Basement Impact Assessment – Revision E – Volume 1 of 2 **31A BELSIZE CRESCENT, LONDON**



31A BELSIZE CRESCENT, LONDON, NW3 5QY Basement Impact Assessment

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31A BELSIZE CRESCENT, LONDON, NW3 5QY Basement Impact Assessment Revision E

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Registration of Amendments

| Revision and Date | Amendment Details | Revision Prepared By | Revision Approved By | |
|----------------------|--|-------------------------|-------------------------|--|
| Rev. A | Inclusion of Appendix G – technical note detailing | AW | СВ | |
| 09.11.2022 | Network Rail asset location | Avv | СБ | |
| Rev. B | Updated with final NR correspondence confirming | AW | СВ | |
| 23.11.2022 | their asset will not be impacted by the proposal | Avv | СВ | |
| Rev. C | Updated following Campbell Reith review | AW | СВ | |
| 13.11.2023 | opuated following campbell keith review | Avv | СВ | |
| Rev D | Inclusion of ADD colors annondiv L | A)A/ | CD | |
| 05.02.2024 | Inclusion of ABP calcs. – appendix J | AW | СВ | |
| Rev E | Pavised CMA (Appendix E) | AW | СВ | |
| 30.05.2024 | Revised GMA (Appendix F) | AVV | СВ | |

1.0 NON-TECHNICAL SUMMARY

1.1 The site location is located at 31A Belsize Crescent, London NW3 5QY. A location plan is presented as Figure 1.1, below.



Figure 1.1: Location Plan

- 1.2 The application Site is located within the lower ground floor of a five storey (including lower ground floor and roof space) Victorian era terraced residential property of east facing aspect. A small tree is located within the front garden of the property. The area of interest is a thin finger, protruding to the front of the property. A plan detailing the Site is presented as Figure 2.1.
- 1.3 The proposed development is to include lowering of the existing basement area by approximately 900mm to accommodate further living space (bathroom and utility room) at the property. With allowance for insulation, heave protection and a thicker slab, it is anticipated the maximum excavation depth will be 1.20m.

- 1.4 The following assessments are presented:
 - Desk Study;
 - Screening;
 - Scoping;
 - Additional evidence/assessments:
 - Ground movement assessment; and
 - Consultation with adjacent infrastructure/asset owners.
 - Impact Assessment.
- 1.5 The authors of the assessments are:

Andrew Warren BSc (Hons), MSc, FGS (15 years experience); Colin Buchanan BSc (Hons), FGS (>30 years experience); Sean Smeltzer, BSc (Hons), MSc, CEng, MICE (>30 years experience); and Robert Griffiths BSc (Hons), MSc, FGS, CGeol (25 years experience).

- 1.6 The ground and groundwater conditions beneath the site are London Clay, with no groundwater within the near surface deposits.
- 1.7 The construction methods proposed are top-down, hit and miss planar wall construction with temporary props.
- 1.8 A structural monitoring strategy to control the works and impacts to neighbouring structures is recommended.
- 1.9 The BIA has assessed land stability and the impacts of the proposed development on neighbouring structures will be negligible (Burland Scale Category 0).
- 1.10 There are no impacts to the development or neighbouring properties from slope stability issues.
- 1.11 The proposed development is not within 250m of a surface watercourse. The culverted River Tyburn is indicated to be located approximately 200m west of the Site. The proposed development will not increase the percentage of hardstanding at the Site and therefore will not impact on any surface water.
- 1.12 Due to the presence of impermeable London Clay beneath the Site, there will be no hydrogeological impacts from the proposed development.
- 1.13 The BIA has identified a very low flood risk for the proposed development.

2.0 INTRODUCTION

Brief

2.1 Create Consulting Engineers Ltd (CCE) have been instructed by P-AD Architects, on behalf of Ellis Naidoo, to undertake a Basement Impact Assessment in support of a planning application for 31A Belsize Crescent, London NW3 5QY.

Planning

- 2.2 This Basement Impact Assessment (BIA) has been produced to support the planning application, reference: 2021/3159/P.
- 2.3 The BIA has been produced in accordance with the guidance laid out by Camden Borough Council in: Planning Guidance – Basements (Camden Planning Guidance, March 2018) – and the Local Plan (A5 Basements, July 2017).
- 2.4 The key elements are:
 - Desk Study assessing the history, geology, hydrogeology, hydrology and buildings local to the Site;
 - Screening;
 - Scoping;
 - Ground investigation;
 - Ground movement assessment; and
 - Impact Assessment.

Authors

2.5 The BIA has been authored by:

Andrew Warren BSc (Hons), MSc, FGS; Colin Buchanan BSc (Hons), FGS; Sean Smeltzer, BSc (Hons), MSc, CEng, MICE; and Robert Griffiths BSc (Hons), MSc, FGS, CGeol.

Sources of Information

- 2.6 The following baseline data have been referenced to complete the BIA in relation to the proposed development:
 - Geological mapping (British Geological Survey 1:50,000 solid and drift, Sheet 256, North London);

- Hydrogeological data (Magic Map: https://magic.defra.gov.uk/MagicMap.aspx);
- Current/historical hydrological data (https://www.gov.uk);
- Flood risk mapping (https://www.gov.uk);
- LB Camden, Strategic Flood Risk Assessment (produced by URS, 2014);
- LB Camden, Floods in Camden, Report of the Floods Scrutiny Panel (2013);
- LB Camden, Planning Guidance (CPG) Basements (March 2018);
- LB Camden, Camden Geological, Hydrogeological and Hydrological Study Guidance for Subterranean Development (produced by Arup, 2010);
- LB Camden, Local Plan Policy A5 Basements (2017);
- LB Camden's Audit Process Terms of Reference;

Existing and Proposed Development

2.7 The application Site is located within the lower ground floor of a five storey (including lower ground floor and roof space) Victorian era terraced residential property of east facing aspect, see Plate P1, Appendix B. A small tree is located within the front garden of the property. Photographs taken during the Site walkover (dated 19.05.2022), are presented in Appendix B. The area of interest is a thin finger, protruding to the front of the property. A plan detailing the Site is presented as Figure 2.1, below.

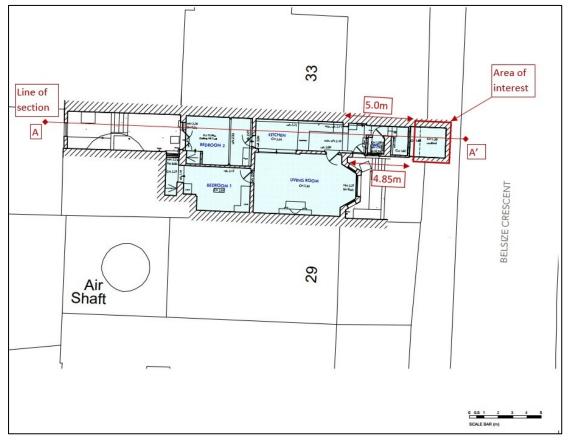


Figure 2.1: Site Plan (Extract from P-AD Architects, drawing number 7335/01 rev. B)

- 2.8 The adjacent properties are also Victorian in age, with lower ground floors. The general area of the Site slopes gently from north to south at an approximate angle of 4°. An air shaft associated with a Network Rail tunnel is located approximately twenty metres southwest of the proposed application. The Network Rail tunnel, trending east-west is indicated to pass directly beneath the Site.
- 2.9 The proposed development is to include lowering of the existing basement area (see Plate P2, Appendix C), by approximately 900mm to accommodate further living space (bathroom and utility room) at the property.
- 2.10 This will involve reducing a section of the existing basement space within the building footprint. Architects drawings are presented in full in Appendix A, with extracts detailing the existing layout (Figure 2.2a) and proposed layout (Figure 2.2b), below.

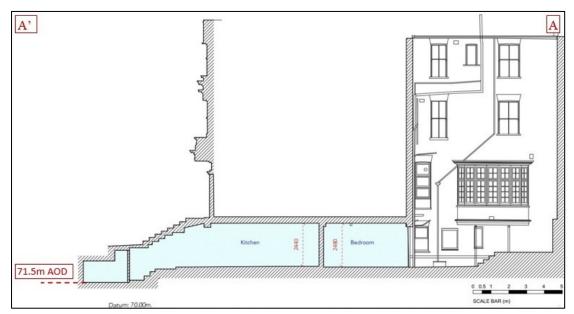


Figure 2.2a: Existing Cross-Section (Extract from P-AD Architects Drawing: 7335/04 Rev B)

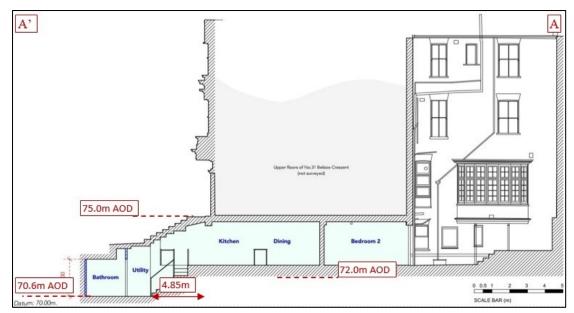


Figure 2.2b: Proposed Cross-Section (Extract from P-AD Architects Drawing: 7335/08 Rev B)

2.11 The proposed method of construction is for hit and miss underpinning of the existing wall.

3.0 DESK STUDY

Site History

- 3.1 A review of the historical plans for the Site indicates that at the time of the first historical plan of the area (six inch to one mile, dated 1866), the Site comprised farmland. By this time, the Network Rail tunnel and associated shaft had been constructed and residential properties were located 130m to the southeast.
- 3.2 The historical plan of 1894 to 1896 details the Site and surrounding area to have been developed to the current configuration.

Geology

3.3 Reference has been made to the 1:50,000 BGS plans for the Site (Sheet 256, North London, dated 2006), which indicates the Site is directly underlain by solid strata of the London Clay. However, based on the topography, there is potential for Head deposits at the Site. The British Lexicon of Named Rock Units details Head deposits as comprising poorly sorted and poorly stratified, angular rock debris and/or clayey hillwash and soil creep. Any Head deposits will therefore comprise upslope material and be of low strength.

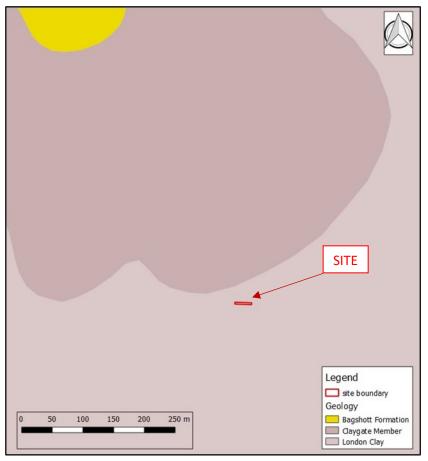


Figure 3.1: Geological Plans (Ref. British Geological Society)

Hydrogeology

- 3.4 The London Clay formation is classified as unproductive strata. The Environment Agency states that unproductive strata are unable to provide usable water supplies and are unlikely to have surface water and wetland ecosystems dependent on them.
- 3.5 The Site is not located within a source protection zone. The nearest source protection zone to the Site is an outer source protection zone, located approximately 400m south of the Site, as detailed in Figure 3.2, below.



Figure 3.2: Source Protection Zone (ref. Magicmap)

Hydrology

- 3.6 There are no surface water bodies indicated within 250m of the Site.
- 3.7 The Lost Rivers of London map, an extract of which is reproduced as Figure 3.3, below indicates the Tyburn River approximately 200m west and 200m southeast of the Site.

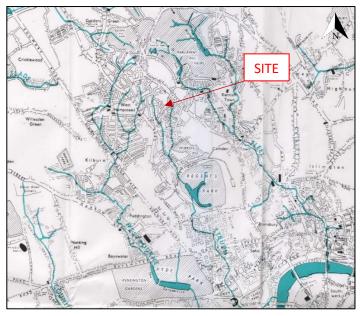


Figure 3.3: Lost Rivers of London (Barton)

3.8 The River Tyburn was culverted sometime between 1866 and 1894, when the local area was developed.

Flood Risk

- 3.9 A Flood Risk Assessment has been completed in accordance with the National Planning Policy Framework (2021) and Planning Practice Guidance in order to meet the requirements of a basement Impact Assessment as required in The London Borough of Camden flood risk management strategy, Campden Planning Guidance and Camden Development policies.
- 3.10 The assessment concluded that there are no significant sources of flooding that would warrant a more detailed Flood Risk Assessment. A number of mitigation measures are recommended to address the low risk of surface waterflooding and groundwater flooding and residual risks of flooding from public sewers, site drainage and public and internal water supply pipe work/storage. These risks can be managed by the design of the site drainage, by regular inspection and maintenance of the public and private sewer and water supply network.
- 3.11 A copy of this report is provided in Appendix D.

Historic Buildings

3.12 A search of the Historic England website indicates No.24 Belsize Crescent comprises a Grade II listed building, including walls and gate piers, as detailed in Figure 3.4, below.

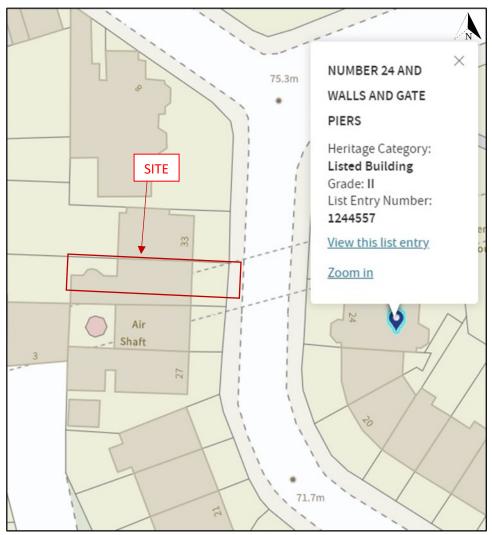


Figure 3.4: Listed Buildings (Historic England)

Utilities

3.13 Services plans, as reproduced in Appendix C indicate gas, electric, foul water and water mains within Belsize Crescent.

4.0 SCREENING

4.1 A screening process has been undertaken and the findings are described below.

| Question | Response | Details |
|---|----------|---|
| 1a. Is the site located directly above an | No | Underlying geology is unproductive strata of |
| aquifer? | | London Clay Formation. |
| 1b. Will the proposed basement extend | No | Underlying strata comprises the aquiclude, |
| beneath the water table surface? | | London Clay. |
| 2. Is the site within 100m of a watercourse, | No | No. Closest watercourse is greater than 250m |
| well (used/disused) or potential spring line? | | from the Site. The Site is not near a spring line |
| | | as the surrounding geology is London Clay. |
| 3. Is the site within the catchment of the pond | No | The Hampstead Heath Ponds are 900m to the |
| chains on Hampstead Heath? | | northeast. |
| 4. Will the proposed basement development | No | The proportion of hardstanding at the Site |
| result in a change in the proportion of hard | | will not change. |
| surfaced/paved areas? | | |
| 5. As part of site drainage, will more surface | No | SUDS are not proposed as part of the |
| water (e.g. rainfall and run-off) than at present | | development. |
| be discharged to the ground (e.g. via | | |
| soakaways and/or SUDS)? | | |
| 6. Is the lowest point of the proposed | No | There are no surface water features within |
| excavation (allowing for any drainage and | | 250m of the Site. |
| foundation space under the basement floor) | | |
| close to, or lower than, the mean water level | | |
| in any local pond (not just the pond chains on | | |
| Hampstead Heath) or spring line? | | |

Table 4.1: Groundwater and Flooding Screening

Slope Stability

| Question | Response | Details |
|---|----------|--|
| 1. Does the existing site include slopes, natural | No | The local topography slopes to the south and |
| or man-made greater than 7 degrees | | east at approximately 4°. |
| (approximately 1 in 8)? | | |
| 2. Will the proposed re-profiling of landscaping | No | No landscaping will be undertaken as part of |
| at the site change slopes at the property | | this project. |
| boundary to more than 7 degrees | | |
| (approximately 1 in 8)? | | |
| 3. Does the development neighbour land, | No | There are no railway cuttings within the |
| including railway cuttings and the like, with a | | vicinity of the Site. |
| slope greater than 7 degrees (approximately 1 | | |
| in 8)? | | |
| 4. Is the site within a wider hillside setting in | No | The local topography slopes at approximately |
| which the general slope is greater than 7 | | 4°. |
| degrees (approximately1 in 8)? | | |
| 5. Is the London Clay the shallowest strata at | Yes | See BGS Sheet 256, North London and |
| the site? | | findings of Site investigation (Section 6.0 of |
| | | this report). |

| Question | Response | Details |
|--|----------|---|
| 6. Will any trees be felled as part of the | No | No trees will be felled as part of the proposed |
| development and/or are any works proposed | | works. And, there are no tree protection |
| within any tree protection zones where trees | | zones within the area of proposed work. |
| are to be retained? | | |
| 7. Is there a history of seasonal shrink-swell | No | The existing lower ground floor is 1.20m |
| subsidence in the local area and/or evidence of | | below ground level and therefore outwith |
| such effects at the site?` | | the zone of influence from seasonal |
| | | fluctuations in moisture levels. |
| 8. Is the site within 100m of a watercourse or a | No | Closest watercourse is >250m. The local |
| potential spring line? | | geology is London Clay |
| 9. Is the site within an area of previously | No | Historical maps do not indicate the Site is |
| worked ground? | | located in an area of worked ground. |
| 10. Is the site within an aquifer. If so, will the | No | Underlying geology comprises London Clay. |
| proposed basement extend beneath the water | | |
| table such that dewatering may be required | | |
| during construction? | | |
| 11. Is the site within 50m of the Hampstead | No | The Hampstead Heath Ponds are >900m to |
| Heath Ponds? | | the northeast. |
| 12. Is the site within 5m of a highway or | Yes | The Site is located directly beneath a |
| pedestrian right of way? | | pedestrian right of way. |
| 13. Will the proposed basement significantly | No | The maximum excavation depth is 1.20m. |
| increase the differential depth of foundations | | |
| relative to neighbouring properties? | | |
| 14. Is the site over (or within the exclusion | Yes | See correspondence with Network Rail, |
| zone of) any tunnels, e.g., railway lines? | | Appendix G and technical note detailing |
| | | tunnel location, Appendix H. |

Table 4.2: Slope Stability Screening

Surface Water and Flooding

| Question | Response | Details |
|--|----------|---|
| 1. Is the site within the catchment of the ponds | No | The Hampstead Heath Ponds are 900m |
| chains on Hampstead Heath? | | northeast of the Site. |
| 2. As part of the proposed site drainage, will | No | Hardstanding at the Site will not change. The |
| surface water flows (e.g., volume of rainfall | | proposed development will not increase the |
| and peak run-off) be materially changed from | | footprint of the existing structure. |
| the existing route? | | |
| 3. Will the proposed basement development | No | Hardstanding at the Site will not change. The |
| result in a change in the proportion of hard | | proposed development will not increase the |
| surfaced/paved external areas? | | footprint of the existing structure. |
| 4. Will the proposed basement result in | No | Hardstanding at the Site will not change. The |
| changes to the profile of the inflows | | proposed development will not increase the |
| (instantaneous and long-term) of surface | | footprint of the existing structure. |
| water being received by adjacent properties or | | |
| downstream watercourses? | | |
| 5. Will the proposed basement result in | No | Hardstanding at the Site will not change. The |
| changes to the quality of surface water being | | proposed development will not increase the |
| received by adjacent properties or | | footprint of the existing structure. |
| downstream watercourses? | | |

| Question | Response | Details |
|--|----------|--|
| 6. Is the site in an area identified to have | No | The nearest surface water feature is greater |
| surface water flood risk according to either the | | than 250m from the Site. |
| Local Flood Risk Management Strategy or the | | |
| Strategic Flood Risk Assessment or is it at risk | | |
| from flooding, for example because the | | |
| proposed basement is below the static water | | |
| level of nearby surface water feature. | | |

 Table 4.3: Surface Water and Flooding Screening

Non-Technical Summary of Screening Process

- 4.2 The screening process identifies the following issues to be carried forward to scoping for further assessment:
 - The shallowest strata at the Site is the London Clay Formation.
 - The proposed development is located directly beneath a pedestrian walkway.
 - The proposed development is located directly over a Network Rail asset.
 - A listed building is located approximately 10m from the proposed development.
 - Below ground services are located within the adjacent road.
- 4.3 The other potential concerns considered within the screening process have been demonstrated to be not applicable or not significant when applied to the proposed development.

5.0 SCOPING

5.1 The following issues have been brought forward from the Screening process for further assessment:

The shallowest strata at the Site is London Clay Formation

5.2 There is potential for heave to occur as a result of excavations undertaken within the London Clay Formation. The proposed development is for a maximum excavation of 1.20m. Therefore, floor slab design will account for heave potential.

The proposed development is located directly beneath a pedestrian walkway

- 5.3 The existing lower ground floor is located directly beneath a pedestrian highway. The proposed development will not increase the footprint of the existing structure.
- 5.4 No further action required.

The proposed development is located directly over a Network Rail Asset

- 5.5 The technical note presented in Appendix H details the tunnel location with respect to the proposed development. And demonstrates that based on proposed elevations and tunnel elevations, the proposed development will not impact upon the Network Rail asset.
- 5.6 The Network Rail Asset Protection team have confirmed the proposed development will not impact upon their asset. Correspondence with Network Rail is presented within Appendix G.

Listed Building

5.7 There is potential for the proposed development to impact upon the listed building, approximately 10m west of the proposed development.

Below Ground Services

5.8 Below ground services are indicated within the adjacent road.

6.0 SITE INVESTIGATION AND ADDITIONAL ASSESSMENTS

Description of Fieldwork

- 6.1 Site investigation works were undertaken on 15th September 2023 and comprised:
 - Drilling of 1No. windowless sample borehole (WS01) to a depth of 5.0m below ground level (bgl), to determine ground and groundwater conditions, provide soil strength information and enable the collection of soil samples for laboratory geotechnical testing;
 - Installation of a groundwater monitoring standpipe and subsequent monitoring of groundwater levels; and
 - Laboratory geotechnical testing of Site soils.
- 6.2 The soil arisings were logged by a suitably qualified Engineer, in line with the relevant British Standard (BS 5930 and Eurocode 7). The borehole log is included within Appendix E.
- 6.3 All works were undertaken in accordance with the CCE Health and Safety Policy and within the framework of a Health and Safety plan.
- 6.4 The borehole was undertaken within the front garden of the property at a level of 73.60m aOD, approximately 200mm below the adjacent footpath. An exploratory hole location plan is presented in Appendix E.

Laboratory Geotechnical Testing

- 6.5 Selected soil samples were subjected to testing within a UKAS accredited geotechnical laboratory. The results of this testing, along with laboratory certificates are included as Appendix E. Laboratory testing comprise:
 - 2No. plastic limit tests;
 - 2No. moisture content tests; and
 - 3No. pH and sulphate tests.

Ground Conditions

Topsoil / Made Ground

6.6 Topsoil was recorded to 0.50m. The Topsoil was underlain by Made Ground to 1.40m bgl (72.2m aOD), at a level approximate to the base of the lower ground floor steps. The Made Ground was recorded as: '*soft brown black silty clay, track rounded fine to medium flint and brick.*'

6.7 A single chemical test undertaken on a sample from the Made Ground, recorded a pH of 8.1, SO₄ of 25mg/l and total sulphur of 0.058%.

Weathered London Clay

- 6.8 The Made Ground was underlain by soils considered to represent weathered London Clay, to the maximum extent of the borehole, 5.0m bgl (68.60m aOD). The weathered London Clay was described as: *'firm to stiff orange-brown, mottled grey silty clay'*.
- 6.9 Strength testing on recovered soils samples recorded material strengths in the range of 78kPa to 158kPa, as detailed in the strength dept plot, Appendix E.

| Test | No. of tests | Result |
|------------------|--------------|-------------------|
| Moisture Content | 2 | 31.3% and 32.5% |
| Plasticity Index | 2 | 43% and 53% |
| рН | 2 | 8.1 and 8.2 |
| SO4 2 | | 10mg/l and 95mg/l |
| Total sulphur | 2 | 0.01% |

 Table 6.1: Weathered London Clay summary of geotechnical test results

6.10 Plotting the results on the plasticity chart, Appendix E, indicates the material is a clay of very high plasticity and therefore of high volume change potential.

<u>Groundwater</u>

6.11 During exploratory hole formation, groundwater was not encountered. Subsequent monitoring of the installed standpipe (28th September 2023) did not record groundwater.

Allowable Bearing Pressure

6.12 Based on the recorded ground conditions, the following ground model and design parameters have been determined, as summarized in Table 6.2.

| Lithology | Depth to base | Y (kN/m³) | PI (%) | C' (Kn/m²) | Phi (°) | mv (MN/m²) | E (MN/m²) |
|--------------------------|-----------------------|--------------|-----------|---------------|------------|---------------|--------------|
| Made Ground | 1.4m, 72.2m aOD | 17 | 20 | 24 | 5 | - | - |
| Weathered London Clay | >5.0m, 68.6m aOD | 18 | 45 | 98 | 20* | 0.141 | 20,000 |
| Groundwater | >4.0m bgl (69.6m aOD) | | | | | | |

* BS8002

Table 6.2: Ground Model Design Parameters

Design Factors

- Permanent design load: EC7 DA1b
- tan ø, 1.25
- c'*,* 1.6
- c_u, 1.4
- q_u, 1.4

Allowable Bearing Pressure

6.13 On the basis of the ground model, the following is calculated at basement formation level:

$$q_a = \frac{1}{F} \left(c.N_c + \sigma_0.N_q + \frac{1}{2}.B.\gamma.N_\gamma \right)$$

where;

 $N_c,\,N_q$ and N_Y are bearing capacity factors

c' is the cohesive strength of the soil (kN/m^2)

B is the width of the foundation (m)

 Υ is the unit weight of the soil (kN/m³)

 σ_0 is the overburden pressure

F is a factor of safety against bearing capacity failure

6.14 With formation level at approximately 2.30m bgl (71.3m aOD), an allowable bearing pressure of **198kN/m²** is calculated. The calculations for this are provided in Appendix J.

Heave

6.15 Due to the reduction in pressure from overburden, excavations in clay will result in heave. A conservative estimate of long-term heave as a result of excavations is 0.15%. Therefore, for an excavation of 1.20m, an allowance of 1.8mm heave protection should be incorporated within the slab design.

Buried Concrete

- 6.16 Based on the chemical laboratory test results (BRE SD1 suite) and in accordance with BRE Special Digest 1: 2005 (Concrete in Aggressive Ground), the following criteria have been determined. A total of 3No. BRE SD1 chemical suits were undertaken on soil samples from the Made Ground and London Clay horizons.
- 6.17 The site is classified as 'natural ground' and the local geology (weathered London Clay) is indicated to contain pyrite (i.e., sulphide) and groundwater conditions are considered to be 'static'. Laboratory chemical testing recorded water soluble sulphate concentrations in the soil of between 10mg/l and 95mg/l. Therefore, the Design Sulphate Class for the site is considered to be "**DS-1**". pH values of between 8.1 and 8.2 were also recorded. Therefore, the

"Aggressive Chemical Environment for Concrete (ACEC)" class for concrete in the ground is indicated to be **AC-1**.

Ground Movement Assessment

- 6.18 The existing lower ground floor at No.31A Belsize Crescent extends to the front of the building.
- 6.19 The proposal is to create additional living space and lower formation level by a maximum of 1.20m bgl.
- 6.20 Ground movement during basement construction can occur as a result of the basement construction (underpinning) and due to the excavation of soil from the basement area, which results in a bending moment on the wall from earth pressure. The methodology for calculating ground movement in CIRIA document C760 'Guidance on Embedded Retaining Wall Design, 2017', has been followed, with calculation sheets presented in Appendix F.
- 6.21 The existing foundation profile is not known this will be determined when construction works begin and a section of the basement is broken out. Therefore, for the analysis, a worst-case scenario approach has been taken.
- 6.22 The maximum excavation depth will be 1.20m. The proposed method of wall construction will be by underpinning. Based on the method of construction, an estimate of 5mm vertical and horizontal movement has been determined by Campbell Reith who undertake technical reviews of BIAs for Camden Borough.
- 6.23 Notwithstanding, with a maximum depth of excavation of 1.20m below existing formation level and at a distance of greater than 4.80m from the proposed excavation, the façades of No.31 and 33 Belsize Crescent are outwith the zone of influence from ground movement due to basement construction, as detailed in the calculation sheets provided in Appendix F.
- 6.24 Ground movements as a result of basement construction are within tolerable limits and will not adversely impact upon buried services within the adjacent road.

7.0 CONSTRUCTION METHODOLOGY/ENGINEERING STATEMENTS

Existing Basement Structure

- 7.1 The house is part of a terrace of similar houses, although the section of basement under consideration here projects from the front of the terrace and does not include party walls on either side (Fig. 7.1). The wall on one side does form the boundary with the adjacent plot. It has been noted that the adjacent property is extending their basement below their front garden, although the extent of this work is unknown.
- 7.2 The proposed works impact only a portion of the existing basement, being the section below the front access path and steps, including a section which projects below the public footpath. The ground level either side of the path and steps is lower than that of the footpath (Figs. 7.2 and 7.3).
- 7.3 The walls are of solid brick construction. The existing foundations are not known at this stage, but are likely to be brick footings, possibly with corbels to spread the load.

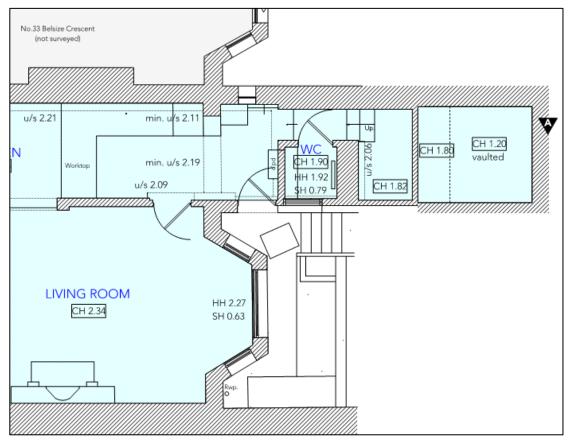


Figure 7.1: Existing basement footprint (Partial)

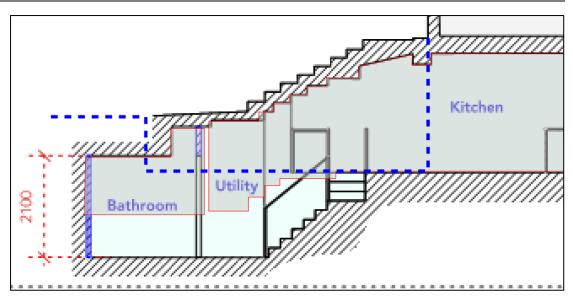


Figure 7.2: Proposed basement part long section with existing overlaid and assumed ground levels (blue dashed line)

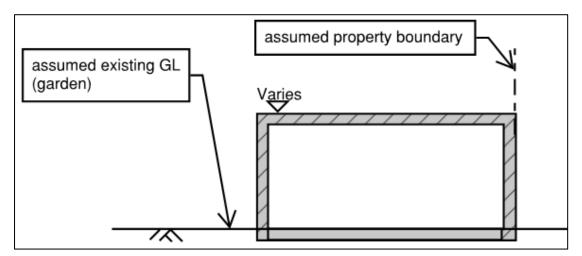


Figure 7.3: Simplified section showing existing construction

Proposed Basement Structure

- 7.4 The proposals are intended to lower the floor of the projecting basement portion, to improve headroom and make the space more inhabitable.
- 7.5 The simplest way to construct the extended basement would be to demolish the access steps and path, and to construct the new sections of basement, followed by reconstructing the portions above the garden level. However, this option has initially been rejected.
- 7.6 Given the desire to retain the existing path, steps, and wall, i.e., the roof and sides of the existing basement, the proposed solution is to remove the existing slab and to construct new concrete walls below the existing walls, using an underpinning type solution, followed by constructing a monolithic slab connecting the walls (Fig. 7.4). It should be noted that the

portion of reduced floor level closest to the house only extends across half the width of the basement. This must be taken into account when designing the structural works.

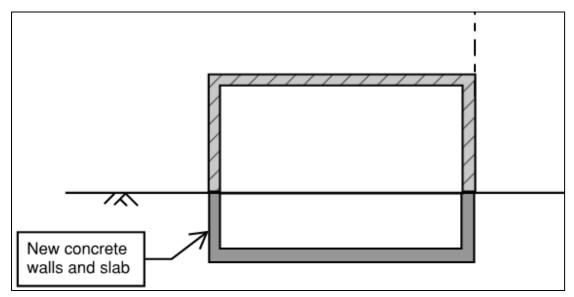


Figure 7.4: Simplified section showing proposed construction

Proposed Sequence of Working

- 7.7 Priority has to be given to the stability of the building at all times. There are several options for sequencing the construction of this basement extension, and the final choice will be made following discussion with the Contractor and the Structural Engineer.
- 7.8 Note the following elements of the structural works (Letters refer to Fig. 7.5):
 - A. This area of floor is dropped by a varying amount, 1, 2, or 3 steps. Construction details should be agreed on site.
 - B. This wall forms part of the slightly dropped floor in A. Together with the wall on the opposite side, it can either be the first or last section to be built (but see note below)
 - C. The floor in this area is raised slightly from the original. It should be built up off the original floor.
 - D. This wall will be stepped at the edge of section A and between sections 1 and 2.
 - E. This wall underpins part of the solid brick pier (as does part of wall D).
 - F. This is the end wall.

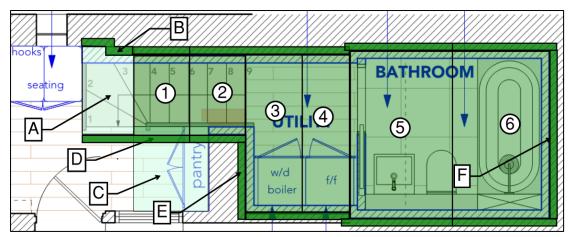


Figure 7.5: Construction sequence and structural elements

- 7.9 One possible sequence is outlined below:
 - 1. Starting with section 1 (Fig. 7.5).
 - 2. Excavate locally below and between existing walls and trim corbelled footings if necessary Note that the footings may not be corbelled masonry, and may be strip/mass concrete which would be treated in the same way. Foundation trimming should be by saw cutting and not by hydraulic breaker, to minimise potential for damage to the retained portions of the structure..
 - 3. Construct short sections of RC slab (approx. 1.0m) across width of basement.
 - 4. Construct retaining walls between the new slab and the existing walls.
 - 5. Once concrete has reached sufficient strength dry pack existing walls to ensure full and even load transfer.
 - 6. Repeat steps 1 4, working away from the house. Waterproofing between adjacent sections should be achieved by the use of water bars.

Note: If desired, a hit and miss sequence can be adopted, whereby sections 1 and 4 are constructed simultaneously, followed by sections 2 and 5 and finally sections 3 and 6.

7.10 Figure 7.6, below, provides a simplified section of underpinning wall and slab. Depending on the length of wall in a given section, temporary support may be required. This can be provided in one of several ways. For example, a continuous angle of PFC section can be bolted to the wall, above the excavation. If the footing is corbelled brickwork, this member should be fixed to the cut face. Temporary props would then support this member either side of each section, while it is being excavated. Once the concrete has been placed and cured, and the dry pack installed, the props can be removed and repositioned either side of the next section. Also note that this sequence does not impact the existing path between the public footpath and the house, which can be kept in use during the work. Note that temporary support may be required to the excavations formed as part of the underpinning work. The design, implementation, maintenance and removal of such temporary works are the responsibility of the Contractor under regulation 15 of The Construction (Design and Management) Regulations (CDM) and are not covered by this report.

- 7.11 Note that this is a simplified sequence, prepared to demonstrate viability. It may be possible to progress more than one section at the same time. In this case, at least one, and preferably two sections should be left between the sections being constructed. Any variation to the sequence proposed in section 6.9 must be agreed with the designer before implementing.
- 7.12 It is possible that these works will result in some movement of the building and the footpath, and that such movement will result in minor cracking to the property. This is to be expected as the load paths adjust to the new structural form. It is important that a party wall agreement and pre-start condition survey are in place before the work starts. It is also recommended that regular monitoring takes place during the construction phase, with agreed trigger points and actions.

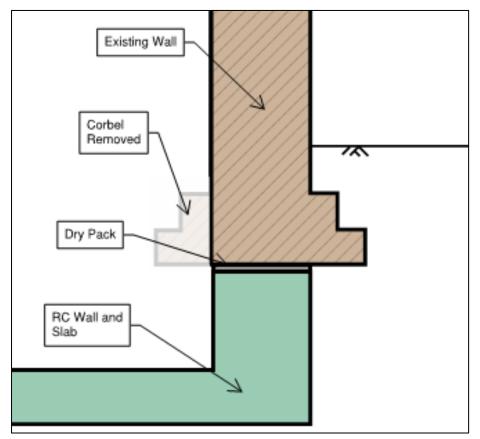


Figure 7.6: Detail of underpinning wall and slab

8.0 BASEMENT IMPACT ASSESSMENT

- 8.1 Based on our understanding of the Site and proposed development, the following conceptual site model (CSM) has been developed.
 - Ground conditions at the Site are indicated to comprise solid strata of the London Clay Formation. Based on the local topography, there is the potential for Head deposits at the Site.
 - Due to the presence of London Clay beneath the Site, groundwater is not anticipated to impact upon the proposed development.
 - The local topography slopes at an angle of 4° to the south/southwest.
 - The existing floor level is indicated to be at 71.5m aOD.
 - Proposed finished floor level is indicated to be at 70.6m aOD. Allowing for insulation, a thicker floor slab and heave protection, anticipated formation level will be at 70.3m aOD.
 - An allowable bearing pressure of 375kN/m² is calculated.
 - A concrete classification of DS-1, AC-1 is calculated.
 - The depths of neighbouring foundations/basements are indicated to be at an equivalent level to existing Site levels.
 - The proposed development is located directly beneath a footpath.
 - The proposed development is located directly over a Network Rail asset.
 - The proposed development is relatively minor and not indicated to impact upon local infrastructure.
 - Proposed mitigation is for good construction methodology.
 - Residual impacts are not anticipated.
 - Groundwater monitoring within the installed standpipe did not record groundwater to 4.0m bgl (69.6m aOD).

Land Stability/Slope Stability

- 8.2 The indicated founding strata at the Site is the London Clay Formation.
- 8.3 The risk of movement and damage to this development due to shrink and swell of the London Clay is negligible considering the minor depth of excavation (1.20m).
- 8.4 Based on the distance from the excavation to building façade, the Ground Movement Assessment has concluded that ground movements caused by the excavation and construction of the proposed development will be negligible. The Damage Impact to surrounding structures within the zone of influence has been assessed as Category 0, in accordance with the Burland Scale.

8.5 The BIA has concluded that there will not be risks or stability impacts to the development or adjacent sites due to slopes. With good construction techniques, comprising hit and miss construction, with propping prior to slab formation, residual impacts are not considered likely.

Hydrogeology and Groundwater Flooding

- 8.6 The BIA has concluded there is a very low risk of groundwater flooding to the proposed development.
- 8.7 The BIA has concluded there are no impacts to the wider hydrogeological environment from the proposed development.

Hydrology, Surface Water Flooding and Sewer Flooding

- 8.8 The BIA has concluded there is a low risk of surface water/sewer flooding to the proposed development.
- 8.9 The BIA has concluded there are no impacts to the wider hydrological environment from the proposed development.

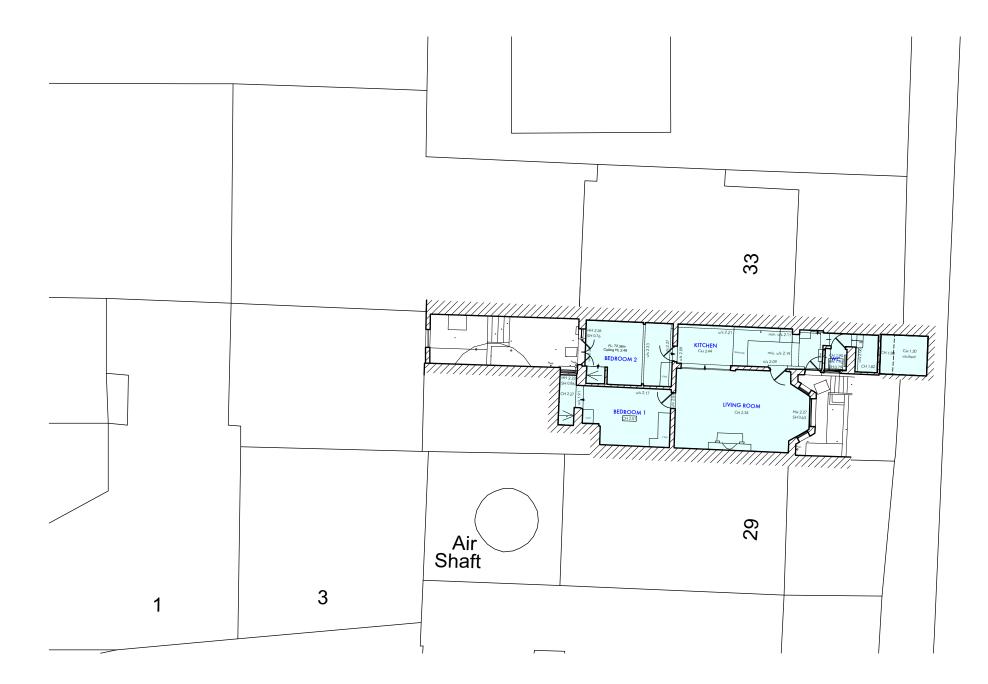
9.0 CONSTRAINTS AND LIMITATIONS

- 9.1 The copyright of this report is vested in Create Consulting Engineers Limited and the Client, Ellis Naidoo. The Client, or their appointed representatives, may copy the report for purposes in connection with the development described herein. It shall not be copied by any other party or used for any other purposes without the written consent of Create Consulting Engineers Limited or the Client.
- 9.2 Create Consulting Engineers Limited accepts no responsibility whatsoever to other parties to whom this report, or any part thereof, is made known. Any such other parties rely upon the report at their own risk.
- 9.3 Create Consulting Engineers Limited has endeavoured to assess all information provided to them during this appraisal. Should additional information become available which may affect the opinions expressed in this report, Create Consulting reserves the right to review this information and, if warranted, to modify the opinions presented in the report accordingly.
- 9.4 The report summarises information from a number of external sources and is unable to offer any guarantees or warranties for the completeness or accuracy of information relied upon. Information from third parties has not been verified by Create Consulting Engineers Limited unless otherwise stated in this report.
- 9.5 It should be noted that the risks which are identified in this report are perceived risks based on the available information at the time of writing and that the actual risks associated can only be assessed following a physical investigation of the site.
- 9.6 The conclusions resulting from this study are not necessarily indicative of future conditions or operating practices at or adjacent to the site.

APPENDICES

APPENDIX A

ARCHITECTURAL DRAWINGS



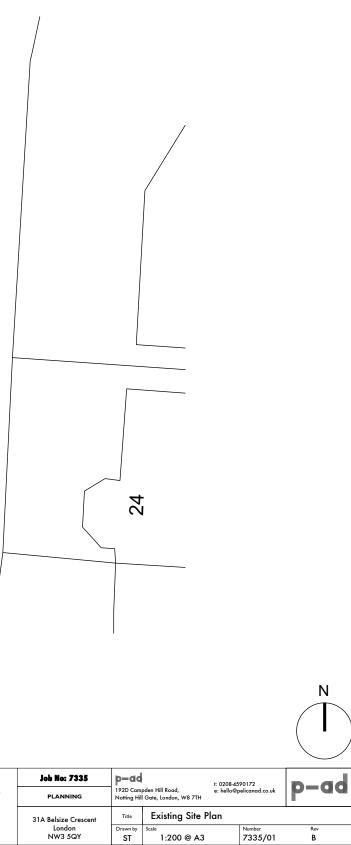
BELSIZE CRESCENT

EXISTING SITE PLAN

WRITTEN DIMENSIONS ONLY TO BE USED ALL DIMENSIONS ARE IN MILLIMETRES ALL DIMENSIONS TO BE VERIFIED ON SITE ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY

0 0.5 1 2 3 4 5 SCALE BAR (m)

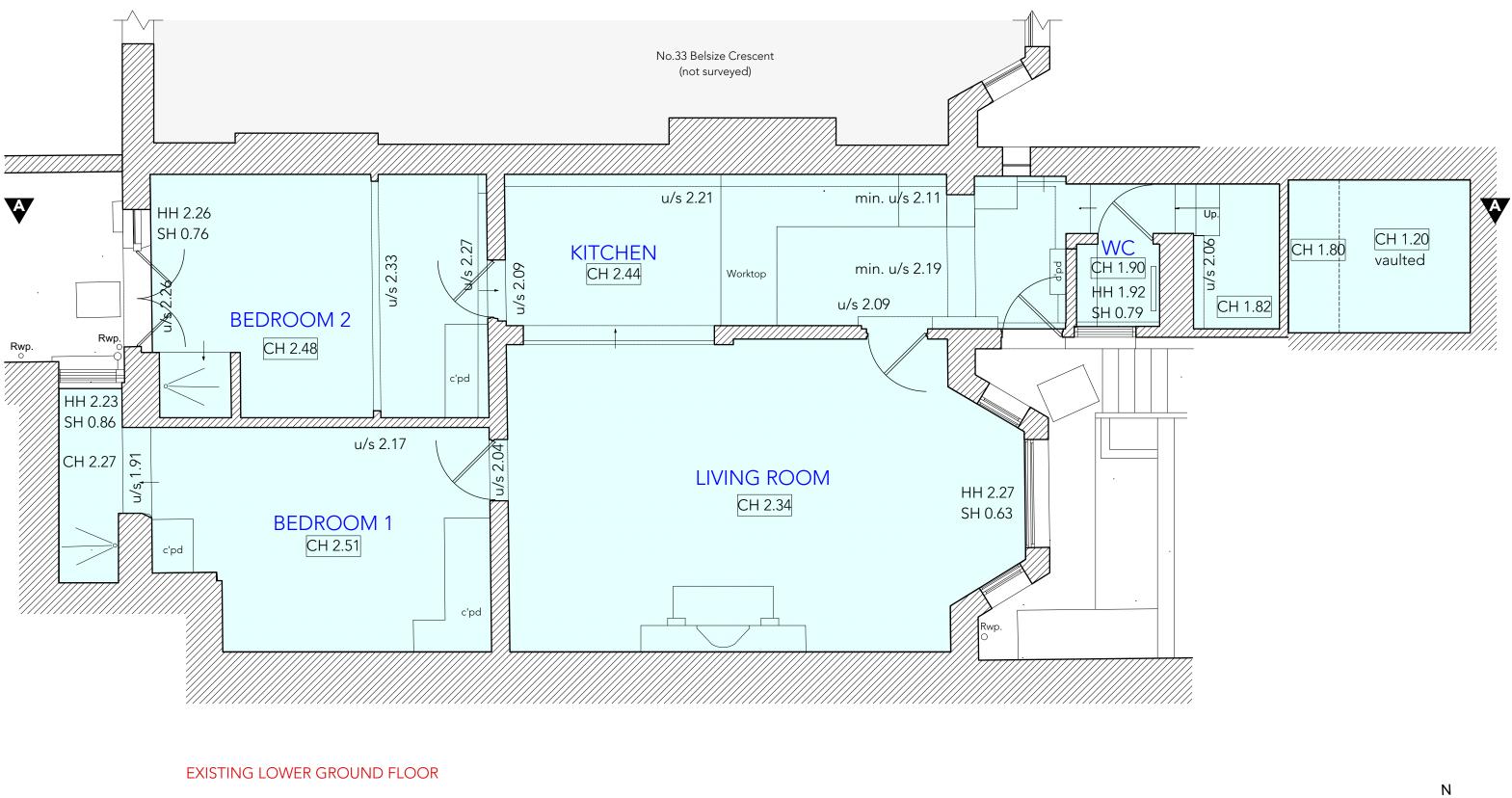
EXISTING SITE PLAN



Number 7335/01

В

1:200 @ A3



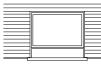


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EXISTING PLAN

| Job No: 7335 | p-ad | | t: 0208-4590172 | n_ad | |
|----------------------|----------|---|--------------------------|------|--|
| PLANNING | | pden Hill Road, I Gate, London, W8 7TH | e: hello@pelicanad.co.uk | p-dd | |
| 31A Belsize Crescent | Title | Existing Plans | | | |
| London | Drawn by | Scale | Number | Rev | |
| NW3 5QY | ST | 1:50 @ A3 | 7335/02 | В | |







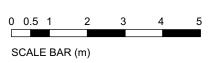




Datum: 70.00m.

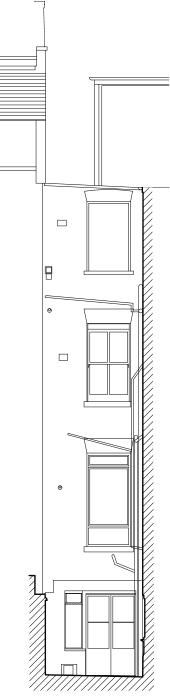
EXISTING FRONT ELEVATION

EXISTING REAR ELEVATION

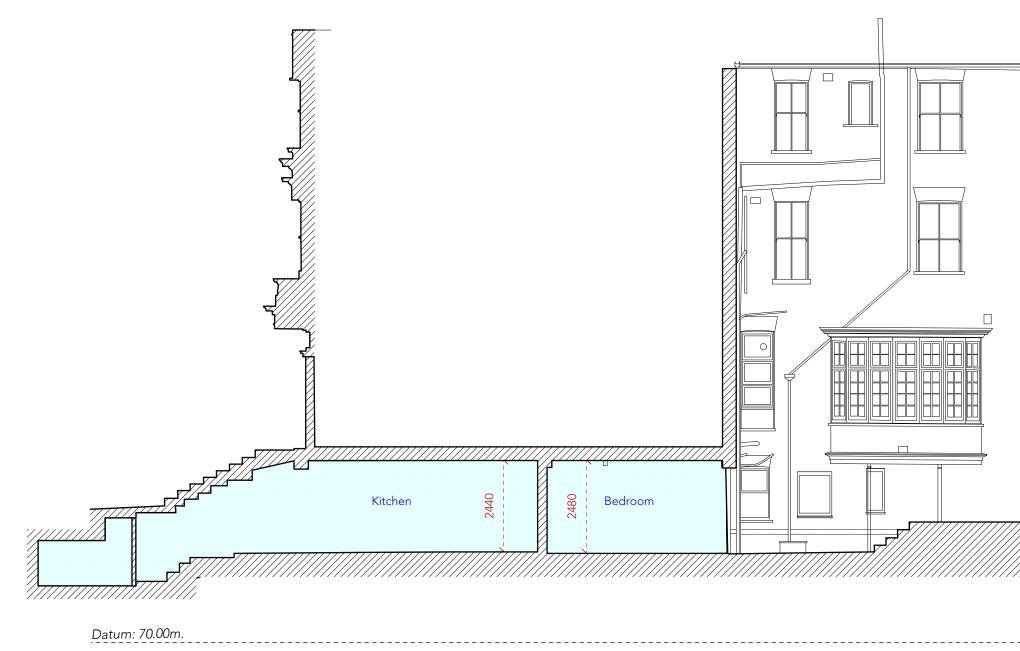




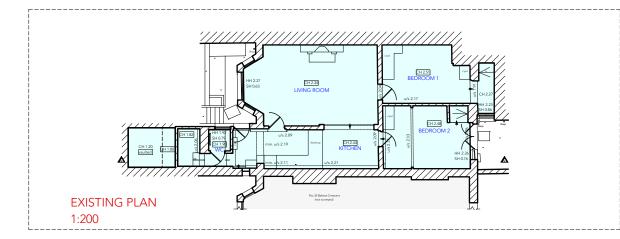
EXISTING ELEVATIONS



| | Job No: 7335 | p-ad | | t: 0208-4590172 | p_ad |
|---|----------------------|----------|---|-------------------------|------|
| | PLANNING | | pden Hill Road, I Gate, London, W8 7TH | e: hello@pelicanad.co.u | P-dd |
| ĺ | 31A Belsize Crescent | Title | Existing Eleve | itions | · |
| | London | Drawn by | Scale | Number | Rev |
| | NW3 5QY | ST | 1:100 @ A3 | 7335/0 | 3 B |



EXISTING SECTION AA



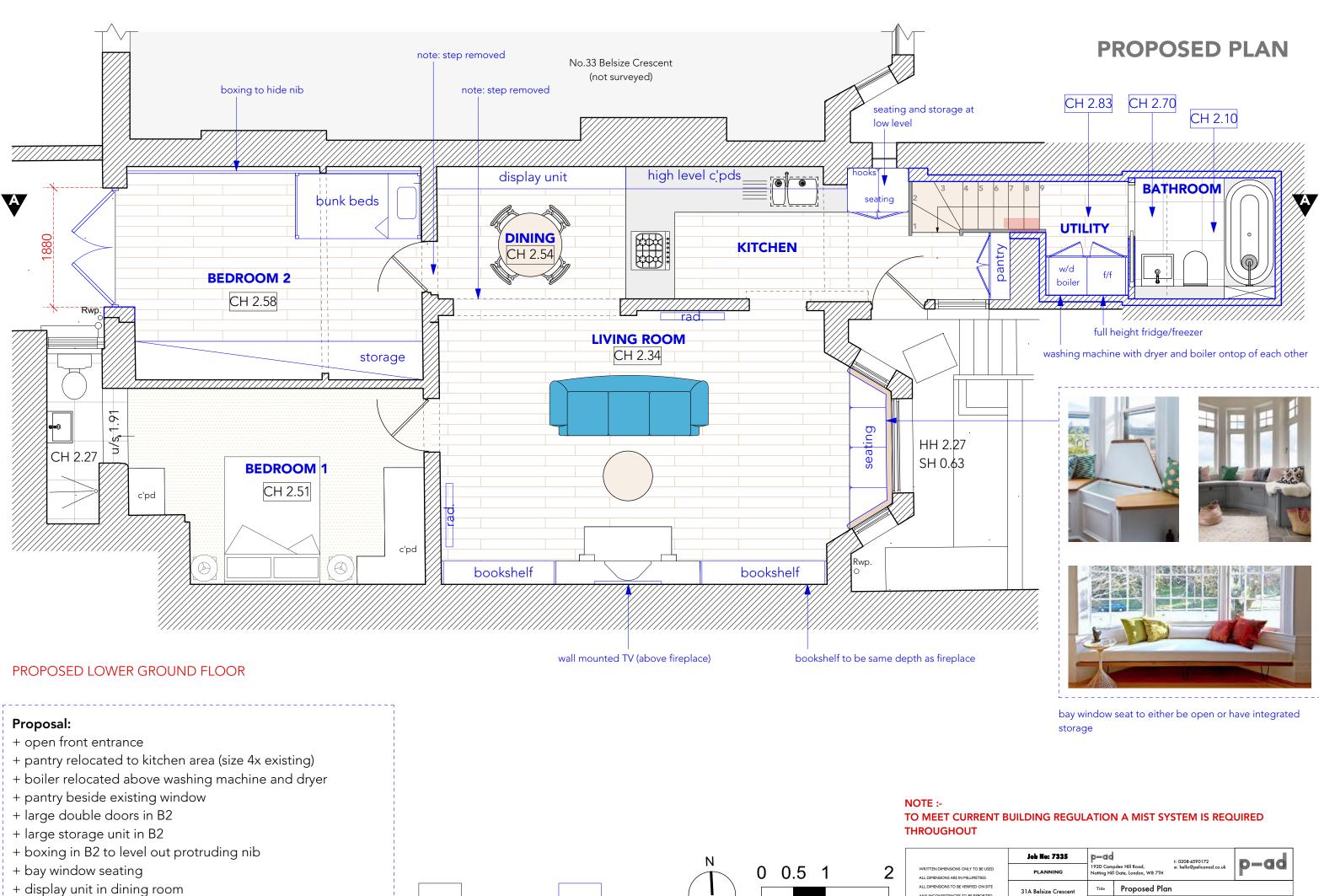


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EXISTING SECTION



| - | Job No: 7335 Planning | | pden Hill Road, I Gate, London, W8 7TH | t: 0208-4590172 e: hello@pelicanad.co.uk | p-ad |
|---|--------------------------|----------------|---|---|----------|
| | 31A Belsize Crescent | Title | Existing Section | on AA | |
| | London NW3 5QY | Drawn by ST | Scale 1:100 @ A3 | Number 7335/04 | Rev B |



PROPOSED

EXISTING

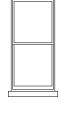
- + bookshelves in living room (same width as fireplace)

| | Job No: 7335 Planning | | pden Hill Road, I Gate, London, W8 7TH | t: 0208-459 e: hello@pe | 0172 Ilicanad.co.uk | p-ad |
|--|---|----------------|---|----------------------------|------------------------|----------|
| | 31A Belsize Crescent London NW3 5QY | Title | Proposed Plan | ı | | |
| | | Drawn by ST | Scale 1:50 @ A3 | | Number 7335/05 | Rev B |

ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY







replacement of existing double doors and window with timber framed double glazed doors

Datum: 70.00m.

PROPOSED FRONT ELEVATION

NOTE: FRONT ELEVATION REMAINS UNCHANGED

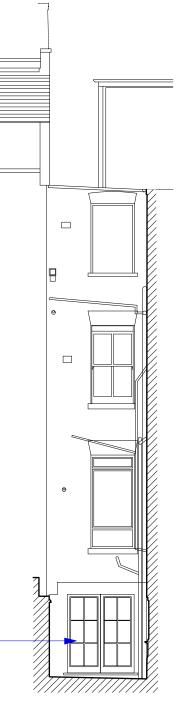


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0 0.5 1 2 3 4

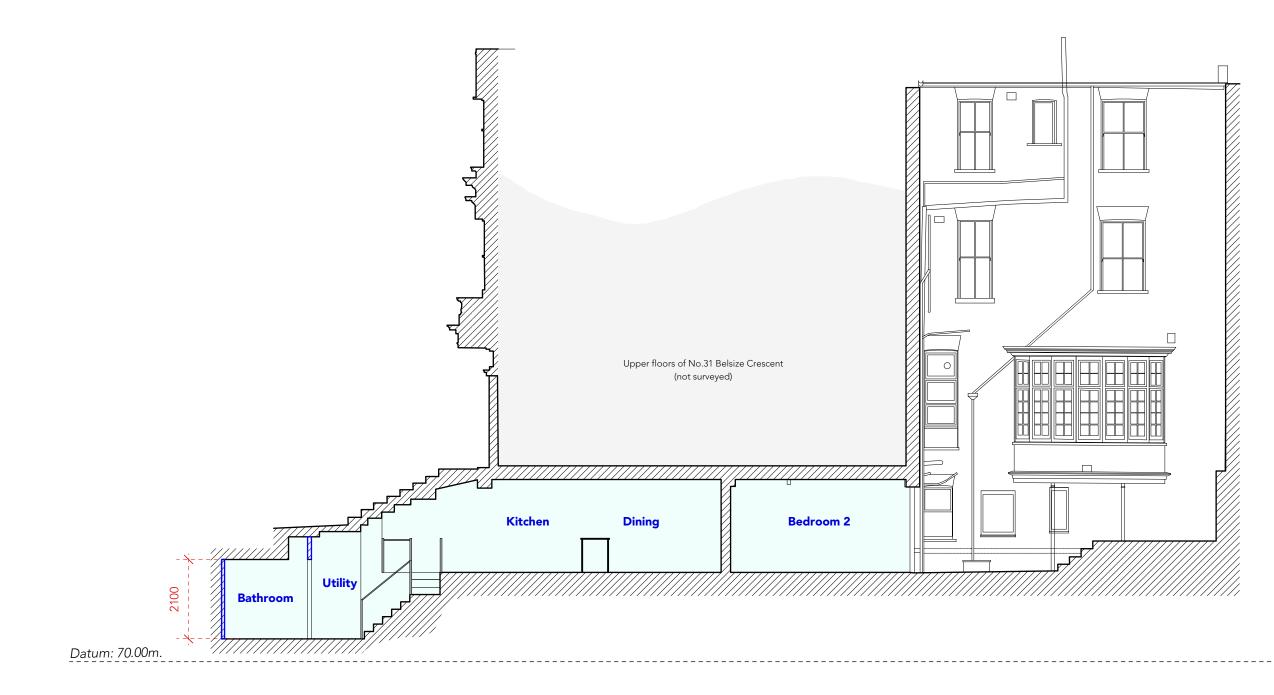


PROPOSED ELEVATIONS

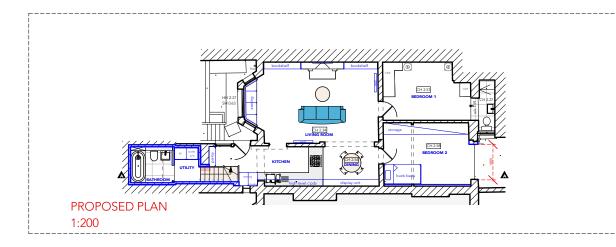


PROPOSED REAR ELEVATION

| Job No: 7335 | | pden Hill Road, I Gate, London, W8 7TH | t: 0208-4590172 e: hello@pelicanad.co.uk | | p-ad | |
|----------------------|----------------|---|---|-------------------|----------|--|
| 31A Belsize Crescent | Title | Proposed Elev | vations | | - | |
| London NW3 5QY | Drawn by ST | Scale 1:100 @ A3 | | Number 7335/07 | Rev B | |



PROPOSED SECTION AA





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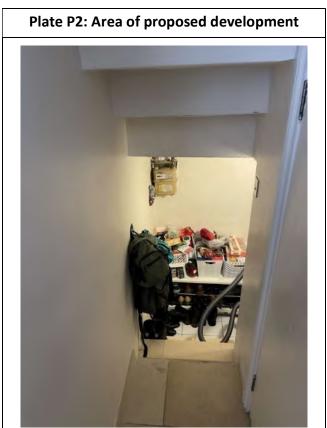
PROPOSED SECTION

| Job No: 7335 | p-dd | pden Hill Road, | t: 0208-4590172 e: hello@pelicanad.co.uk | | p_ad | |
|----------------------|-------------|------------------------|---|----------|------|--|
| PLANNING | | l Gate, London, W8 7TH | | | | |
| 31A Belsize Crescent | Title | Proposed Sec | tion AA | \ | | |
| London | Drawn by | Scale | | Number | Rev | |
| NW3 5QY | ST | 1:100 @ A3 | | 7335/08 | В | |

APPENDIX B PHOTOS

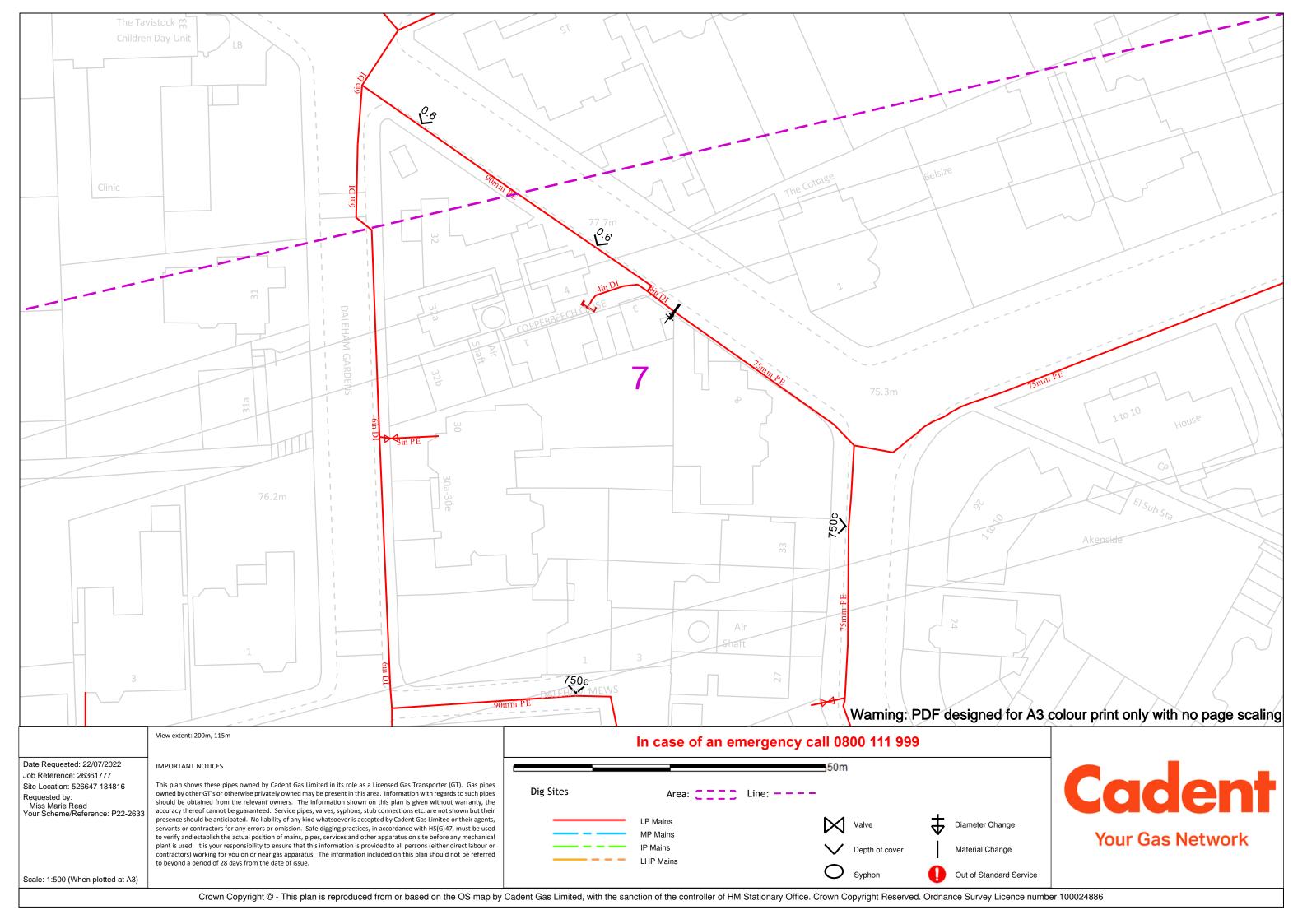


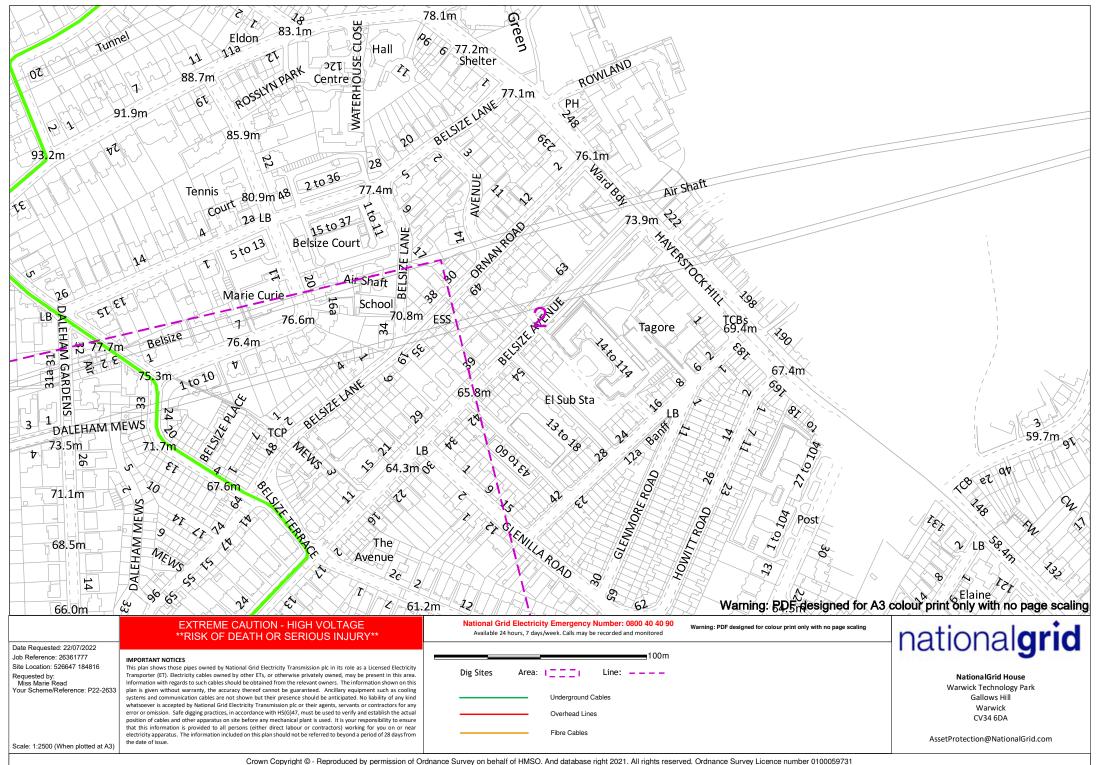


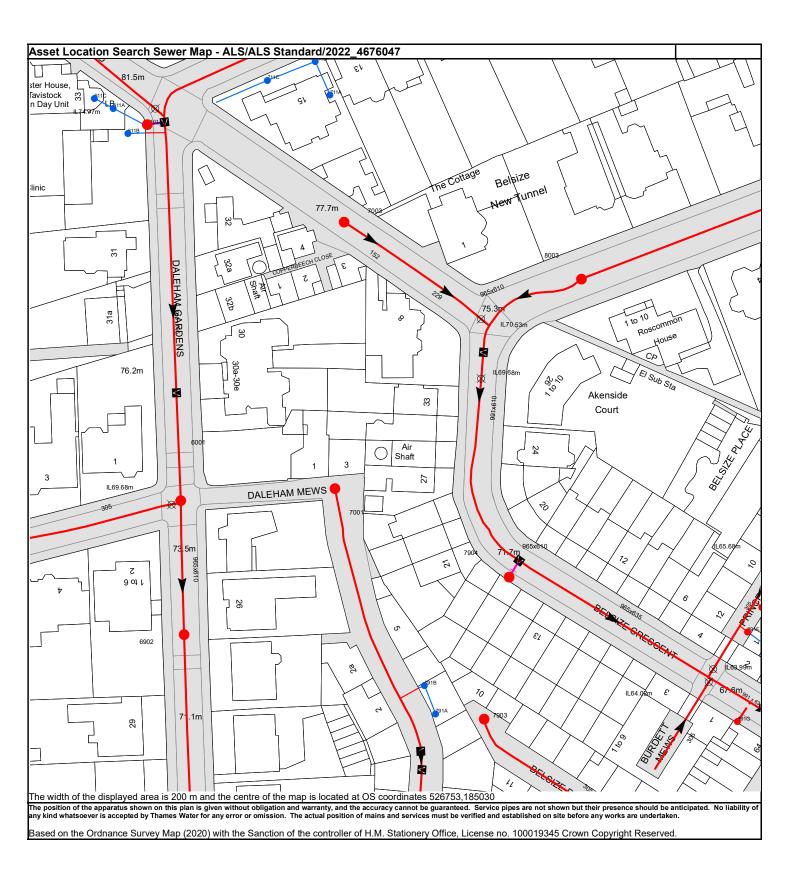


APPENDIX C

SERVICE PLANS

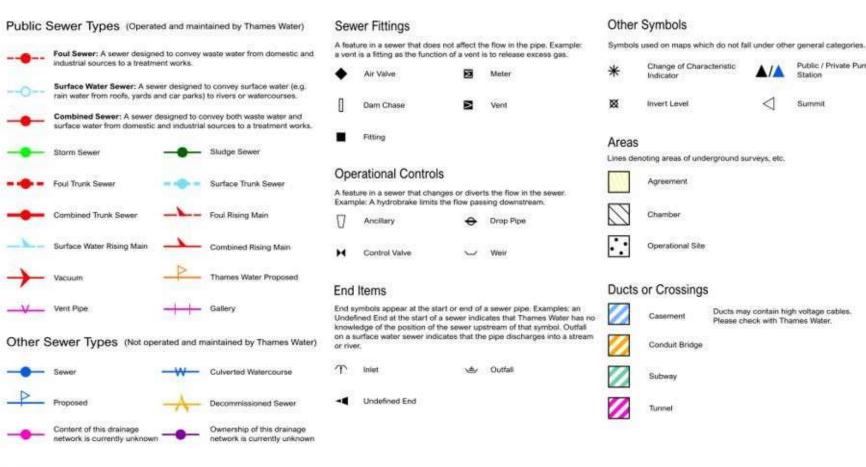








Asset Location Search - Sewer Key



Notes:

1) All levels associated with the plans are to Ordnance Datum Newlyn.

2) All measurements on the plan are metric.

3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate the direction of flow. 4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded. 5) 'na' or '0' on a manhole indicates that data is unavailable.

6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in millimeters. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology, please contact Property Searches on 0800 009 4540.

Thames Water Utilities Ltd, Property Searches, PO Box 3189, Slough SL1 4W, DX 151280 Slough 13 T 0800 009 4540 E searches@thameswater.co.uk | www.thameswater-propertysearches.co.uk

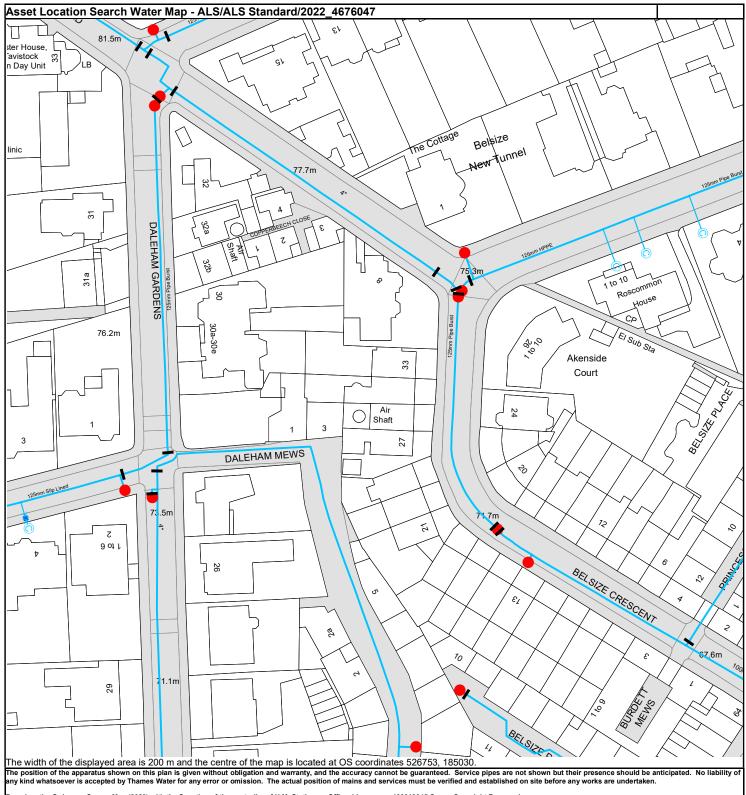
Public / Private Pumping

Station

Summit

Ducts may contain high voltage cables.

Please check with Thames Water.



Based on the Ordnance Survey Map (2020) with the Sanction of the controller of H.M. Stationery Office, License no. 100019345 Crown Copyright Reserved.



Asset Location Search - Water Key

| Water F | Pipes (Operate | d & Maintained by Tharnes Water) | Valves | Oper |
|--|---|---|--|------------|
| e | | in: The most common pipe shown on water maps. | General PurposeValve | - 0 |
| | With few excep distribution main | bions, domestic connections are only made to | Air Valve | -0 |
| | | | Pressure ControlValve | - 0 |
| | treatmentplanto to another. Also | nain carrying water from a source of supply to a r reservoir, or from one treatmentplant or reservoir a main transferring water in bulk to smaller water supplying individual customers. | X Customer Valve | |
| T AUTOL | | supply main indicates that the water main is used a single property or group of properties. | Hydrants Single Hydrant | |
| 2745 | Fire Main: When be displayed alc | re a pipe is used as a fire supply, the word FIRE will ong the pipe. | | — • — 凤 |
| Meterod Pipe: A metered main indicates that the pipe in question supplies water for a single property or group of properties and that quantity of water passing through the pipe is metered even though there may be no meter symbol shown. | | or a single property or group of properties and that passing through the pipe is metered even though | Meters Meter | Othe |
| _ | tunnels are buri | funnel: A very large diameter water pipe. Most ed very deep underground. These pipes are not ct the structural integrity of buildings shown on the | End Items Symbol indicating what happens at the end of a water main. | |
| | process of bein | : A main that is still in the planningstages or in the g laid. More details of the proposed main and its er are generally included near the main. | Blank Flange Capped End Emptying Pit | Other |
| | and the second se | | Undefined End | - |
| PIPE DIAM | COLUMN AND | DEPTH BELOW GROUND | Manifold | |
| Up to 300mm | | 900mm (3') | Customer Supply | |
| 300mm - 600m | un (12° - 24°) | 1100mm (3' 8') | | |

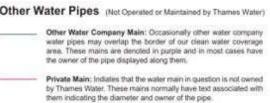
erational Sites



her Symbols Data Logger



Casement: Ducts may contain high voltage cables. Please check with Thames Water.



APPENDIX D

FLOOD RISK ASSESSMENT AND DRAINAGE STRATEGY



Our ref: GB/CS/P22-2633/01

BY EMAIL ONLY

26 August 2022

Eilis Naidoo 31A Belsize Crescent London NW3 5QY

Dear Eilis,

31A BELSIZE CRESCENT, LONDON NW3 5QY - FLOOD RISK ASSESSMENT AND DRAINAGE STRATEGY

As requested, we have undertaken a desk-based Flood Risk Assessment (FRA) and Drainage Strategy for the above project. Please find a summary of our key findings below.

Project Context

31A Belsize Crescent is located in north London, to the northwest of Belsize Park. The site (Figure 1) is bound by Belsize Crescent to the east and Akenside Road to the north. The application involves various elements of conversions and extensions to the existing property. The proposed development involves the lowering of the floor level in the basement/vault area under the front garden and pavement however, excluding any of main building or surrounding areas. This will include an extension of the basement of approximately 9.0 m², to extend under the footpath to the front of the property. The existing and proposed plans and proposed elevations are enclosed within this report.

Objectives

To undertake a Flood Risk Assessment in accordance with the National Planning Policy Framework¹ (NPPF), updated 2021, and Planning Practice Guidance to the NPPF² in order to meet the requirements of a Basement Impact Assessment as required in The London Borough of Camden flood risk management strategy³, Camden Planning Guidance (CPG)⁴ and Camden Development Policies ⁵.

^{3 London} Borough of Camden Flood risk Management Strategy accessed online (August, 2022)

¹ NPPF accessed online (August 2022) <u>https://www.gov.uk/government/publications/national-planning-policy-framework--2</u> ² PPG accessed online (August 2022) <u>http://planningguidance.planningportal.gov.uk/</u>

https://www.camden.gov.uk/documents/20142/1458280/Camden Flood Risk Management Strategy.pdf/9e739029-02e5-59c7-e9a4-64d3622f2475

⁴ Camden Planning Guidance accessed online (August, 2022)

https://www.camden.gov.uk/documents/20142/4823269/Water+and+Flooding+CPG+-+March+2019.pdf/c7633c7d-2b93-cb52-ee01-717fa0416e84

⁵ Camden Development Policies accessed online (August, 2022)

-2-Eilis Naidoo 26 August 2022

The key policies from these local guidance documents include:

- Core Strategy Policy CS13.
- CPG 1
- CPG 4 Basements and Lightwells
- CPG Sustainability

The report will also assess the suitability of the scheme in relation to all sources of flooding, and the flood risk posed by the scheme once it is complete and operational. It will then go on to suggest mitigation measures in order to reduce any residual risks to acceptable levels, and progression of an appropriate surface and foul water drainage strategy.

Sources of Information

As part of this study, the following documents have been obtained and reviewed:

- Existing and Proposed drawings by p-ad Architects (16th May 2022);
- Thames Water Asset Plans (Received 4/7/22);
- Thames Water Sewer Flooding History Enquiry (Received 4/7/22);
- The London Borough of Camden flood risk management strategy (2013)
- Camden Local Plan (2017)
- Camden Planning Guidance (CPG)
- Camden Development Policies
- Camden Surface Water Management Plan (2013)
- Camden PFRA (2011)
- Camden Surface Water Management Plan (2011)
- Camden Strategic Flood Risk Assessment (2014)
- The London Plan: The Spatial Development Strategy for Greater London (Greater London Authority, March 2021);
- CIRIA SuDS Manual C753 (CIRIA, 2015);
- Environment Agency Website available at gov.uk (Accessed August 2022); and
- British Geological Survey GeoIndex (Accessed August 2022).
- Thames Water Asset Plans (1461588)
- Thames Water Flooding History Enquiry (1501578)
- Existing Drawings (733/01)
- Proposed Drawings (733/05)
- Indicative Foul Water Drainage Strategy (2633/02/001)
- Topographic Survey (39478_01-02_PES_RevB)

Consultation

The agencies and individuals consulted as part of this exercise to obtain records or seek input to the proposals as part of this FRA are listed in Table 1 and key records are enclosed with this report.

| Consultee | Form of Consultation | Topics Discussed and Actions Agreed |
|---------------|-------------------------|--|
| Thames Water | Request for | Clean water and wastewater asset plans were requested in order to inform |
| Property | Asset Plans | the proposals at the Site. |
| Searches Team | | |

| Consultee | Form of Consultation | Topics Discussed and Actions Agreed | |
|-----------|---|---|--|
| | | The asset plans (enclosed) dated 4 th July show both waste and freshwater assets in the vicinity of the Site. | |
| | Request for Sewer Flooding History Report | A Thames Water Sewer Flooding History Enquiry dated 5 th July confirms that there have been no incidents of flooding in the area as a result of surcharging public sewers. | |

Table 1: List of key consultations

Flood Risk to the Development (Key Findings):

Flood Zones, Flood Levels & Defence Protection – River Thames

The Site lies within the Environment Agency's (EA) Flood Zone 1, as shown in Figure 2, which is described within the NPPF Technical Guidance as having a 1 in 1000 or less annual probability of river or sea flooding (<0.1%) in any one year.

The Draft Strategic Flood Risk Assessment shows this site is not at risk of flooding in the event if a breach of the flood defences associated with the River Thames. Additionally, the Camden flood risk management strategy details that Camden is not at risk from flooding from the sea or rivers.

After considering the relevant information, the Site is therefore considered to be at a low risk of fluvial/tidal flooding and this source is not considered further in this report.

Surface Water Flooding

The EA modelled surface water mapping (Figures 3-6) show that the site is at risk from flooding during the 1 in 1000 (low risk) event only (Figure 6). During this event the flood waters reach up to 1200mm in the western extent of the site and less than 150mm in the eastern extent of the site, adjoining Belsize Crescent.

Flood risk from this source is therefore deemed to be low to medium. The site currently consists of 100% hardstanding areas, comprising of impermeable paving. However, the proposed development (detailed on enclosed plans 733/05) will not change these hardstanding areas and therefore will not increase the surface water flow into the drainage system.

As there are no proposed changes to the surfacing or external building areas, there is little opportunity to retrofit mitigation measures for this flood risk. This risk is considered to be low to medium but will not worsen as a result of the proposed development for the aforementioned reasons, therefore, this flood risk is not considered further within this report.

Groundwater Flooding

BGS mapping (Figure 7) shows the bedrock geology at the Site as the London Clay formation- clay, silt and sand. There are no superficial deposits at the location of the site.

There are several borehole records in the vicinity of the site, the nearest is 0.35 km to the southeast of the site (TQ28SE3106) which confirms the obtained bedrock record. An additional borehole record (TQ28NE449) details that bedrock was found 5 m below ground level, this was 0.82km northwest from the site. According to the Camden Flood Risk Management Strategy, the area experiences a low risk

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from groundwater flooding, however according to proximal BGS borehole records to the site, water levels have not been detailed. This risk will need to be considered further and possibly mitigated due to the proposed development involving basement development/lowering.

Groundwater flooding to proposed substructures therefore remains a residual risk and appropriate mitigation measures are included later in this report.

Flood Risk from Public Sewers

Thames Water asset plans show that there are only combined sewers in the vicinity of the site, a 152 mm combined sewer flows southeast connecting to a 229 mm sewer adjoining Belsize Crescent at a manhole (IL:70.53 m) which runs south along Belsize Crescent. There are also two vents; located just to the north of the site boundary and then one further south in Belsize Crescent. There has been no history of flooding reported in the Thames Water Sewer Flooding History Report (see enclosed).

Flooding from public sewers remains a residual risk which will be mitigated through the development of an appropriate drainage strategy in addition to other mitigation measures, as outlined later in this report.

Flood Risk from Water Mains

Flood risk from this source is considered to be a residual risk, with no existing mains shown crossing the Site within the supplied Thames Water asset plans. There is a distribution main running from Akenside south along Belsize Crescent, with valves and hydrants along it. Flooding from water mains remains a residual risk with appropriate mitigation measures included later in this report.

Flood Risk from Reservoirs/Permanent Water Bodies

The site is located approximately 900 m southwest from the Hamstead Heath pond and is not located within the region affected by a breach of these ponds, flood risk mapping resultant of these ponds is shown in Figure 8. Therefore, this risk is considered negligible and is not considered any further within this report.

Flood History

A Thames Water Sewer Flooding History Enquiry confirms there have been no records of sewer flooding events at the Site (appended). A review of the local information indicates that there have been two major floods recorded in Camden in 1975 and 2002 (see Figure 5.2 in the Camden Flood Risk management strategy). The 1975 storm was caused by a severe storm that affected Gospel oak, Kentish Town, Belsize Park and Camden. The flooding was caused by poor drainage capacity at the time. The 2002 flooding was less severe but still affected many locations within Camden.

Flood Risk Summary

The Site is at a low risk of flooding from most sources, however a low to medium risk from surface water flooding is present within the site. Due to the nature of the proposals flood risks associated with groundwater flooding to substructures also remains a consideration, albeit residual risk. Residual risks are also associated with water mains, sewer surcharging and surface water.

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A number of mitigation measures are recommended below to manage the risks associated with these identified forms of flooding.

Mitigation

A number of mitigation measures are recommended below to address and manage the residual risks from the identified forms of flooding:

- Finished floor levels should be raised as high as reasonably practicable;
- An appropriate foul water drainage strategy had been developed along with a management and maintenance plan for all drains assets. This has been provided below;
- Routine inspection of the public sewers, site drainage, and public and internal water supply pipe work/storage should be undertaken by Thames Water and the site owner during the construction and operational phase;
- Flow routing should be considered to ensure all surface water flows are directed away from any new building entrances.
- Consider the need for dewatering during the construction phase.
- Consider the need for waterproofing substructures and any below ground services as part of the detailed design.
- Carry out de-watering as necessary through the construction phase.
- Where proposed drainage networks are to be placed within any water bearing strata, they should be constructed such that water ingress cannot occur.

Flood Risk From the Development

Existing Foul Water Drainage

There is existing foul infrastructure that serves the current property on the Site, this is shown in the appended asset plans. This is assumed to currently serve the site with a private network connecting to it from the site.

Proposed Foul Water Drainage Strategy

Drawing 2633/02/001 illustrates that there will a proposed pipe running beneath the building leading from the new bathroom and exiting the building just west of the pantry outwards to the assumed foul/combined manhole located near the front steps of the building within the site boundary (IL: 71.14 and CL: 72.29). This is then assumed to connect to the existing public sewer located in Belsize Crescent, (See enclosed asset plans).

Given the location of the existing manholes into the public Thames Water sewer network, it is assumed that an existing connection to the public sewer is present here, however, this will need to be confirmed as part of the later design stages.

After considering the topography and invert levels available on the asset plans, it is assumed that the proposed foul water drainage will connect via gravity to the existing infrastructure in Belsize Crescent, although as above this will need to be confirmed at the detailed design stage.

Existing Surface Water Drainage

There are no existing surface water sewers in the immediate vicinity of the site according to the asset plans appended to this report. However, there is existing private drainage seen on the existing site plan topographic survey (Drawing 39478_01-02_PES_RevB).

Proposed Surface Water Drainage Strategy

There is no proposed surface water drainage as the proposed developments will not alter the hardstanding areas of the current property. However, it is recommended that water butts should be installed in the back garden in addition to increased planting as this will act to reduce surface water runoff for the development (see Drawing 2633/02/001).

Management and Maintenance Plan

Regular inspection and maintenance of adjacent highway drainage, public and private drainage by the London Borough of Camden, Thames Water, and site management respectively, will minimise the residual risks associated with surface water/sewers.

The following maintenance schedule (Table 3) for the proposed SUDS should be adhered to in order to ensure efficient operation and to prevent failure.

| Drainage Feature | Maintenance | Maintenance Period |
|----------------------|---|--|
| Manholes | Inspect and identify any areas that are not operating correctly. If required, take remedial action | Monthly for first three months, then annually. |
| | Check free from silt and debris and water discharging freely through. | Every 6 months |
| | Jet/clear out as necessary. | |
| | Repair / rehabilitate inlets, outlet, overflows and vents | As required |
| Drainage Pipework | CCTV inspection/condition survey prior to connection to the existing network. | Every 5 years |
| | Sewer jetting | As required |

Table 3: Outline SuDS Management and Maintenance Schedule

Flood Risk from the Development/Drainage Assessment

Impact on Surrounding Areas

The proposed development will increase foul flows from the current state give the proposals include a new toilet and shower block, these will be accommodated within the foul sewer network via the existing connection which will connect to the public sewer located in Belsize Crescent and any increases in flow are not considered to be of a magnitude that would impact sewer capacity.

There will be no alterations to the current surface water drainage for the site as the proposals do not increase any hard standing areas. Therefore, the proposed development is unlikely to have a

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significant impact on the local public sewer network or increase the risk of flooding to surrounding development.

Conclusions and Recommendations

We conclude that no significant sources of flooding have been identified as part of this study that warrant further consideration as part of a more detailed Flood Risk Assessment.

A number of mitigation measures are recommended to address the low risk of surface water flooding and groundwater flooding and residual risks of flooding from public sewers, site drainage, and public and internal water supply pipe work/storage. These risks can be managed by the design of the site drainage, by regular inspection and maintenance of the public and private sewer and water supply network.

Yours sincerely,

-ar Land

Grace Beard Graduate Flood Risk Consultant

Encs

6. Linda

Graham Sinclair Technical Director

- Constraints and Limitations
- References
- Figure 1: Site Location Plan
- Figure 2: EA Fluvial/Tidal Flood Zone Map
- Figure 3: EA Surface Water Extent
- Figure 4: EA Surface Water Flood Risk Map (1 in 30 Year Event)
- Figure 5: EA Surface Water Flood Risk Map (1 in 100 year Event)
- Figure 6: EA Surface Water Flood Risk Map (1 in 1000 Year Event)
- Figure 7: BGS Bedrock Deposit Geological Mapping and Borehole records
- Figure 8: EA Risk of Reservoir Flooding
- Thames Water Asset Plans (1461588)
- Thames Water Flooding History Enquiry (1501578)
- Existing Drawings (733/01)
- Proposed Drawings (733/05)
- Indicative Foul Water Drainage Strategy (2633_02_001)
- Topographic Survey (39478_01-02_PES_RevB)

Constraints and Limitations

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Create Consulting Engineers Ltd accepts no responsibility whatsoever to other parties to whom this report, or any part thereof, is made known. Any such other parties rely upon the report at their own risk.

This report has been undertaken with the assumption that the Site will be developed in accordance with the above proposals without significant change. The conclusions resulting from this study are not necessarily indicative of future conditions or operating practices at or adjacent to the Site.

Create Consulting Engineers Ltd has endeavoured to assess all information provided to them during this appraisal. The report summarises information from a number of external sources and cannot offer any guarantees or warranties for the completeness or accuracy or information relied upon. Information from third parties has not been verified by Create Consulting Engineers Ltd unless otherwise stated in this report.

References

- i. British Geological Survey GeoIndex (Accessed August, 2022).
- ii. CIRIA. (2015). CIRIA SuDS Manual C753.
- iii. Camden Strategic Flood Risk Assessment
- iv. Camden Preliminary Flood Risk Assessment
- v. Camden Local Plan (2017)
- vi. Camden Planning Guidance (CPG)
- vii. Camden Development Policies
- viii. Camden Surface Water Management Plan (2013)
- ix. Camden PFRA (2011)
- x. Camden Surface Water Management Plan (2011)
- xi. Camden Strategic Flood Risk Assessment (2014)
- xii. The London Borough of Camden flood risk management strategy (2013)
- xiii. Data.gov.uk. (2022). Flood Maps, Surface Water, Groundwater Maps and Reservoir Flood Maps. (Accessed August, 2022).
- xiv. Greater London Authority. (2021) The London Plan: The Spatial Development Strategy for Greater London.

ENCLOSURES







Figure 2: EA Fluvial/Tidal Flood Zone Map

Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>)



Figure 3: EA Surface Water Extent

Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>)



Figure 4: EA Surface Water Depth (High Risk Event)

Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>)



Figure 5: EA Surface Water Depth (Medium Risk Event *Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>)*



Figure 6: EA Surface Water Depth (Low Risk Event) Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>)

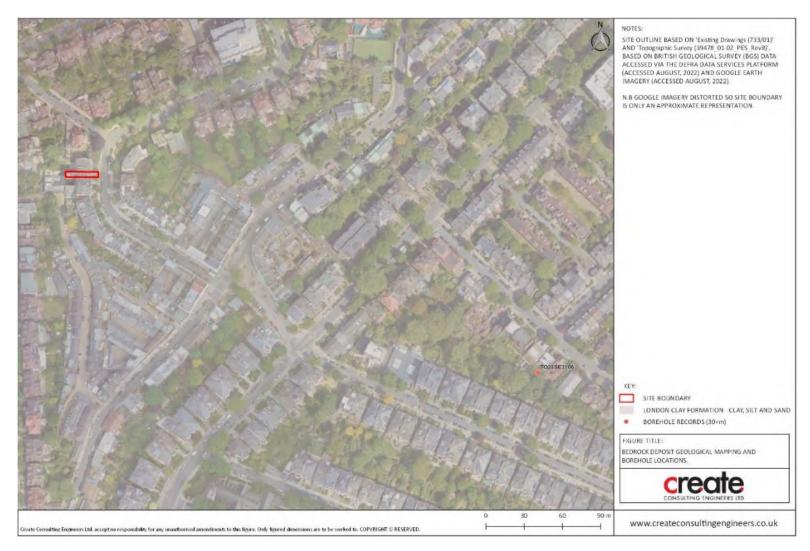


Figure 7: BGS Bedrock Deposit Geological Mapping and Borehole records *Source: BGS Geoindex (<u>https://mapapps2.bgs.ac.uk/geoindex/home.html</u>)*



Figure 8: EA Risk of Reservoir Flooding

Source: Defra Data Services Platform (<u>https://environment.data.gov.uk/</u>

Asset location search



Create Consulting Engineers Ltd NORWICH NR3 1AF

Search address supplied

31a Belsize Crescent London NW3 5QY

Your reference

P22-2633

Our reference

ALS/ALS Standard/2022_4676047

Search date

4 July 2022

Knowledge of features below the surface is essential for every development

The benefits of this knowledge not only include ensuring due diligence and avoiding risk, but also being able to ascertain the feasibility of any development.

Did you know that Thames Water Property Searches can also provide a variety of utility searches including a more comprehensive view of utility providers' assets (across up to 35-45 different providers), as well as more focused searches relating to specific major utility companies such as National Grid (gas and electric).

Contact us to find out more.



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0800 009 4540





Search address supplied: 31a, Belsize Crescent, London, NW3 5QY

Dear Sir / Madam

An Asset Location Search is recommended when undertaking a site development. It is essential to obtain information on the size and location of clean water and sewerage assets to safeguard against expensive damage and allow cost-effective service design.

The following records were searched in compiling this report: - the map of public sewers & the map of waterworks. Thames Water Utilities Ltd (TWUL) holds all of these.

This searchprovides maps showing the position, size of Thames Water assets close to the proposed development and also manhole cover and invert levels, where available.

Please note that none of the charges made for this report relate to the provision of Ordnance Survey mapping information. The replies contained in this letter are given following inspection of the public service records available to this company. No responsibility can be accepted for any error or omission in the replies.

You should be aware that the information contained on these plans is current only on the day that the plans are issued. The plans should only be used for the duration of the work that is being carried out at the present time. Under no circumstances should this data be copied or transmitted to parties other than those for whom the current work is being carried out.

Thames Water do update these service plans on a regular basis and failure to observe the above conditions could lead to damage arising to new or diverted services at a later date.

Contact Us

If you have any further queries regarding this enquiry please feel free to contact a member of the team on 0800 009 4540, or use the address below:

Thames Water Utilities Ltd Property Searches PO Box 3189 Slough SL1 4WW

Email: <u>searches@thameswater.co.uk</u> Web: <u>www.thameswater-propertysearches.co.uk</u>

Asset location search



Waste Water Services

Please provide a copy extract from the public sewer map.

Enclosed is a map showing the approximate lines of our sewers. Our plans do not show sewer connections from individual properties or any sewers not owned by Thames Water unless specifically annotated otherwise. Records such as "private" pipework are in some cases available from the Building Control Department of the relevant Local Authority.

Where the Local Authority does not hold such plans it might be advisable to consult the property deeds for the site or contact neighbouring landowners.

This report relates only to sewerage apparatus of Thames Water Utilities Ltd, it does not disclose details of cables and or communications equipment that may be running through or around such apparatus.

The sewer level information contained in this response represents all of the level data available in our existing records. Should you require any further Information, please refer to the relevant section within the 'Further Contacts' page found later in this document.

For your guidance:

- The Company is not generally responsible for rivers, watercourses, ponds, culverts or highway drains. If any of these are shown on the copy extract they are shown for information only.
- Any private sewers or lateral drains which are indicated on the extract of the public sewer map as being subject to an agreement under Section 104 of the Water Industry Act 1991 are not an 'as constructed' record. It is recommended these details be checked with the developer.

Clean Water Services

Please provide a copy extract from the public water main map.

Enclosed is a map showing the approximate positions of our water mains and associated apparatus. Please note that records are not kept of the positions of individual domestic supplies.

For your information, there will be a pressure of at least 10m head at the outside stop valve. If you would like to know the static pressure, please contact our Customer Centre on 0800 316 9800. The Customer Centre can also arrange for a full flow and pressure test to be carried out for a fee.

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For your guidance:

- Assets other than vested water mains may be shown on the plan, for information only.
- If an extract of the public water main record is enclosed, this will show known public water mains in the vicinity of the property. It should be possible to estimate the likely length and route of any private water supply pipe connecting the property to the public water network.

Payment for this Search

A charge will be added to your suppliers account.





Further contacts:

Waste Water queries

Should you require verification of the invert levels of public sewers, by site measurement, you will need to approach the relevant Thames Water Area Network Office for permission to lift the appropriate covers. This permission will usually involve you completing a TWOSA form. For further information please contact our Customer Centre on Tel: 0845 920 0800. Alternatively, a survey can be arranged, for a fee, through our Customer Centre on the above number.

If you have any questions regarding sewer connections, budget estimates, diversions, building over issues or any other questions regarding operational issues please direct them to our service desk. Which can be contacted by writing to:

Developer Services (Waste Water) Thames Water Clearwater Court Vastern Road Reading RG1 8DB

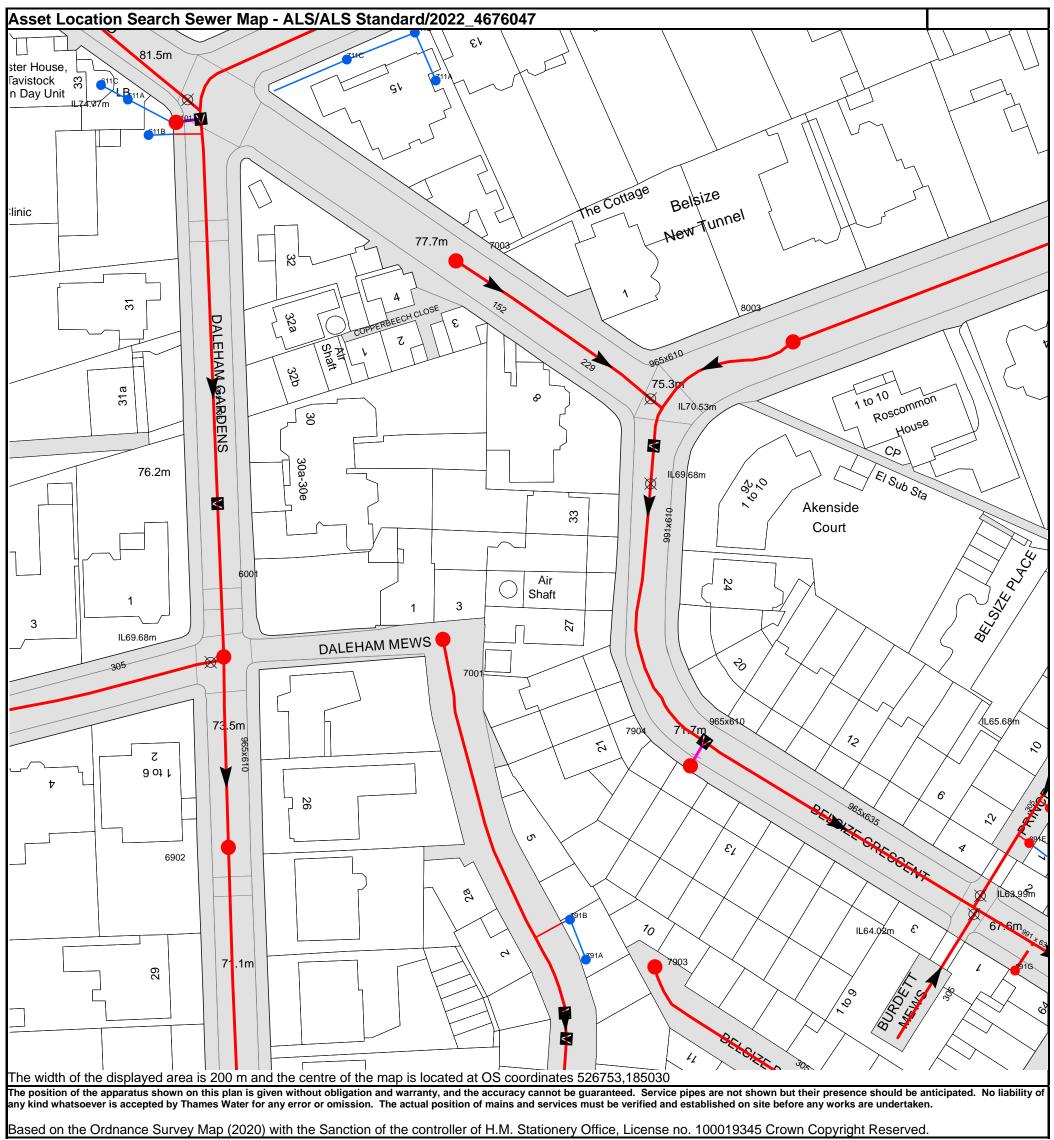
Tel: 0800 009 3921 Email: developer.services@thameswater.co.uk

Clean Water queries

Should you require any advice concerning clean water operational issues or clean water connections, please contact:

Developer Services (Clean Water) Thames Water Clearwater Court Vastern Road Reading RG1 8DB

Tel: 0800 009 3921 Email: developer.services@thameswater.co.uk



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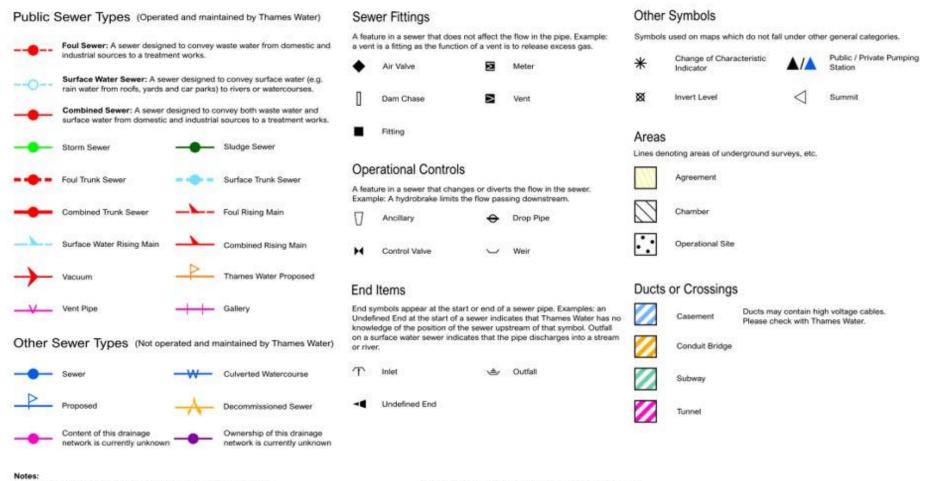
NB. Levels quoted in metres Ordnance Newlyn Datum. The value -9999.00 indicates that no survey information is available

| Manhole Reference | Manhole Cover Level | Manhole Invert Level |
|--|--|---|
| 6902 | 72.66 | 67.74 |
| 891E | n/a | n/a |
| 891F | n/a | n/a |
| 7904 | n/a | n/a |
| 6001 | 74.76 | 69.43 |
| 7001 | 73.07 | 70.63 |
| 8003 | 75.89 | 71.06 |
| 7003 | 76.9 | 74 |
| 611B | n/a | n/a |
| 6101 | n/a | n/a |
| 611A | n/a | n/a |
| 611C | n/a | n/a |
| 711A | n/a | n/a |
| 711C | n/a | n/a |
| 711B | n/a | n/a |
| 791B | n/a | n/a |
| 791A | n/a | n/a |
| 7903 | 67.07 | 65.75 |
| 891G | n/a | n/a |
| | | |
| The necition of the ennerotus -house - | this plan is given without obligation and warrents | d the accuracy cannot be guaranteed. Service pipes are no |

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.



Asset Location Search - Sewer Key



1) All levels associated with the plans are to Ordnance Datum Newlyn.

2) All measurements on the plan are metric.

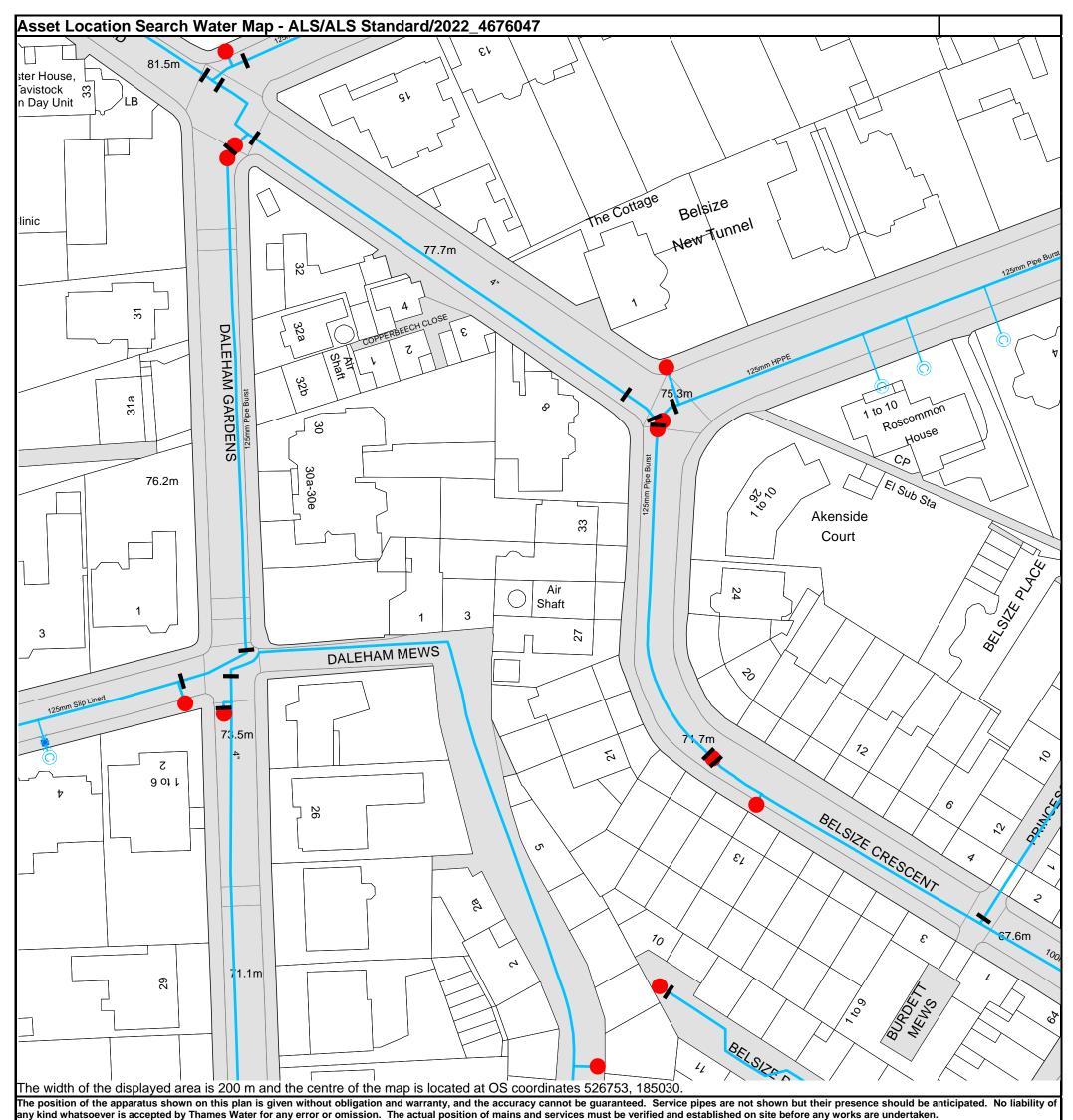
3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate the direction of flow.

4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded.

5) 'na' or '0' on a manhole indicates that data is unavailable.

6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in millimeters. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology, please contact Property Searches on 0800 009 4540.

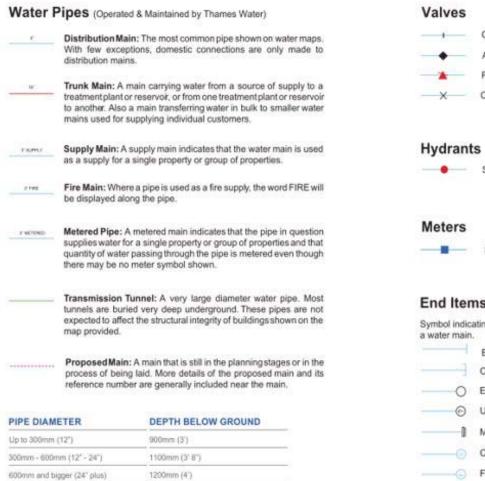
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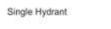


Asset Location Search - Water Key



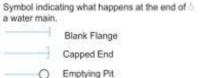






Meter

End Items



Undefined End

Manifold

Customer Supply

Fire Supply

Operational Sites

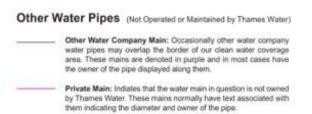


Other Symbols

Data Logger



Casement: Ducts may contain high voltage cables. Please check with Thames Water.



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If you are unhappy with our service you can speak to your original goods or customer service provider. If you are not satisfied with the response, your complaint will be reviewed by the Customer Services Director. You can write to her at: Thames Water Utilities Ltd. PO Box 492, Swindon, SN38 8TU.

If the Goods or Services covered by this invoice falls under the regulation of the 1991 Water Industry Act, and you remain dissatisfied you can refer your complaint to Consumer Council for Water on 0121 345 1000 or write to them at Consumer Council for Water, 1st Floor, Victoria Square House, Victoria Square, Birmingham, B2 4AJ.

| Credit Card | BACS Payment | Telephone Banking | Cheque |
|--|--|---|--|
| Call 0800 009 4540 quoting your invoice number starting CBA or ADS / OSS | Account number 90478703 Sort code 60-00-01 A remittance advice must be sent to: Thames Water Utilities Ltd., PO Box 3189, Slough SL1 4WW. or email ps.billing@thameswater. co.uk | By calling your bank and quoting: Account number 90478703 Sort code 60-00-01 and your invoice number | Made payable to ' Thames Water Utilities Ltd' Write your Thames Water account number on the back. Send to: Thames Water Utilities Ltd., PO Box 3189, Slough SL1 4WW or by DX to 151280 Slough 13 |

Ways to pay your bill

Thames Water Utilities Ltd Registered in England & Wales No. 2366661 Registered Office Clearwater Court, Vastern Rd, Reading, Berks, RG1 8DB.





Create Consulting Engineers Ltd Norwich Norwich Princes Street

Search address supplied 31a Belsize Crescent London NW3 5QY

| Your reference | P22-2633 |
|----------------|-------------------------------|
| Our reference | SFH/SFH Standard/2022_4676049 |
| Received date | 4 July 2022 |
| Search date | 4 July 2022 |



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searches@thameswater.co.uk www.thameswater-propertysearches.co.uk



0800 009 4540





Search address supplied: 31a,Belsize Crescent,London,NW3 5QY

This search is recommended to check for any sewer flooding in a specific address or area

- TWUL, trading as Property Searches, are responsible in respect of the following:-
- (i) any negligent or incorrect entry in the records searched;
- (ii) any negligent or incorrect interpretation of the records searched;
- (iii) and any negligent or incorrect recording of that interpretation in the search report
- (iv) compensation payments



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History of Sewer Flooding

Is the requested address or area at risk of flooding due to overloaded public sewers?

The flooding records held by Thames Water indicate that there have been no incidents of flooding in the requested area as a result of surcharging public sewers.

For your guidance:

- A sewer is "overloaded" when the flow from a storm is unable to pass through it due to a permanent problem (e.g. flat gradient, small diameter). Flooding as a result of temporary problems such as blockages, siltation, collapses and equipment or operational failures are excluded.
- "Internal flooding" from public sewers is defined as flooding, which enters a building or passes below a suspended floor. For reporting purposes, buildings are restricted to those normally occupied and used for residential, public, commercial, business or industrial purposes.
- "At Risk" properties are those that the water company is required to include in the Regulatory Register that is presented annually to the Director General of Water Services. These are defined as properties that have suffered, or are likely to suffer, internal flooding from public foul, combined or surface water sewers due to overloading of the sewerage system more frequently than the relevant reference period (either once or twice in ten years) as determined by the Company's reporting procedure.
- Flooding as a result of storm events proven to be exceptional and beyond the reference period of one in ten years are not included on the At Risk Register.
- Properties may be at risk of flooding but not included on the Register where flooding incidents have not been reported to the Company.
- Public Sewers are defined as those for which the Company holds statutory responsibility under the Water Industry Act 1991.
- It should be noted that flooding can occur from private sewers and drains which are not the responsibility of the Company. This report excludes flooding from private sewers and drains and the Company makes no comment upon this matter.
- For further information please contact Thames Water on Tel: 0800 316 9800 or website www.thameswater.co.uk

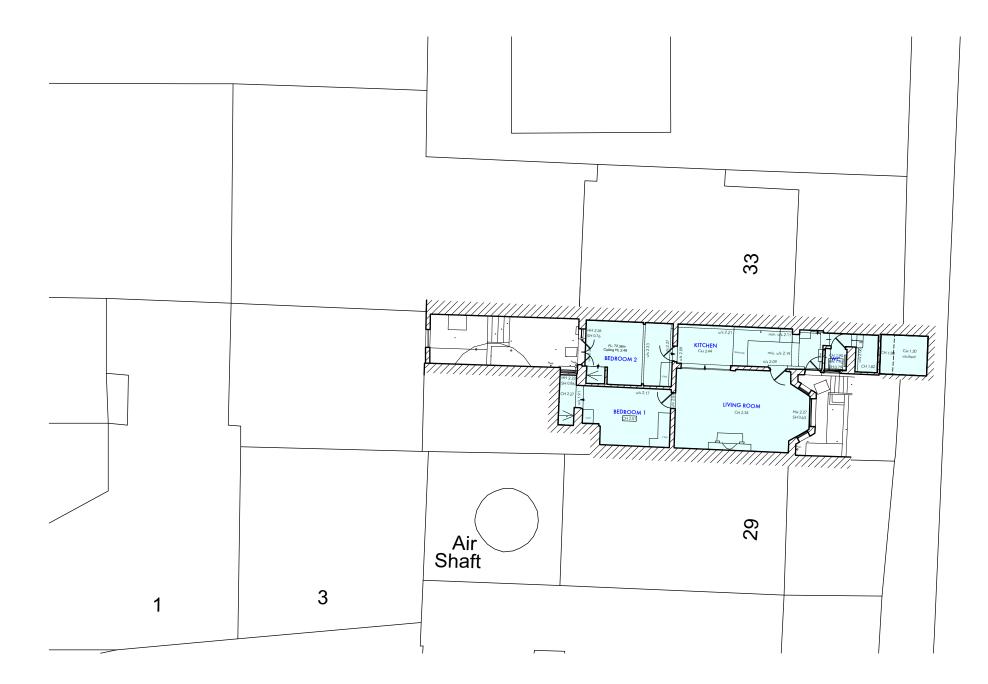


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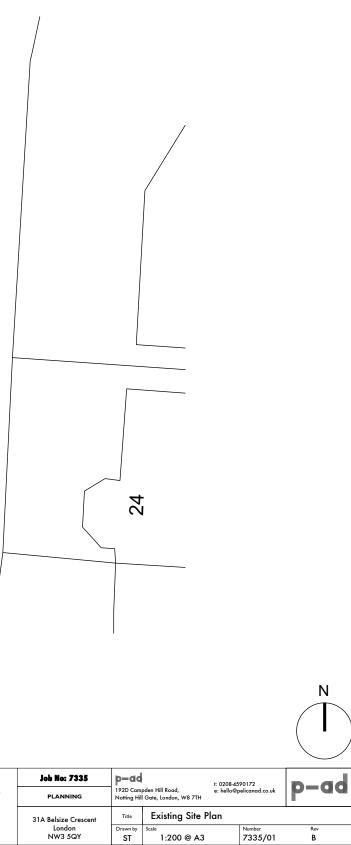
BELSIZE CRESCENT

EXISTING SITE PLAN

WRITTEN DIMENSIONS ONLY TO BE USED ALL DIMENSIONS ARE IN MILLIMETRES ALL DIMENSIONS TO BE VERIFIED ON SITE ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY

0 0.5 1 2 3 4 5 SCALE BAR (m)

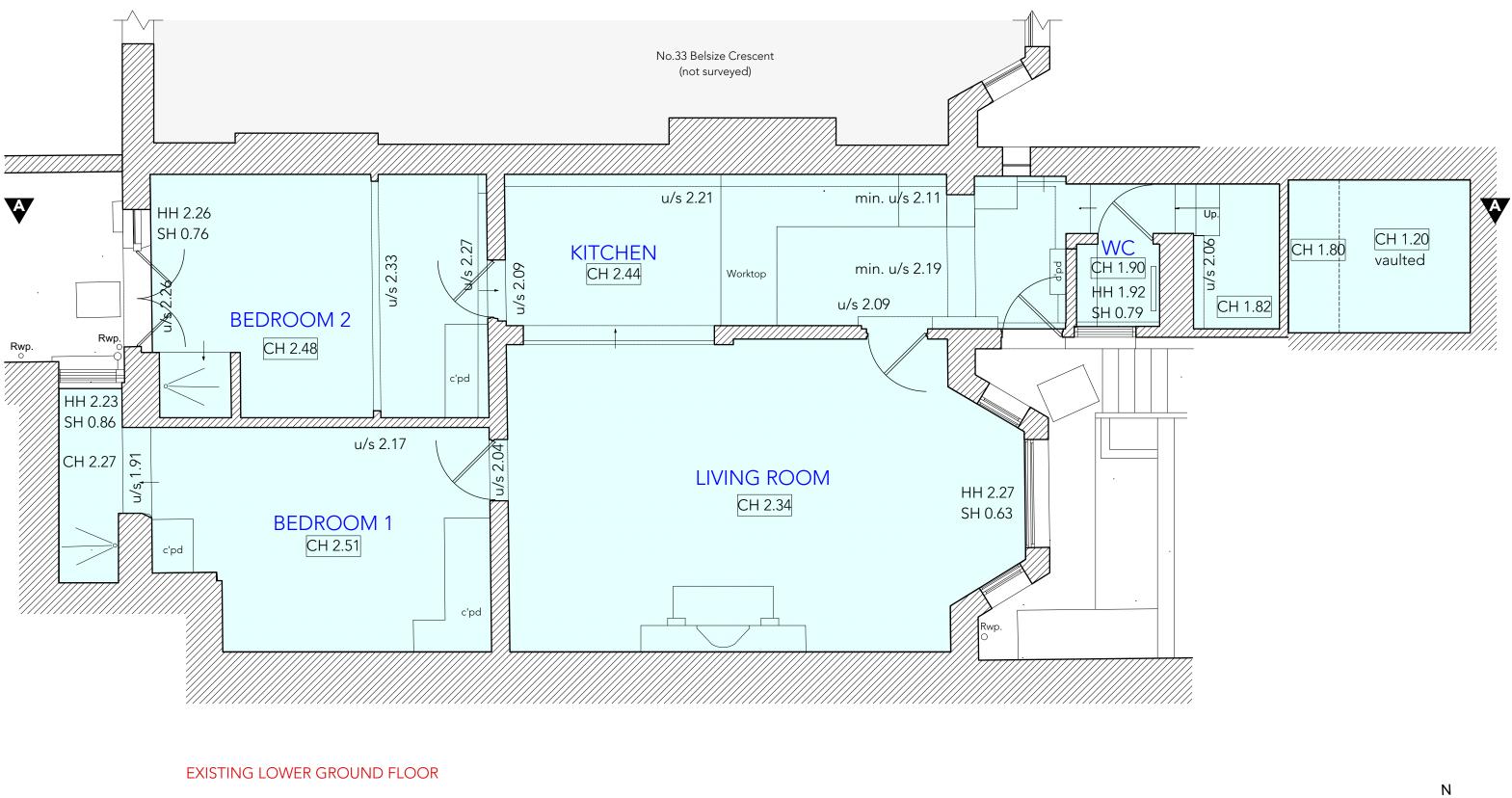
EXISTING SITE PLAN



Number 7335/01

В

1:200 @ A3



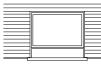


WRITTEN DIMENSIONS ONLY TO BE USED ALL DIMENSIONS ARE IN MILLIMETRES ALL DIMENSIONS TO BE VERIFIED ON SITE ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY

EXISTING PLAN

| Job No: 7335 | p-ad | t: 0208-4590172 D Campden Hill Road, e: hello@pelicanad.co.uk | | n-ad | |
|----------------------|--|--|--------------------------|------|--|
| PLANNING | 192D Campden Hill Road, Notting Hill Gate, London, W8 7TH | | e: hello@pelicanad.co.uk | p-dd | |
| 31A Belsize Crescent | Title | Existing Plans | | | |
| London | Drawn by | Scale | Number | Rev | |
| NW3 5QY | ST | 1:50 @ A3 | 7335/02 | В | |







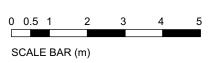




Datum: 70.00m.

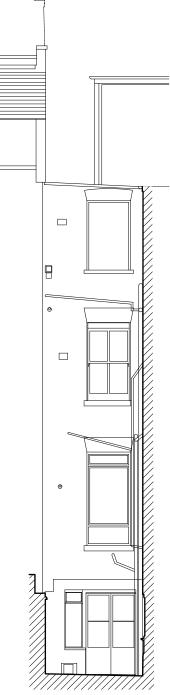
EXISTING FRONT ELEVATION

EXISTING REAR ELEVATION

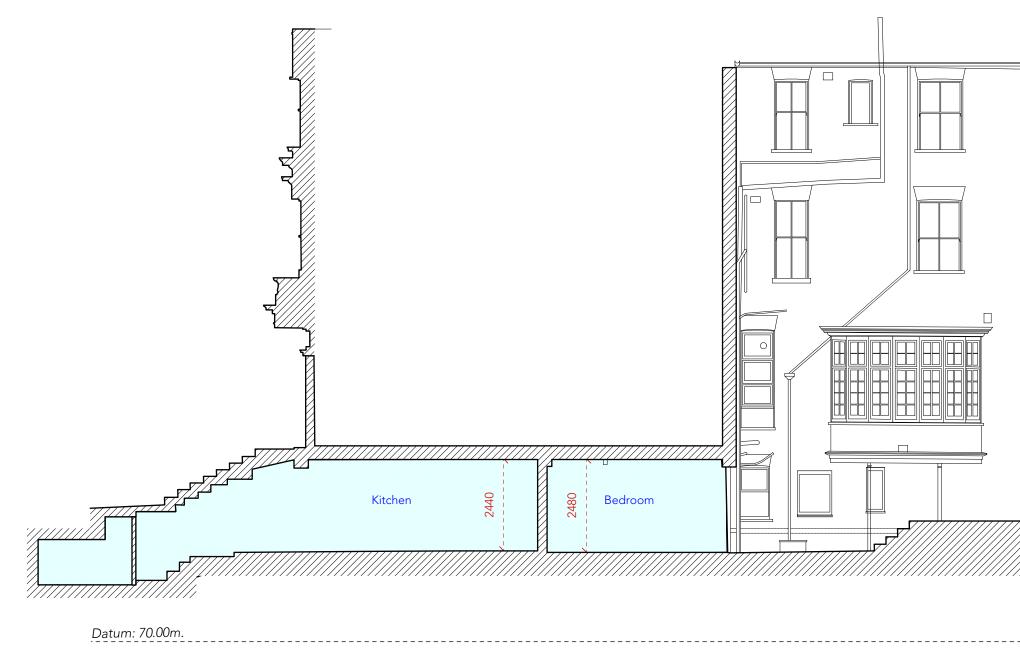




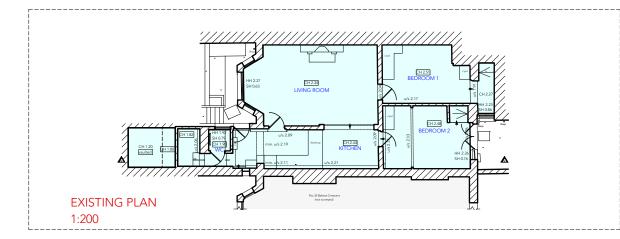
EXISTING ELEVATIONS

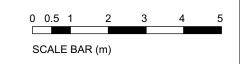


| Job No: 7335 | p-ad | | t: 0208-4590172 | n_ad | |
|----------------------|--|----------------|--------------------------|------|--|
| PLANNING | 192D Campden Hill Road, Notting Hill Gate, London, W8 7TH | | e: hello@pelicanad.co.uk | p-dd | |
| 31A Belsize Crescent | Title | Existing Elevo | itions | · | |
| London | Drawn by | Scale | Number | Rev | |
| NW3 5QY | ST | 1:100 @ A3 | 7335/03 | В | |



EXISTING SECTION AA



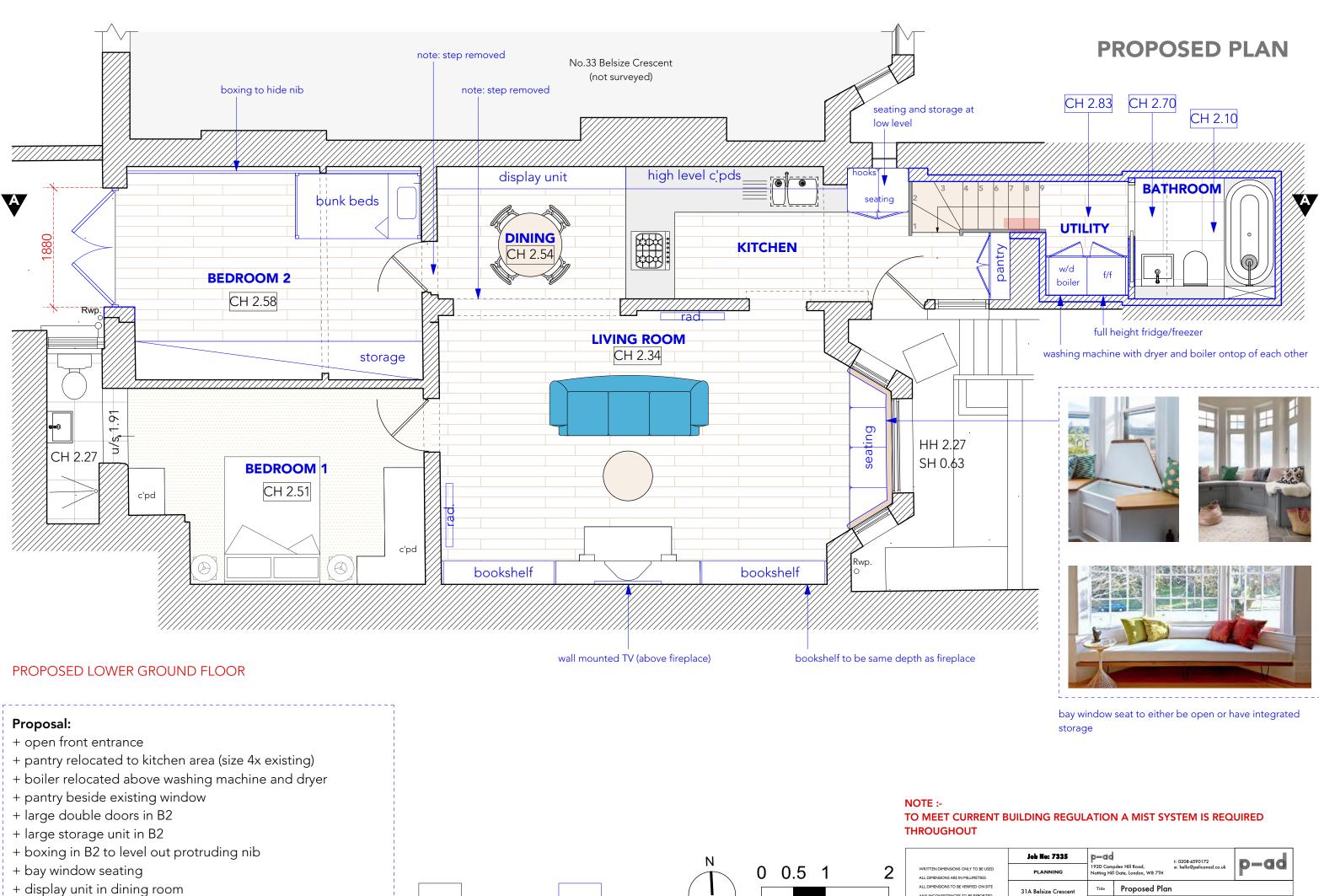


WRITTEN DIMENSIONS ONLY TO BE USED ALL DIMENSIONS ARE IN MILLIMETRES ALL DIMENSIONS TO BE VERIFIED ON SITE ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY

EXISTING SECTION



| - | Job No: 7335 Planning | | pden Hill Road, I Gate, London, W8 7TH | t: 0208-4590172 e: hello@pelicanad.co.uk | p-ad |
|---|--------------------------|----------------|---|---|----------|
| | 31A Belsize Crescent | Title | Existing Section | on AA | |
| | London NW3 5QY | Drawn by ST | Scale 1:100 @ A3 | Number 7335/04 | Rev B |



PROPOSED

EXISTING

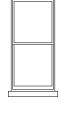
- + bookshelves in living room (same width as fireplace)

| Job No: 7335 Planning | | | | p-ad | |
|---------------------------------|----------------|--------------------|---|-------------------|----------|
| 31A Belsize Crescent | Title | Proposed Plan | ı | | |
| London NW3 5QY | Drawn by ST | Scale 1:50 @ A3 | | Number 7335/05 | Rev B |

ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY







replacement of existing double doors and window with timber framed double glazed doors

Datum: 70.00m.

PROPOSED FRONT ELEVATION

NOTE: FRONT ELEVATION REMAINS UNCHANGED

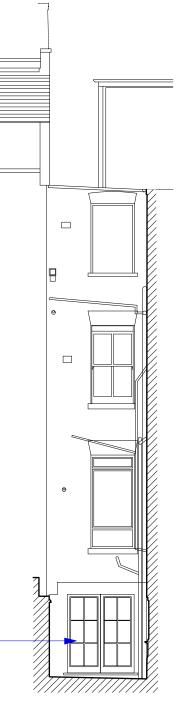


5

0 0.5 1 2 3 4

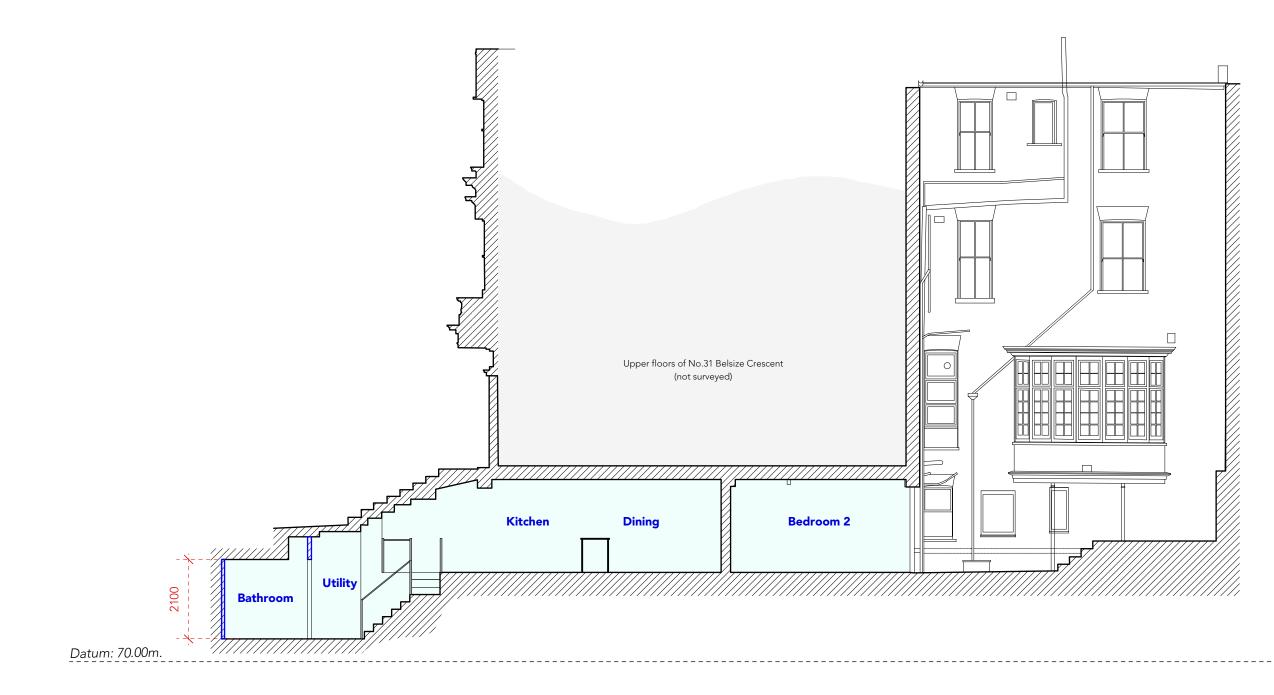


PROPOSED ELEVATIONS

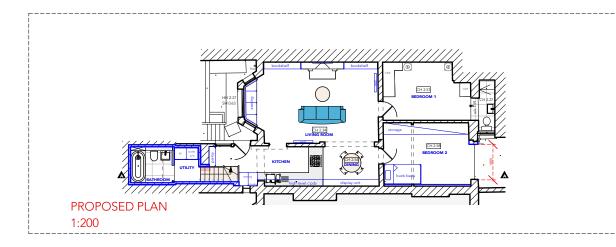


PROPOSED REAR ELEVATION

| Job No: 7335 | | pden Hill Road, I Gate, London, W8 7TH | | | p-ad | |
|----------------------|----------------|---|--|-------------------|----------|--|
| 31A Belsize Crescent | Title | Proposed Elev | | | | |
| London NW3 5QY | Drawn by ST | Scale 1:100 @ A3 | | Number 7335/07 | Rev B | |



PROPOSED SECTION AA

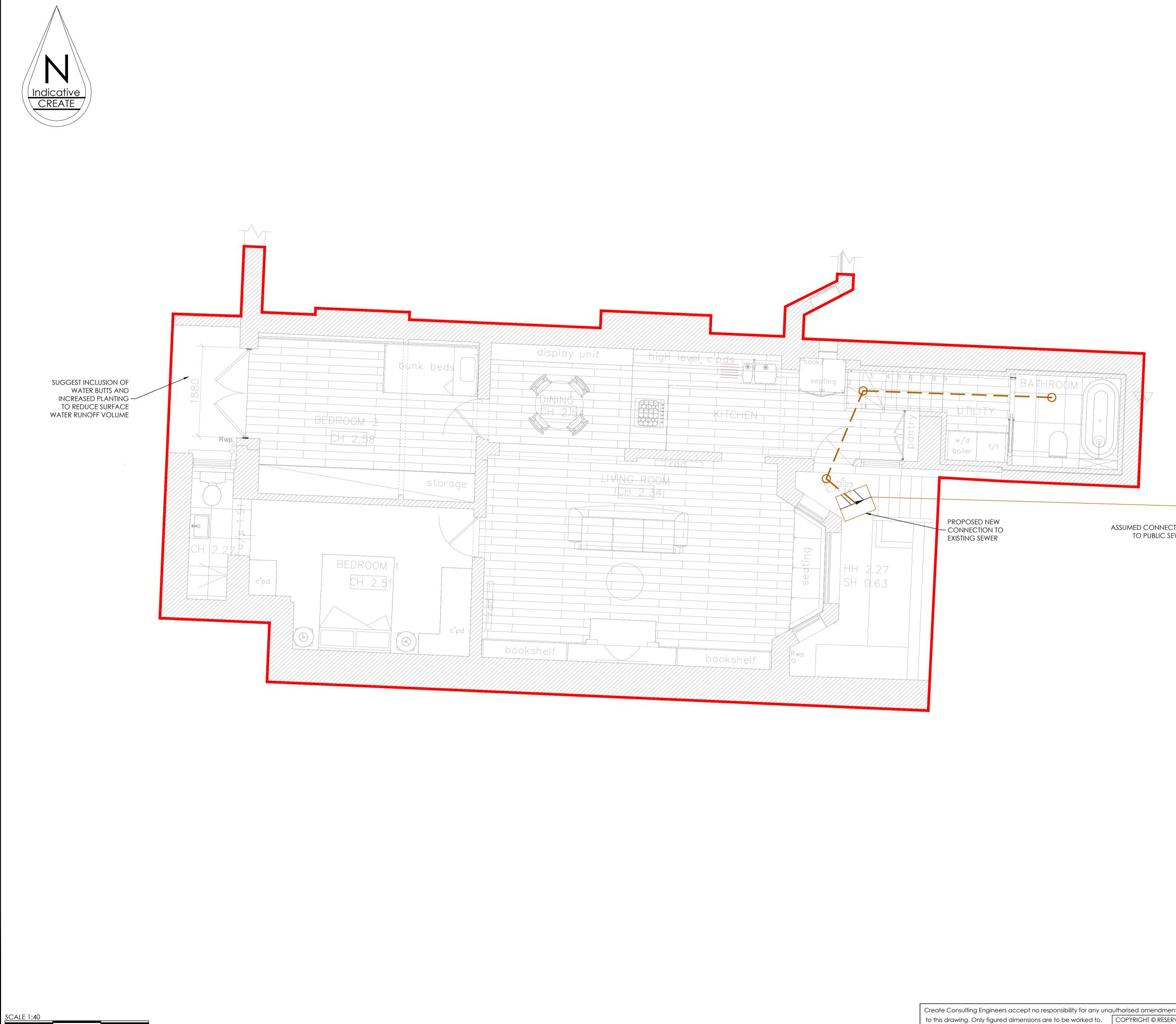




WRITTEN DIMENSIONS ONLY TO BE USED ALL DIMENSIONS ARE IN MILLIMETRES ALL DIMENSIONS TO BE VERIFIED ON SITE ANY INCONSISTENCIES TO BE REPORTED TO THE ARCHITECT IMMEDIATELY

PROPOSED SECTION

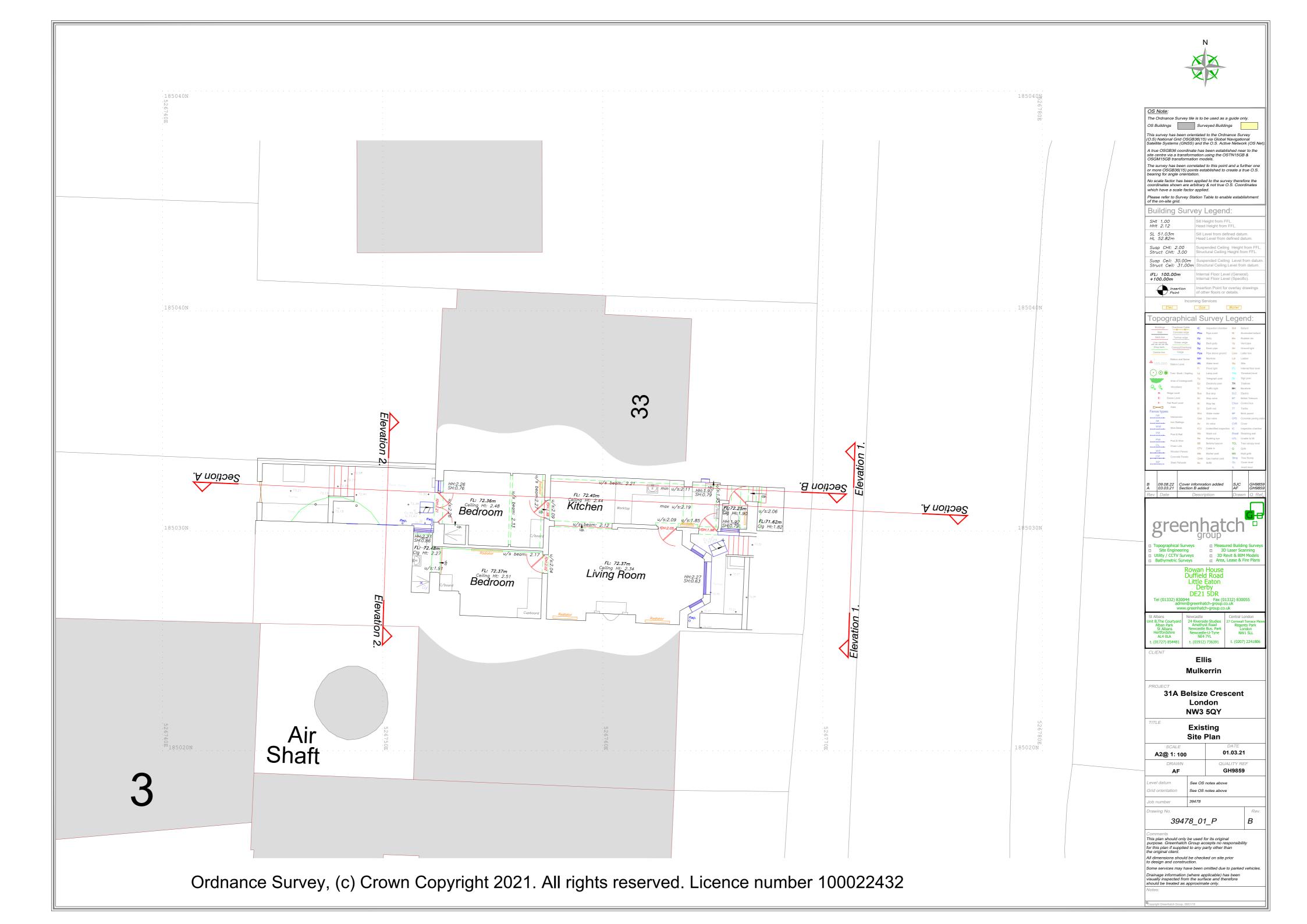
| Job No: 7335 | p-dd | pden Hill Road, | t: 0208-4590172 e: hello@pelicanad.co.uk | | p-ad | |
|----------------------|-------------|-----------------------------------|---|---------|------|--|
| PLANNING | | Notting Hill Gate, London, W8 7TH | | | | |
| 31A Belsize Crescent | Title | Title Proposed Section AA | | | | |
| London | Drawn by | Scale | | Number | Rev | |
| NW3 5QY | ST | 1:100 @ A3 | | 7335/08 | В | |

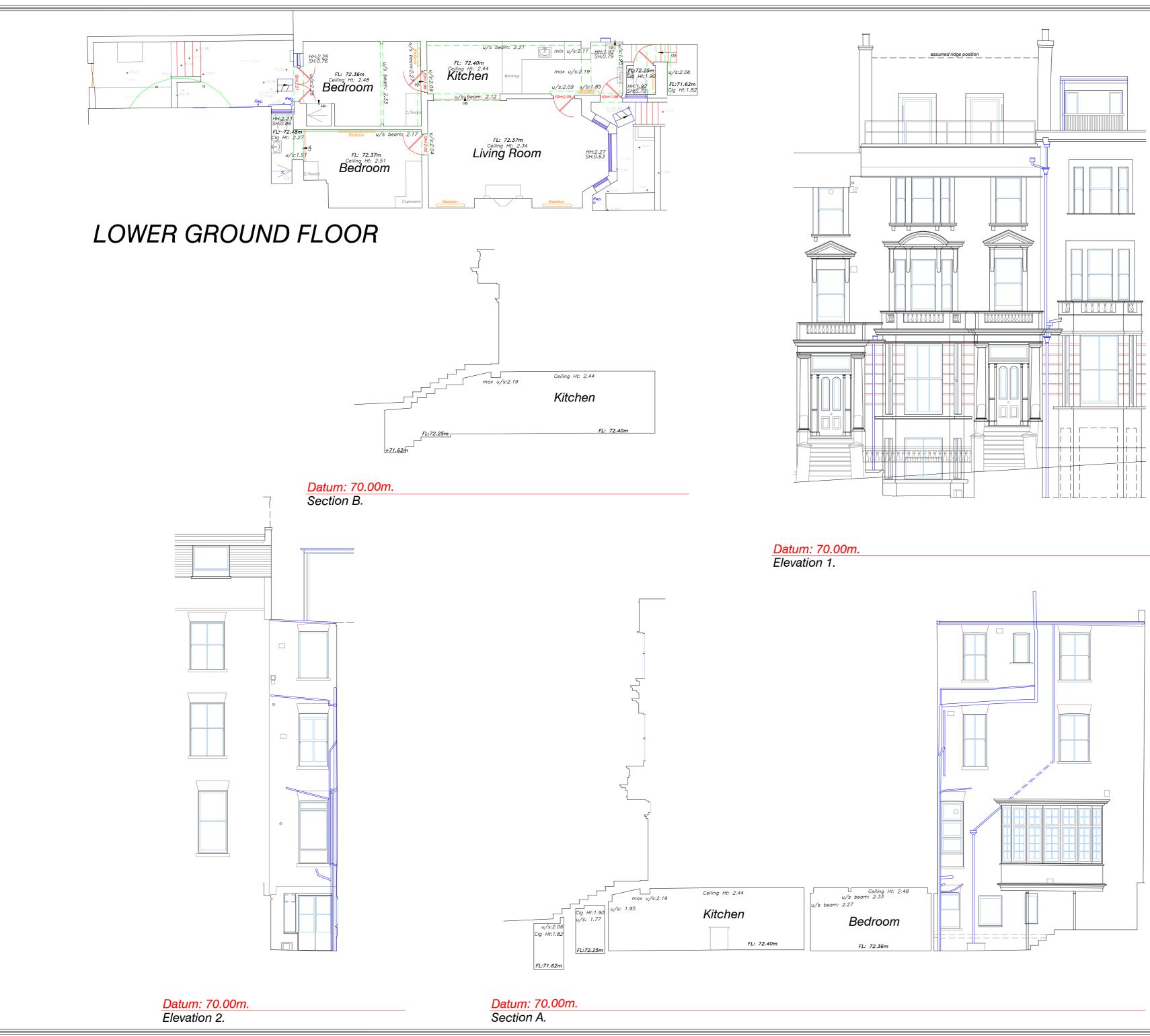


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| 1 | | | | |
|-----|----------------------------------|---|--|---------------------|
| | | | | |
| 1. | PAD ARCH BY GREENH MANHOLE | IITECTS DATED 16/05/ HATCH GROUP SURVE | IL OR COMBINED SEWER. THIS WILL | 9478_01-02_PES_RevB |
| KEY | POSITION (DRAINAGE | DF PROPOSED FOUL S | SEWER SHOWN SUBJECT TO EXISTIN | |
| PUI | wer Blic foul w Wer | | BOUNDARY | |
| | | | | |
| | | | | |
| | | | | |
| RE | EV DATE | AMEND/ | MENT DETAILS | DRAWN APPROVED |
| | PROJECT 31 A BELSIZE | | DATE DRAWING STATUS 10/08/22 INFORMATION SCALE(S) DESIGNED DRAWN 1:40 GB GB | |
| | PROJECT | CRESCENT | DATE DRAWING STATUS 10/08/22 INFORMATION SCALE(S) DESIGNED DRAWN | |

DO NOT SCALE



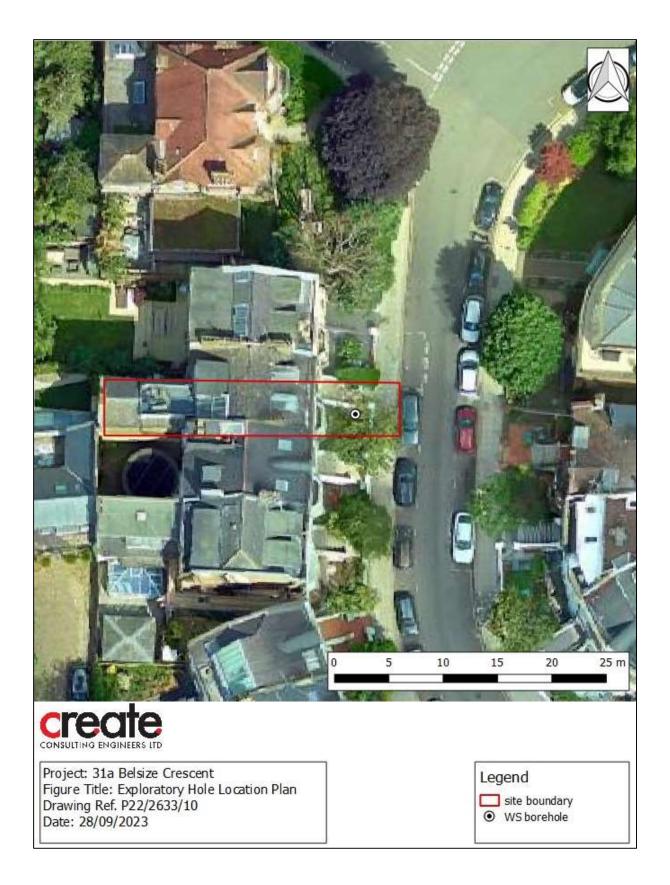




| OS Note: The Ordnance Survey tile | e is to be used as a guide only. |
|---|--|
| OS Buildings | Surveyed Buildings |
| (O.S) National Grid OSGE Satellite Systems (GNSS) | entated to the Ordnance Survey B36(15) via Global Navigational and the O.S. Active Network (OS I |
| site centre via a transform OSGM15GB transformati | ate has been established near to the mation using the OSTN15GB & tion models. related to this point and a further on |
| or more OSGB36(15) poi bearing for angle orientat | ints established to create a true O.S |
| coordinates shown are al which have a scale factor | rbitrary & not true O.S. Coordinates |
| Building Surv | |
| SHt 1.00 HHt 2.12 | Sill Height from FFL. Head Height from FFL. |
| SL 51.03m HL 52.82m | Sill Level from defined datum. Head Level from defined datum. |
| Susp CHt: 2.00 Struct CHt: 3.00 | Suspended Ceiling Height from FF Structural Ceiling Height from FFL. |
| Susp Ceil: 30.00m Struct Ceil: 31.00m IFL: 100.00m | Suspended Ceiling Level from date Structural Ceiling Level from datum Internal Floor Level (General). |
| +100.00m | Internal Floor Level (Specific). Insertion Point for overlay drawings |
| | of other floors or details. |
| Topographica | al Survey Legend: |
| Buildings Overhead Cable Wall Concrete edge | IC Inspection chamber Boll Bollard Pinv Pipe invert IB Illuminated boll |
| Kerb line Tarmac edge Line marking Grass verge Drop kerb Canopy/Overhang Centre line Verge | Gy Gully Bin Rubbish bin Bg Back gully Vp Vent pipe Dp Down pipe Grl Ground light |
| ▲ 1 Station and Name 100.000 Station Level | Pipe Pipe above ground Lbox Letter box MH Manhole Ldr Ladder WL Water level Sty Stile FI Flood light IFL Internal floor level |
| CORE Tree / Bush / Sapling | Lp Lamp post THL Threshold level Tn Telegraph post So Sign post |
| R: Ridge Level | TI Traffic light BH Borehole Bus stop ELC Electric Sv Stop valve BT British Telecom |
| F: Flat Roof Level Gate Fence types: | St Stop tap Cbox Control box Er Earth rod TT Tactile Wm Water meter BP Brick paved |
| IN Interwoven | Gas Gas valve CPS Concrete pavin Av Air valve CVR Cover ICU Undentified inspection IC Inspection char |
| PIR Post & Rail PIW Post & Wire CIL Chain Link | Wo Wash out R/wall Retaining wall Re Rodding eye UTL Unable to lift BB Belisha beacon TCL Tree canopy let |
| WP Wooden Panels CP Concrete Panels SVP Steel Palisade | CTV Cable tv G: Girth Mkr Marker post MG Multi girth Gmkr Gas marker post Stmp Tree Stump Sn Soffit CL: Cover level |
| A 03.03.21 Section | nformation added SJC GH9 B added AF GH9 |
| Rev Date | Description Drawn Q. F |
| <u>araa</u> | abatah G |
| greer | nhatch ⁻ |
| Topographical Surveys Site Engineering Utility / CCTV Surveys | rs |
| Bathymetric Surveys Roy | Area, Lease & Fire Plar |
| Du | iffield Road ittle Eaton Derby |
| Tel (01332) 830044 | DE21 5DR Fax (01332) 830055 reenhatch-group.co.uk |
| St Albans New | vcastle Central London |
| St Albans Ne | Riverside Studios Amethyst Road ewcastle Bus. Park ewcastle -U-Tyne NE4 7YL |
| t. (01727) 854481 t. | NE4 /YL (01912) 736391 t. (0207) 224180 |
| CLIENT | Ellis |
| М | ulkerrin |
| PROJECT 31A Bel | size Crescent |
| | ₋ondon W3 5QY |
| TITLE | g Floor Plans |
| | ons & Section |
| | DATE |
| SCALE A2@ 1: 100 | 01.03.21 |
| | 01.03.21 QUALITY REF GH9859 |
| A2@ 1: 100 DRAWN AF Level datum | QUALITY REF GH9859 |
| A2@ 1: 100 DRAWN AF | QUALITY REF GH9859 |
| A2@ 1: 100 DRAWN AF Level datum Grid orientation Job number Drawing No. | QUALITY REF GH9859 See OS notes above see OS notes above 9478 |
| A2@ 1: 100 DRAWN AF Level datum s Grid orientation s Job number 36 Drawing No. 39478 Comments | QUALITY REF GH9859 iee OS notes above ee OS notes above 9478 B_02_PES |
| A2@ 1: 100 DRAWN AF Level datum Grid orientation Job number 35 Drawing No. 39478 Comments This plan should only be purpose. Greenhatch Gr for this plan if supplied to | QUALITY REF GH9859 iee OS notes above ee OS notes above 9478 B_02_PES used for its original oup accepts no responsibility |
| A2@ 1: 100 DRAWN AF Level datum Grid orientation Job number Brawing No. 39478 Comments This plan should only be purpose. Greenhatch Gr for this plan if supplied to the original client. | QUALITY REF GH9859 Re A Used for its original roup accepts no responsibility on any party other than a checked on site prior an. |
| A2@ 1: 100 DRAWN AF Level datum Grid orientation Job number 35 Drawing No. 39478 Comments This plan should only be purpose. Greenhatch Gr for this plan if supplied to the original client. All dimensions should be to design and constructio Some services may have | QUALITY REF GH9859 GH285 GH285 GH285 GH285 GH285 GH285 GH285 |

APPENDIX E

GROUND INVESTIGATION LOGS AND GEOTECHNICAL RESULTS





Borehole Log

WS01

AGS

| CONSO | LTING ENGINEERS | LTD | | _ | | CIIC | | | | |
|--------------|--|--|--|-------------|----------|--------|--------------|------------------|--|---------------|
| | | | | | | | | | | Sheet 1 |
| roject: 31/ | A Belsize Cre | scent | Pro | oject No: F | P22-2633 | | Co-ords: | E526769.4 | 0 N185030.50 | Hole Ty WS |
| ocation: Lor | don | | | | | | Level: | 73.60m aO | | Scale |
| | | | | | | | | 75.0011 40 | 0 | 1:25 |
| ient: Eili | s Nadoo | | | | | | Date: | 15/09/2023 | | Logge AW |
| Vell Water | Sample | and In Si | tu Testing | Depth | Level | Logond | | Stratum | Description | |
| Strikes | Depth (m) | Туре | Results | (m) | (m) | Legend | | | Description | |
| | | | | 0.50 | 73.10 | | 10mm to | 15mm. TOPSO | mic silty clay. With IL. h black silty clay. Tr lint. Trace fine bric | ace |
| | 1.10 | D | 85 kPa | 1.40 | 72.20 | | Firm to st | tiff orange-brow | n mottled grey silty | CLAY. |
| | $\begin{array}{c} 1.50 - 1.60 \\ 1.50 \\ 1.60 \\ 1.70 \\ 1.80 - 1.90 \\ 1.80 \\ 1.90 \\ 2.00 \\ 2.10 \\ 2.20 \\ 2.30 - 2.40 \\ 2.30 \\ 2.40 \end{array}$ | D HPT HPT D HPT HPT HPT HPT HPT HPT HPT | 85 kPa 84 kPa 78 kPa 78 kPa 84 kPa 90 kPa 103 kPa 102 kPa 102 kPa | | | | | RED LONDON | CLAY. | |
| | 2.50 2.60 2.70 - 2.80 2.90 3.00 3.10 3.20 3.30 3.40 3.50 3.60 3.70 | HPT HPT D HPT HPT HPT HPT HPT HPT HPT HPT HPT | 110 kPa 122 kPa 98 kPa 78 kPa 103 kPa 116 kPa 131 kPa 132 kPa 139 kPa 118 kPa 98 kPa 146 kPa 118 kPa | 2.60 | 71.00 | | 2.60m - becc | oming stiff. | | |
| | 3.80 3.90 4.00 4.10 4.20 4.30 4.40 4.50 4.60 4.70 4.80 4.90 | HPT HPT HPT HPT HPT HPT HPT HPT HPT HPT | 116 kPa 129 kPa 96 kPa 136 kPa 138 kPa 138 kPa 139 kPa 132 kPa 154 kPa 140 kPa 158 kPa 146 kPa | | | | | | | |
| | 4.90 5.00 | HPT | 139 kPa | 5.00 | 68.60 | × | 2 | | | |
| | 5.00 | 1 | | | | | | End of Bo | rehole at 5.00m | |

1. WS01 terminated at 5.0m, target depth.
 2. Groundwater not encountered.



GEOLABS Limited **Bucknalls Lane** Garston Watford Hertfordshire WD25 9XX Tel: +44(0) 1923 892 190 Fax: +44(0) 1923 892 191

email: admin@geolabs.co.uk web: www.geolabs.co.uk

> 16 October 2023 Report No : GEO/38908/01 Page 1 of 1

| | | Date samples received | 20/09/2023 |
|----------|-------------|------------------------------------|------------|
| | | Date written instructions received | 22/09/2023 |
| Our ref | GEO / 38908 | Date testing commenced | 23/09/2023 |
| Your Ref | P22-2633 | Date of sample disposal | 13/11/2023 |
| | | | |

Project **31A BELSIZE CRESCENT**

Mr A Warren

Create Consulting Engineers 109-112 Temple Chambers

3-7 Temple Avenue

For the attention of

London

EC4Y 0HP

Further to your instructions we have pleasure in enclosing the results of the tests you requested in the attached figures.

LABORATORY TEST REPORT

| Item No | Test Quantity | Description |
|-------------|-----------------------|--|
| 1 2 3 | ~ ~ 2 2 ~ | Geotechnical Test Summary Liquid & Plastic Limits Summary Water Content Liquid & Plastic Limits Geochemical Test Summary |
| | 3 | pH Value & Water Soluble Sulphate Content as SO4 & Total Sulphate |

Any opinions or interpretations expressed herein are outside the scope of UKAS accreditation. All results contained in this report are provisional unless signed by an approved signatory. The results contained in this report relate only to samples received in the laboratory and are tested 'as received' unless otherwise stated. This report should not be reproduced, except in full, without the written approval of the laboratory. The results reported are applicable only to the test items received by the laboratory.

All the necessary data required by the documented test procedures has been recorded and will be stored for a period of not less than 6 years. This data will be issued to yourselves at your request. All samples will be disposed of after the date shown above. Written confirmation will be required to retain the samples beyond this period and a storage charge may be applied.

We trust that the above meets your requirements and should you require any further information or assistance, please do not hesitate to contact us.

Yours faithfully on behalf of GEOLABS Limited

J Sturges **Operations Manager**













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SUMMARY OF GEOTECHNICAL TESTING

| | | Samp | ole details | (| Classi | ficati | ion Te | sts | Densit | / Tests | ι | Jndrained Tr | iaxial Com | pression | Ch | emical T | ests | |
|----------|--------------|-----------------|---|---------|---------|--------|--------|-----|---------------|--------------|-----------|-------------------------|---------------------------|------------------------|----|--------------------------|-------------------|--------------------------|
| Location | Depth (m) | Sample Ref Type | Description | wc % | LL % | | | μm | Bulk Mg/m³ | Dry Mg/m³ | Condition | Cell Pressure kPa | Deviator Stress kPa | Shear Stress kPa | рН | 2:1 W/S SO4 g/L | W/S Mg mg/L | Other tests and comments |
| WS01 | 1.10 | D | | | | | | | | | | | | | | | | Chemical |
| WS01 | 1.50-1.60 | D | Brown and orangish brown slightly sandy CLAY. Sand is fine. | 31.3 | 75 | 32 | 43 | 100 | | | | | | | | | | Chemical |
| WS01 | 1.80-1.90 | D | Brown slightly sandy CLAY. Sand is fine. | 32.5 | 76 | 23 | 53 | 100 | | | | | | | | | | Chemical |
| | | | | | | | | | | | _ | | | | | | | |
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Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

| GEO / 38908 Project Name: GEO / 38908 GEOLABS GEOLABS | Checked and Approved by | Project Number: | |
|---|--|-----------------|---------|
| J Sturges - Operations Manager | 101 | | |
| J Sturges - Operations Manager D22_2623 | LEJ - | | GEOLABS |
| | J Sturges - Operations Manager 16/10/2023 | P22-2633 | |

Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX

Client : Create Consulting Engineers, 109-112 Temple Chambers, 3-7 Temple Avenue, London, EC4Y 0HP

1115 - PI Summary - 38908.XLSM

| 38908.XLSM | | | SUM | MARY | BS EN ISO 17892-12 : 2018+ OF LIQUID AND PLA | | LIMI | T TES | STS | | | | | | |
|--------------------------------|-----------------------------|---|---------------|----------------|---|--|----------------|-----------------|--------------------|----------------------------|--------------------------|-----------------|------------------|--|--|
| 1115 - PI Summary - 38908.XLSM | Location | Depth m | Sample Ref | Sample Type | Description | Water Content & BS EN ISO 17892-1 : 2014 +A1:2022 | % Liquid Limit | % Plastic Limit | % Plasticity Index | % Percentage Passing 425µm | Atterberg Classification | Test Type | Sample Condition | | |
| | WS01 | 1.50-1.60 | | D | Brown and orangish brown slightly sandy CLAY. Sand is fine. | 31.3 | 75 | 32 | 43 | 100 | CV | 1 | 1 | | |
| | WS01 | 1.80-1.90 | | D | Brown slightly sandy CLAY. Sand is fine. | 32.5 | 76 | 23 | 53 | 100 | CV | 1 | 1 | | |
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| | | | | | | | | | | | | | | | |
| 15 | 2 - 4 point 80 | g / 30° fall cone g / 30° fall cone c determination | method. | | 1 - A 2 - A | ple conditions Is Received Ir Dried Vashed & Ai | | | | | | · | | | |
| 3810-11 | Checked and | | 1 | Number: | | | | | | | | | | | |
| GL Version 70.230810-1115 | f. | A | Project | Name: | GEO / 3890 31A BELSIZE CR | | ΙТ | | | | | | | | |
| GL Ve | J Sturges - Opera 16/10/ | | | | P22-2633 | | | | | | TEST 19 | AS ING 82 | | | |

Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX Client : Create Consulting Engineers, 109-112 Temple Chambers, 3-7 Temple Avenue, London, EC4Y 0HP 1240 - Chemical Test Summary - 38908.XLSM

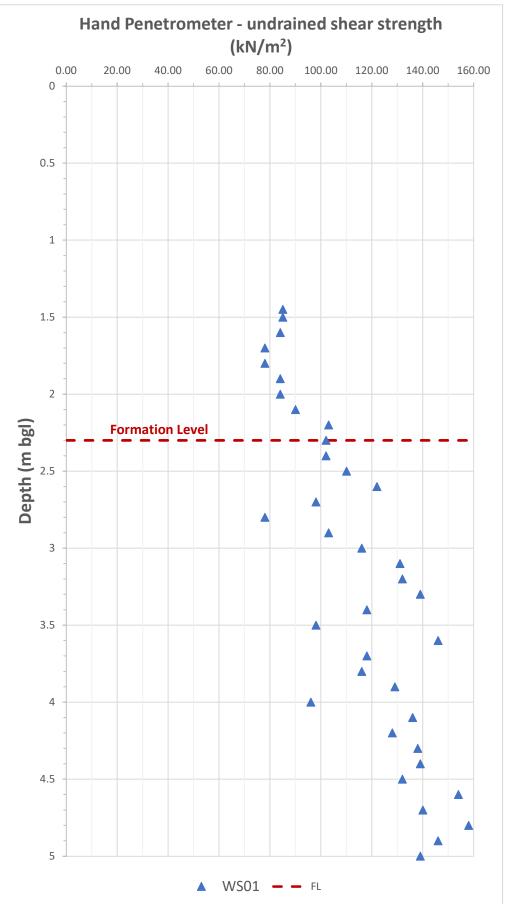
SUMMARY OF CHEMICAL TESTS ON SOIL

| Location | Depth m | Sample Ref | Sample Type | pH Value | Total Acid Soluble Sulphate as SO4 | بع Water Soluble Sulphate as SO4 ≥:1 Water:Soil Extract | % Total Sulphur | ⇔ Water Soluble Chloride | ⇔ Water Soluble Nitrate | ل الم | % Organic Content | % Mass Loss on Ignition | % Carbonate Content |
|----------|------------|---------------|----------------|----------|---------------------------------------|--|-----------------|--------------------------|-------------------------|----------|-------------------|-------------------------|---------------------|
| WS01 | 1.10 | | D | 8.1 | 0.058 | 0.025 | - | - | - | - | - | - | - |
| WS01 | 1.50-1.60 | | D | 8.2 | < 0.010 | < 0.010 | - | - | - | - | - | - | - |
| WS01 | 1.80-1.90 | | D | 8.1 | < 0.010 | 0.095 | - | - | - | - | - | - | - |
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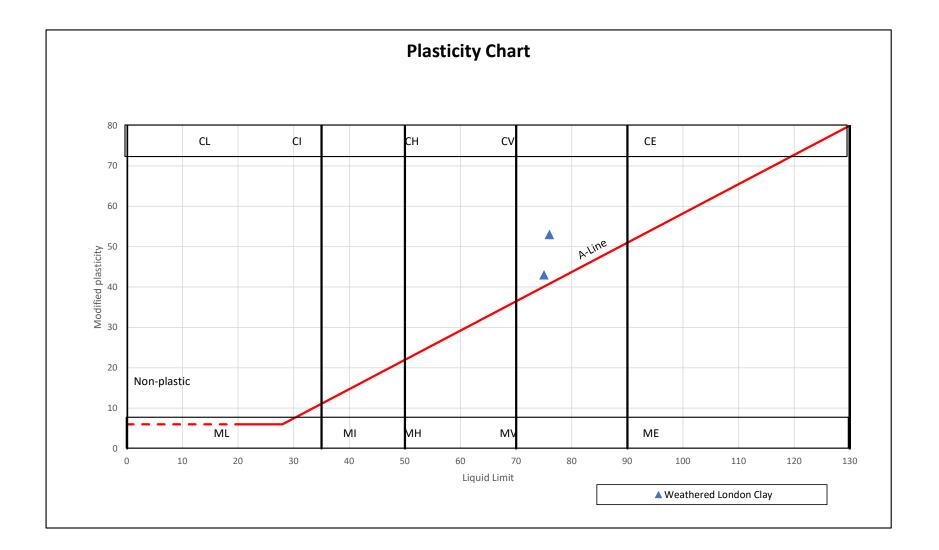
| Checked and Approved by: | GEO / 38908 | GEOLABS |
|--|---|---------|
| J Sturges - Operations Manager 16/10/2023 | Project Name: 31A BELSIZE CRESCENT P22-2633 | GEOLABS |

Test Report By GEOLABS Limited Bucknalls Lane, Garston, Watford, Hertfordshire, WD25 9XX Client : Create Consulting Engineers, 109-112 Temple Chambers, 3-7 Temple Avenue, London, EC4Y 0HP









Ground Gas and Groundwater Monitoring Record Sheet

| JOB DETAILS Site: Date: | 31a Belsi 28.09.202 | | ent | | | | | | | | Visit No: Operator | | 1 AW | of | 1 | |
|-------------------------------|------------------------|----------|----------------------------|----------|-----------------|-----------------|----------|------|-----------------------|---------|-----------------------|-------------------|---|--------------|----------------------|----------|
| | | | | GA | S CONCE | NTRATIO | ONS | | | FLOW | DATA | VOLA | TILES | WELL | | |
| Monitoring Point | Time | | ie (%v/v) | (% | Dioxide v/v) | | n (%v/v) | | .EL | | ate (l/hr) | PID Peak (ppm) | Product thickness (mm) | Water | R DATA Well Depth | COMMENTS |
| | | Peak | Steady | Peak | Steady | Min. | Steady | Peak | Steady | Peak | Steady | | (,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | level (mbgl) | | |
| WS01 | 08:15 | | | | | | | | | ļ | | | | - | 4.0m | |
| | | | | | | | | | | | | | | | | |
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| | | | | | | | | | | | | | | | | |
| Max | | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | DRY | NR | |
| Min | | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | NA | 0.00 | 0.00 | |
| METEOROLOGI | | NA - | Not recorde Non applica | able | | | | | | | | | | | | |
| State of ground: | | | | | | Dry | | | Moist | | [| Wet | | | Snow | Frozen |
| Wind: | | | | | | Calm | | | Light | | | Moderate | • | | Strong | |
| Cloud cover: | | | | | | None | | | Slight | | | Cloudy | | | Overcast | |
| Precipitation: | | | | | | None | | | Slight | | | Moderate | ; | | Heavy | |
| Time monitoring p | performed: | | | | | | | | Start | | | - | | | End | |
| Barometric press | ure (mbar): | : | | | | | | | Start | | | _ | | | End | |
| Pressure trend (D |)aily): | | | | | | | | Falling | | | Steady | | | Rising | |
| Source: | | | | | GA5000 | | | | - | | | | | r | - | |
| Air Temperature (| (Deg. C): | | | | | | | | Before | | | | | | After | |
| INSTRUMENTAT | | INICAL S | | TIONS: | | | | | | | | | | | | |
| Ground gas met | er: | | GA5000 | | | | | | | | | | | | | |
| Gas Range: | | | - | 0 - 100% | 1 | CO ₂ | 0 - 100% | | O ₂ | 0 - 25% | | | | | | |
| Gas Flow range: | | | +100/-50 | | | | | | | | | | | | | |
| Differential Press | ure: | | (+/-) 100 |) Pa | | | | | | | | | | | | |
| Ambient air cheo | ck: | | CH₄ | 0.0 | |] CO2 | 0.1 | |] O ₂ | 20.9 | |] | | | | |

APPENDIX F

GROUND MOVEMENT ASSESSMENT



CALCULATION SHEET

| Project: | 31A B | elsize Crescent, London NW3 5QY | Job No: P22-2633 | She | et No: 1 | Re |
|----------|-------|--|---------------------|-----|-------------|--------|
| ubject: | | | AW | Che | ecked b | y: CB |
| | Groun | d Movement Assessment | Date: 24.05.24 | Dat | | 5/2024 |
| | | <u>Ground movement calculations for proposed lowering of</u> <u>At 31A Belsize Crescent, London NW3 5QY</u> | | | | |
| | | To accommodate the proposed lower ground floor living spa to lower existing floor levels within a section of the property beyond the front of the building. | | | | |
| | | Ground movement estimates have been made using the gui Embedded Retaining Walls Guidance for Economic Design, G 2017. Some tables and graphs are reproduced in the calcula | Gaba et al., CIRIA, | | | |
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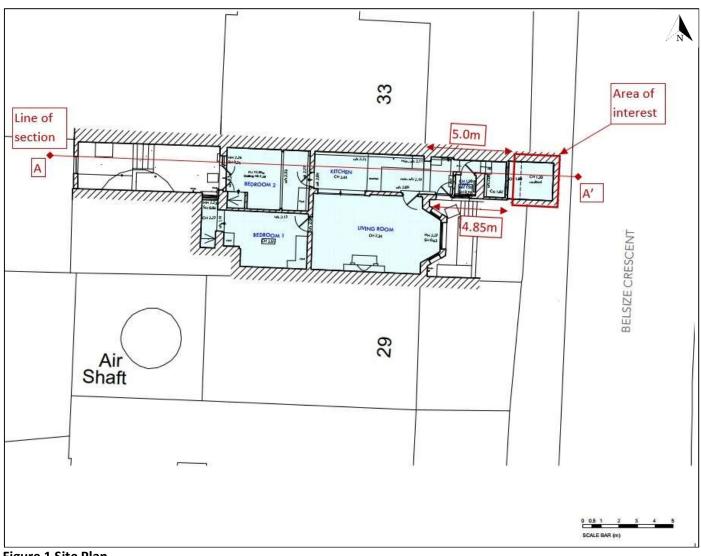


Figure 1 Site Plan



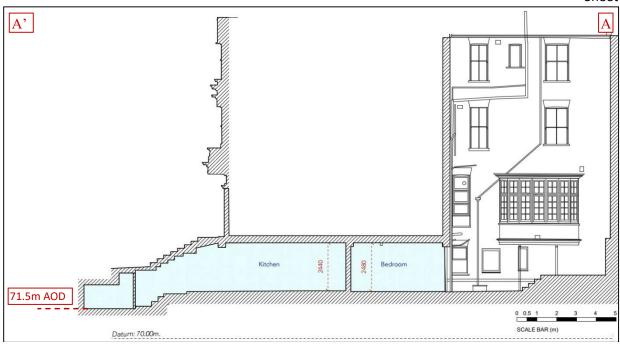


Figure 2 Existing cross-section (PAD Architects)

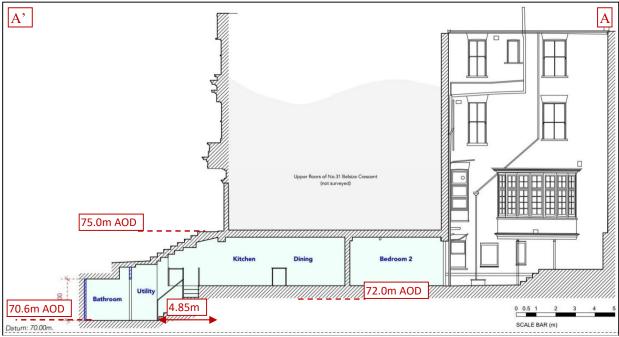


Figure 3 Proposed cross-section (PAD Architects)

CALCULATION SHEET

Project: Job No: Sheet No: Rev. P22-2633 4 31A Belsize Crescent, London NW3 5QY AW Subject: Checked by: CB **Ground Movement Assessment** Date: Date: 24.05.24 28/05/2024 Table 6 1 Ground surface movements due to bored pile and diaphragm wall installation in stiff clay Horizontal movements Vertical movements Wall type Surface movement Distance behind wall to Surface movement Distance behind wall to negligible movement negligible movement at wall (per cent at wall (per cent of wall depth) of wall depth) (multiple of wall depth) (multiple of wall depth) Bored piles 2 Contiguous 0.04 1.5 0.04 Secant 0.08 1.5 0.05 2 Diaphragm walls Planar 0.05 15 0.05 15 Counterfort 0.1 1.5 0.05 1.5 Notes 1 Maximum surface movement occurs close to the wall and is calculated as a percentage of the pile depth/diaphragm wall trench depth, as appropriate. 2 Extent of movement is calculated non-dimensionally by dividing by the pile depth/diaphragm wall trench depth, as appropriate. A planar wall represents the underpinned wall method of construction. Table 6.2 Support stiffness categories (after Carder, 1995) Support stiffness Description/examples High Top-down construction, temporary props installed before permanent props at high level. Moderate Temporary props of high stiffness installed before permanent props at low level Low Cantilever walls, temporary props of low stiffness or temporary props installed at low level. High support stiffness is a reflection of the shallow excavation depth. Table 6.3 Ground surface movements due to excavation in front of bored pile, diaphragm wall and sheet pile walls wholly embedded in competent ground (stiff clavs) Low support stiffness High support stiffness (high propped wall, top-down construction) (cantilever or low-stiffness temporary props or temporary props installed at low level) Movement type Distance behind wall to Distance behind wall to Surface movement at Surface movement at negligible movement negligible movement wall (per cent of max wall (per cent of max (multiple of max (multiple of max excavation depth) excavation depth) excavation depth) excavation depth) Horizontal 0.15 4 0.4 4 3.5 0.1 0.35 3.5 Vertical Notes 1 Maximum surface movement occurs close to the wall and is expressed as a percentage of maximum excavation depth in front of the wall. 2 Extent of movement is calculated non-dimensionally by dividing by maximum excavation depth. 3 Movements exclude those arising from wall installation effects. 4 Movements are for good workmanship and walls wholly embedded in stiff clays, retaining stiff clays or other competent soils. 5 Movements will be greater where soft soils are encountered at formation level (see Figure 6.14 and Appendix A6).





CALCULATION SHEET

| Project: 31A | Belsize Crescent, London NW3 5QY | Job No: P22-2633 | Sh | eet No 5 |): | Rev. |
|------------------------|--|---------------------|----|---------------|-------|---------|
| Subject: Grou | und Movement Assessment | AW | Ch | ecked | by: C | I CB |
| | | Date: 24.05.24 | | te: /05/20 | 024 | |
| CIRIA C760 | The site specific borehole indicates the Site is underlain by v | weathered London | | | | |
| | Clay. | | | | | |
| | The existing floor level is indicated to be 71.5m AOD. The pr | oposed floor level | | | | |
| | is indicated to be 70.6m AOD, a lowering of floor levels by 0 | .90m. Allowing for | | | | |
| | a thicker floor slab and increased insulation, the maximum i | increase in | | | | |
| | excavation is expected to be 1.20m. | | | | | |
| | Ground Surface Movement Due To Underpin | ning | | | | |
| | Max depth of underpinned wall = 1.20m below existing leve | els. | | | | |
| | Horizontal surface movement due to underpinning = <u>5mm</u> , Campbell Reith. | as determined by | | | | |
| Table 6.3 | Distance to negligible horizontal movement = 4 x 1.20m = 4 | | | | | |
| | Vertical surface movement due to underpinning = <u>5mm</u> , as Campbell Reith | determined by | | | | |
| Table 6.3 | Distance to negligible vertical movement = $3.5 \times 1.20 \text{m} = 4.3$ | 20m | | | | |
| | At a distance of 4.85m, the building façade is outwith the zo | one of influence | | | | |
| | from ground movement as a result of underpinning for the | proposed | | | | |
| | basement. | | | | | |
| | Check on services within road | | | | | |
| | Predicted vertical and horizontal movements adjacent to th | e excavation of | | | | |
| | 5mm and 5mm respectively are within tolerable limits and v upon services within Belsize Crescent. | | | | | |