

## SITE INVESTIGATION FACTUAL REPORT



Client: Sedgwick International UK - Maidstone

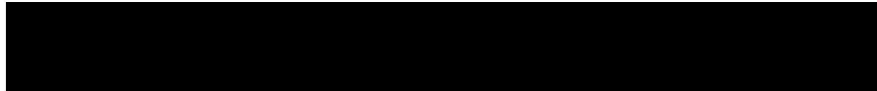
Site: 3a Ainsworth Way, Camden



Date of Visit: 10/11/2022



Home Emergency Response - Subsidence Investigation - Drainage Services – Crack & Level Monitoring – Property Video Surveys



# Investigation Layout Plan

Sheet: 1 of 1

Job No: [REDACTED]

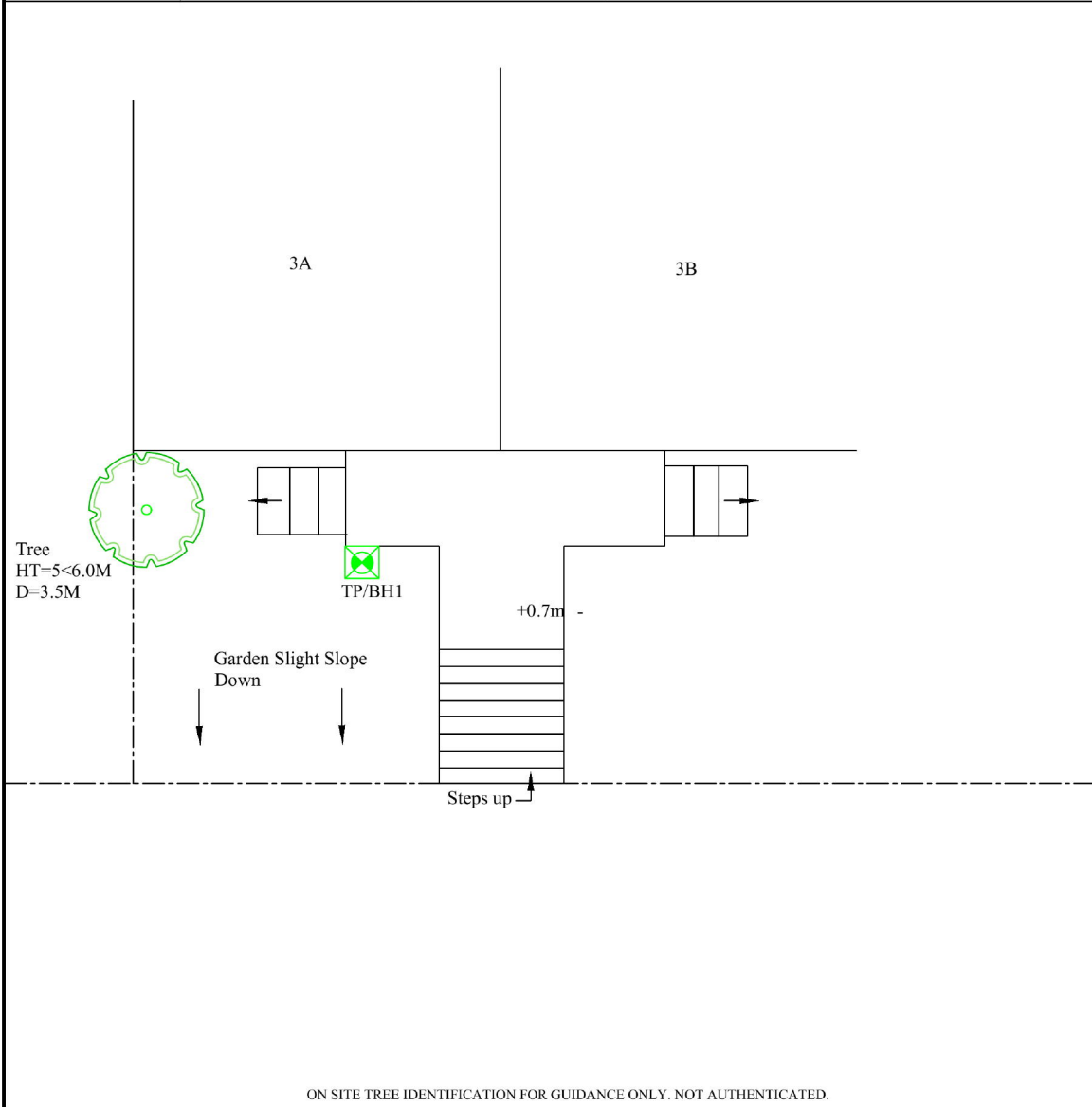
Date: 10.11.22

Site: 3A, Ainsworth Way

Work carried out for: Sedgwick International UK

SP (SI) PS (Checked) JMC (Drawn)

Weather:



ON SITE TREE IDENTIFICATION FOR GUIDANCE ONLY. NOT AUTHENTICATED.

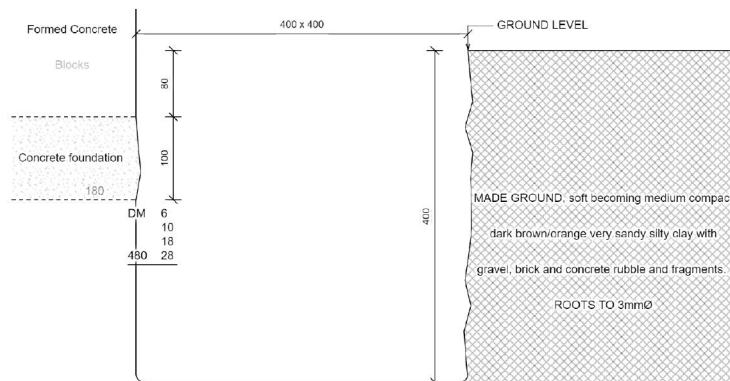
Remarks:

Key:

Combined Gully	RWWG	Surface Water Drain	
Manhole	MH	Foul Water Drain	
Rain Water Pipe	RWP	Tree / Bush	
Rain Water Gully	RWG	(approx. ht in m)	
Soil Vent Pipe	SVP	Trial Pit	
Waste Gully	WG	Borehole	
Waste Pipe	WP		

Scale: N.T.S.

**TEST REPORT:** Trial Pit  
**REPORT NUMBER:** [REDACTED]  
**TRIAL PIT REF:** TP1 **DATE:** 15/11/2022  
**CLIENT:** Sedgwick International UK **SITE:** 3A AINSWORTH WAY  
**JOB NO:** [REDACTED] **WEATHER:** N/A  
**EXCAVATION METHOD:** Hand tools



For Strata below 400mm see Bore Hole log

















**Key:**  
 D Small disturbed sample J Jar sample  
 B Bulk disturbed sample V Pilcon vane (kPa)  
 W Water sample M Mackintosh probe  
 TDTD Too dense to drive

**Remarks:**  
 Test results reported relate only to the items tested.  
 This report shall not be reproduced except in full without approval of the Laboratory.  
 Opinions and interpretations expressed herein are outside the scope of UKAS Accreditation.  
 The laboratory does not apply a conformity statement to test reports as standard, unless specifically requested by the customer.

For and on behalf of CTS  
 Mark Duffield - SI

Approved Signatory  
 Report date 06-Dec-22

Construction Testing Solutions Ltd.  
 Registered in England No. 05998333

Borehole		1		Sheet:	1 of 1 <th>Site:</th> <td colspan="3">3A AINSWORTH WAY</td>		Site:	3A AINSWORTH WAY				
Boring Method:		Hand Auger		Job No:								
Diameter (mm):		75		Date:	10/11/2022							
Weather:		dry		Ground Level:			Client:	SEDGWICK INTERNATIONAL UK				
Depth	Soil Description						Thickness	Legend	Samples and Tests			
(m)									Depth	Type	Result	
0.00	See Trial Pit						0.40					
0.40	MADEGROUND medium compact brown silty sandy clay with gravel and brick fragments						0.40		0.50	DM	24	
											25	
											43	
0.80	Very stiff fragmented orange-brown silty CLAY						1.50				48	
												
									1.00	DV	140+	
											140+	
												
												
												
									1.50	DV	140+	
											140+	
												
												
												
									2.00	DV	140+	
											140+	
												
2.30	End of BH											
Remarks: BH ends 2.3m, too hard to hand auger. BH dry and open on completion.							Key:			To	Max	
							D - Disturbed Sample	Depth	Dia			
							B - Bulk Sample	(m)	(mm)			
							W - Water Sample	Roots	1.70	20		
							J - Jar Sample	Roots	2.30	1		
							V - Pilcon Shear Vane (kPa)	Roots				
							M - Mackintosh Probe	Depth to Water (m)				
							TDTD - Too Dense To Drive					
Logged:	SP	AM	Checked:	Approved:	Version	V1.0 28/01/16			N.T.S.			



**SITE INVESTIGATION  
LABORATORY TEST REPORT**

**SI REPORT NUMBER:** [REDACTED]

**CLIENT :** CET Property Assurance (Sedgwick International UK)

**SITE:**  
3A Answorth Way  
Camden  
London  
NW8 0SR

**DATE OF SITE VISIT:**  
10/11/2022

**DATE RECEIVED BY LABORATORY:**  
11/11/2022

L. Kirby  
Compiled by : .....  
L. Kirby - Senior Laboratory Technician (B)  
[REDACTED]  
Approved by : .....  
L. Marshall - Laboratory Manager (B)

**DATE REPORTED:** 5-Dec-2022

# Laboratory Summary Results

Our Ref: [REDACTED]

Date Sampled: 10/11/2022

Location: 2A Ainsworth Way, Camden, London, NW8 0SR

Date Received: 11/11/2022

Client: CET Property Assurance (Crawford Claims Management)

Date Tested: 14/11/2022

Address: [REDACTED]

Date of Report: 05/12/2022

TP/BH No	Sample Ref Depth (m)	Type	Moisture Content (%) [1]	Soil Fraction > 0.425mm (%) [2]	Liquid Limit (%) [3]	Plastic Limit (%) [4]	Plasticity Index (%) [5]	Liquidity Index [3]	Modified Plasticity Index (%) [6]	Soil Class [7]	Filter Paper Contact Time (d)	Soil Sample Suction (kPa) [8]	Oedometer Strain [9]	Estimated * Heave Potential (Dd) (mm) [10]	In situ * Shear Vane Strength (kPa) [11]	Organic * Content (%) [12]	pH * Value [13]	Sulphate Content * (g/l)		* Class [16]
																		SO <sub>3</sub> [14]	SO <sub>4</sub> [15]	
1	U/S 0.18	D	22	40	42	24	18	-0.09	11	CI	Unsuitable									
	0.5	D	25	26							Unsuitable									
	1.0	D	29	<5	72	27	45	0.06	45	CV	7	494			>140					
	1.5	D	27	<5							7	958			>140					
	2.0	D	27	<5	69	26	43	0.01	43	CH	7	1020			>140					

**Test Methods / Notes**

[1] BS 1377: Part 2: 1990, Test No 3.2  
 [2] Estimated if <5%, otherwise measured  
 [3] BS 1377: Part 2: 1990, Test No 6.4  
 [4] BS 1377: Part 2: 1990, Test No 5.3  
 [5] BS 1377: Part 2: 1990, Test No 5.4  
 [6] BS 1377: Part 2: 1990, Test No 9  
 [7] BS 5930: 2018: Figure 8 - Plasticity Chart for the classification of fine soils  
 Test results reported relate only to the items tested. This report shall not be reproduced except in full without approval of the laboratory. The laboratory does not apply a conformity statement to test reports as standard, unless specifically requested by the customer. Opinions and interpretations expressed herein are outside of the scope of UKAS accreditation.

[8] Building Research Establishment Information Paper 6/93  
 [9] In accordance with BS 1377-5: 1990 - Clause 3  
 [10] Estimated Heave Potential (Dd)  
 [11] Values of shear strength were determined in situ by CTS using a piston head cone or Cone vane (CV).  
 [12] BS 1377: Part 3: 1990, Test No 4  
 [13] BS 1377: Part 3: 1990, Test No 9  
 [14] BS 1377: Part 3: 1990, Test No 2.6  
 [15] SO<sub>4</sub> = 1.2 x SO<sub>3</sub>

[16] BS 5930: 2018: Figure 8 - Plasticity Chart for the classification of fine soils  
 Note that if the SO<sub>4</sub> content falls into the DS-4 or DS-5 class, it would be prudent to consider the sample as falling into the DS-4M or DS-5M class respectively unless water soluble magnesium testing is undertaken to prove otherwise.  
 PSD Chart - BS 1377: Part 2: 1996, Test No 9.2  
 \* These tests are not UKAS accredited.  
 Full reports can be provided upon request.

**Key**

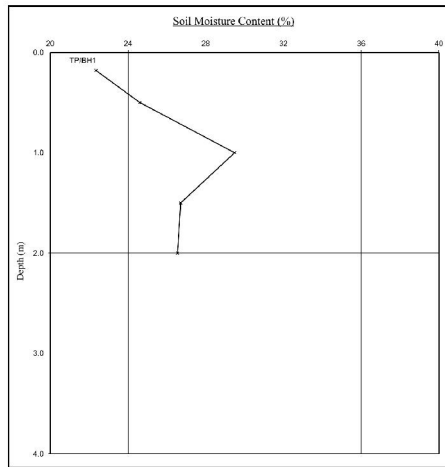
D Disturbed sample (small)  
 B Disturbed sample (bulk)  
 U Undisturbed sample  
 W Groundwater sample  
 FSNP Facially Non-Plastic by inspection  
 US Upside of Foundation



4161

### Moisture Content Profiles

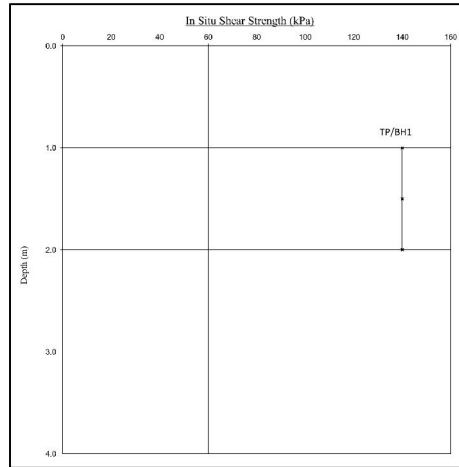
Our Ref: [REDACTED]  
 Location: 2A Ainsworth Way, Camden, London, N1W 0SR  
 Work carried out for: CET Property Assurance (Crawford Claims Management)



Notes:  
 1. If plotted 0.4 LL and PL 2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.  
 2. Unless specifically noted the profiles have not been related to a site datum.

### Shear Strength Profiles

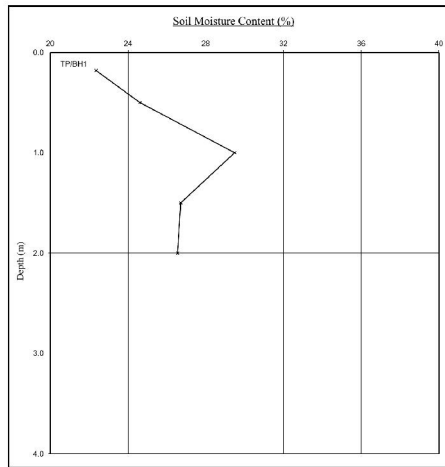
Date Sampled: 10/11/2022  
 Date Received: 11/11/2022  
 Date Tested: 14/11/2022  
 Date of Report: 05/12/2022



Note:  
 1. Unless otherwise stated, values of Shear Strength were determined in situ by CTS using a PKW Hand Vane the calibration of which is limited to a maximum reading of 130 kPa.  
 2. Unless specifically noted the profiles have not been related to a site datum.

## Moisture Content Profiles

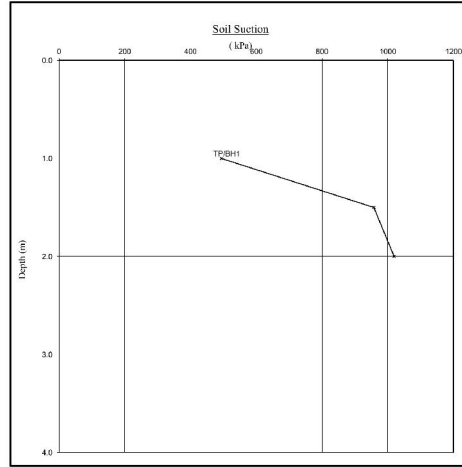
Our Ref: [REDACTED]  
 Location: 2A Ainsworth Way, Camden, London, NW8 0SR  
 Work carried out for: CET Property Assurance (Crawford Claims Management)



Notes  
 1. If plotted 0.4 LL and PL 2 (after Driscoll, 1983) should only be applied to London Clay (and similarly overconsolidated clay) at shallow depths.  
 2. Unless specifically noted the profiles have not been related to a site datum.

## Soil Suction Profiles

Date Sampled: 10/11/2022  
 Date Received: 11/11/2022  
 Date Tested: 14/11/2022  
 Date of Report: 05/12/2022



Notes  
 When shown, the theoretical equilibrium suction profiles are based on conventional assumptions associated with London Clay (and similarly overconsolidated clay) at shallow depths. Note that the suction disturbance component is dependent on the method of sampling and any subsequent recompaction. The above plots show this to be 100kPa which is the value suggested by the BRE on the basis of their limited number of tests on recompact samples. This may or may not be appropriate in this instance and judgement should be exercised.



Construction Testing Solutions



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# ROOT IDENTIFICATION

## 3a Ainsworth Way

Client Reference: [Redacted]  
Report Date: 21 November 2022  
Our Ref: [Redacted]

Sub Sample	Species Identified		Root Diameter	Starch
<b>TP1:</b>				
USF	<i>Fraxinus</i> spp.		3 mm	Moderate
USF	broadleaved species, too decayed for positive identification	1	6 mm	Absent
<b>BH1:</b>				
to 2.3m	<i>Fraxinus</i> spp.	2	16 mm	Abundant

**Comments:**

- 1 - Plus 1 other the same.
- 2 - Plus 2 others also identified as *Fraxinus* spp.

*Fraxinus* spp. include common ash.

**Signed:** R Shaw

Unless we are otherwise instructed in writing, the above sample material will normally be disposed of 6 years after the date of this report.