| Tekla Tedds<br>Alan Baxter Ltd<br>75 Cowcross Street<br>London<br>EC1M 6EL | Project<br>British Museum - ECP | | | | Job no.<br>1910/41 | |
|---|---|---|---|---|---|---|
| | Calcs for<br>1 in 1 year, 5 min duration - existing discharge rate | | | | Start page no./Revision<br>1 | |
| | Calcs by<br>CM | Calcs date<br>13/10/2023 | Checked by<br>DLa | Checked date<br>16/10/2023 | Approved by | Approved date |

## DESIGN RAINFALL

**In accordance with the Wallingford Procedure**

Tedds calculation version 2.0.02

**Design rainfall intensity**

| | |
|---|---|
| Location of catchment area | London |
| Storm duration | D = **5** min |
| Return period | Period = **1** yr |
| Ratio 60 min to 2 day rainfall of 5 yr return period | r = **0.440** |
| 5-year return period rainfall of 60 minutes duration | M5_60min = **20.0** mm |
| Increase of rainfall intensity due to global warming | $p_{climate}$ = **0** % |
| Factor Z1 (Wallingford procedure) | Z1 = **0.39** |
| Rainfall for 5min storm with 5 year return period | $M5\_5min_i$ = Z1 × M5_60min = **7.7** mm |
| Factor Z2 (Wallingford procedure) | Z2 = **0.61** |
| Rainfall for 5min storm with 1 year return period | M1_5min = Z2 × $M5\_5min_i$ = **4.8** mm |
| Design rainfall intensity | $I_{max}$ = M1_5min / D = **57.0** mm/hr |

**Maximum surface water runoff**

| | |
|---|---|
| Catchment area | $A_{catch}$ = **530** $m^2$ |
| Percentage of area that is impermeable | p = **100** % |
| Maximum surface water runoff | $Q_{max} = A_{catch} \times p \times I_{max}$ = **8.4** l/s |

## DESIGN RAINFALL

**In accordance with the Wallingford Procedure**

Tedds calculation version 2.0.02

**Design rainfall intensity**

| | |
|---|---|
| Location of catchment area | London |
| Storm duration | D = **5** min |
| Return period | Period = **30** yr |
| Ratio 60 min to 2 day rainfall of 5 yr return period | r = **0.440** |
| 5-year return period rainfall of 60 minutes duration | M5_60min = **20.0** mm |
| Increase of rainfall intensity due to global warming | $p_{climate}$ = **0** % |
| Factor Z1 (Wallingford procedure) | Z1 = **0.39** |
| Rainfall for 5min storm with 5 year return period | $M5\_5min_i = Z1 \times M5\_60min$ = **7.7** mm |
| Factor Z2 (Wallingford procedure) | Z2 = **1.46** |
| Rainfall for 5min storm with 30 year return period | $M30\_5min = Z2 \times M5\_5min_i$ = **11.3** mm |
| Design rainfall intensity | $I_{max} = M30\_5min / D$ = **135.6** mm/hr |

**Maximum surface water runoff**

| | |
|---|---|
| Catchment area | $A_{catch}$ = **530** $m^2$ |
| Percentage of area that is impermeable | p = **100** % |
| Maximum surface water runoff | $Q_{max} = A_{catch} \times p \times I_{max}$ = **20.0** l/s |

| Tekla Tedds<br>Alan Baxter Ltd<br>75 Cowcross Street<br>London<br>EC1M 6EL | Project<br>British Museum - ECP | | | Job no.<br>1910/41 | |
|---|---|---|---|---|---|
| | Calcs for<br>1 in 100 years, 5 min duration - existing discharge rate | | | Start page no./Revision<br>1 | |
| | Calcs by<br>CM | Calcs date<br>16/10/2023 | Checked by<br>DLa | Checked date<br>16/10/2023 | Approved by | Approved date |

## DESIGN RAINFALL

**In accordance with the Wallingford Procedure**

Tedds calculation version 2.0.02

**Design rainfall intensity**

| | |
|---|---|
| Location of catchment area | London |
| Storm duration | D = **5** min |
| Return period | Period = **100** yr |
| Ratio 60 min to 2 day rainfall of 5 yr return period | r = **0.440** |
| 5-year return period rainfall of 60 minutes duration | M5_60min = **20.0** mm |
| Increase of rainfall intensity due to global warming | $p_{climate}$ = **0** % |
| Factor Z1 (Wallingford procedure) | Z1 = **0.39** |
| Rainfall for 5min storm with 5 year return period | $M5\_5min_i = Z1 \times M5\_60min$ = **7.7** mm |
| Factor Z2 (Wallingford procedure) | Z2 = **1.86** |
| Rainfall for 5min storm with 100 year return period | $M100\_5min = Z2 \times M5\_5min_i$ = **14.3** mm |
| Design rainfall intensity | $I_{max} = M100\_5min / D$ = **172.2** mm/hr |

**Maximum surface water runoff**

| | |
|---|---|
| Catchment area | $A_{catch}$ = **530** $m^2$ |
| Percentage of area that is impermeable | p = **100** % |
| Maximum surface water runoff | $Q_{max} = A_{catch} \times p \times I_{max}$ = **25.4** l/s |

## DESIGN RAINFALL

**In accordance with the Wallingford Procedure**

Tedds calculation version 2.0.02

**Design rainfall intensity**

| | |
|---|---|
| Location of catchment area | London |
| Storm duration | D = **6** hr |
| Return period | Period = **100** yr |
| Ratio 60 min to 2 day rainfall of 5 yr return period | r = **0.440** |
| 5-year return period rainfall of 60 minutes duration | M5_60min = **20.0** mm |
| Increase of rainfall intensity due to global warming | $p_{climate}$ = **40** % |
| Factor Z1 (Wallingford procedure) | Z1 = **1.53** |
| Rainfall for 6hr storm with 5 year return period | $M5\_6hr_i = Z1 \times M5\_60min \times (1 + p_{climate})$ = **42.8** mm |
| Factor Z2 (Wallingford procedure) | Z2 = **1.87** |
| Rainfall for 6hr storm with 100 year return period | $M100\_6hr = Z2 \times M5\_6hr_i$ = **80.0** mm |
| Design rainfall intensity | $I_{max} = M100\_6hr / D$ = **13.3** mm/hr |

**Maximum surface water runoff**

| | |
|---|---|
| Catchment area | $A_{catch}$ = **530** $m^2$ |
| Percentage of area that is impermeable | p = **100** % |
| Maximum surface water runoff | $Q_{max} = A_{catch} \times p \times I_{max}$ = **2.0** l/s |

Surface water runoff volume | Volume = M100_6hr x Acatch = 42.4 $m^3$

## ATTENUATION DESIGN

**In accordance with CIRIA publication C753 - The SUDS Manual**

Tedds calculation version 1.0.04

**EA_Defra method**

**Site characteristics**

Location — London

Hydrological region — 6

Soil type (Wallingford Procedure W.R.A.P map) — 2

Standard percentage runoff — SPR = **0.30**

Average annual rainfall — SAAR = **600** mm

5 year return period rainfall of 60 minute duration — M5_60min = **20.0** mm

Ratio 60-minute to 2 day rainfalls of 5 year return — r = **0.44**

Rainfall intensity increase due to global warming — $p_{climate}$ = **0%**

Impervious area req. attenuation storage — $\alpha$ = **100.0** %

**Catchment details**

| Subcatchment | Name | Area (ha) | PIMP (%) | Impermeable. area (ha) |
|---|---|---|---|---|
| 1; | SWEC; | 0.05; | 100.0 | 0.05; |
| | **Total** | 0.05; | 100.0 | 0.05; |

**Greenfield runoff rates**

Catchment area — AREA = **50.00** hectare

Greenfield runoff rate (50 hectare site) — $\bar{Q}_{rural} = 0.00108\text{m}^3/\text{s} \times (\text{AREA}/1\text{km}^2)^{0.89} \times (\text{SAAR}/1\text{mm})^{1.17} \times \text{SPR}^{2.17}$ = **76.1** l / s

Greenfield runoff rate — $\bar{Q} = \bar{Q}_{rural} / \text{AREA} \times A$ = **0.1** l / s

Greenfield runoff rate per unit area — $\bar{Q}_A = \bar{Q} / A$ = **1.5** l / s / hectare

**Estimated site discharges**

FSR growth rate (1 year) — $FSR_{1yr}$ = **0.85**

Discharge (1 year) — $Q_{1yr} = \bar{Q} \times FSR_{1yr}$ = **0.1 l/s**

FSR growth rate (30 year) — $FSR_{30yr}$ = **2.30**

Discharge (30 year) — $Q_{30yr} = \bar{Q} \times FSR_{30yr}$ = **0.2 l/s**

FSR growth rate (100 year) — $FSR_{100yr}$ = **3.19**

Discharge (100 year) — $Q_{100yr} = \bar{Q} \times FSR_{100yr}$ = **0.3 l/s**

Greenfield run-off rates with 40% climate change allowance.

**Estimated attenuation volume - 1 year**

Attenuation storage vol (fig A7.1 - A7.8) — $Uvol_{1yr}$ = **205.0** m$^3$ / hectare

Basic storage volume — $BSV_{1yr} = Uvol_{1yr} \times \alpha \times A$ = **10.87** m$^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) — $FF_{1yr}$ = **0.90**

Storage volume ratio (fig A8.1 - A8.8) — $SVR_{1yr}$ = **1.13**

Adjusted storage volume — $ASV_{1yr} = SVR_{1yr} \times BSV_{1yr}$ = **12.28** m$^3$

Hydrological regional volume ratio (fig A9.1) — $HR_{1yr}$ = **1.01**

Final estimated attenuation storage — $Vol_{1yr} = HR_{1yr} \times ASV_{1yr}$ = **12.35** m$^3$

Library item: Estimated attenuation output

**Estimated attenuation volume - 30 year**

Attenuation storage vol (fig A7.1 - A7.8) — $Uvol_{30yr}$ = **420.0** m$^3$ / hectare

Basic storage volume — $BSV_{30yr} = Uvol_{30yr} \times \alpha \times A$ = **22.26** m$^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) — $FF_{30yr}$ = **0.80**

Storage volume ratio (fig A8.1 - A8.8) $SVR_{30yr}$ = **1.32**

Adjusted storage volume $ASV_{30yr} = SVR_{30yr} \times BSV_{30yr}$ = **29.36** $m^3$

Hydrological regional volume ratio (fig A9.1) $HR_{30yr}$ = **1.01**

Final estimated attenuation storage $Vol_{30yr} = HR_{30yr} \times ASV_{30yr}$ = **29.74** $m^3$

Library item: Estimated attenuation output

**Estimated attenuation volume - 100 year**

Attenuation storage vol (fig A7.1 - A7.8) $Uvol_{100yr}$ = **525.0** $m^3$ / hectare

Basic storage volume $BSV_{100yr} = Uvol_{100yr} \times \alpha \times A$ = **27.83** $m^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) $FF_{100yr}$ = **0.75**

Storage volume ratio (fig A8.1 - A8.8) $SVR_{100yr}$ = **1.46**

Adjusted storage volume $ASV_{100yr} = SVR_{100yr} \times BSV_{100yr}$ = **40.61** $m^3$

Hydrological regional volume ratio (fig A9.1) $HR_{100yr}$ = **1.02**

Final estimated attenuation storage $Vol_{100yr} = HR_{100yr} \times ASV_{100yr}$ = **41.49** $m^3$

Library item: Estimated attenuation output

**Attenuation storage required**

Vol. increase due to head-discharge relationship $p_{hydro}$ = **1.25**

Maximum attenuation storage required $V_{req\_max} = Vol_{30yr} \times p_{hydro}$ = **37.2** $m^3$

This is the theoretical attenuation volume required to limit flows to greenfield rates. However, it is not possible to limit flows as low as greenfield rates.

**Interception storage**

Interception rainfall depth $d_{int}$ = **0** mm

Volume of interception storage required $V_{int\_req} = 0.8 \times A_{imp} \times d_{int}$ = **0.00** $m^3$

**Long term storage**

Proportion of paved area draining in to network $\alpha$ = **1.0**

Proportion of pervious area draining in to network $\beta$ = **0.5**

Rainfall depth for 100years, 6 hour event RD = M100_360 = **60.1** mm

Extra runoff vol of dev.runoff over greenfield runoff $Vol_{xs} = \max(RD \times A \times (PIMP \times \alpha \times 0.8 + ((1 - PIMP) \times \beta \times SPR) - SPR), 0m^3)$ = **15.94** $m^3$

**Treatment volume**

Treatment volume (assume 80% runoff) $T_{vol} = 0.8 \times A \times 15mm \times PIMP$ = **6.36** $m^3$

## ATTENUATION DESIGN

**In accordance with CIRIA publication C753 - The SUDS Manual**

Tedds calculation version 1.0.04

**EA_Defra method**

**Site characteristics**

Location London

Hydrological region 6

Soil type (Wallingford Procedure W.R.A.P map) 2

Standard percentage runoff SPR = **0.30**

Average annual rainfall SAAR = **600** mm

5 year return period rainfall of 60 minute duration M5_60min = **20.0** mm

Ratio 60-minute to 2 day rainfalls of 5 year return r = **0.44**

Rainfall intensity increase due to global warming $p_{climate}$ = **40%**

Impervious area req. attenuation storage $\alpha$ = **100.0** %

**Catchment details**

| Subcatchment | Name | Area (ha) | PIMP (%) | Impermeable. area (ha) |
|---|---|---|---|---|
| 1; | SWEC; | 0.05; | 100.0 | 0.05; |
| | **Total** | 0.05; | 100.0 | 0.05; |

**Greenfield runoff rates**

Catchment area AREA = **50.00** hectare

Greenfield runoff rate (50 hectare site) $\bar{Q}_{rural} = 0.00108m^3/s \times (AREA/1km^2)^{0.89} \times (SAAR/1mm)^{1.17} \times SPR^{2.17}$ = **76.1** l / s

Greenfield runoff rate $\bar{Q} = \bar{Q}_{rural} / AREA \times A$ = **0.1** l / s

Greenfield runoff rate per unit area $\bar{Q}_A = \bar{Q} / A$ = **1.5** l / s / hectare

**Estimated site discharges**

FSR growth rate (1 year) $FSR_{1yr}$ = **0.85**

Discharge (1 year) $Q_{1yr} = \bar{Q} \times FSR_{1yr}$ = **0.1 l/s**

FSR growth rate (30 year) $FSR_{30yr}$ = **2.30**

Discharge (30 year) $Q_{30yr} = \bar{Q} \times FSR_{30yr}$ = **0.2 l/s**

FSR growth rate (100 year) $FSR_{100yr}$ = **3.19**

Discharge (100 year) $Q_{100yr} = \bar{Q} \times FSR_{100yr}$ = **0.3 l/s**

Greenfield run-off rates with 40% climate change allowance.

**Estimated attenuation volume - 1 year**

Attenuation storage vol (fig A7.1 - A7.8) $Uvol_{1yr}$ = **205.0** $m^3$ / hectare

Basic storage volume $BSV_{1yr} = Uvol_{1yr} \times \alpha \times A$ = **10.87** $m^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) $FF_{1yr}$ = **0.90**

Storage volume ratio (fig A8.1 - A8.8) $SVR_{1yr}$ = **1.74**

Adjusted storage volume $ASV_{1yr} = SVR_{1yr} \times BSV_{1yr}$ = **18.91** $m^3$

Hydrological regional volume ratio (fig A9.1) $HR_{1yr}$ = **1.01**

Final estimated attenuation storage $Vol_{1yr} = HR_{1yr} \times ASV_{1yr}$ = **19.00** $m^3$

Library item: Estimated attenuation output

**Estimated attenuation volume - 30 year**

Attenuation storage vol (fig A7.1 - A7.8) $Uvol_{30yr}$ = **420.0** $m^3$ / hectare

Basic storage volume $BSV_{30yr} = Uvol_{30yr} \times \alpha \times A$ = **22.26** $m^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) $FF_{30yr}$ = **0.80**

Storage volume ratio (fig A8.1 - A8.8) $SVR_{30yr}$ = **1.74**

Adjusted storage volume $ASV_{30yr} = SVR_{30yr} \times BSV_{30yr}$ = **38.73** $m^3$

Hydrological regional volume ratio (fig A9.1) $HR_{30yr}$ = **1.01**

Final estimated attenuation storage $Vol_{30yr} = HR_{30yr} \times ASV_{30yr}$ = **39.23** $m^3$

Library item: Estimated attenuation output

**Estimated attenuation volume - 100 year**

Attenuation storage vol (fig A7.1 - A7.8) $Uvol_{100yr}$ = **525.0** $m^3$ / hectare

Basic storage volume $BSV_{100yr} = Uvol_{100yr} \times \alpha \times A$ = **27.83** $m^3$

FEH rainfall factor (figs A11.1, A6.1.1 - A6.3.4) $FF_{100yr}$ = **0.75**

Storage volume ratio (fig A8.1 - A8.8) $SVR_{100yr}$ = **1.74**

Adjusted storage volume $ASV_{100yr} = SVR_{100yr} \times BSV_{100yr}$ = **48.42** $m^3$

Hydrological regional volume ratio (fig A9.1) $HR_{100yr}$ = **1.02**

Final estimated attenuation storage $Vol_{100yr} = HR_{100yr} \times ASV_{100yr}$ = **49.47** $m^3$

Library item: Estimated attenuation output

**Attenuation storage required**

Vol. increase due to head-discharge relationship $p_{hydro}$ = **1.25**

Maximum attenuation storage required $V_{req\_max} = Vol_{30yr} \times p_{hydro}$ = **49.0** $m^3$

This is the theoretical attenuation volume required to limit flows to greenfield rates. However, it is not possible to limit flows as low as greenfield rates.

**Interception storage**

Interception rainfall depth $d_{int}$ = **0** mm

Volume of interception storage required $V_{int\_req} = 0.8 \times A_{imp} \times d_{int}$ = **0.00** $m^3$

**Long term storage**

Proportion of paved area draining in to network $\alpha$ = **1.0**

Proportion of pervious area draining in to network $\beta$ = **0.5**

Rainfall depth for 100years, 6 hour event RD = M100_360 = **80.0** mm

Extra runoff vol of dev.runoff over greenfield runoff $Vol_{xs} = \max(RD \times A \times (PIMP \times \alpha \times 0.8 + ((1 - PIMP) \times \beta \times SPR) - SPR), 0m^3)$ = **21.20** $m^3$

**Treatment volume**

Treatment volume (assume 80% runoff) $T_{vol} = 0.8 \times A \times 15mm \times PIMP$ = **6.36** $m^3$

| | |
|---|---|
| **Calculated by:** | Cara Malcolm |
| **Site name:** | British Museum - ECP |
| **Site location:** | London |

Calculation to size the attenuation tank for a 1:100 year storm + 40% climate change with discharge rate of 2 l/s

## Site Details

| | |
|---|---|
| **Latitude:** | Site latitude |
| **Longitude:** | Site longitude |

This is an estimation of the storage volume requirements that are needed to meet normal
best practice criteria in line with Environment Agency guidance "Rainfall runoff management
for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and
the non-statutory standards for SuDS (Defra, 2015). It is not to be used for detailed design
of drainage systems. It is recommended that hydraulic modelling software is used to calculate
volume requirements and design details before finalising the design of the drainage scheme.

| | |
|---|---|
| **Reference:** | 4042322780 |
| **Date:** | Oct 16 2023 15:41 |

## Site characteristics

| | |
|---|---|
| **Total site area (ha):** | 0.053 |
| **Significant public open space (ha):** | 0 |
| **Area positively drained (ha):** | 0.053 |
| **Impermeable area (ha):** | 0.053 |
| **Percentage of drained area that is impermeable (%):** | 100 |
| **Impervious area drained via infiltration (ha):** | 0 |
| **Return period for infiltration system design (year):** | 10 |
| **Impervious area drained to rainwater harvesting (ha):** | 0 |
| **Return period for rainwater harvesting system (year):** | 10 |
| **Compliance factor for rainwater harvesting system (%):** | 66 |
| **Net site area for storage volume design (ha):** | 0.05 |
| **Net impermable area for storage volume design (ha):** | 0.05 |
| **Pervious area contribution to runoff (%):** | 0 |

* where rainwater harvesting or infiltration has been used for managing surface water runoff such that the effective impermeable area is less than 50% of the 'area positively drained', the 'net site area' and the estimates of $Q_{BAR}$ and other flow rates will have been reduced accordingly.

## Design criteria

| | |
|---|---|
| **Climate change allowance factor:** | 1.4 |

## Methodology

| | |
|---|---|
| **esti** | IH124 |
| **$Q_{BAR}$ estimation method:** | Specify Qbar manually |
| **SPR estimation method:** | Calculate from SOIL type |

## Soil characteristics

| | Default | Edited |
|---|---|---|
| **SOIL type:** | | 2 |
| **SPR:** | | 0.3 |

## Hydrological characteristics

| | Default | Edited |
|---|---|---|
| **Rainfall 100 yrs 6 hrs:** | -- | 60 |
| **Rainfall 100 yrs 12 hrs:** | -- | 103.95 |
| **FEH / FSR conversion factor:** | - | 1.35 |
| **SAAR (mm):** | - | 600 |
| **M5-60 Rainfall Depth (mm):** | - | 20 |
| **'r' Ratio M5-60/M5-2 day:** | - | 0.4 |
| **Hydological region:** | - | - |
| **Growth curve factor 1 year:** | - | 0.85 |
| **Growth curve factor 10 year:** | - | 1.62 |
| **Growth curve factor 30 year:** | - | 2.3 |
| **Growth curve factor 100 years:** | - | 3.19 |
| **$Q_{BAR}$ for total site area (l/s):** | -- | 0.08 |

| | | | | |
|---|---|---|---|---|
| Urban creep allowance factor: | 1 | $Q_{BAR}$ for net site area (l/s): | -- | 0.08 |
| Volume control approach | Flow control to max of 2 l/s/ha or Qbar | | | |
| Interception rainfall depth (mm): | 0 | | | |
| Minimum flow rate (l/s): | 2 | | | |

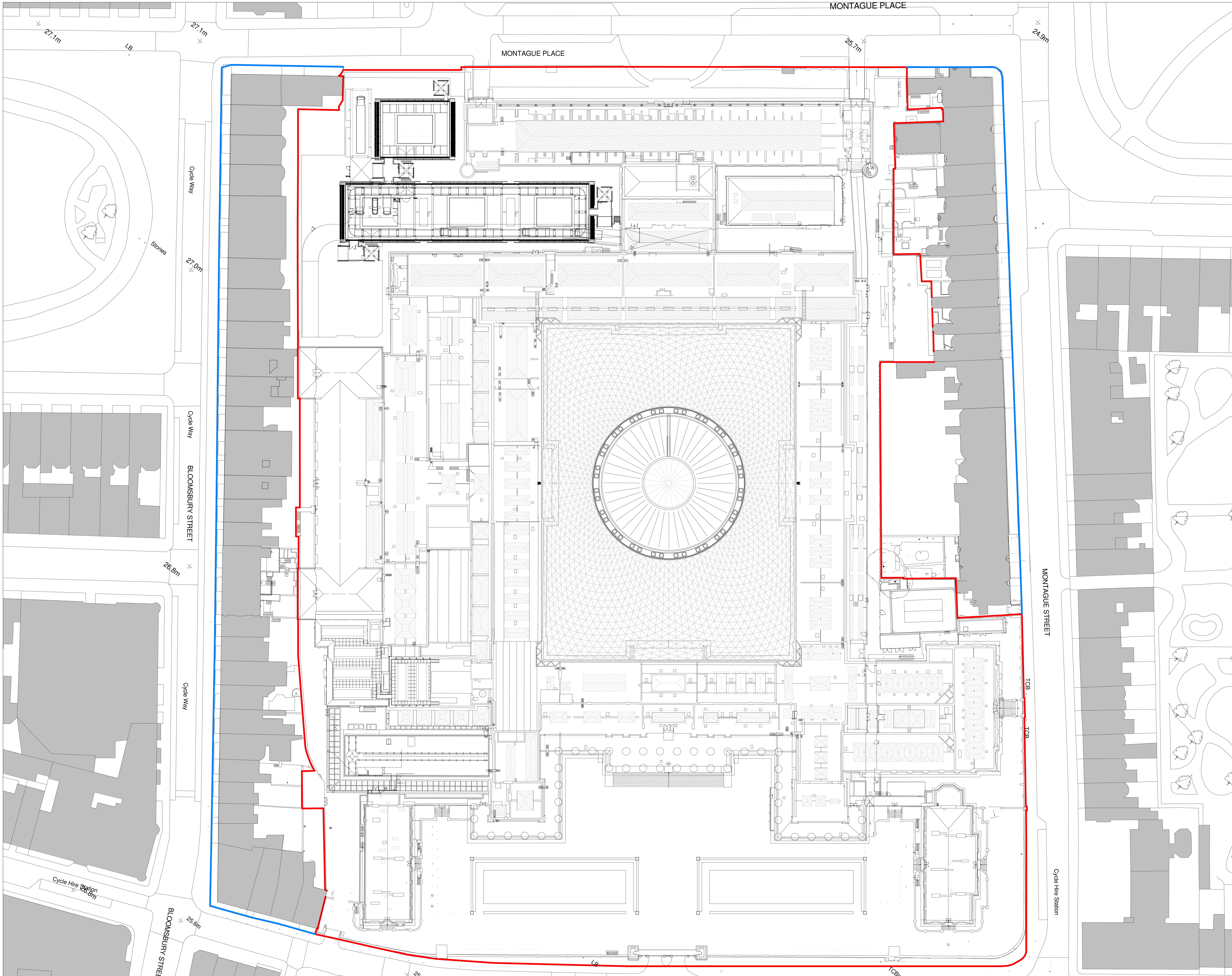
## Site discharge rates

| | Default | Edited |
|---|---|---|
| 1 in 1 year (l/s): | -- | 2 |
| 1 in 30 years (l/s): | -- | 2 |
| 1 in 100 year (l/s): | -- | 2 |

## Estimated storage volumes

| | Default | Edited |
|---|---|---|
| Attenuation storage 1/100 years ($m^3$): | -- | 24 |
| Long term storage 1/100 years ($m^3$): | 0 | 0 |
| Total storage 1/100 years ($m^3$): | -- | 24 |

# Appendix D: Architect's Proposed Layout Drawings




Key Plan

This drawing is not to be scaled. Dimensions govern.
Wright & Wright Architects shall be notified in writing of any discrepancies.
All dimensions are in millimeters unless noted otherwise.
This drawing issued at First Contract is indicative of design intent and should be read in association with all specifications, schedules and consultants information.
The Contractor is required to complete the detailed design, final coordination and interfaces. Contractor and Sub-Contractors design are subject to Architects acceptance.

**Key:**

- Land within applicants ownership
- Proposed Red line boundary

| Rev | Note | Date | Issued By | Issued To |
|---|---|---|---|---|
| P4 | Stage 03 / Planning | 28.09.2023 | JB | BM |
| P3 | Draft Planning Issue | 01.09.2023 | JB | BM |
| P2 | Draft Stage 03 Issue | 28.07.2023 | JB | BM |
| P1 | Draft Stage 03 Issue for Costing | 14.07.2023 | JB | DT |


WRIGHT & WRIGHT ARCHITECTS

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E ww@wrightandwright.co.uk


0 5000 10000 20000 40000 mm

Client
British Museum

Project Name
ECP: South-West Energy Centre

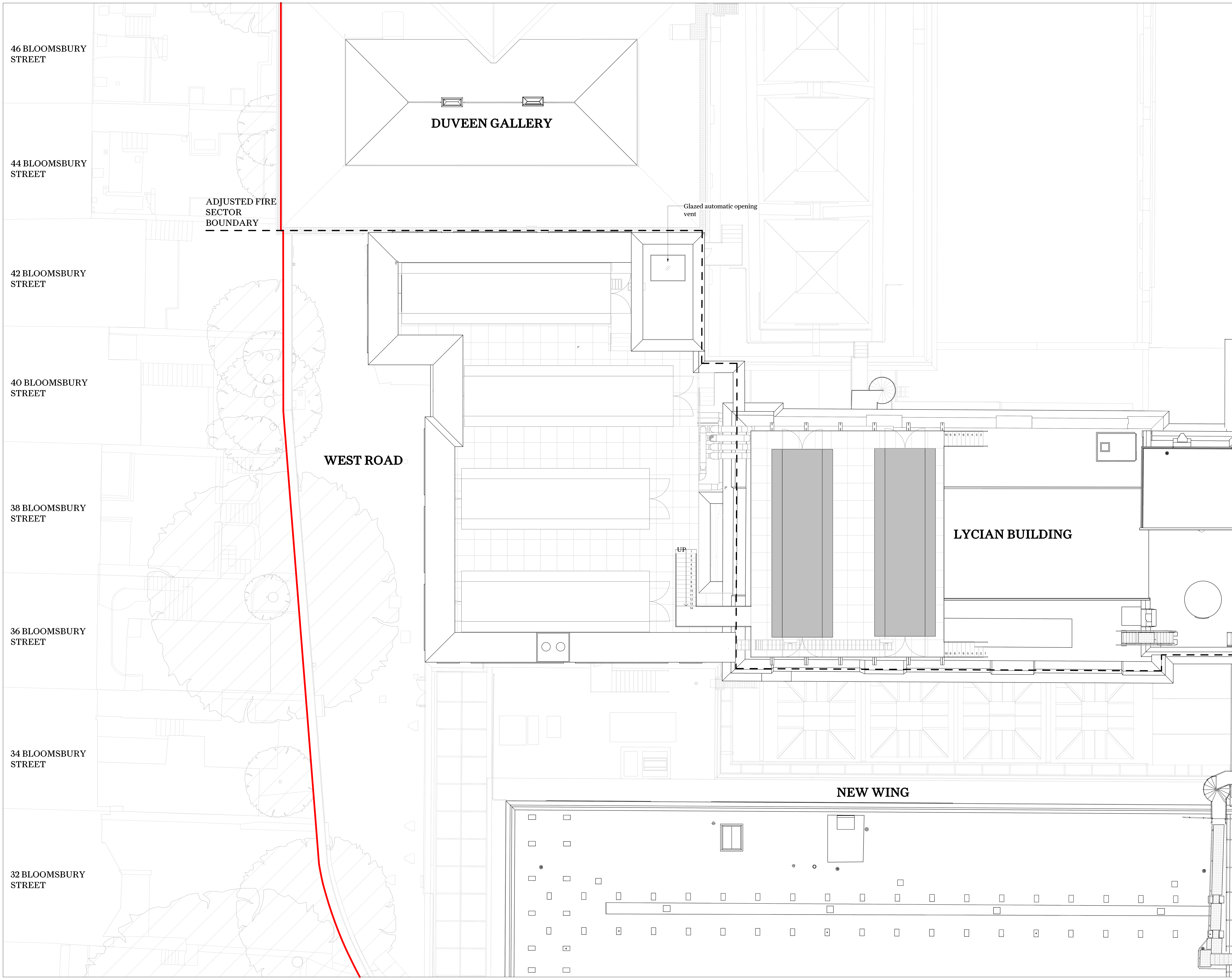
Stage
RIBA Stage 03

Drawing Title
# Proposed Site Plan

Drawing Number
10771-WW-SW-RF-DR -A-2031

| Scale | Sheet Size | Issue Date |
|---|---|---|
| 1:500 | A1 | 28.09.2023 |

| Revision | Status |
|---|---|
| P4 | S4 - For Approval |




Key Plan

This drawing is not to be scaled. Dimensions govern.
Wright & Wright Architects shall be notified in writing of any discrepancies.
All dimensions are in millimeters unless noted otherwise.
This drawing issued at First Contract is indicative of design intent and should be read in association with all specifications, schedules and consultants information.
The Contractor is required to complete the detailed design, final coordination and interfaces. Contractor and Sub-Contractors design are subject to Architects acceptance.

Key:

- Site Boundary
- Adjusted Fire Sector Boundary

Note:

| Rev | Note | Date | Issued By | Issued To |
|---|---|---|---|---|
| P4 | Stage 03 / Planning | 28.09.2023 | JB | BM |
| P3 | Draft Planning Issue | 01.09.2023 | JB | BM |
| P2 | Draft Stage 03 Issue | 28.07.2023 | JB | BM |
| P1 | Draft Stage 03 Issue for Costing | 14.07.2023 | JB | DT |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0 1000 2000 5000 7500 mm

Client
British Museum

Project Name
ECP: South-West Energy Centre

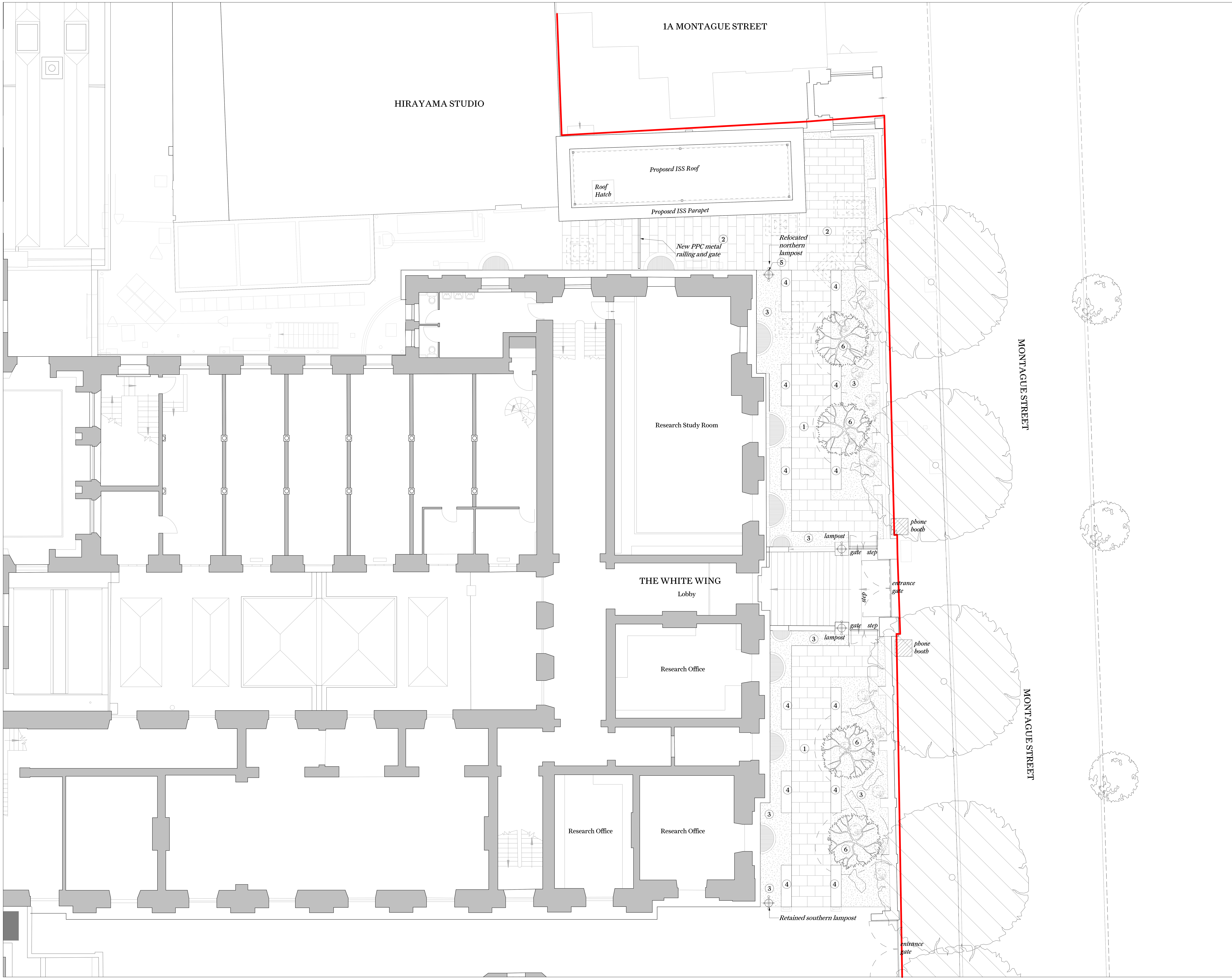
Stage
RIBA Stage 03

Drawing Title
Proposed SWEC Roof Plan - Phase 02

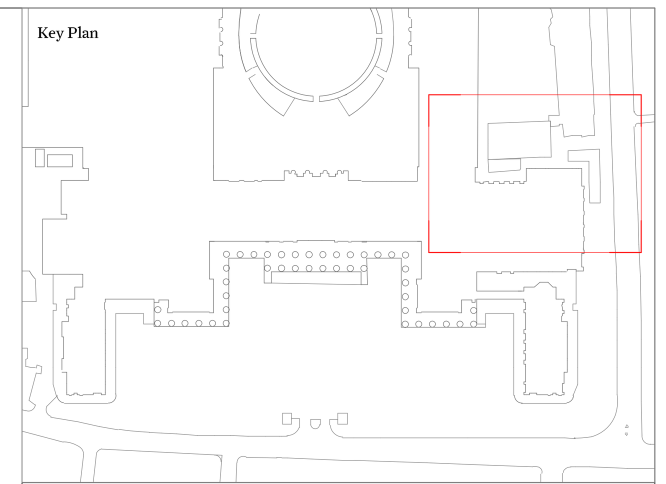
Drawing Number
10771-WW-SW-RF-DR -A-2147

| Scale | Sheet Size | Issue Date |
|---|---|---|
| 1:100 | A1 | 28.09.2023 |

| Revision | Status |
|---|---|
| P4 | S4 - For Approval |




Key Plan



This drawing is not to be scaled. Dimensions govern.
Wright & Wright Architects shall be notified in writing of any discrepancies.
All dimensions are in millimeters unless noted otherwise.
This drawing issued at First Contract is indicative of design intent and should be read in association with all specifications, schedules and consultants information.
The Contractor is required to complete the detailed design, final coordination and interfaces. Contractor and Sub-Contractors design are subject to Architects acceptance.

## Key:

- Site Boundary
- Fire Sector Boundary
- Underground drainage manhole / cover

## Landscape Key:

1. Large format natural stone paving
2. Medium format natural stone paving
3. Edge gravel
4. Natual stone outdoor bench seating
5. Relocated external northern lampost
6. New trees and planting in gravel on topsoil. Feature sculptural objects/stones in soft landscaping

| Rev | Note | Date | Issued By | Issued To |
|---|---|---|---|---|
| P6 | Planning Addendum | 07.02.2024 | JB | BM |
| P5 | Stage 3 / Planning | 28.09.2023 | JB | BM |
| P4 | Draft Planning Issue | 01.09.2023 | JB | DT |
| P3 | Draft Stage 03 Issue | 28.07.2023 | JB | DT |
| P2 | Draft Stage 03 Issue For Costing | 14.07.2023 | JB | DT |
| P1 | Draft WIP Stage 3 Issue | 31.05.2023 | JB | DT |

Security Classification: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Client

British Museum

Project Name

ECP: Incoming Substation (ISS)

Stage

RIBA Stage 04

Drawing Title

Proposed ISS Landscape Plan

Drawing Number

10771-WW-SE-RF-DR -A-2174

| Scale | Sheet Size | Issue Date |
|---|---|---|
| 1:100 | A1 | 07.02.2024 |

| Revision | Status |
|---|---|
| P6 | S4 - For Approval |

Alan Baxter

**Prepared by** Cara Malcolm
**Reviewed by** Lloyd Kershaw
**Issued** October 2023

T:\1910\1910-041\12 DTP Data\BGD Report (from Stage 3 report)\1910-41_British Museum - Below Ground Drainage and SuDS for planning submission.indd

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