse Form: Inland river Watercourse Length: 37.2 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NW (N)	984	5	526820 186671
	OS Water Network Lines				
35	Watercourse Form: Inland river Watercourse Length: 9.8 Watercourse Level: On ground surface Permanent: True Watercourse Name: Not Supplied Catchment Name: Thames Primacy: 1	A18NW (N)	984	5	526820 186671



Waste

Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Local Authority La	ndfill Coverage				
	Name:	London Borough of Camden - Has no landfill data to supply		0	6	526851 185664
	Potentially Infilled	Land (Non-Water)				
36	Bearing Ref: Use: Date of Mapping:	SW Unknown Filled Ground (Pit, quarry etc) 1996	A8NW (SW)	414	8	526616 185296
	Potentially Infilled	Land (Non-Water)				
37	Bearing Ref: Use: Date of Mapping:	SE Unknown Filled Ground (Pit, quarry etc) 1996	A9NW (SE)	601	8	527284 185228
	Potentially Infilled	Land (Non-Water)				
38	Bearing Ref: Use: Date of Mapping:	S Unknown Filled Ground (Pit, quarry etc) 1996	A8NW (S)	617	8	526763 185029
	Potentially Infilled	Land (Non-Water)				
39	Bearing Ref: Use: Date of Mapping:	SE Unknown Filled Ground (Pit, quarry etc) 1996	A9NW (SE)	674	8	527347 185189
	Potentially Infilled	Land (Non-Water)				
40	Bearing Ref: Use: Date of Mapping:	NE Unknown Filled Ground (Pit, quarry etc) 1996	A19SW (NE)	678	8	527250 186231
	Potentially Infilled	Land (Non-Water)				
41	Bearing Ref: Use: Date of Mapping:	SE Unknown Filled Ground (Pit, quarry etc) 1996	A9NW (SE)	728	8	527473 185261
	Potentially Infilled	Land (Non-Water)				
42	Bearing Ref: Use: Date of Mapping:	SW Unknown Filled Ground (Pit, quarry etc) 1991	A7NE (SW)	745	8	526467 184999
	Potentially Infilled	Land (Water)				
43	Use: Date of Mapping:	Unknown Filled Ground (Pond, marsh, river, stream, dock etc) 1873	A18SW (N)	322	8	526813 186007
	Potentially Infilled	Land (Water)				
44	Use: Date of Mapping:	Unknown Filled Ground (Pond, marsh, river, stream, dock etc) 1873	A14NW (E)	362	8	527228 185721



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	BGS 1:625,000 Solid					
	Description:	Thames Group	A13NW (NE)	0	1	526851 185664
	BGS Estimated Soil	Chemistry				
	No data available					
	BGS Measured Urba Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	British Geological Survey, National Geoscience Information Service 526732, 185657 Topsoil London 40.30 mg/kg 0.60 mg/kg	A13SW (W)	107	1	526732 185657
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Chromium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A14NW (E)	365	1	527233 185694
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	96.90 mg/kg 626.10 mg/kg 27.60 mg/kg	A14SW (SE)	465	1	527216 185357
	BGS Measured Urba	-	A ON 1147	40.4	_	506700
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A8NW (S)	494	1	526763 185153



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration:		A18SW (N)	585	1	526737 186262
	Concentration: Lead Measured Concentration: Nickel Measured Concentration:	104.40 mg/kg 7.80 mg/kg				
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A12SE (W)	616	1	526223 185630
	BGS Measured Urba Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	British Geological Survey, National Geoscience Information Service 526278, 185352 Topsoil London 25.30 mg/kg 0.50 mg/kg	A12SE (SW)	638	1	526278 185352
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Chromium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A19SW (NE)	705	1	527297 186229
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A8SE (S)	893	1	527169 184808



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:		A14NE (E)	898	1	527766 185717
	BGS Measured Urba Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Chromium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	British Geological Survey, National Geoscience Information Service 526219, 186357 Topsoil London 15.20 mg/kg 0.30 mg/kg	A17NE (NW)	918	1	526219 186357
	BGS Measured Urba	an Soil Chemistry				
	Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Chromium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	British Geological Survey, National Geoscience Information Service 527669, 185211 Topsoil London 18.20 mg/kg 99.60 mg/kg 936.90 mg/kg 25.60 mg/kg	A9NE (SE)	921	1	527669 185211
	BGS Measured Urba Source: Grid: Soil Sample Type: Sample Area: Arsenic Measured Concentration: Cadmium Measured Concentration: Chromium Measured Concentration: Lead Measured Concentration: Nickel Measured Concentration:	British Geological Survey, National Geoscience Information Service 526703, 184701 Topsoil London 32.80 mg/kg 0.70 mg/kg	A8SW (S)	950	1	526703 184701

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BGS Urban Soil Chemistry Averages A13N Source: British Geological Survey, National Geoscience Information Service A13N Sample Area: London Count Id: 7209 Arsenic Minimum 1.00 mg/kg Concentration: Arsenic Average Arsenic Average 17.00 mg/kg Concentration: Arsenic Maximum Arsenic Maximum 161.00 mg/kg Concentration: Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Cadmium Average Cadmium Average 0.90 mg/kg Concentration: Cadmium Maximum Cadmium Maximum 165.20 mg/kg		1	526851 185664
Source: British Geological Survey, National Geoscience Information Service A13N Sample Area: London Count Id: 7209 Arsenic Minimum 1.00 mg/kg Concentration: Arsenic Average Arsenic Average 17.00 mg/kg Concentration: Arsenic Maximum Arsenic Maximum 161.00 mg/kg Concentration: Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Concentration: Cadmium Average 0.90 mg/kg Concentration: 0.90 mg/kg		1	
Count Id: 7209 Arsenic Minimum 1.00 mg/kg Concentration: Arsenic Average Arsenic Average 17.00 mg/kg Concentration: Arsenic Maximum Arsenic Maximum 161.00 mg/kg Concentration: Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Concentration: Cadmium Average 0.90 mg/kg Concentration: Concentration:	:)		185664
Arsenic Minimum 1.00 mg/kg Concentration: 4rsenic Average Arsenic Average 17.00 mg/kg Concentration: 4rsenic Maximum Arsenic Maximum 161.00 mg/kg Concentration: 6.10 mg/kg Concentration: 0.10 mg/kg Concentration: 0.10 mg/kg Concentration: 0.90 mg/kg Concentration: 0.90 mg/kg			
Concentration: Arsenic Average 17.00 mg/kg Concentration: Arsenic Maximum 161.00 mg/kg Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Cadmium Average 0.90 mg/kg Concentration: Cadmium Average 0.90 mg/kg			
Concentration: Arsenic Maximum 161.00 mg/kg Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Cadmium Average 0.90 mg/kg Concentration:			
Arsenic Maximum 161.00 mg/kg Concentration: Cadmium Minimum 0.10 mg/kg Concentration: Cadmium Average 0.90 mg/kg Concentration:			
Cadmium Minimum 0.10 mg/kg Concentration: Cadmium Average 0.90 mg/kg Concentration:			
Concentration: Cadmium Average 0.90 mg/kg Concentration:			
Concentration:			
Cadmium Maximum 165 20 mg/kg			
Concentration:			
Chromium Minimum 13.00 mg/kg			
Concentration: Chromium Average 79.00 mg/kg			
Concentration:			
Chromium Maximum 2094.00 mg/kg Concentration:			
Lead Minimum 11.00 mg/kg			
Concentration: Lead Average 280.00 mg/kg			
Concentration: Lead Maximum 10000.00 mg/kg			
Concentration:			
Nickel Minimum 2.00 mg/kg Concentration:			
Nickel Average 28.00 mg/kg			
Concentration: Nickel Maximum 506.00 mg/kg			
Concentration:			
Coal Mining Affected Areas In an area that might not be affected by coal mining			
Non Coal Mining Areas of Great Britain			
No Hazard			
Potential for Collapsible Ground Stability Hazards			
Hazard Potential: Very Low A13N		1	526851
Source: British Geological Survey, National Geoscience Information Service (NE)		185664
Potential for Compressible Ground Stability Hazards			
Hazard Potential: No Hazard A13N Source: British Geological Survey, National Geoscience Information Service (NE		1	526851 185664
Potential for Ground Dissolution Stability Hazards	·/		
Hazard Potential: No Hazard A13N	w o	1	526851
Source: British Geological Survey, National Geoscience Information Service (NE			185664
Potential for Landslide Ground Stability Hazards			
Hazard Potential: Very Low A13N		1	526851
Source: British Geological Survey, National Geoscience Information Service (NE	:)		185664
Potential for Landslide Ground Stability Hazards			500054
Hazard Potential: Low A13N Source: British Geological Survey, National Geoscience Information Service (E)		1	526951 185705
Potential for Landslide Ground Stability Hazards			
Hazard Potential: Low A135		1	526980
Source: British Geological Survey, National Geoscience Information Service (SE	.)		185575
Potential for Running Sand Ground Stability Hazards			
Hazard Potential: Very Low A13N Source: British Geological Survey, National Geoscience Information Service (NE		1	526851 185664
Potential for Shrinking or Swelling Clay Ground Stability Hazards	·		-
Hazard Potential: Moderate A13N	o w	1	526851
Source: British Geological Survey, National Geoscience Information Service (NE	.)		185664
Radon Potential - Radon Affected Areas			
Affected Area: The property is in a Lower probability radon area (less than 1% of homes are A13N estimated to be at or above the Action Level). (NE		1	526851 185664
Source: British Geological Survey, National Geoscience Information Service	·,		100004
Radon Potential - Radon Protection Measures			
Protection Measure: No radon protective measures are necessary in the construction of new A13N		1	526851
dwellings or extensions (NE Source: British Geological Survey, National Geoscience Information Service	:)		185664



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
45	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Lily'S Kitchen 6, Rosslyn Mews, London, NW3 1NN Pet Foods & Animal Feeds Inactive Automatically positioned to the address	A13SW (SW)	83	-	526769 185611
45	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Bang & Olufsen 44, Rosslyn Hill, London, NW3 1NH Electrical Goods Sales, Manufacturers & Wholesalers Inactive Automatically positioned to the address	A13SW (SW)	93	-	526764 185598
45	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Cleaning Services Hampstead 58a, Rosslyn Hill, London, NW3 1ND Carpet, Curtain & Upholstery Cleaners Inactive Automatically positioned to the address	A13SW (W)	126	-	526723 185614
45	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Farrow & Ball Ltd 58, Rosslyn Hill, London, NW3 1ND Wallpapers & Wall Coverings Active Automatically positioned to the address	A13SW (W)	126	-	526723 185614
46	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Interior Couture 14a, Downshire Hill, LONDON, NW3 1NR Wallpapers & Wall Coverings Inactive Automatically positioned to the address	A13NE (NE)	98	-	526950 185723
47	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Fast Cash 4 Scrap Cars London Aeg 64, Rosslyn Hill, London, NW3 1ND Car Breakers & Dismantlers Inactive Automatically positioned to the address	A13SW (W)	140	-	526708 185619
47	Contemporary Trad Name: Location: Classification: Status:		A13SW (W)	159	-	526685 185626
48	Contemporary Trad Name: Location: Classification: Status:		A13SW (SW)	150	-	526714 185571
49	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Tenancy Cleaners London 4, Shepherds Walk, London, NW3 5UE Cleaning Services - Domestic Inactive Automatically positioned to the address	A13SW (SW)	164	-	526744 185512
50	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Radici Plastics Uk 6a, Hampstead High Street, London, NW3 1PR Plaster Manufacturers & Suppliers Inactive Automatically positioned to the address	A13SW (W)	213	-	526626 185654
50	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Cleaners Hampstead 8, Hampstead High Street, London, NW3 1PR Cleaning Services - Domestic Inactive Automatically positioned to the address	A13SW (W)	224	-	526614 185656
51	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Belsize Park Carpet Cleaners 12 Gayton Crescent, Camden, London, NW3 1TT Carpet, Curtain & Upholstery Cleaners Active Automatically positioned to the address	A13NW (NW)	214	-	526693 185837



Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
51	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Skipwith Consulting 37, Willow Road, London, NW3 1TN Commercial Cleaning Services Inactive Automatically positioned to the address	A13NW (NW)	215	-	526726 185866
52	Contemporary Trad Name: Location: Classification: Status:		A13NE (NE)	219	-	527034 185812
53	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Cleaners Of Hampstead 15, Hampstead High Street, London, NW3 1PX Cleaning Services - Domestic Inactive Automatically positioned to the address	A13NW (W)	265	-	526573 185667
53	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Cleaners Of Hampstead 15, Hampstead High Street, London, NW3 1PX Cleaning Services - Domestic Inactive Automatically positioned to the address	A13NW (W)	265	-	526573 185667
54	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Bri-Clean Laundries 57, South End Road, London, NW3 2QB Laundries & Launderettes Inactive Automatically positioned to the address	A13NE (E)	319	-	527188 185678
54	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Padma Davu House, 2b, Heath Hurst Road, LONDON, NW3 2RX Textile Manufacturing Inactive Automatically positioned to the address	A14SW (E)	336	-	527204 185637
55	Contemporary Trad Name: Location: Classification: Status:		A8NW (S)	366	-	526829 185274
56	Contemporary Trad Name: Location: Classification: Status:		A12NE (W)	366	-	526475 185717
56	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Xyz 10, Flask Walk, London, NW3 1HE Ceramic Manufacturers, Supplies & Services Inactive Manually positioned to the address or location	A12NE (W)	401	-	526445 185756
57	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries American Dry Cleaning 29, South End Road, London, NW3 2PT Dry Cleaners Active Automatically positioned to the address	A14SW (E)	376	-	527235 185581
57	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries House Of Mistry 15, South End Road, LONDON, NW3 2PT Pharmaceutical Manufacturers & Distributors Inactive Automatically positioned to the address	A14SW (E)	400	-	527251 185547
57	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Bevan Scaffolding 14, SOUTH END ROAD, LONDON, NW3 2QE Scaffolding & Work Platforms Active Automatically positioned to the address	A14SW (E)	418	-	527275 185569



Map ID	Details			Estimated Distance From Site	Contact	NGR
108	Contemporary Trad Name: Location: Classification: Status: Positional Accuracy:	e Directory Entries Executive Clean 148, FINCHLEY ROAD, LONDON, NW3 5HS Dry Cleaners Active Automatically positioned to the address	A7SE (SW)	995	-	526261 184836
109	Fuel Station Entries Name: Location: Brand: Premises Type: Status: Positional Accuracy:	Belsize Park Service Station 215, Haverstock Hill , Belsize Park , London, Inner London, NW3 4QE BP Petrol Station Open Automatically positioned to the address	A8NE (SE)	536	-	527188 185227
110	Name: Location: Category: Class Code:	Commercial Services A V Auto Locksmiths 38 Willow Road, London, NW3 1TN Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A13NW (NW)	216	7	526722 185864
111	Name: Location: Category: Class Code:	Commercial Services Car Wash Belzier Park Service Station 215, Haverstock Hill, London, NW3 4QE Personal, Consumer and other Services Vehicle Cleaning Services Positioned to address or location	A8NE (SE)	535	7	527187 185227
111	Name: Location: Category: Class Code:	Commercial Services B P Car Wash 215 Haverstock Hill, London, NW3 4QE Personal, Consumer and other Services Vehicle Cleaning Services Positioned to address or location	A8NE (SE)	536	7	527188 185227
112	Name: Location: Category: Class Code:	Commercial Services Targus Seatrade 201 Haverstock Hill, London, NW3 4QG Transport, Storage and Delivery Distribution and Haulage Positioned to address or location	A9NW (SE)	671	7	527272 185121
113	Name: Location: Category: Class Code:	Commercial Services Comac Motors 13 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	707	7	526773 184937
113	Name: Location: Category: Class Code:	Commercial Services Continental Autos 10 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	730	7	526749 184917
113	Name: Location: Category: Class Code:	Commercial Services Continental Autos 10 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	730	7	526749 184917
113	Name: Location: Category: Class Code:	Commercial Services Comac Motors 19 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	733	7	526770 184911
113	Name: Location: Category: Class Code:	Commercial Services Daleham Garage 14 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	752	7	526749 184894
113	Name: Location: Category: Class Code:	Commercial Services Daleham Garage 14 Daleham Mews, London, NW3 5DB Repair and Servicing Vehicle Repair, Testing and Servicing Positioned to address or location	A8SW (S)	752	7	526749 184894



Map ID	Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
125	Points of Interest - Manufacturing and Production Name: Shaft Location: NW3 Category: Extractive Industries Class Code: Unspecified Quarries Or Mines Positional Accuracy: Positioned to an adjacent address or location	A8NW (S)	628	7	526752 185019
126	Points of Interest - Manufacturing and Production Name: Zarka Marble Ltd Location: 43 Belsize Lane, London, NW3 5AU Category: Extractive Industries Class Code: Stone Quarrying and Preparation Positional Accuracy: Positioned to address or location	A8SE (S)	723	7	526861 184917
126	Points of Interest - Manufacturing and Production Name: Zarka Marble Ltd Location: 43 Belsize Lane, London, NW3 5AU Category: Extractive Industries Class Code: Stone Quarrying and Preparation Positional Accuracy: Positioned to address or location	A8SE (S)	723	7	526861 184917
127	Points of Interest - Manufacturing and Production Name: Air Shaft Location: NW3 Category: Extractive Industries Class Code: Unspecified Quarries Or Mines Positional Accuracy: Positioned to an adjacent address or location	A9NW (SE)	724	7	527482 185282
128	Points of Interest - Manufacturing and Production Name: Air Shaft Location: NW3 Category: Extractive Industries Class Code: Unspecified Quarries Or Mines Positional Accuracy: Positioned to an adjacent address or location	A7NE (SW)	747	7	526472 184994
129	Points of Interest - Manufacturing and Production Name: Air Shaft Location: NW3 Category: Extractive Industries Class Code: Unspecified Quarries Or Mines Positional Accuracy: Positioned to an adjacent address or location	A9NE (SE)	942	7	527732 185289
130	Points of Interest - Manufacturing and Production Name: Sand Pit Location: NW3 Category: Extractive Industries Class Code: Sand, Gravel and Clay Extraction and Merchants Positional Accuracy: Positioned to an adjacent address or location	A14NE (E)	980	7	527840 185798
131	Points of Interest - Public Infrastructure Name: Metropolitan Police Service Hampstead Location: Hampstead Police Station 26, Rosslyn Hill, London, NW3 1PD Category: Central and Local Government Class Code: Police Stations Positional Accuracy: Positioned to address or location	A13SE (S)	102	7	526866 185540
131	Points of Interest - Public Infrastructure Name: Hampstead Police Station Location: Hampstead Police Station 26, Rosslyn Hill, London, NW3 1PD Category: Central and Local Government Class Code: Police Stations Positional Accuracy: Positioned to address or location	A13SE (S)	108	7	526883 185539
132	Points of Interest - Public Infrastructure Name: Hampstead Heath Rail Station Location: South End Road, NW3 Category: Public Transport, Stations and Infrastructure Class Code: Railway Stations, Junctions and Halts Positional Accuracy: Positioned to address or location	A14SW (E)	382	7	527250 185634
132	Points of Interest - Public Infrastructure Name: Hampstead Heath Station Location: South End Road, NW3 Category: Public Transport, Stations and Infrastructure Class Code: Railway Stations, Junctions and Halts Positional Accuracy: Positioned to address or location	A14SW (E)	382	7	527250 185634
133	Points of Interest - Public Infrastructure Name: Sluice Location: NW3 Category: Water Class Code: Weirs, Sluices and Dams Positional Accuracy: Positioned to an adjacent address or location	A14NW (NE)	486	7	527231 185992



Map ID	Details			Estimated Distance From Site	Contact	NGR
	Points of Interest -	Recreational and Environmental				
139	Name: Location: Category: Class Code: Positional Accuracy:	Play Area NW3 Recreational Playgrounds Positioned to an adjacent address or location	A13NE (NE)	287	7	527055 185886
140	Name: Location: Category: Class Code:	Recreational and Environmental Playground Not Supplied Recreational Playgrounds Positioned to an adjacent address or location	A14SW (E)	486	7	527351 185607
140	Name: Location: Category: Class Code:	Recreational and Environmental Playground St Crispins Close, NW3 Recreational Playgrounds Positioned to an adjacent address or location	A14SW (E)	486	7	527351 185608
141	Name: Location: Category: Class Code:	Recreational and Environmental Play Area NW3 Recreational Playgrounds Positioned to an adjacent address or location	A18SW (N)	627	7	526752 186307
142	Name: Location: Category: Class Code:	Recreational and Environmental Play Area NW3 Recreational Playgrounds Positioned to an adjacent address or location	A14SE (E)	827	7	527635 185355
	Points of Interest -	Recreational and Environmental				
143	Name: Location: Category: Class Code: Positional Accuracy:	Playground Not Supplied Recreational Playgrounds Positioned to an adjacent address or location	A14NE (E)	983	7	527841 185818
143	Name: Location: Category: Class Code:	Recreational and Environmental Playground Savernake Road, NW3 Recreational Playgrounds Positioned to address or location	A14NE (E)	983	7	527840 185823
144	Underground Electr Unique Feature Identifier: Cable Status: Cable Type: Record Last Updated:	rical Cables 10006073 Electrically Decommissioned Alternating Current 27th October 2017	A13SW (SW)	96	8	526791 185562
	Underground Electr	ical Cables				
145	Unique Feature Identifier: Cable Status: Cable Type: Record Last Updated:	10005912 Electrically Decommissioned Alternating Current 27th October 2017	A13SW (SW)	96	8	526791 185562
	Underground Electr	rical Cables				
146	Unique Feature Identifier: Cable Status: Cable Type: Record Last Updated:	10005913 Electrically Decommissioned Alternating Current 27th October 2017	A13SW (SW)	140	8	526711 185598
	Underground Electr	ical Cables				
147	Unique Feature Identifier: Cable Status: Cable Type: Record Last Updated:	10006070 Electrically Decommissioned Alternating Current 27th October 2017	A13SW (SW)	140	8	526711 185599



Sensitive Land Use

Map ID		Details		Estimated Distance From Site	Contact	NGR
	Local Nature Rese	rves				
158	Name: Multiple Area: Area (m2): Source: Designation Date:	Belsize Wood N 2723 Natural England 1st October 2004	A9NW (SE)	715	9	527487 185309



A selection of organisations who provide data within this report

Data Supplier	Data Supplier Logo
Ordnance Survey	Map data
Environment Agency	Environment Agency
Scottish Environment Protection Agency	SEP PAPE Scottish Environment Protection Agency
The Coal Authority	The Coal Authority
British Geological Survey	British Geological Survey
Centre for Ecology and Hydrology	Centre for Ecology & Hydrology NATURAL ENVIRONMENT RESEARCH COUNCIL
Natural Resources Wales	Cyfoeth Naturiol Cymru Natural Resources Wales
Scottish Natural Heritage	SCOTTISH NATURAL HERITAGE
Natural England	NATURAL ENGLAND
Public Health England	Public Health England
Ove Arup	ARUP
Stantec UK Ltd	Stantec

Useful Contacts

Contact	Name and Address	Contact Details	
1	British Geological Survey - Enquiry Service British Geological Survey, Environmental Science Centre, Keyworth, Nottingham, Nottinghamshire, NG12 5GG	Telephone: 0115 936 3143 Fax: 0115 936 3276 Email: enquiries@bgs.ac.uk Website: www.bgs.ac.uk	
2	Environment Agency - National Customer Contact Centre (NCCC)	Telephone: 03708 506 506 Email: enquiries@environment-agency.gov.uk	
	PO Box 544, Templeborough, Rotherham, S60 1BY		
3	London Borough of Camden - Pollution Projects Team Seventh Floor, Town Hall Extension, Argyle Street, London, WC1H 8EQ	Telephone: 020 7278 4444 Fax: 020 7860 5713 Website: www.camden.gov.uk	
4	Environment Agency - Head Office Rio House, Waterside Drive, Aztec West, Almondsbury, Bristol, Avon, BS32 4UD	Telephone: 01454 624400 Fax: 01454 624409	
5	Ordnance Survey Adanac Drive, Southampton, Hampshire, SO16 0AS	Telephone: 03456 05 05 05 Email: customerservices@ordnancesurvey.co.uk Website: www.ordnancesurvey.gov.uk	
6	London Borough of Camden Town Hall, Judd Street, London, WC1H 9JE	Telephone: 020 7974 4444 Fax: 020 7974 6866 Email: info@camden.gov.uk Website: www.camden.gov.uk	
7	PointX 7 Abbey Court, Eagle Way, Sowton, Exeter, Devon, EX2 7HY	Website: www.pointx.co.uk	
8	Landmark Information Group Limited Imperium, Imperial Way, Reading, Berkshire, RG2 0TD	Telephone: 0844 844 9966 Fax: 0844 844 9951 Email: helpdesk@landmark.co.uk Website: www.landmark.co.uk	
9	Natural England County Hall, Spetchley Road, Worcester, WR5 2NP	Telephone: 0300 060 3900 Email: enquiries@naturalengland.org.uk Website: www.naturalengland.org.uk	
-	Public Health England - Radon Survey, Centre for Radiation, Chemical and Environmental Hazards Chilton, Didcot, Oxfordshire, OX11 0RQ	Telephone: 01235 822622 Fax: 01235 833891 Email: radon@phe.gov.uk Website: www.ukradon.org	
-	Landmark Information Group Limited Imperium, Imperial Way, Reading, Berkshire, RG2 0TD	Telephone: 0844 844 9952 Fax: 0844 844 9951 Email: customerservices@landmarkinfo.co.uk Website: www.landmarkinfo.co.uk	

Please note that the Environment Agency / Natural Resources Wales / SEPA have a charging policy in place for enquiries.

GEA

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Geology 1:50,000 Maps Legends

Artificial Ground and Landslip

Map Colour	Lex Code	Rock Name	Rock Type	Min and Max Age
	WGR	Worked Ground (Undivided)	Void	Not Supplied - Holocene

Superficial Geology

Map Colour	Lex Code	Rock Name	Rock Type	Min and Max Age
	DHGR	Dollis Hill Gravel Member	Sand and Gravel	Not Supplied - Cromerian
	STGR	Stanmore Gravel Formation	Sand and Gravel	Not Supplied - Pleistocene

Bedrock and Faults

Map Colour	Lex Code	Rock Name	Rock Type	Min and Max Age
	CLGB	Claygate Member	Clay, Silt and Sand	Not Supplied - Ypresian
	LC	London Clay Formation	Clay, Silt and Sand	Not Supplied - Ypresian
	BGS	Bagshot Formation	Sand	Not Supplied - Ypresian



Geology 1:50,000 Maps

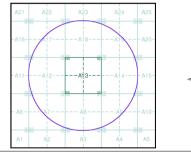
This report contains geological map extracts taken from the BGS Digital Geological map of Great Britain at 1:50,000 scale and is designed for users carrying out preliminary site assessments who require geological maps for the area around the site. This mapping may be more up to date than previously published paper maps.

The various geological layers - artificial and landslip deposits, superficial geology and solid (bedrock) geology are displayed in separate maps, but superimposed on the final 'Combined Surface Geology' may All map legends feature on this page. Not all layers have complete nationwide coverage, so availability of data for relevant map sheets is indicated below.

Geology 1:50,000 Maps Coverage Map ID:

Map ID:	1
Map Sheet No:	256
Map Name:	North London
Map Date:	2006
Bedrock Geology:	Available
Superficial Geology:	Available
Artificial Geology:	Available
Faults:	Not Supplied
Landslip:	Available
Rock Segments:	Not Supplied

Geology 1:50,000 Maps - Slice A



Order Details:

Order Number: Customer Reference: National Grid Reference: Slice: А Site Area (Ha): Search Buffer (m): 0.08 1000

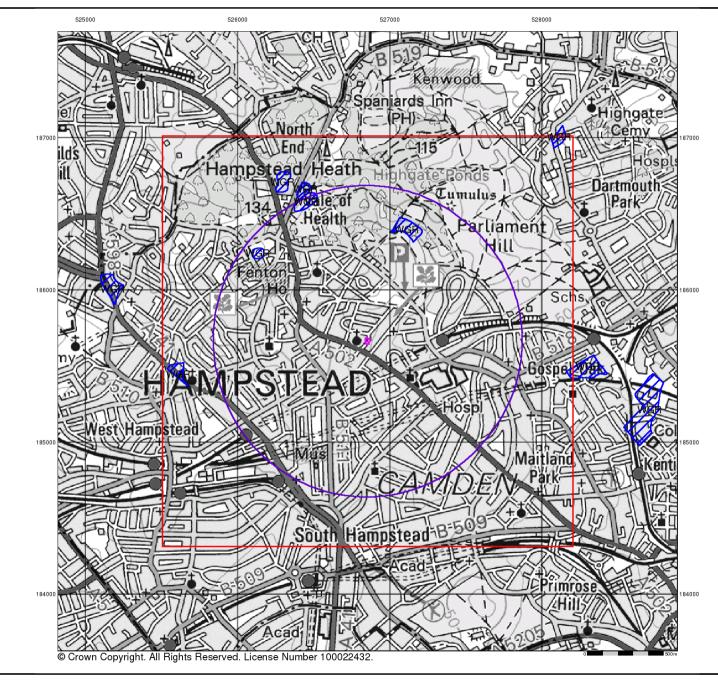
287612059 1 1 J21282 526850, 185660

Site Details: 12, Pilgrims Lane, LONDON, NW3 1SN





Page 1 of 5





Artificial Ground and Landslip

Artificial ground is a term used by BGS for those areas where the ground surface has been significantly modified by human activity. Information about previously developed ground is especially important, as it is often associated with potentially contaminated material, unpredictable engineering conditions and unstable ground.

Artificial ground includes:

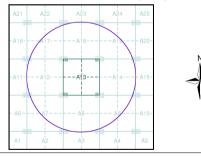
Made ground - man-made deposits such as embankments and spoil heaps on the natural ground surface.
Worked ground - areas where the ground has been cut away such as

quarries and road cuttings. - Infilled ground - areas where the ground has been cut away then wholly or partially backfilled.

 Landscaped ground - areas where the surface has been reshaped.
 Disturbed ground - areas of ill-defined shallow or near surface mineral workings where it is impracticable to map made and worked ground separately.

Mass movement (landslip) deposits on BGS geological maps are primarily superficial deposits that have moved down slope under gravity to form landslips. These affect bedrock, other superficial deposits and artificial ground. The dataset also includes foundered strata, where the ground has collapsed due to subsidence.

Artificial Ground and Landslip Map - Slice A



Order Details:

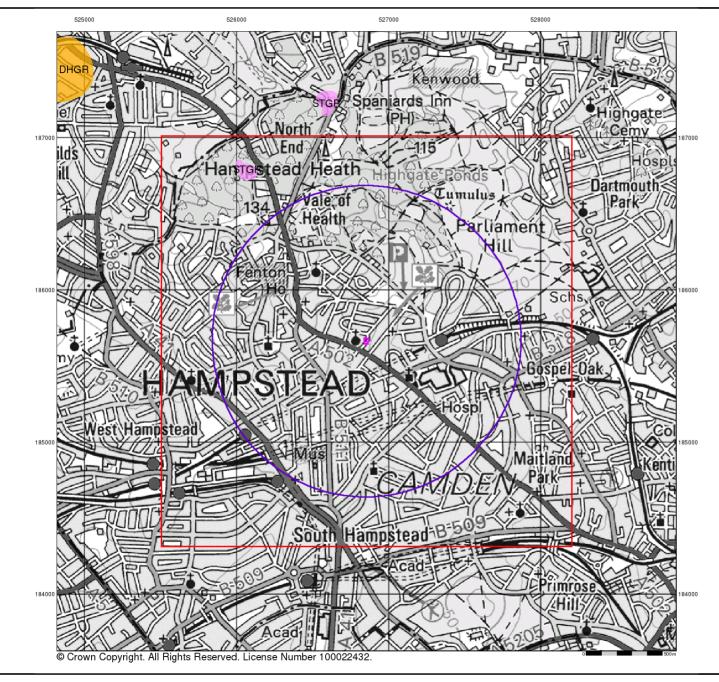
v15.0 11-Nov-2021

Order Number: 28761 Customer Reference: J2128 National Grid Reference: 52685 Slice: A Site Area (Ha): 0.08 Search Buffer (m): 1000

287612059_1_1 J21282 526850, 185660 A 0.08 1000

Site Details: 12, Pilgrims Lane, LONDON, NW3 1SN







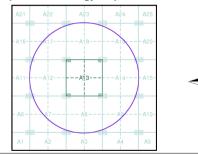
Superficial Geology

Superficial Deposits are the youngest geological deposits formed during the most recent period of geological time, the Quaternary, which extends back about 1.8 million years from the present.

They rest on older deposits or rocks referred to as Bedrock. This dataset contains Superficial deposits that are of natural origin and 'in place'. Other superficial strata may be held in the Mass Movement dataset where they have been moved, or in the Artificial Ground dataset where they are of man-made origin.

Most of these Superficial deposits are unconsolidated sediments such as gravel, sand, silt and clay, and onshore they form relatively thin, often discontinuous patches or larger spreads.

Superficial Geology Map - Slice A



Order Details:

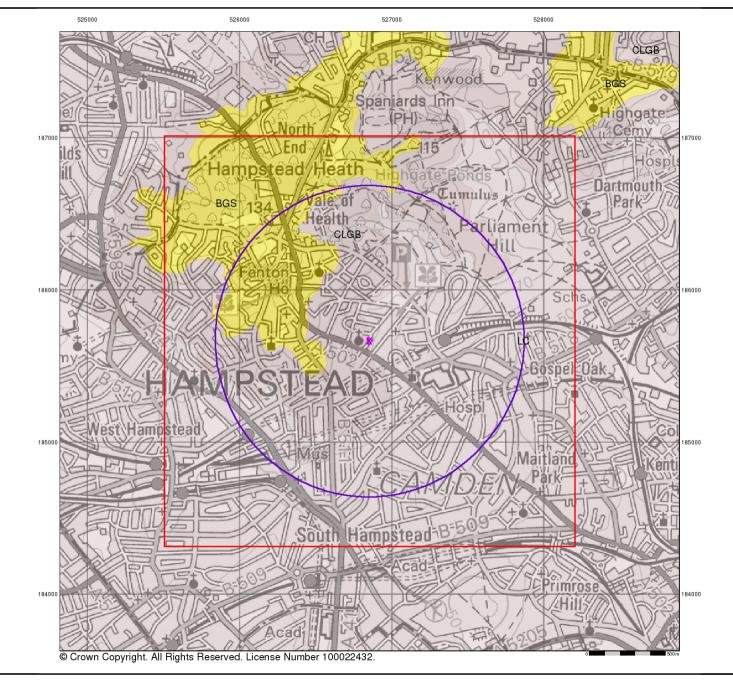
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287612059_1_1 J21282 : 526850, 185660 A 0.08 1000

Site Details: 12, Pilgrims Lane, LONDON, NW3 1SN



v15.0 11-Nov-2021





Bedrock and Faults

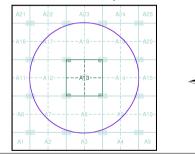
Bedrock geology is a term used for the main mass of rocks forming the Earth and are present everywhere, whether exposed at the surface in outcrops or concealed beneath superficial deposits or water.

The bedrock has formed over vast lengths of geological time ranging from ancient and highly altered rocks of the Proterozoic, some 2500 million years ago, or older, up to the relatively young Pliocene, 1.8 million years ago.

The bedrock geology includes many lithologies, often classified into three types based on origin: igneous, metamorphic and sedimentary.

The BGS Faults and Rock Segments dataset includes geological faults (e.g. normal, thrust), and thin beds mapped as lines (e.g. coal seam, gypsum bed). Some of these are linked to other particular 1:50,000 Geology datasets, for example, coal seams are part of the bedrock sequence, most faults and mineral veins primarily affect the bedrock but cut across the strata and post date its deposition.

Bedrock and Faults Map - Slice A



Order Details:

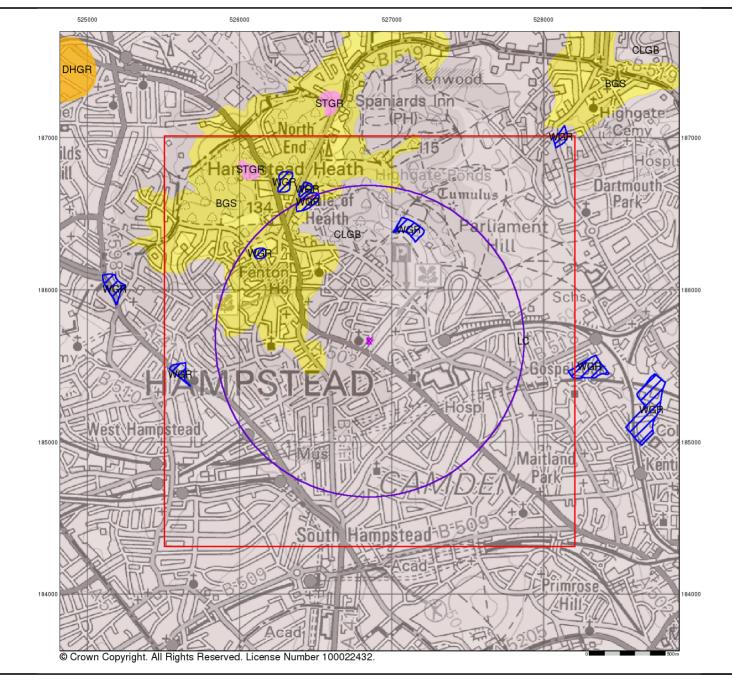
v15.0 11-Nov-2021

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287612059_1_1 J21282 526850, 185660 A 0.08 1000

Site Details: 12, Pilgrims Lane, LONDON, NW3 1SN







Combined Surface Geology

The Combined Surface Geology map combines all the previous maps into one combined geological overview of your site.

Please consult the legends to the previous maps to interpret the Combined "Surface Geology" map.

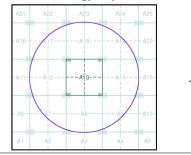
Additional Information

More information on 1:50,000 Geological mapping and explanations of rock dassifications can be found on the BCS website. Using the LEX Codes in this report, further descriptions of rock types can be obtained by interrogating the 'BGS Lexicon of Named Rock Units'. This database can be accessed by following the 'Information and Data' link on the BGS website.

Contact

British Geological Survey Kingsley Dunham Centre Keyworth Nottingham NG12 5GG Telephone: 0115 936 3143 Fax: 0115 936 3276 email: enquiries@bgs.ac.uk website: www.bgs.ac.uk

Combined Geology Map - Slice A



Order Details:

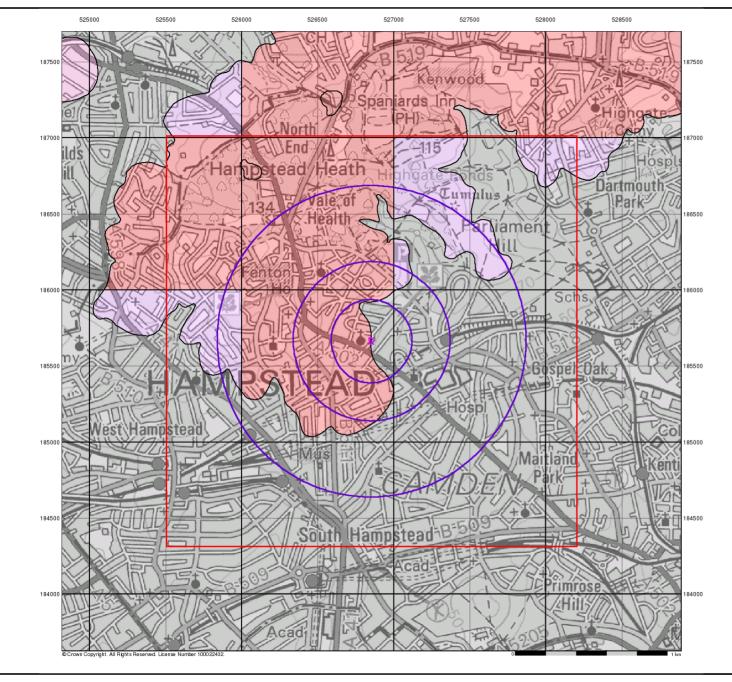
v15.0 11-Nov-2021

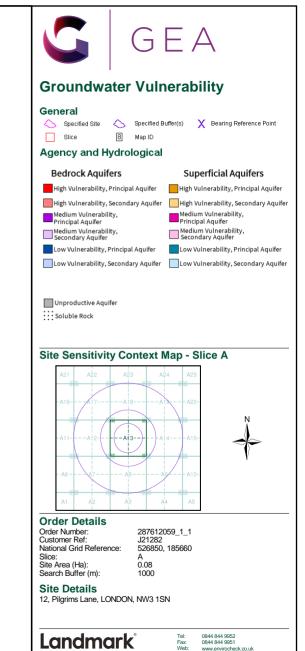
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287612059_1_1 J21282 : 526850, 185660 A 0.08 1000

Site Details: 12, Pilgrims Lane, LONDON, NW3 1SN



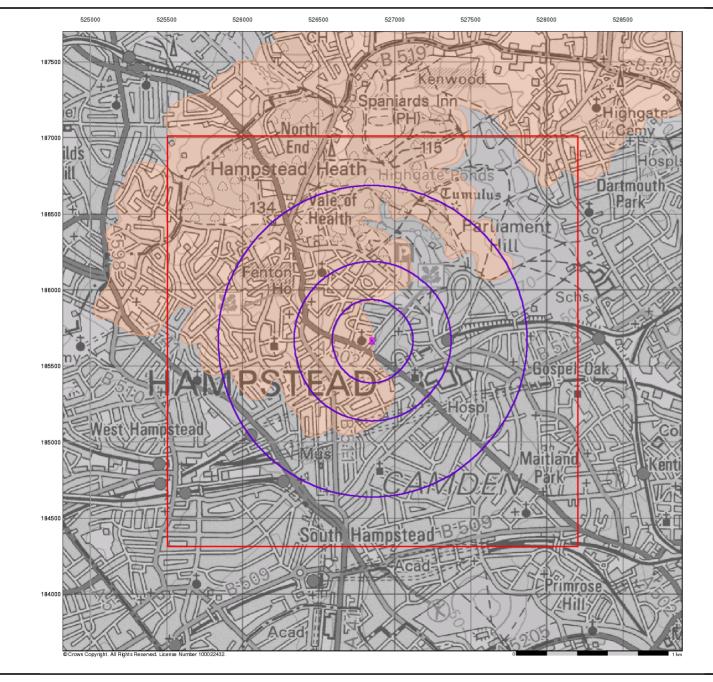


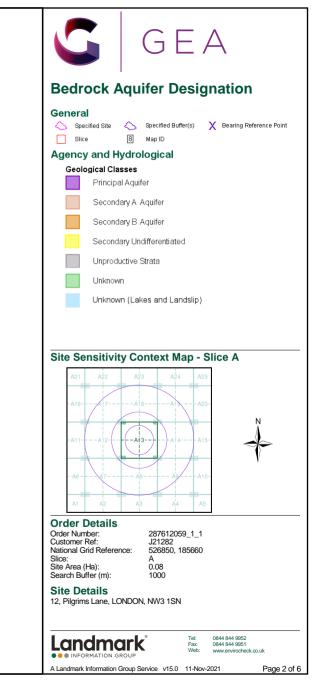


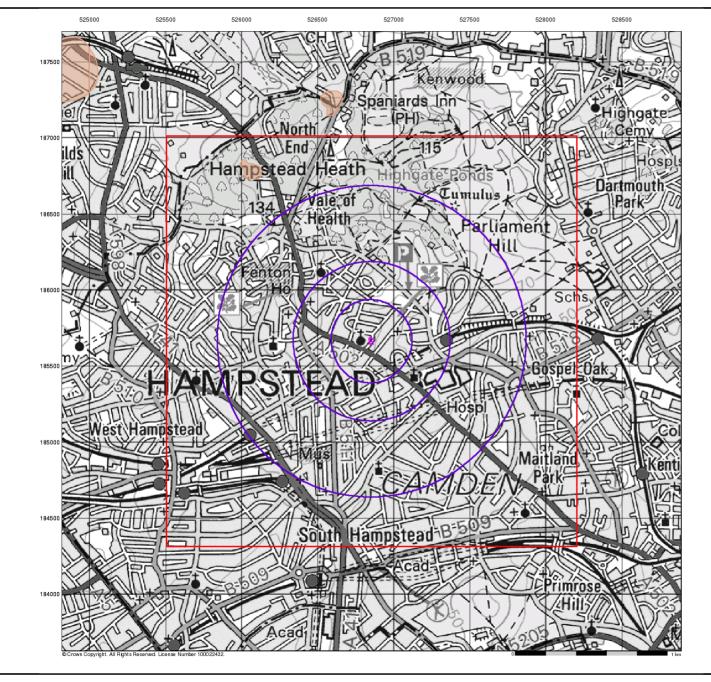
A Landmark Information Group Service v15.0 11-Nov-2021

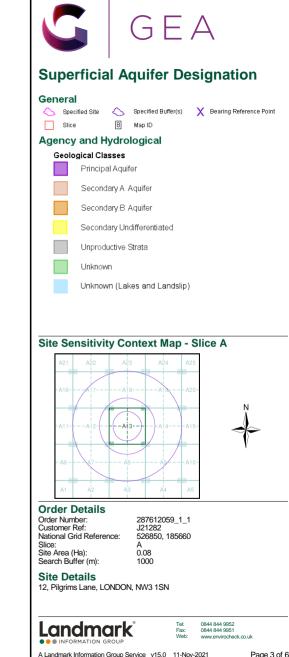
Page 1 of 6

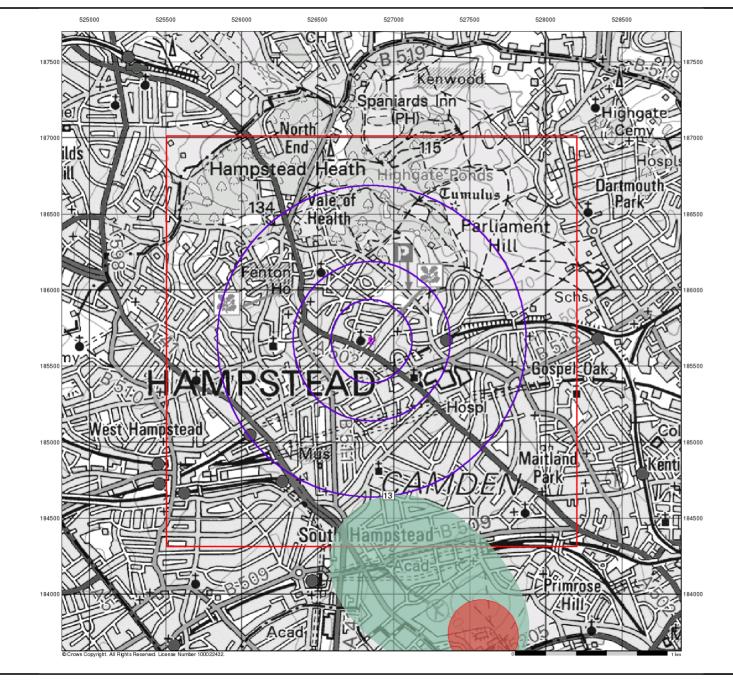
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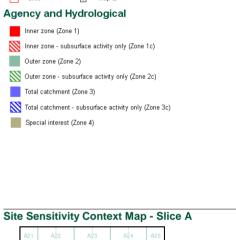


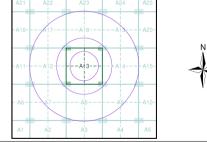












Order Details

Site Details

Landmark

. INFORMATION GROU

Slice:

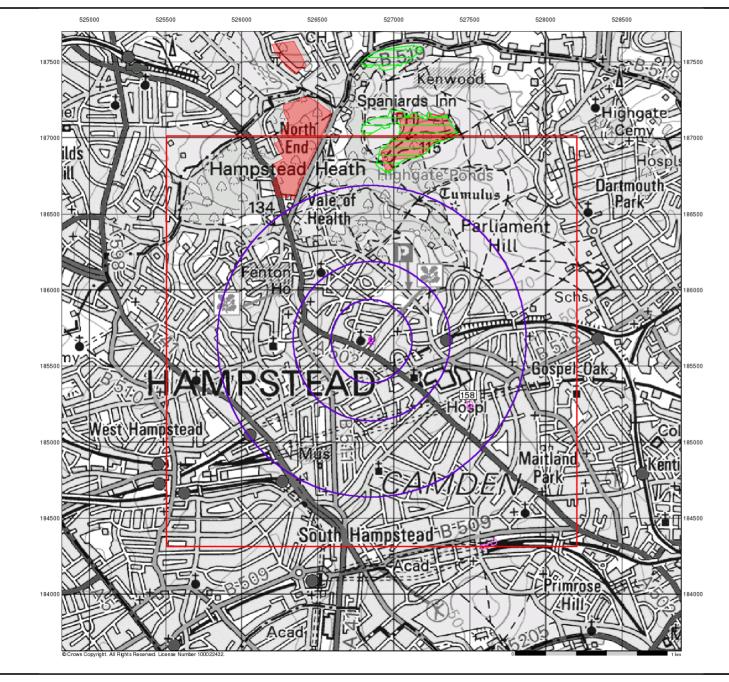
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12, Pilgrims Lane, LONDON, NW3 1SN

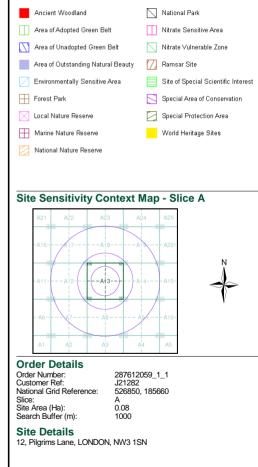
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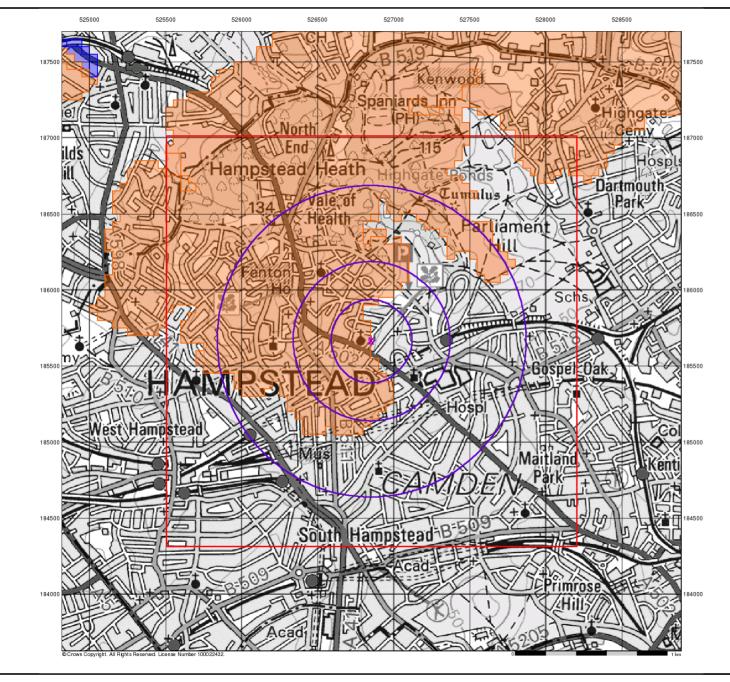


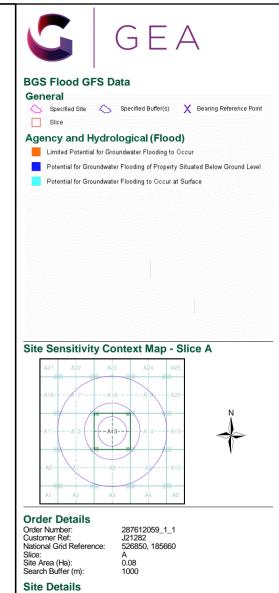






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