ElliottWood

12 Pilgrim's Lane Camden, London, NW3 1SN

Sustainable Drainage Strategy Report

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Executive Summarv 1.

Elliott Wood Partnership Ltd have prepared this Sustainable Drainage Strategy to support the full planning application for the proposed redevelopment of 12 Pilgrim's Lane. The site is located within the London Borough of Camden (LBC) who are also the Lead Local Flood Authority (LLFA).

The existing development is a two-storey semi-detached residential building, The site includes a garden to the south, east, and west of the building. In 2022, a planning application was approved (reference: 2022/2398/P) for the extension and refurbishment of the existing property. This incorporated a new lower ground floor, with a floor area of approximately 205m², which contained a pool, plant room, gym and new bedroom

An updated scheme has now been proposed. Whilst the majority of the approved scheme is being kept, there have been minor amendments to massing on the right and left sides of the building, and a reduction in the footprint of the new lower ground floor. This report has been prepared to support the planning application for this updated scheme,

Surface water runoff from the proposed development will be managed through the use of permeable paving and below ground geo-cellular attenuation, with the peak discharge rate restricted to 2.0 l/s, for all storm events including the 1-in-100-year plus 40% climate change allowance. In order to achieve the proposed discharge rates, an attenuation volume of approximately 11.5 m³ is required. This has been incorporated in the form of a 9.6m² geocellular attenuation tank, and a 54.5m² area of permeable paving, with a 250mm deep lined porous sub-base beneath, with an attenuation volume of 4.1m³.

All foul water drainage from above ground floor will offset at high-level within the building, as designed by the M&E engineer, and drop to the below ground drainage network. All ground floor drainage will be connected to this network. All foul drainage below ground floor level will be positively pumped. discharging to the external below ground drainage network.

Introduction 2.

The purpose of this report is to explain the approach taken with regards to the below ground drainage strategy. It evaluates the selection of SuDS devices and highlights how the drainage disposal hierarchy has been followed.

3. Policy

This report has been prepared in accordance with the National Planning Policy Framework (NPPF) 2023, GOV.UK Sustainable Drainage Systems: Nonstatutory Technical Standards, London Local Plan 2021, and the London Borough of Camden (LBC) Camden Local Plan (Adopted 2017).

The NPPF states the following:

"Plans should take a proactive approach to mitigating and adapting to climate change, taking into account the long-term implications for flood risk, coastal change, water supply, biodiversity and landscapes, and the risk of overheating from rising temperatures."

The London Local Plan 2021, Policy SI 13: Sustainable Drainage, states the followina:

"Development proposals should aim to achieve greenfield run-off rates and ensure that surface water run-off is managed as close to its source as possible. There should also be a preference for green over grey features, in line with the following drainage hierarchy:

> 1) rainwater use as a resource (for example rainwater harvesting, blue roofs for irrigation)

2) rainwater infiltration to ground at or close to source

3) rainwater attenuation in green infrastructure features for gradual release (for example green roofs, rain gardens)

4) rainwater discharge direct to a watercourse (unless not appropriate)

5) controlled rainwater discharge to a surface water sewer or drain

6) controlled rainwater discharge to a combined sewer"

The London Borough of Camden [LBC] Local Plan (Adopted 2017), Policy CC3: Water and Flooding states the following:

"[LBC] will require development to:

- a) incorporates water efficiency measures;
- avoid harm to the water environment and improve water quality; b) considers the impact of development in areas at risk of flooding c)
- (including drainage);
- d) incorporates flood resilient measures in areas prone to flooding;
- utilises Sustainable Drainage Systems (SuDS) in line with the drainage hierarchy to achieve a greenfield run-off rate where feasible; and
- does not locate vulnerable development in flood-prone areas".

4.1

Site Location

The site is located on Pilgrims Lane, within Hampstead Town which is situated within the London Borough of Camden (LBC). The site centred OS grid reference is 526850E: 185679N and the total site boundary is approximately 785m² (0.0785ha). The area surrounding the site consists of residential properties with associated hard and soft landscaping.

The closest stations to the site are Hampstead Underground Station, which is approximately 575m to the west and Hampstead Heath Overground Station which is located 675m to the east. The site is bounded by Pilgrim's Lane to the west and private residential developments to the north, east and south.

The Lead Local Flood Authority (LLFA), responsible for all flood risk matters that do not relate directly to designated Main Rivers is the LBC. The Environment Agency (EA) are responsible for flood risk related to the nearby watercourses. The Statutory Sewerage Undertaker for the area is Thames Water (TW). The site is located within the Hampstead Conservation Area.



4.2 Existing Development

The existing development is a two-storey semi-detached residential building. oriented south to north. The site includes a garden to the south, east, and west of the building.

Existing development floor plans and elevations can be found in **Appendix A**.



Figure 2: Photograph of Existing Development

4.3 **Existing Site Topography**

A Measured Building Survey was undertaken by Target Surveys in September 2021, which can be found in Appendix B.

External levels show that the site is largely flat but with differing topographical features on the west and east sides of the building. The westside falls to the southeast towards the building, with levels starting at 99.90m AOD and falling to 99.70m AOD. The eastern side has a fall to the southeast away from the building, with levels starting at approximately 97.55m AOD and falling to approximately 97.25m AOD at the boundary of the garden.

Underlying Site Geology and Hydrology 4.4

4.4.1 Geology

In accordance with British Geological Survey (BGS) online mapping tool, the underlying ground conditions for the site should consist of bedrock deposits of Claygate member and London clay member, with no superficial deposits recorded. Additionally, site specific ground investigations where undertaken by Geotechnical & Environmental Associates Ltd. (GEA) in September 2021. These investigations encountered ground conditions consistent with those published in BGS records and online mapping tools. The investigations confirmed that beneath a moderate thickness of made ground (between 0.60m and 2.00m), clay was encountered. GEA's site specific borehole logs can be found in Appendix C.

4.4.2 Hydrology

The borehole investigations carried out by Geotechnical & Environmental Associates Ltd. (GEA) in September 2021 state that no groundwater was encountered during the borehole investigations. Standpipes were installed into a selection of the boreholes and the results indicate that the groundwater was not encountered and hence "unlikely to be encountered within basement excavations".

Note that groundwater levels are subject to variations caused by local drainage and seasonal effects.

4.5 Flood Risk Assessment

A site-specific Flood Risk Assessment (FRA) prepared by Elliott Wood Partnership Ltd. has been carried out and is to be read in conjunction with this Sustainable Drainage Strategy report. (Report reference: 2210419-EWP-ZZ-XX-RP-C-00001).

Existing Drainage 5.

Public Sewer

5.1

Thames Water is the principal sewerage provider for the area. A copy of the Thames Water asset maps can be found in **Appendix D**. These show that the area is served by a network of combined sewers.

The asset map indicates a 940mm x 635mm combined water sewer beneath Pilgrim's Lane, running in a north-eastern direction, with a recorded depth of 3.15m from Thames Water manhole 8606 (located adjacent to No. 12). Additionally, a 305mm diameter combined sewer runs beneath Pilgrim's Lane in a south-eastern direction, with a recorded depth of 2.04m from Thames Water manhole 8602 (situated on the southern corner of No. 12).

The development boundary is over 6.0m away from the Thames Water sewer on plan, and as such there is no need for a sewer build near or build over agreement. Confirmation of this has been included in Appendix E.



5.2 **Private Drainage**

located beneath Pilgrim's Lane.

The below-ground CCTV drainage survey can be found in **Appendix F**.

Figure 3: Extract from Thames Water Asset Records – Sewer Map

A below-ground CCTV drainage survey was undertaken by GO Drainage Services Ltd. in June 2022. The CCTV survey shows the existing site to be served by a combined below ground drainage network which discharges via a 150mm diameter vitrified clay outfall to the Thames Water combined sewer

Existing Surface Water Run-off 6. **Rates**

6.1 **Existing Surface Water Run-off Rates**

The surface water run-off rates for the existing site have been calculated using the Modified Rational Method equation below (based on CIRIA C697) and are shown in Table 1:

Q = 2.78C.i.A

Where Q = Existing peak runoff (I/s), C = non-dimensional runoff coefficient=1.0, I = Rainfall intensity (see table 1) and A = total catchment area being drained = 0.033ha

Table 1: Existing Surface Water Run-off Rates

Storm Return Period	Average Rainfall Intensity (mm/hr)	Brownfield Run-off Rate (I/s)
1-in-1-year event	31.74	2.8
1-in-30-year event	79.87	7.1
1-in-100-year event	101.88	9.1
1-in-100-year + 40% climate change event	142.6	12.8

Note that the rainfall intensities used in the above calculations have been based on average rainfall intensities for a 15-minute storm using the Wallingford Procedure. The existing run-off rates have been calculated assuming there are no current flow restrictions on surface water run-off leaving the site.

The calculations of the existing surface water runoff from the brownfield site can be found in **Appendix G.**

Greenfield Run-off Rates 6.2

The London Plan guidance states that developments should aim to achieve greenfield runoff rates. Therefore, the greenfield runoff for the site has been calculated using the HR Wallingford Online Tool.

The results are shown in **Table 2** and are based on a total site area of 0.033ha.

The greenfield run-off rate calculations can be found in Appendix H.

Table 2: Greenfield Run-off Rate Estimations

Storm return period	Greenfield Run-off Rate (I/s)
Qbar	0.44
1-in-1-year event	0.38
1-in-30-year event	1.02
1-in-100-year event	1.41
1-in-200-year event	1.66

Proposed Development 7.

In 2022, a planning application was approved (reference: 2022/2398/P) for the extension and refurbishment of the existing property. This incorporated a new lower ground floor, with a floor area of approximately 205m², which contained a pool, plant room, gym and new bedroom.

An updated scheme has now been proposed. Whilst the majority of the approved scheme is being kept, there have been minor amendments to massing on the right and left sides of the building, and a reduction in the footprint of the new lower ground floor. This report has been prepared to support the planning application for this updated scheme.

The proposed development plans and elevations can be found in **Appendix I**.



Figure 4: Proposed Development (Front Elevation)



Figure 5: Proposed Development (Rear Elevation)

8. **Proposed Surface Water Drainage** Strategy & SuDS

SuDS / Surface Water Drainage Appraisal 8.1

The surface water drainage strategy has been designed in accordance with the requirements of National Planning Policy Framework (NPPF) and the Planning Practice Guidance (PPG), the London Plan, and the COL Local Plan (2015) and Draft City Plan 2024. The following drainage hierarchy has therefore been considered:

- 1.

- 5.
- 6.

8.1.1 Appraising the use of Rainwater Re-Use

It is not proposed to implement rainwater harvesting. The works that would be required to the existing structure and water supply system in order to accommodate a harvesting system are not considered feasible, given the proposed scope of works. Low flow appliances will be installed to reduce the building's demand on potable water.

8.1.2 Appraising the use of Infiltration Techniques

In order to comply with building regulations, infiltration techniques such as soakaways must not be installed within 5m of a building or highway. Due to the nature of the building taking up so much of the site there is not sufficient space for infiltration techniques to be practicable.

The underlying geology comprises clay and silty gravels, with made ground above which does not lend itself to effective infiltration techniques.

Based on the above, infiltration has not been deemed feasible for this site.

Appraising the use of Open Water Features 8.1.3

There is little external area on site available and space for open water features is limited. Open water features are deemed not to be feasible due to the proposed usage of the site.

Appraising the use of Above/Below Ground Attenuation 8.1.4

A blue roof system restricts surface water at the rainwater outlets and provides temporary attenuation at roof level through the use of a layer of geocellular crate. As the proposed development has a pitched roof, there is insufficient flat space throughout to incorporate a blue roof for water attenuation.

There is an opportunity to locate a below ground geocellular attenuation tank beneath the parking area in the northwest of the site. The area and depth available for this tank is limited by tree root protection areas, the building foundations, and the existing drainage outfall depth.

8.1.5 Appraising the use of Permeable Surfacing

The proposed development includes approximately 70m² of open, external hardstanding areas, comprising parking and the building entrance. Permeable paving with a lined porous sub-base will be utilised in this area to provide attenuation storage for run-off from the paved areas. The permeable paving will help control surface water runoff at source, providing attenuation and filtration of runoff in these areas.

Rainwater uses as a resource (for example rainwater harvesting)

2. Rainwater infiltration to ground at or close to source

3. Rainwater attenuation in green infrastructure features for gradual release (for example blue/green roofs, rain gardens)

4. Rainwater discharge direct to a watercourse (unless not appropriate)

Controlled rainwater discharge to a surface water sewer or drain

Controlled rainwater discharge to a combined sewer

8.1.6 Appraising Draining to a Watercourse

There are no nearby accessible water courses, therefore surface water generated from the development will discharge to the local Thames Water sewer network, at a restricted rate.

8.1.7 Appraising Draining to a Public Water Sewer

There is no surface water sewers located in the vicinity of the development, and so it is proposed to discharge surface water at a controlled rate to the combined Thames Water sewer network.

8.2 **Evaluation of SuDS Techniques**

The evaluation of SuDS is demonstrated in Table 3 below.

Table 3: Evaluation of SuDS Techniques

SuDS Techniques	Inclusion	Comments
Rainwater Re-Use	Ν	Rainwater reuse is not proposed for the development, low flow appliances to be installed.
Infiltration Devices	Ν	Soakaways are not deemed feasible for this site due to restricted space on site. The underlying ground conditions are also not conducive to infiltration
Attenuation in Open Water Features	Ν	The confined nature of the development makes open water features unfeasible.
Attenuation in Blue Roofs	Ν	Blue roofs are not proposed for the site as there is insufficient flat roof space.
Attenuation in Green Roofs	Ν	A green roof is not proposed for this development.
Discharge Direct to a Watercourse	Ν	There are no nearby accessible watercourses.
Attenuation with Permeable Surfacing	Y	The proposed development has sufficient external hardstanding area to the front of the building to introduce permeable surfaces.
Attenuation within a Below Ground Tanked System	Y	A buried tank will help achieve suitable water attenuation for the development.

Proposed Surface Water Drainage Strategy 8.3

The development will use a combination of permeable surfacing and a below ground storage tank with vortex flow control to provide sufficient attenuation to achieve the peak surface water discharge rate of 2.0 l/s for all storms up to and including the 1-in-100-year return + 40% climate change allowance.

Table 4 highlights the proposed development areas, showing that the belowground surface water drainage network serves a total of 358 m². This area includes contributions from the building footprint, permeable paving, and an underdrain to the rear of the property at lower ground floor level serving the pedestal paving. This underdrain is pumped to the ground floor level surface water network, where it is then attenuated via a geo-cellular attenuation tank.

Table 4: Proposed Development Area's

	m2	ha
Total Site Boundary Area	782	0.00782
Building Footprint	272	0.00272
Permeable Paved Area	54.6	0.00546
Area Served by Underdrain	31.4	0.0031
Total	358	0.0358

While the greenfield runoff rates are lower for the 1-in-1-year, 1-in-30-year and 1-in-100-year return periods, the area available for attenuation is limited by root protection areas and building foundations, and the increased restriction of the flow control devices would increase the risk of blockage and subsequent flooding.

The permeable surfacing with porous subbase adheres to the CIRIA guidelines, providing improvements to water guality via filtration through the gravels and permeable materials while also increasing the time of entry into the accepting sewer.

In order to achieve the proposed discharge rates, an attenuation volume of approximately 11.5 m³ is required. This has been incorporated in the form of a 9.6m² geocellular attenuation tank, and a 250mm granulation sub-base beneath permeable surfacing with an area of 54.5m² (attenuation volume of 13.6m³).

Refer to Appendix J for MicroDrainage hydraulic modelling calculations.

Proposed Surface Water discharge rates for the full site are presented in

Table 5, alongside a comparison against the existing scheme.

Table 5: Pre and Post Development Surface Water Discharge Rate Comparison

Return Period	Brownfield / Existing Run-off Rate (I/s)	Greenfield Run-off Rate (I/s)	Proposed Run-off Rate (I/s)	Percentage Betterment on Existing Run-off Rate
1-in-1-year event	2.8	0.38	1.7	35.7%
1-in-30-year event	7.1	1.02	2.0	71.8%
1-in-100-year event	9.1	1.41	2.0	78.0%
1-in-100-year + 40% climate change event	12.8		2.0	84.4%

As can be seen in the table above, although it is not possible to achieve greenfield runoff rates a significant betterment can be achieved over the existing runoff rates. The proposed SuDS strategy reduces surface water runoff by 78% in the 1-in-100-year return event.

Refer to Appendix K for the London Borough of Camden SuDS Pro-Forma.

Refer to Appendix L for the proposed below ground drainage layout.

Proposed Surface Water Outfalls 8.4

The CCTV survey illustrates that the existing site is served by a combined below ground drainage network which eventually discharges via a 150mm diameter vitrified clay outfall connection to the Thames Water combined sewer located beneath Pilgrim's Lane. The proposed below-ground drainage works involve removing all existing drainage and replacing it with new systems. However, the existing demarcation manhole and combined water outfall to the Thames Water sewer will be retained, as they are deemed to be in suitable condition for reuse, provided minor remediation works are undertaken.

Proposed Foul Water Strategy 9.

All foul water drainage from above ground floor will offset at high-level within the building, as designed by the M&E engineer, and drop to the below ground drainage network. All ground floor drainage will be connected to this network.

To protect the building from flooding due to sewer surcharge all foul drainage below ground floor level will be positively pumped, discharging to the highlevel suspended gravity network. Pumping stations are to include dual vortex pumps (duty and standby), non-return valves located in an accessible location to protect against public sewer surcharge, alarms, and telemetry.

It is proposed that foul water will outfall from the site via the existing 150mm diameter combined outfall to Pilgrim's Lane. A Pre-planning enquiry has been submitted to Thames Water to confirm the capacity of the downstream sewer network. No issues are foreseen as there is no change in use.

Thames Water Consultation 10.

A pre-planning enquiry has been submitted to Thames Water to agree points of connection (including reuse of existing connections) and proposed flow rates to the sewer network. A response is currently awaited.

The reuse of the existing connections to the public sewer will be subject to a S106 agreement with Thames Water, to be secured by way of a planning condition.

Due to the existence of many combined sewers and water mains within the site vicinity, should extensive substructure or excavation works be required, Thames Water should be consulted to satisfy that their assets will not be compromised as a result of the development works.

Pollution Prevention and Surface 11_ Water Treatment

Pollution control and treatment has been developed in line with the recommendations and guidance within the CIRIA SuDS Manual C753.

The Simple Index Approach has been used to evaluate the site's pollution hazards and proposed mitigation. The Simple Index Approach assigns pollution indices to different land use classifications and mitigation indices to SuDS elements. Depending on whether surface water is being discharged to the ground or to surface waters, the mitigation indices can be used to provide a total mitigation index which is equal to or higher than the pollution hazard index.

Pollution hazard indices for land usage have been evaluated in accordance with the CIRIA SuDS Manual - refer to Table 6. Residential roofs represent a very low pollution hazard level, with individual property driveways representing a low pollution hazard level.

Manual Table 26.2 Annex 5)

Land use	Pollution Hazard Level	Total suspended solids (TSS)	Metals	Hydrocarbons
Residential Roof	Very Low	0.2	0.2	0.05
Individual Property Driveways	Low	0.5	0.4	0.4

Reference is made to Table 26.3 of the SuDS Manual for pollution mitigation indices for permeable pavement and proprietary treatment systems for discharges to surface waters (Table 7).

Manual Annex 5)

SuDS component	Total suspended solids (TSS)	Metals	Hydrocarbons
Permeable Paving	0.7	0.6	0.7

catchment. runoff.

Table 6: Pollution Hazard Indices for Various Land Uses (taken from the SuDS

Table 7: SuDS Mitigation Indices for Permeable Paving (taken from the SuDS

As per the SuDS Manual, provided that all roof water downpipes are sealed against pollutants entering the system from polluted surface runoff, it is deemed acceptable to discharge surface water captured on a residential roof to groundwater, and subsequently, it can be assumed acceptable to discharge to a combined public sewer. Additionally, mitigation indices for the permeable payement are shown to be higher than the pollution indices for its specific

Based on the SuDS mitigation indices for the proposed SuDS features it is considered that adequate treatment is provided on site for all surface water

Operation and Maintenance 12. Requirements

During the construction phase of the development, the responsibility for the maintenance of below ground drainage lies with the developer. Postconstruction, all proposed drainage within the site is to be private (i.e., does not lie on public land) therefore all below ground drainage will be maintained by the site owner for the lifetime of the development.

The drainage system should be regularly inspected and maintained to ensure that is kept clear of silt and debris. Permanent inspection records should be kept by the site owner or management company, recording any previous issues and future work to be carried out.

There are three categories of maintenance activities, as follows:

- a) Regular maintenance (including inspections and monitoring)
- b) Occasional maintenance
- c) Remedial maintenance

Regular maintenance consists of basic tasks done on a frequent and predictable schedule, including vegetation management, litter and debris removal, and inspections.

Occasional maintenance comprises tasks that are likely to be required periodically, but on a much less frequent and predictable basis than the regular tasks (e.g., sediment removal or filter replacement). Error! Reference source not found., Error! Reference source not found., Table 9, and Table 10 summarises the likely maintenance activities required for each SuDS component and guidance on specific maintenance activities is given in the following sections.

Remedial maintenance describes the intermittent tasks that may be required to rectify faults associated with the system, although the likelihood of faults can be minimised by good design, construction, and regular maintenance activities. Where remedial work is found to be necessary, it is likely to be due to site-specific characteristics or unforeseen events, and so timings are difficult to predict. Remedial maintenance can comprise activities such as:

- inlet/outlet repairs
- erosion repairs
- reinstatement or realignment of edgings, barriers, rip-rap or other erosion control
- infiltration surface rehabilitation
- replacement of blocked filter fabrics
- construction stage sediment removal (although this activity should have been undertaken before the start of the maintenance contract)
- system rehabilitation immediately following a pollution event.

It is important to note that these remedial activities will not be required for all systems, but for the purpose of estimating whole life maintenance costs, a contingency sum of 15-20% should be added to the annual regular and occasional maintenance costs to cover the risk of these activities being reauired.

The sections below summarise the proposed schedule of maintenance for below ground drainage elements.

Permeable Paving 12.1

Regular inspection and maintenance are important for the effective operation of pervious pavements. Maintenance responsibility for a pervious pavement and its surrounding area should be placed with an appropriate responsible organisation. The facility should be inspected regularly, preferably during and after heavy rainfall to check effective operation and to identify any areas of ponding.

Pervious surfaces need to be regularly cleaned of silt and other sediments to preserve their infiltration capability. Experience in the UK is limited, but advice issued with permeable precast concrete paying has suggested a minimum of three surface sweepings per year. Manufacturers' recommendations should always be followed.

A brush and suction cleaner, which can be a lorry-mounted device or a smaller precinct sweeper, should be used and the sweeping regime should be as follows:

- End of winter (April) to collect winter debris.
- Mid-summer (July/August) to collect dust, flower, and grass-type deposits.
- After autumn leaf fall (November).

Care should be taken in adjusting vacuuming equipment to avoid removal of iointing material. Any lost material should be replaced.

Operation and maintenance requirements for permeable paving are described below:

Schedule	Required Action	Frequency
Regular Maintenance	Brushing and vacuuming.	Three times/year at end of winter, mid-summer, after autumn leaf fall, or as required based on site-specific observations of clogging or manufacturers' recommendations.
Occasional maintenance	Stabilise and mow contributing and adjacent areas.	As required.
	Removal of weed.	As required.
	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50 mm of the level of the paving.	As required.
Remedial actions	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users.	As required.
	Rehabilitation of surface and upper sub-structure.	As required (if infiltration performance is reduced as a result of significant clogging).
	Initial inspection.	Monthly for three months after installation
Monitoring	Inspect for evidence of poor operation and/or weed growth. If required take remedial action.	3-monthly, 48 h after large storms.
monitoring	Inspect silt accumulation rates and establish appropriate brushing frequencies.	Annually.
	Monitor inspection chambers.	Annually.

12.2 **Geocellular Attenuation Systems**

Regular inspection and maintenance are required to ensure the effective longterm operation of attenuation storage systems. Maintenance responsibility for systems should be placed with a responsible organisation. Maintenance requirements for attenuation systems are described in Table 9 below. Maintenance plans and schedules should be developed during the design phase. Specific maintenance needs of the system should be monitored, and maintenance schedules adjusted to suit requirements.

Table 9: Attenuation Systems Maintenance Schedule

Maintenance Schedule	Required Actions	Frequency
	Inspect and identify any areas that are not operating correctly. If required, take remedial action.	Monthly for 3 months, then six monthly
Regular maintenance	Debris removal from catchment surface (where may cause risks to performance)	Monthly
	Remove sediment from pre- treatment structures	Annually, or as required
Remedial actions	Repair/rehabilitation of inlets, outlet, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually and after large storms

Drainage Pipes, Manholes and Silt Traps 12.3

Maintenance requirements of general drainage items is listed in Table 10.

Table 10: Drainage Pipes, Manholes and Silt Traps Maintenance Schedule

Maintenance Schedule	Required Actions	Frequency
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action.	Monthly for 3 months, then six monthly
	Debris removal from catchment surface and chambers (where may cause risks to performance)	Monthly
	Inspection of manholes, silt traps and catchpits to assess silt accumulation	Monthly (and after large storms)
	Removal of accumulated silt from silt trap and catchpit sumps	Annually, or as required
Remedial actions	Cleaning/jetting of pipework if silt/debris is building up in system	As required
		As required
Monitoring	Inspect/check all inlets, outlets, and overflows to ensure that they are in good condition and operating as designed.	Annually and after large storms

NOTE: Manhole covers can be heavy and suitable lifting equipment / procedures should be used. Where possible, personnel should not enter manholes to carry out maintenance.

12.4 **Gullies / Linear Channels**

Inspection and removal of debris from silt trap every 3 months, preferably after leaf fall in the autumn. (Timeframe can be adjusted to suit actual site conditions.) Charge water trap where necessary.

12.5 Pumping Stations

Pumping stations are to be maintained in accordance with the pump supplier/maintenance company requirements and in accordance with British Standards (BS EN 12056-4), i.e. inspections every guarter.

13. Conclusion

Elliott Wood Partnership Ltd have prepared this Sustainable Drainage Strategy to support the full planning application for the proposed redevelopment of 12 Pilgrim's Lane. The site is located within the London Borough of Camden (LBC) who are also the Lead Local Flood Authority (LLFA).

The SuDS Hierarchy has been followed in order to employ the most suitable and practicable SuDS techniques to improve surface water management across the site. The development will provide benefits to surface water runoff and water quality, through the proposal of permeable paving and below ground attenuation.

Surface water runoff from the proposed development will be managed through the use of permeable paying and below ground geo-cellular attenuation, with the peak discharge rate restricted to 2.0 l/s, for all storm events including the 1-in-100-year plus 40% climate change allowance. In order to achieve the proposed discharge rates, an attenuation volume of approximately 11.5 m³ is required. This has been incorporated in the form of a 9.6m² geocellular attenuation tank, and a 54.5m² area of permeable paving, with a 250mm deep lined porous sub-base beneath, with an attenuation volume of 4.1m³.

All foul water drainage from above ground floor will offset at high-level within the building, as designed by the M&E engineer, and drop to the below ground drainage network. All ground floor drainage will be connected to this network. All foul drainage below ground floor level will be positively pumped, discharging to the external below ground drainage network.

ElliottWood

Appendices

Appendix A

Existing Development Floor Plans & Elevations

Α



Existing Site Plan





PLANNING

scale: **1 : 100**

rev no:

^{project:} 12 Pilgrim's Lane Hampstead, London

Existing Site Plan

status:

drawing title:

date:

02/02/22

dwg no: **2160-PL-010**





Existing Lower Ground Floor Plan



0 First Issue 25.05.22 Rev Description Date Drwn Ckd



16 lambton place notting hill london w112sh t 02072293125 f 02072293257 e info@wolffarchitects.co.uk

PLANNING

project: 12 Pilgrim's Lane Hampstead, London

drawing title:

status:

Existing Lower Ground Floor Plan

 date:
 scale:

 11/18/21
 1 : 100

 dwg no:
 rev no:

 2160-PL-151
 0



Existing Ground Floor Plan



Extent of 12A Pilgrims Lane Demise

Common Area

10 Pilgrim's Lane

0 First Issue Rev De Date Drwn Ckd Description WOLFF ARCHITECTS

16 lambton place notting hill london w112sh

t 02072293125
f 02072293257
e info@wolffarchitects.co.uk

PLANNING

status:

project: 12 Pilgrim's Lane

Hampstead, London

drawing title:

Existing Ground Floor Plan

scale: **1 : 100** date:

01/21/22

rev no: 0

dwg no: **2160-PL-152**



Existing First Floor Plan



Extent of 12A Pilgrims Lane Demise

0 First Issue Rev D Date Drwn Ckd Description

WOLFF ARCHITECTS

16 lambton place notting hill london w112sh

t 02072293125
f 02072293257
e info@wolffarchitects.co.uk

status:

PLANNING

project: 12 Pilgrim's Lane

Hampstead, London

drawing title:

Existing First Floor Plan

date: 01/21/22

rev no:

1 : 100

dwg no: **2160-PL-153**

scale:

0



Existing Roof Plan



10 Pilgrim's Lane





WOLFF ARCHITECTS



16 lambton place notting hill london w112sh

status:

drawing title:

date:

01/21/22

dwg no: **2160-PL-154**

project: 12 Pilgrim's Lane

Hampstead, London

Existing Roof Plan

t 02072293125
f 02072293257
e info@wolffarchitects.co.uk

PLANNING

scale: **1 : 100**

0

rev no:

0 First Issue Rev Description



South Elevation - Existing



North Elevation - Existing 1:100











16 lambton place notting hill london w112sh t 02072293125
f 02072293257
e info@wolffarchitects.co.uk

PLANNING

project: **12 Pilgrim's Lane**

status:

Hampstead, London

drawing title:

Existing Front & Rear Elevations

scale: date:

11/19/21

1:100

rev no: 0

dwg no: 2160-PL-160



East Elevation - Existing



West Elevation - Existing 1:100



_	
date: 01/24/22	scale: 1:100
dwg no: 2160-PL-161	rev no:

drawing title: Existing Side Elevations

Hampstead, London

status:

project: 12 Pilgrim's Lane

PLANNING

16 lambton place notting hill london w112sh t 02072293125 f 02072293257 e info@wolffarchitects.co.uk

0 First Issue 25.05.22 Rev Description Date Drwn Ckd







Section AA - Existing





Section CC - Existing

Irawing title:	
Existing Sect	tions 1
ate:	scale:
	4 . 400
01/24/22	1 : 100
01/24/22 wg no:	rev no:

^{project:} 12 Pilgrim's Lane Hampstead, London

status:

PLANNING

16 lambton place notting hill london w112sh t 02072293125 f 02072293257 e info@wolffarchitects.co.uk









Section DD - Existing



Section GG - Existing



Section EE - Existing







Existing sec	tions 2
date: 01/24/22	scale:
dwg no:	rev no:

^{project:} 12 Pilgrim's Lane Hampstead, London

status:

PLANNING

16 lambton place notting hill london w112sh t 02072293125 f 02072293257 e info@wolffarchitects.co.uk





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1 0 1 2 3 4 5 SCALE 1:100 m

Appendix B

Measured Building Survey

В









 \oplus OVERLAY CROSS

BASEMENT FLOOR PLAN

fs 0.89 sh 1.19

(2.63) carpet

103.02

2.63

laminate

fs 0.80 sh 1.30







4.0

<u>Scale = 1 : 50</u>

<u>Datum 96.00m</u>

FRONT ELEVATION



<u>Datum 96.00m</u>

SIDE ELEVATION

OVERHEA SEASONA	עוועא ו E GRID IS CABLES WILL I VARIATION. TH CIES SHOUL A	DASED ON ASSUN BE SHOWN INDICA IESE WILL BE MEA LWAYS BE CONFIE	MED VALUE TIVELY ANI SURED USI MED BY A -	5. DARE SUBJECT TO NG REMOTE METHODS IREE SPECIALIST
SERVICE (ALL INFOR	COVERS WILL BE	E LIFTED AND DETA E TAKEN FROM SU	AIL SHOWN RFACE LEV	WHERE POSSIBLE. 'EL ONLY.
INFORMAT	ION MAY BE OM	ITTED DUE TO OB		IS AT TIME OF SURVEY.
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ffl tl mh ic cl	IEMPORANT BI FINISHED FLOC THRESHOLD LE MANHOLE COV INSPECTION CO COVER LEVEL	ENCH MARK DR LEVEL EVEL ER OVER	g bt catv conc bk	GODLING EYE BRITISH TELECOM CABLE TELEVISION CONCRETE BRICK
ו wl bl ø wfc utl	INVERT LEVEL WATER LEVEL BED LEVEL PIPE DIAMETER WATER FILLED UNABLE TO LI	२ CHAMBER FT	f/b ht tow tof toh ep	FLOWER BED HEIGHT TOP OF WALL TOP OF FENCE TOP OF HEDGE ELECTRICITY POLE
d−c g−ri rwp svp vp	nan DRAINAGE CHA In GULLY RUN RAIN WATER P SOIL VENT PIP VENT PIPE WASTE PIPE	INNEL IPE PE	tp lp ts rs sp	TELEGRAPH POLE LAMP POST TRAFFIC SIGNAL ROAD SIGN SIGN POST NOTICE BOARD
wp cr er sv gv wm	CABLE RISER EARTH ROD STOP VALVE GAS VALVE WATER METER		bol c/b c/l p/r p/w	NOTICE BOARD BOLLARD CLOSEBOARD CHAINLINK POST & RAIL POST & WIRE
MEASURED BL	ILDING SURVEY KE	<u>Y:-</u>	۰/۲ ۶ s	witch
fs sh fh dh	ROOF SLOPE (E FLOOR TO SIL SILL TO HEAD FLOOR TO HE DOOR HEIGHT	DOWN) L AD	√ a Y s Y d	ouble switch ocket ouble socket
u/s t/s s/c rl fp sd	UNDERSIDE TOPSIDE SUSPENDED C ROOFLIGHT FIREPLACE SLIDING DOOR	EILING	- ⊢ ti ⊢ t	nermostat elephone socket
fb cup elec htr hwt	d CUPBOARDS d CUPBOARD ELECTRICS RADIATOR HEATER HOT WATER T/	ANK	L t τv t ⊗ f	v socket use
	STATI	ON COOF		TES
STATION	EASTINGS	NORTHINGS	LEVEL	REMARKS
	+	+		
		DEV/IOIO	NC	
	Date		Remarks	
Revision				
Revision				
Revision				
Revision		rc		
Revision		rq	E	
Revision		rg	SL	arveys
Revision	12 PILGRIM	S LANE, LONDON		arveys





<u>Datum 96.00m</u>





SECTION Α-Α



Appendix C

Borehole Logs

С



12 P Job No J212 Client	ilgrims La	ne, London, NV	V3	1SN					I
Job No J212 Client	וסח	Date 21-00-21		C					
J212 Client	้อว	/ / / / / / / / / / / /		Gro	und Lev	/el (m OD)	Co-Ordinates ()		L
Client	202	21-09-21	-						
		•			En	gineer		Sheet	
						Elliott Wo	bod	1 of	1
SAN	/IPLES & 1	TESTS					STRATA		ent
	Type	Test	ater	Reduced		Depth			ume
Depth	No	Result	≥	Level	Legend	(Thick- ness)	DESCRIPTION		lnstr / Bi
0.25 0.50 0.75 1.00 1.20-1.65 1.85 2.00-2.45 2.75 3.00-3.45 3.75 4.00-4.45 4.75 5.00-5.45	D D D D U100 D U100 D U100 D D U100	2,3/3,3,3,4 N60 = 18 2,3/2,3,3,3 N60 = 15 2,2/2,4,3,4				(1.20)	 PAVING SLABS over MADE GROUND (stif desiccated sandy silty clay, with frequent gravel sized fragments of brick and flint. (roots.) Stiff becoming firm slightly desiccated br clay with frequent partings of orange bro sand and grey silt Firm becoming stiff with depth fissured s silty clay with frequent partings of orange sand and grey silt. Occasional blue-grey v Occasional roots. 3.00 Occasional fine gravel sized selenite 3.00 m 4.00 Sand partings are only occasional ar below 4.00 m 	f brown slightly fine to coarse Occasional own sandy silty wn clayey lightly sandy e brown clayey reining. crystals below	
6.00 6.50-6.95 7.50 8.00-8.45	D U100 D D	2,3/4,4,4,5 N60 = 23				6.30	Stiff becoming very stiff with depth fissur silty clay with occasional partings of brov and silt. Frequent fine crystals (mica) and white shell fragments.	red brown-grey vn-grey sand l occasional	
Boring Depth I All dimensic Scale	Progress Date Ti	and Water Ob Tessin Method/ Plant Used CF	DSE g Dia.	rvation mm De	s ater epth	Hand dug No ground Standpipe	GENERAL REMARKS services pit to 1.20 m water encountered installed to depth of 8.0 m	Logged By	
	Depth 0.25 0.50 0.75 1.00 1.20-1.65 1.85 2.00-2.45 3.75 4.00-4.45 4.75 5.00-5.45 6.00 6.50-6.95 7.50 8.00-8.45 Boring Depth A	Depth Type No 0.25 D 0.50 D 0.75 D 1.00 D 1.20-1.65 D 1.85 D 2.75 D 3.00-3.45 D 3.75 D 4.00-4.45 U100 4.75 D 5.00-5.45 D 6.00 D 6.50-6.95 U100 7.50 D 8.00-8.45 D Depth Date Time Depth Time Date All dimensional sin metric	Depth Type No Test Result 0.25 0.50 0.75 1.00 1.20-1.65 D D D D D 1.20-1.65 D D D D D D D D 1.20-1.65 D D D D D D D D D D D D D D D D D D D	DepthType NoTest Result 5 0.25 0.50 1.00 1.20-1.65D D D 1.20-1.65D D D D 1.20-1.65 $2,3/3,3,3,4$ N60 = 18 $2,3/2,3,3,3,4$ N60 = 151.85 2.00-2.45D U100 $2,3/2,3,3,3,3$ N60 = 15 $1,3,3,4$ N60 = 153.75 4.00-4.45D U100 $2,3/2,3,3,3,3$ N60 = 15 $1,4,1,2,3,3,3,3,3,4$ N60 = 153.75 4.00-4.45D U100 $2,3/2,3,3,3,3,3,4$ N60 = 15 $1,4,1,2,3,3,3,3,4,3,4,3,5,3,3,3,4,3,4,3,5,3,3,3,3$	Depth Type No Test Result Image Reduced Result 0.25 D	Depth Type No Test Result Test Result Test Result Test Level Reduced Level Legend 0.25 D 2,3/3,3,3,4 Image: Comparison of the test of tes	Depth Type No Test Result Test Result Reduced Result Depth Level Depth Level Depth (Thick- ress) 0.25 D	Depth Type Test No Test Result Test Level Test Level Test Level Description 0.25 D D D D D D D 0.30 D D D D D D 0.75 D 2,3/3,3,3,4 D D D 1.80 D 2,3/3,3,3,4 D D D 1.85 D 2,3/2,3,3,3 D D D 2.00-2.45 U100 D D D D 3.00-3.45 D 2,3/2,3,3,3 D D D 3.00-3.45 D 2,3/2,3,3,3 D D D 3.00 D 2,3/2,3,3,3 D D D 3.00-3.45 D 2,3/2,3,3,3 D D D 4.00-4.45 U100 D D D D 4.75 D D 2,2/2,4,3,4 D D D 5.00-5.45 D 2,3/4,4,4,5 D D D D 6.50-6.95 U100 D D D D D 7.50 D D 2,3/4,4,4,5 D D	Depth Type No Test Result Test Result Test Result Test Result Description (120) 0.35 0.50 0.50 0.50 0.50 0.50 0.50 0.50



	Project BOREHOLE N						E N	0				
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	Job No		Date 21-09-21	1	Gr	ound Le	evel (m OD)	Co-Ordinates ()			<u> </u>	
	J212	282	21-09-21	Ĺ								
	Client		•		·	En	ngineer	·		Sheet		
							Elliott We	bod		1 of	1	
	SAN	APLES &	TESTS					STRATA		1	ut	=
		Tuno	Tost	ater	Poduco	4	Depth				a me	ackfi
	Depth	No	Result	Š	Level	Legend	d (Thick- ness)	DESCRIPTION			Instr	/ Ba
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	-1.85 2.00-2.45 2.75	D U100 D	NOU = 9					Firm becoming stiff with depth fissi silty clay with frequent partings of sand and grey silt. Occasional blue- Occasional roots. 2.50 Occasional coarse sand to fine crystals below 2.50 m	ured sorang grey	slightly sandy ge brown clayey veining. rel sized selenite	00000000	
	3.00-3.45 3.75 4.00-4.45	D D U100	1,2/3,4,3,3 N60 = 18					3.50 Sand partings are only occasio below 3.50 m	nal a	nd no roots		
November 2021	4.75 5.00-5.45 6.00	D D D	1,2/2,3,4,4 N60 = 18								00000000	
BRARY.GLB Date: 12	6.50-6.95	U100					(2.15)	Stiff becoming very stiff with depth silty clay with occasional partings o and silt. Frequent fine crystals (mic white shell fragments.	fissu f bro a) an	ired brown-grey wn-grey sand d occasional		
Library: GEA LI	8.00-8.45	D	2,3/3,4,4,6 N60 = 23									
21282 - 12 PILGRIMS LANE.GPJ	Boring	Progress	s and Water Ok	Dse	rvatior	15 Vater		GENERAL				
ID: CABLE PERCUSSION Project: J2	Deptn	Dia.	<u>mm Ľ</u>	<u>Depth</u>	Hand dug No ground Standpipe	services pit to 1.20 m dwater encountered installed to depth of 8.0 m						
Report I	All dimensions in metres Scale 1:62.5Method/ Plant Used CFA / Percussive samplerLogged By AG											



Project									BOREHOL	E No
12 P	ilgrims La	ne, London, N	W3	1SN					рца)
Job No		Date 24-09-2	1	Gr	ound L	evel (m OD)	Co-Ordinates ()		ВПЗ	
J212	282	24-09-2	1							
Client					E	Ingineer			Sheet	
						Elliott Wo	bod		1 of	1
SAN	MPLES &	TESTS					STRATA			ient fill
Depth	Type No	Test Result	Wate	Reduce Level	d _{Leger}	Depth nd (Thick- ness)	DESCRIPTIO	N		lnstrum / Back
0.50	D					(1.00)	TURF over MADE GROUND (brow frequent fine to coarse gravel siz flint and rare charcoal. Occasion	vn sandy ed fragr al tree re	y silty clay with nents of brick, oots)	
1.30	D						Firm becoming stiff with depth b orange-brown fissured silty clay sand and silt. Rare blue-grey veir	rown m with par ning.	ottled tings of clayey	
2.00	D					< - - - - - - - - - - - - -				
- 2.80 - 3.20	D						3.00 Occasional coarse sand size below 3.00 m	d selenit	te crystals	
	D	and Water O	hse	rvatio	ns		CENEDA			
Depth	Date T	ime Casi Depth	ng Dia.	mm [Water Depth	Hand dug No ground Standpipe	GENERA REMARK services pit to 1.20 m dwater encountered installed to depth of 4.0 m	S		
All dimension Scale	ons in metro 1:62.5	es Method/ Plant Used H	anc	l-held	windc	w sample			Logged By AG	



Project	Project BOREHOLE No				
12 Pilgrims Lane, London, NV	V3 1SN	J			
Job No Date 24-09-21	L	Ground Lev	vel (m OD)	Co-Ordinates ()	DП4
J21282 24-09-21	L				
Client		Eng	gineer		Sheet
		E	Elliott Wo	ood	1 of 1
SAMPLES & TESTS	5			STRATA	
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Boring Progress and Water Of	g	Water		GENERAL REMARKS	
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All dimensions in metres Method/ Scale 1:62.5 Plant Used Ha	and-hel	ld window	sampler		Logged By AG

Appendix D

Thames Water Asset Maps

D



Based on the Ordnance Survey Map (2020) with the Sanction of the controller of H.M. Stationery Office, License no. 100019345 Crown Copyright Reserved.

Thames Water Utilities Ltd, Property Searches, PO Box 3189, Slough SL1 4W, DX 151280 Slough 13 T 0800 009 4540 E searches@thameswater.co.uk I www.thameswater-propertysearches.co.uk

NB. Levels quoted in metres Ordnance Newlyn Datur	n. The value -9999.00 indicates that no survey information is available
---	---

Manhole Reference	Manhole Cover Level	Manhole Invert Level				
8602	81.66	79.62				
851A	n/a	n/a				
8511	n/a	n/a				
8701	n/a	n/a				
871B	n/a	n/a				
871C	n/a	n/a				
8604	76.28	75.87				
8601	76.83	71.48				
8603	76.27	74.59				
8605	77.22	75.79				
961B	n/a	n/a				
961A	n/a	n/a				
9601	75.48	73.59				
961C	n/a	n/a				
7502	85.03	78.75				
8510	n/a	n/a				
8504	83.95	79.05				
7601	87.09	86.02				
8606	83.06	79.91				
761A	n/a	n/a				
871A	n/a	n/a				
8703	85.06	81.88				
7702	86.83	83.6				
7703	89.01	85.67				
The position of the apparetue shown on this plan i	a given without obligation and warranty, and the acc	hursey connet he guaranteed. Service nines are not				
shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position						
of mains and services must be verified and established on site before any works are undertaken.						



Asset Location Search - Sewer Key



1) All levels associated with the plans are to Ordnance Datum Newlyn.

2) All measurements on the plan are metric.

3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate the direction of flow.

Most private pipes are not shown on our plans, as in the past, this information has not been recorded.

5) 'na' or '0' on a manhole indicates that data is unavailable.

6) The text appearing alongside a server line indicates the internal diameter of the pipe in millimeters. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology, please contact Property Searches on 0800 009 4540.



The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.

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Asset Location Search - Water Key



General PurposeValve Air Valve

Other Other (Proposed) Pumping Station Service Reservoir Shaft Inspection Treatment Works Unknown Water Tower

Operational Sites

Booster Station

Other Symbols

Data Logger



Casement: Ducts may contain high voltage cables. Please check with Thames Water.



Appendix E

Thames Water Build-Over Correspondence

Е

55 Whitfield Street London W1T 4AH 020 7499 5888 | DDI: 020 3982 7926 Lines operational as usual

<u>Website</u> <u>Twitter</u> Instagram LinkedIn

Discover our new co-working space for the built environment at The Building Society

TIME TO DO MORE Let's not go back to 'normal'. It's time for us all to do more to engineer a better, healthier society. Watch our video here on giving back to the industry and join the discussion #EngineeringaBetterSociety

From: <u>DEVELOPER.SERVICES@THAMESWATER.CO.U</u> <<u>DEVELOPER.SERVICES@THAMESWATER.CO.UK</u>> Sent: 28 June 2022 09:59 To: Keri Trimmer <<u>k.trimmer@elliottwood.co.uk</u>> Subject: 12 Pilgrims Lane NW3 1SN

You don't need to apply for a build over agreement

Dear Keri Trimmer

Thank you for your email dated 21st June 2022, regarding **12 Pilgrims Lane, London, NW3 1SN**. We've reviewed your drawing and are satisfied that no further action is required, as the proposed work won't be within three metres of a public sewer or one metre of a lateral drain.

This means you can go ahead with the work, without the need to enter into an agreement with us.

However, please note that if you find a shared drainage pipe within three metres of your proposed building once you've started work, you need to tell us immediately so that we can review any new information.

If you've any further questions, please contact our helpdesk on 0800 009 3921, selecting Option 1, or email us at <u>developer.services@thameswater.co.uk</u>.

Regards

Karla Denton

Pre App Build Over Team, (Previously LA Team) Part H4 Consultations, Buildovers, Developer Services Wastewater Clearwater Court, Vastern Road, Reading, RG1 8DB Helpdesk 0800 009 3921, email developer.services@thameswater.co.uk Apply to build within 3m or connect to a public sewer online - www.thameswater.co.uk/buildover

Original Text

From:	Keri Trimmer < <u>k.trimmer@elliottwood.co.uk</u> >
То:	BUILDOVERS@THAMESWATER.CO.UK
	Rishi Bodhani < <u>Rishi.Bodhani@stature.london</u> >;
CC:	;DEVELOPER.SERVICES@THAMESWATER.CO.U
	< <u>DEVELOPER.SERVICES@THAMESWATER.CO.UK</u> >
Sent:	21.06.22 16:37:55

Subject: TW Build-Over: 12 PILGRIMS LANE LONDON NW3 1SN

Dear Sir / Madam,

We are the civil and structural engineers working on a development at the above address and have recently received the below email from the Thames Water build-over team.

We would like to confirm that the combined sewers beneath Pilgrims Lane are over 6m away from the development boundary, therefore a build-near or build-over application should not be required. We have also had a CCTV drainage survey undertaken which shows that all drainage on site is private, with no third-party sewers passing through the site footprint, the CCTV survey report is attached FYI. We will be reviewing the drainage strategy for the site in due course and will submit any relevant S106 applications during detailed design.

In the interim, could you please confirm that we do not need a sewer build-near or build-over agreement as part of any works on site.



Kind Regards

Keri Trimmer MEng CEng MICE Associate



Engineering a better society

55 Whitfield Street London W1T 4AH 020 7499 5888 | DDI: 020 3982 7926 Lines operational as usual

<u>Website</u> <u>Twitter</u> <u>Instagram</u> <u>LinkedIn</u>

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TIME TO DO MORE Let's not go back to 'normal'. It's time for us all to do more to engineer a better, healthier society. Watch our video here on giving back to the industry and join the discussion #EngineeringaBetterSociety

From: BUILDOVERS@THAMESWATER.CO.UK < BUILDOVERS@THAMESWATER.CO.UK >

Sent: 17 June 2022 12:03 To: Rishi Bodhani <<u>Rishi.Bodhani@stature.london</u>> Subject: Building over or within 3 metres of a sewer

You don't often get email from buildovers@thameswater.co.uk. Learn why this is important

Building over or near sewers. Site location: 12 PILGRIMS LANE LONDON NW3 1SN

Dear Mr BODHANI,

Thank you for your enquiry.

If you're planning a new building, a conservatory, a garage or any other extension to your home, it's important you let us know. We need to make sure your work doesn't accidentally affect a sewer, or limit our access if we have to repair it. I've provided some information below about build over agreements, which I hope you find helpful.

Types of build over agreements

If you're building over or near a domestic sewer with a diameter of **160mm or less**, you can potentially apply for a **self-certified build over agreement**. We may grant this if you're able to confirm that your plans pose little risk to the pipe. This free service is only available online, and

requires you to complete a questionnaire at www.thameswater.co.uk/buildover.

If your answers don't meet our requirements, we'll redirect you to apply for an **approved build over agreement**, for which you'd need to pay. You'll need to apply for this full agreement if you're unsure of any of the answers, or if you want to build over or near a sewer with a diameter **above 160mm**.

If you're applying for an **approved build over agreement**, in addition to contact information, you'll need to provide the following:

- A drawing showing the cross-sectional foundation plan, including clearance distances and size of the sewer being built over
- A drawing showing the location of the sewer relative to the existing property and proposed work
- A copy of the drawings submitted for Building Regulations approval
- Credit or debit card details, in order to pay the required deposit and charges

Ownership of sewers

The start of the drain is the responsibility of the property owner until it crosses the boundary into land owned by someone else. At that point it becomes a 'public lateral drain'. Once a second property connects into the public lateral drain, it becomes a 'public sewer'. We're responsible for all public sewers and lateral drains in our region. You can view typical examples of the different types of drains and sewers on page 2 of our guidance booklet attached.

How to apply

You can <u>apply online</u> on our website . Alternatively download the application form from our <u>website</u> which then can be emailed to us at <u>developer.services@thameswater.co.uk</u> or posted to us at Developer Services, Clearwater Court, Vastern Road, Reading RG1 8DB.

Next steps

If you apply for a self-certified build over agreement, you'll find out whether you qualify as soon as you submit the completed questionnaire online.

If you apply for an approved build over agreement, we'll review your application and contact you within three weeks providing we have all relevant plans and payment.

If you'd like to speak to me about this, please call me on 0800 009 3921 between 8am and 5pm, Monday to Friday.

Yours sincerely,

Jordan

Customer Service Advisor Developer Services

Appendix F

Below Ground Drainage Survey

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